Appendix A

SHOALING



US Army Corps of Engineers New Orleans District

TECHNICAL REPORTS CALCASIEU RIVER AND PASS, LOUISIANA DREDGED MATERIAL MANAGEMENT PLAN PHASE II



January 2009

TECHNICAL REPORTS SHOALING STUDY

CALCASIEU RIVER AND PASS, LOUISIANA DREDGED MATERIAL MANAGEMENT PLAN PHASE II

Contract No. DACW27-03-D-0005 Delivery Order No. CZ03 GEC Project No. 27321CZ03

PREPARED FOR:



U.S. Army Corps of Engineers **New Orleans District** New Orleans, Louisiana

PREPARED BY:



GAHAGAN & BRYANT ASSOCIATES, INC.

10631 South Sam Houston Parkway West Suite 190 Houston, Texas 77071-3140 Phone (832) 518-2112 Fax (832) 518-1496

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1 INTRODUCTION

1.1 **Project Location**

The Project is located on the Calcasieu River from channel mile 5.0 to mile 36.0 in Calcasieu and Cameron Parishes and within Calcasieu Lake.

1.2 Purpose

The United States Army Corps of Engineers (USACE), Mississippi Valley Division, New Orleans District (MVN) is preparing a Dredged Material Management Plan (DMMP) and the corresponding Environmental Impact Statement (EIS) for the disposal of dredged material from the continued maintenance of the Calcasieu River and Pass, Louisiana project. The DMMP study investigates alternatives for managing dredged material for the next 20 years, including confined, aquatic (open water or ocean), within banks, beach nourishment, and other beneficial-use placement areas. As part of the development of the DMMP and EIS, technical and environmental studies to support the design, construction, and operation of dredged material disposal areas, or beneficial-use sites, are being conducted. The purpose of this technical memorandum is to provide the estimated amount of dredged materials for the next 20 years from mile 5 to mile 36 and programmatically identify sites that can be expanded or added to meet the needs of the DMMP.

1.3 Scope of Work

Historical dredging records and aerial photos were used to support statistical analysis by channel reach of the historical dredging and placement of materials from channel mile 5 to 36. This report, nor its developed scope, is intended to be an engineering design-level document. This evaluation was conducted to support preliminary planning of Placement Areas (PAs).

2 BACKGROUND

2.1 Channel Configurations

The Calcasieu Ship Channel was authorized under the River and Harbor Act of 24 July 1946. Subsequent additions and modernizations occurred under the River and Harbor Act of 14 July 1960, and 23 October 1962, among others to bring the channel to its current configuration. The Calcasieu Ship Channel consists of three main configurations:

- A -42-foot-deep and 800-foot-wide approach channel from the Gulf of Mexico to the Jetties.
- A -40-foot-deep and 400-foot-wide ship channel extending from the jetties to channel mile marker 34.1.
- A -35-foot-deep and 250-foot-wide ship channel from channel mile marker 34.1 to 36 in Lake Charles.
- Mile 0.0 is approximately where the channel makes landfall at the jetties.

Additionally, the authorized channel has the following improvements:

- A mooring basin at channel mile marker 3.
- A -40-foot-deep turning basin at mile 29.6.
- A 1,200 by 1,400-foot turning basin and -40-foot-deep by 400-foot- wide channel at Devil's Elbow.
- A -40-foot by 200-foot channel with a 1,000-foot turning basin at Coon Island.

2.2 Historical Dredging Projections by USACE MVN and ERDC

Dredged materials from ship channel mile marker 5 to 36 are dredged with a cutterhead suction dredge and placed in upland confined or semi-confined placement areas. Some materials have been beneficially used in Black Lake, Brown Lake, and the Sabine National Wildlife Refuge. Dredged material placement areas are discussed further in Section 4.

Review of the several previous reports prepared by USACE MVN and the USACE Engineer Research and Development Center (ERDC) indicate differing quantities for dredging from mile marker 5 to 36 between 1984 and 1994. The USACE MVN "Dredged Material Management Plan Preliminary Assessment", 2 October 1995, divides this study area into 2 reaches. The first reach ranges from mile 5 to 22 and is dredged on an average of once every 5 years with an average of 2,152,000 CY/year. The second reach, from mile 22 to 36, is also dredged every 5 years. A small portion, near Devil's Elbow, is dredged every 2.5 years. An average of 1,262,000 CY/year is removed from this reach. The ten-year projection for each reach is 21,520,000 CY and 12,620,000 CY for the lower and upper reaches respectively. The ten-year total is 34,140,000 CY with a 20-year projection of 68,280,000 CY.

The USACE ERDC, "Calcasieu River and Pass Dredged Material Sedimentation Study Phase 2 Study", August 2005 divides the study area into three reaches: the lower reach from mile 5 to 14, the middle reach from mile 14 to 24, and the upper reach from mile 24 to 36. The ERDC conceptual 20-year plan consists of approximately 4,000,000 CY of material to be dredged every other year in the lower reach, 4,500,000 every other year in the middle reach; and 6,500,000 CY every 5 years for the upper reach. The total dredging for 20 years in the lower reach is estimated at 44,000,000 CY, 49,500,000 in the middle reach, and 32,500,000 in the upper reach for a grand total of 126,000,000 CY of material. The ERDC projected volume is almost double the USACE MVN projected volume taken from actual dredging over 5 years. Review of the ERDC report indicates that the volumes used were taken from the "Calcasieu River Sediment Removal Study", August 1994, by Roy Wade. Review of the Wade Report does not indicate how the volumes were projected but lists assumptions made to illustrate how to use the information in the study for conceptual design of Confined Disposal Facilities (CDFs) and are not actual dredging records for the area. Volumes projected in the ERDC Report were not based on actual site specific dredging data. The historic dredging projections were not evaluated for the entrance and bar channel reaches of the ship channel.

3 SHOALING AND DREDGING EVALUATION

The currents and shoaling evaluation has primarily utilized historic dredging records to determine shoaling rates. The sources of sediment shoaling have not been identified as part of this particular study. A limited Hydrodynamic and Sediment Modeling Study has been completed that indicates the circulation patterns and high energy areas within the project and is included as Appendix C of the overall DMMP. Sources of material consist of riverine deposits, lake and bay bottom erosion, and possible recycling of materials from eroding CDFs or PAs and minimal site management during dredging operations. Figures 1-10 show the reaches of channel

dredged and the corresponding disposal areas. The projected rates of shoaling and required dredging are discussed in the following subsections.

3.1 Data Collection

In order to develop the projected 20-year dredging quantity, historical data from USACE MVN dredging contracts from 1994 to 2005 were reviewed and dredging contractors were interviewed. Dredging contract records were provided by USACE MVN and the USACE Lafayette Area Office. The dredging quantities by contract number and mile are shown in **Table 1**.

3.2 20-Year Dredging Forecast

Using the 11 years of dredging data compiled in Table 1, the projected gross dredging volumes for the next 20 years within the Federal Channel are outlined below.

Channel Reach (by mile)	20-Year Dredging Quantity (CY)
5-12	22,470,000
12-22	34,655,000
22-36	39,590,000
Total	96,715,000

The dredging forecast does not account for bulking during dredging or shrinkage during consolidation. Issues relating to bulking and shrinkage will be addressed in the geotechnical report and future conceptual designs under separate cover. However, a 70% bulking factor was added for the purposes of preliminary planning of disposal sites for each dredging event as discussed in Section 4 of this report. Table 2 outlines the 10 and 20-year dredging need with bulking factors by dredging cycle. Dredged materials from these reaches are placed in various confined, semi-confined, and beneficial-use sites in the project area.

The gross dredging volume is defined as the total material removed, as a result of the dredging process, to meet the required contract lines and grades or dredging template. The gross dredging volume is calculated as the area between the before and after dredging hydrographic survey lines.

3.2.1 Historic Dredging Analysis

The historic dredging completion reports from 1994 to 2005 were reviewed and compiled as shown in Table 1. Due to limited Operation and Maintenance funds available for dredging and PA capacity limitations, the 2003 to present dredging contracts reduced the overall channel width dimensions from 400-feet-wide to 350-feet-wide from approximate mile marker 11 to 22, and to 300-feet-wide around mile marker 15. During initial review of the 20-year dredging need quantities for the project DMMP, the Project Design Team (PDT) recommended that the quantities in this segment be changed to reflect the quantities that would have been dredged and placed, without capacity limitations.

Three methods were discussed by the team to forecast the quantities: 1) Use the before dredge (BD) survey from the last contract and compare that to the required grade template if the channel was dredged to full dimension; 2). Use the after dredge (AD) survey from the last contract that fully maintained the channel width and depth and compare that to the most recent contract before

DACW27-03-D-005, D.O. CZ03 dredge (BD) survey; and 3) evaluate the statistics for the most recent dredging event for overdepth and width by applying them to the wider template or compiling a theoretical template based on the AD survey for that contract, by repeating centerline data in the middle of the channel to simulate an AD survey for a wider required dredging template and compare that to the BD survey.

The PDT did not recommend utilizing the BD surveys to the restricted AD template as it would not reflect the actual amount of material dredged in a box cut to meet grade. They recommended using the AD survey for the last full template contract and comparing it to the BD surveys for the most recent restricted contract in order to estimate the amount of dredging needed.

The AD survey for the last full template contract was available in paper form. However, the electronic xyz data set could not be found by USACE MVN. The AD surveys for the most recent dredging event were available. Therefore, the AD surveys for the narrowed cuts were compiled and reviewed from approximately mile 11 to mile 22. The AD survey data was modified by cutting the data set at the channel centerline, then adding 25 feet of data on either side of the centerline. The data added on either side of the channel centerline was a repeat of the trend of three to four soundings either side of the centerline. The resultant sections are shown in Appendix B.

The analysis method simulated the "box" cuts typical of the industry and for the dredge used in the various contracts. A direct comparison of the BD and AD surveys was conducted to determine the amount of material dredged (or would have been dredged) using the current dredging practices in the area irrespective of the template. Using visual and statistical analysis of the entire segment from mile 11 to 22, it was determined that the contractor has typically swung approximately 235 feet from the centerline on each side on average for the entire segment to get his "box" cut. The average accounts for the extremes in width and narrowness along the entire mile segment analyzed.

Using the above analyses, the cubic yard quantities by mile were modified to reflect a theoretical 400 foot wide channel cut for contracts that had been dredged to a reduced dimension. On average, the difference between the 300 to 350-foot-wide restricted-width channel dredged and the theoretical 400 foot wide dredged channel increased the anticipated volume of 21% to 31% in quantity over the reach. The additional yards were distributed based on a more detailed analysis of each mile from approximately mile 11 to 22, with minor changes at the extremes. The contracts modified were:

- 1. 2003-C-0044
- 2. 2004-C-0048
- 3. 2005-C-0045

3.2.2 Private Dredging Needs

The Port of Lake Charles Harbor and Terminal District (PLCHTD), Trunkline LNG, Sempra LNG Cheniere LNG, Citgo, and Conoco are the only private dredging needs identified during this study.

3.2.2.1 Port of Lake Charles Harbor and Terminal District (PLCHD)

The PLCHTD owns and operates 4 terminals. The City Docks are comprised of several terminals, approximately 8,000 linear feet of berthing space. The dredging need varies from 65,000 to 95,000 CY every 7 to 10 years as provided by PLCHD. The last maintenance dredging cycle was in 2006, approximately 170,000 CY. The material may have been deposited as a result of poor operations by the terminal operator who processes sand and gravel as it was granular and contained gravel. No records exist for the previous dredging event.

Bulk Terminal 4 is located at approximately mile 35. A 1,500 foot berthing area requires maintenance dredging every 5 to 7 years, 8,000-18,000 CY per cycle. The last cycle was in 2006, removing 10,000 CY. Again, no records exist for previous dredging contract events.

Bulk Terminal 1 is permitted to be built south of I-210, but has not been constructed. An anticipated 900,000 cubic yards of dredged material will come from the new work construction. This would be new work material and could be used for levee building materials to either increase levee heights at an existing CDF or to create new levees at the Olin Tailing Ponds Facility currently being evaluated. Therefore, this quantity was not added to the dredging amount needed for disposal capacity. However, geotechnical properties of this material have not been evaluated for levee construction. If not suitable for direct levee pumping, the material could be stockpiled or strategically placed along levee borrow areas necessary for levee expansion at one of the disposal areas. Anticipated shoaling is 12,000 CY per 7-year dredging cycle.

3.2.2.2 Other Private Dredging Needs

Citgo, Conoco, and Trunkline LNG were contacted numerous times by this contractor and the PLCHTD to determine their dredging need. Only Citgo has responded to the requests.

Citgo will require the approximately 30,000 CY of annual dredging at heir Clifton Ridge Facility for a total of 600,000 CY near Channel Mile 27.5. The Citgo Refinery Docks will require approximately 20,000 CY annually for a total of 400,000 CY near Channel Mile 28. They will also attempt to deepen their Refinery Dock with a projected 45,000 CY sometime in the 20 year DMMP.

The Trunkline LNG Terminal is located on Devils Elbow. Dredging to their facility is accounted for in the overall shoaling study. Cheniere is currently constructing their facility around mile 4. Their dredged materials will be placed in their own disposal facility. No other private users were identified during this study period.

Sempra and Cheniere LNG have established their own dredging disposal areas.

3.2.3 Anticipated Future Dredging Needs

As the result of the Calcasieu River and Pass Navigation, Louisiana Reconnaissance Report, May 2006, four alternative plans for the development of anchorages and passing lanes to improve safety on the Calcasieu Ship Channel were developed. The four alternatives include:

- Plan 1 Anchorage Area at Mile 15
- Plan 2 Anchorage Area at Mile 5.1
- Plan 3 Passing Lane at Mile 24
- Plan 4 Passing Lane at Mile 5

These four alternatives will be further evaluated in an ongoing Feasibility Study. Long-term maintenance dredging needs and quantities are not known at this time. However, it should be noted that any new work dredging conducted to construct the anchorages or passing lanes could be stockpiled and utilized as much needed construction material to rehabilitate or raise existing CDF levees and/or create new levees as necessary at PAs M, 12A, 12B, or 13, or could be placed beneficially in Lake Calcasieu or other site identified in the Feasibility Study. Estimates of the new work materials range from 760,000 to 1,250,000 CY. Geotechnical properties of the materials will be determined during the ongoing feasibility study. New work dredging from the anchorages or passing lanes would not deplete existing or future disposal area capacities. Construction of these alternatives is not expected before the next 3 to 5 years and maintenance would not be expected before the next 8 to 10 years.

3.3 Shoreline Loss

Aerial photography of sufficient quality to determine erosion and deposition rates on an annual basis in the area was not available during this study. However, the ERDC report estimates that 5% of the dredging quantities could be caused by recycling of materials. The method for determining the rate of recycling was not provided.

Visual examination of all existing CDFs in July 2006 indicated significant active dike erosion, particularly in the Calcasieu Lake Region from mile 7 to 21 on both the ship channel and lake sides of the CDFs. Photographs documenting some of the erosion are included in Appendix A. Although the amount of material recycled due to erosion could not be easily quantified, it is reasonable to assume the recycling rate is greater than 5%. Recycling of materials could be reduced through shoreline and dike protection measures. A riprap foreshore dike has been designed and constructed on the east side of the ship channel between mile 11.2 and 15.7. The installation of the foreshore dike should help to reduce some of the recycling. However, as shown in the photographs from the site visits, extensive erosion of the CDFs is occurring on the western side of the ship channel and along the lakeside of the CDFs as well.

3.3.1 Limited Shoreline Loss Analysis

A limited historical shoreline change analysis was developed in the Bay Reach along PA 17 to PA E based on historical aerial photographs of the Calcasieu Shorelines dated 1998 and 2008 as provided by the USACE New Orleans District.

In each aerial the shoreline position was digitized based on the vegetation line. The resulting shape file of the shoreline from the 1998 aerial was overlaid on top of the 2008 aerial and

DACW27-03-D-005, D.O. CZ03 compared. Results can be seen in Figures 11-14. Two methods were used to determine the rateof-change for the selected shoreline. The sections below describe the statistical methods used to calculate the loss of shoreline in area and the resulting deposition of materials into the system in cubic yards (CY).

3.3.1.1 Shoreline Change - Difference of Areas Method

The difference of areas method involves the evaluation of the change in area between the two georeferenced shape files that represent the vegetated shorelines between the 1998 and the 2008 aerials and an assumed elevation to determine the volume of materials lost. Limited survey or LIDAR data exists over the entire area. The average elevation of the top of the dikes is approximately +10 MLG. It was assumed that the erosion was limited to a -2 MLG elevation. Therefore the total height lost would be 12 feet. It is difficult with this method to identify areas of accretion or erosion and what the magnitude of the net rate-of-change would be along a specified section of shoreline. Table 3 below shows the calculated loss of area and CY of materials from 1998 to 2008.

	1998 Area	2008 Area	Change in Area (SqFt)	Average Elevation	CY Loss	Per Year
17-20	17,267,118	14,449,449	-2,817,669	12	-1,252,297	-125,230
22-23	27,715,825	23,651,693	-4,064,133	12	-1,806,281	-180,628
D	19,001,995	13,913,844	-5,088,150	12	-2,261,400	-226,140
E	11,205,259	8,428,548	-2,776,711	12	-1,234,094	-123,409
Totals					-6,554,073	-655,407

Table 3 – Shoreline	Change -	- Difference in	Areas Method
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3.3.1.2 Shoreline Change - Average End Point Rate Method

The end point rate method also evaluated the changes in vegetated shoreline between the two georeferenced shape files but was calculated by dividing the distance of shoreline movement by the time elapsed between the oldest and the most recent shorelines. The shorelines were divided into 500 foot sections, and measurements for the loss in area were taken and recorded as positive for shoreline accretion and negative for shoreline erosion. The end point rate allows for independent analysis of the channel shoreline and bay shoreline. The calculation for volume change can be performed based on the determination of the slope of the shoreline and utilization of the average end area method. See Figure 15 below.



Figure 15 Formula to calculate the change in area using the End Point Rate.

The distance to the depth of convergence was based on visual examination in the field and review of aerial photographs. The depth of convergence was determined based on wave type impacting the ship channel and Bay side shorelines. The shorelines on the ship channel are predominantly impacted by ship waves (surge waves); where as the Bayside shorelines are impacted by the wind, wave environment. The depth of convergence was based on conclusions in the technical report "Hydrodynamic and Sediment Transport Study", prepared by Applied Coastal Research and Engineering, Inc. The study indicated that significant ship wave-induced transport along the ship channel side of the PAs occurred when the surge wave flow velocities reached or exceeded 1.5 ft/sec, creating a critical shear stress eroding the bay bottom promoting sediment transport. This typically occurred between the -2 and -4 MLG contour. The same methodology of critical shear stress was used to evaluate the depth of convergence for the bayside shoreline.

Using the above information, it was assumed that the vertical elevation of erosion for the channel sides are +12 MLG and the distance to the depth of conversion was approximately 40 feet. However, on the Bay side there is a long fetch of wind and wave action that causes bay bottom scouring as it approaches the area. The vertical elevation for the Bay side was assumed at +3 MLG but the distance to the depth of convergence was assumed at 200 feet. Table 4 below summarizes the calculated loss of CY of materials from 1998 to 2008 using the End Point Rate Method.

Area	Channel (CY)	Bay (CY)	Total (CY)	Per Year
17-20	-1,091,761	-1,169,643	-2,261,404	-226,140
22-23	-579,471	468,860	-110,611	-11,061
D	-234,758	-1,721,486	-1,956,244	-195,624
Е	-505,544	-1,327,519	-1,833,063	-183,306
Totals	-2,411,535	-3,749,787	-6,161,322	-616,132

Table 4 – Shoreline Change – End Point Rate Method

It is assumed that a majority of eroded materials from these areas remain suspended in the water column and ultimately circulate and fall out into the ship channel. The difference in methods are fairly similar in magnitude and indicate that it is possible that an additional six million CY of

DACW27-03-D-005, D.O. CZ03 material may fall into the ship channel over the ten-year period of review or approximately an additional twelve million CY over the 20 year life of the DMMP. The shoaling rates described in this report indicate that gross quantities dredged over the study period and would include the natural processes occurring here. By placing rock armor along the confined disposal areas in the lake reaches of the ship channel studied in this limited shoreline change analysis from Mile 12 to 20 the amount of dredging necessary could be reduced by approximately twelve million CY.

Initial cost estimates indicate that the CY cost of placing materials in a CDF is approximately 3.27/CY and 5.27/CY to beneficial use sites. The total cost of dredging the "recycled materials" over the 20-year life of the DMMP would be approximately 3.27/CY and 63,240,000 for placement in a beneficial use site. The estimated costs to place rock armoring around the channel and Bay sides of PAs 17, 19, 20, 22, 23 and the backside of D and E was estimated at approximately 27 Million by the New Orleans District. This estimate assumes that rock would be placed from a +8 MLG elevation to a -2 MLG elevation on a 3:1 slope.

Additional concerns have been raised regarding the shoreline on the West side of the ship channel from mile 16.5 to 18.7. A shoreline rate change analysis was not conducted along this shoreline but active erosion is present. The cost of placing rock armor along this section of channel is estimated at approximately \$3.5 Million.

4 PRELIMINARY PLACEMENT AREA PLANNING

4.1 Existing Placement Area Capacities

Review of existing LIDAR survey data of the study area and visual examination of all of the existing PAs within the current federal standard conducted in July 2006 indicates that the majority of the PAs are at or near capacity. A combination of visual examination and LIDAR data was used to estimate the remaining disposal capacities of the PAs in their present condition as shown in Table 5. The remaining total capacity of all of the current PAs is estimated at approximately 5 MCY. Figures 1-10 show the location of the existing PAs. Expanding existing PA capacity, alternative disposal, and beneficial-use alternatives have been analyzed and are quantified in the DMMP in Section 2 for each of the alternatives evaluated. PA expansion, alternative disposal area capacity shortfalls. Implementation of the selected alternative to include existing PA vertical and horizontal expansions, beneficial use of dredged materials, dredged material disposal management, and site management are discussed in Section 5.

5 CONCLUSIONS

- The 20-year dredging forecast indicates a dredged material capacity need of approximately 97 million cubic yards.
- The existing CDFs do not provide sufficient capacity for the 20-year dredging forecast. A combination of site rehabilitation, dike construction, active CDF site management, contractor quality control, and a combination of beneficial-use placement will be needed to meet the dredged material disposal capacity need.

- The dredged material disposal capacity need can be met from mile 21 to 36 with dike rehabilitation, dike raising, and active site management. In addition to these measures, innovative placement and beneficial-use of dredged materials at the Sabine National Wildlife Refuge, Browns Lake, Black Lake, the Cameron Wildlife Refuge, and in the Calcasieu Lake will be needed to meet the dredging needs between mile 6 and 21.
- There is a minimal private dredging need.
- Placing rock armoring along the eroding PAs 17, 19, 20, 22, 23 and the backside of D and E could reduce the dredging need by approximately 12 Million CY over the life of the DMMP and provide a cost savings from 36 to 12 Million dollars depending on the disposal method used.

6 **BIBLIOGRAPHY**

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"Calcasieu River and Pass Dredged Material Sedimentation Study", USACE Environmental Processes and Engineering Division, May 2004.

"Calcasieu River Sediment Removal Study", Roy Wade, USACE Waterways Experiment Station, August 1994.

"Calcasieu River and Pass Environmental Impact Statement" USACE, 1976

Personal Interviews:

Mike McMahan – Mike Hooks Charles Harris – formerly with TL James Dredging and Weeks Marine Herbert Juneau – former Area Engineer, Lafayette Area Office

Contract Completion Reports for Maintenance Dredging for Calcasieu River and Pass, Louisiana, USACE MVN, 1994-2005.

Aile Mile	4-12.3 12.3-13.3 Subtotal	1,056,407	0,000 700,000 1,919,057	- 616,481	1,081,670	0	6,993 314,378 2,611,032	1,270,993	3,684 528,611 4,417,511	999,220	0,125 370,956 3,362,052	2,070,470	
Mile N	10.4-11.4 11.		360,000 75				304,044 310		796,694 56.		551		1 100 730 2 16
Mile	9.5-10.4		109,057	119,826			199,853		560,608				000 244
Mile	8.5-9.5			76,407			266,417		539,385		422,557		771 VUC 1
Mile	7.6-8.5			289,570			706,743		826,221		610,432		
Mile	6.6-7.6			94,312			321,391		336,549		328,241		1 000 402
Mile	5.7-6.6			36,366			133,581		265,759		117,439		377 633
Mile	5-5.7						15,003				7,521		10 E 0 I
Nav Gap	~ Mile 8.5						32,629						002.00
Turning	Basin											326,765	
Clooney	Isl. Loop							207832				347,925	בבב חבח
Devil's	Elbow	1,056,407			1,081,670			1,063,161		999,220	954,781	1,395,780	. <i>EE</i> 1 010
Dredged	Dimensions	400**'X-42.0	400**'X-42.0	400**'X-42.0	400'X-42.0	400**'X-42.0	400'X-42.0	400'X-42.0	400'X-42.0	400'X-42.0	400'X-42.0	400'X-42.0	SUMMATION BY
Contract	Complete	8-Mar-06	23-Feb-05	29-Sep-04	25-Jan-04	29-Nov-03	20-Jan-02	14-Aug-01	12-Nov-99	8-Jul-99	26-Oct-96	30-May-95	
Contract	Date	12-Jul-05	12-Aug-04	30-Jun-04	11-Sep-03	19-Jun-03	4-Apr-01	13-Sep-00	16-Jul-99	18-Aug-98	10-Apr-96	12-Sep-94	
Contract #		05-C-0045	04-C-0048	04-C-0041	03-C-0058	03-C-0044	01-C-0038	00-C-0092	99-C-0050	98-C-0065	96-C-0039	94-C-0099	

	total	3,000	3,225	0	9,209	8,000	,465	1,356	,731	9,200	4,687	3,325	2,198
	Sub	2,60	3,03;		1,16	3,118	250	4,78.	569	5,619	$5,20_{4}$	1,23,	27,58
Mile	24.6-25.6	119,000	109,225					357,733		163,300		157,740	906,998
Mile	23.7-24.6				420,560			357,733		455,785		355,585	1,589,663
Mile	22.7-23.7	421,000			440,072			500,000		512,200		486,705	2,359,977
Mile	21.8-22.7	760,000			308,577	100,000		421,877		663,800	242,343	233,295	2,729,892
Mile	20.8-21.8	145,000				438,000		421,877		578,900	645,644		2,229,421
Mile	19.90-20.8	145,000				334,000		469,934		502,650	542,776		1,994,360
Mile	18.9-19.90	384,000				234,000		182,679		426,900	460,245		1,687,824
Mile	18.0-18.9	287,000				236,000		182,679		302,400	495,592		1,503,671
Mile	17.1-18.0	342,000				347,000		281,406		456,700	606,790		2,033,896
Mile	16.1-17.1		750,000	-		437,000	-	513,258		444,500	578,232		2,722,990
Mile	15.2-16.1		788,000	-		501,000	-	662,016		606,400	520,722		3,078,138
Mile	14.2-15.2		663,000	-		439,000	-	430,164		505,665	564,739		2,602,568
Mile	13.3-14.2		723,000	I		52,000	250,465		569,731		547,604		2,142,800
Dredged	Dimensions	400**'X-42.0	400**'X-42.0	400**'X-42.0	400'X-42.0	400**'X-42.0	400'X-42.0	400'X-42.0	400'X-42.0	400'X-42.0	400'X-42.0	400'X-42.0	SUMMATION BY MILE
Contract	Complete	8-Mar-06	23-Feb-05	29-Sep-04	25-Jan-04	29-Nov-03	20-Jan-02	14-Aug-01	12-Nov-99	8-Jul-99	26-Oct-96	30-May-95	
Contract	Date	12-Jul-05	12-Aug-04	30-Jun-04	11-Sep-03	19-Jun-03	4-Apr-01	13-Sep-00	16-Jul-99	18-Aug-98	10-Apr-96	12-Sep-94	
Contract #		05-C-0045	04-C-0048	04-C-0041	03-C-0058	03-C-0044	01-C-0038	00-C-0092	99-C-0050	98-C-0065	96-C-0039	94-C-0099	

Table 1 – 10 Year Historical Dredging Quantities by Contract

Contract #	Contract	Contract	Dredged	Mile	Mile	Mile	Mile	Mile	Mile	Mile	Mile	Mile	Mile	Mile		Total	
	Date	Complete	Dimensions	25.6-26.5	26.5-27.5	27.5-28.4	28.4-29.4	29.4-30.3	30.3-31.3	31.3-32.2	32.2-33.1	33.1-34.1	34.1-35.0	35.0-36.0	Subtotal	Contact	
05-C-0045	12-Jul-05	8-Mar-06	400**'X-42.0	265,000	200,000	202,000	193,000	79,000							939,000	4,598,407	
04-C-0048	12-Aug-04	23-Feb-05	400**'X-42.0	169,675									51,237	51,237	272,149	5,224,431	
04-C-0041	30-Jun-04	29-Sep-04	400**'X-42.0												0	616,481	
03-C-0058	11-Sep-03	25-Jan-04	400'X-42.0												0	2,250,879	
03-C-0044	19-Jun-03	29-Nov-03	400**'X-42.0												0	3,118,000	
01-C-0038	4-Apr-01	20-Jan-02	400'X-42.0												0	2,861,497	
00-C-0092	13-Sep-00	14-Aug-01	400'X-42.0	465,536		474,320	538,267	351,851							1,829,974	7,882,323	
99-C-0050	16-Jul-99	12-Nov-99	400'X-42.0												0	4,987,242	
98-C-0065	18-Aug-98	8-Jul-99	400'X-42.0												0	6,618,420	
96-C-0039	10-Apr-96	26-Oct-96	400'X-42.0												0	8,566,739	
94-C-0099	12-Sep-94	30-May-95	400'X-42.0	151,785	56,735	170,625	138,310	105,620	172,840	117,100	95,005	77,400	106,885	213,015	1,405,320	4,709,115	
			SUMMATION BY														
			MILE	1,051,996	256,735	846,945	869,577	536,471	172,840	117,100	95,005	77,400	158,122	264,252	4,446,443	51,433,534	
** Modified	by analysis to	include quantitie	es for full dimensions		Modified Ce.	llData modifie	ed to reflect 40)0 foot cut							Modified	Unmodified	Delta
													Total 94-				
													05		51,433,534	47,483,959	3,949,575
													Sum of 95 to	0 05	46,724,419	42,774,844	3,949,575
													Sum of 94 to	0 04	46,562,978	42,939,139	3,623,839

1/7/2009

Cont. Table 1 – 10 Year Historical Dredging Quantities by Contract

	10-YR Gross Quantity	Est. 20- Year Required	Quantity	Dredging Cycle	Quantity	70% Bulking
Mile	(CY)	Capacity	Per Year	(yrs)	Per Cycle	Capacity/Cycle
34 to 36	422,374	844,748	42,237	10	422,374	718,036
Port	176,000	352,000	17,600	5	88,000	149,600
Turning Basin	326,765	653,530	32,677	10	326,765	555,501
Clooney Is. Loop	555,757	1,111,514	55,576	5	277,879	472,393
30 to 34	462,345	924,690	46,235	10	462,345	785,987
26 to 30	2,938,649	5,877,298	293,865	6	1,763,189	2,997,422
22 to 26	6,353,205	12,706,410	635,321	2	1,270,641	2,160,090
21 to 22	2,229,421	4,458,842	222,942	2	445,884	758,003
Devil's Elbow	5,155,239	10,310,479	515,524	2	1,031,048	1,752,781
16 to 21	9,942,741	19,885,482	994,274	2.5	2,485,685	4,225,665
12 to 16	9,737,451	19,474,902	973,745	2.5	2,434,363	4,138,417
9.5 to 12	4,630,884	9,261,768	463,088	3	1,389,265	2,361,751
5 to 9.5	5,426,523	10,853,046	542,652	3	1,627,957	2,767,527
Total:	48,357,354	96,714,709	4,835,735			

Table 2 – 20-year Dredging Forecast

Notes:

*Quantities used were from the 95 to 05 contracts for all reaches except miles 30 to 36, the Turning Basin and Clooney Island Loop.

*The 94 to 04 contract summation was used to capture the mile 30 to 36 reach, the Turning Basin and Clooney Island Loop quantities.

		Vertical	Remaining	Existing	Perimeter
Placement		Capacity	Freeboard	Capacity	Levees
Area	Acres	(CY/FT)	(FT)	(CY)	(FT)
1	50	80,600	4	80,600	6,300
2	44	71,000	4	71,000	5,400
3	113	182,300	5	364,600	11,000
7	257	414,600	4	414,600	13,000
8	192	309,700	2	0	14,000
9	170	274,200	2/3	0	12,500
10	135	217,800	2/3	0	10,000
11	135	217,800	4	217,800	10,000
12A	160	258,100	3	0	12,000
12B	433	698,500	6	2,095,500	24,000
13	710	1,145,400	2/3	0	25,000
15	181	292,000	4/5	584,000	12,000
16N	168	271,000	2/3	0	11,000
17	192	309,700	3/4	309,700	18,000
22	133	214,500	3/4	214,500	12,000
23	116	187,100	4/5	374,200	15,000
D	247	398,500	3/4	398,500	25,000
Е	155	250,000	2/3	0	16,000
Н	142	229,000	4/5	458,000	13,000
Μ	392	632,400	2/3	0	18,000
Ν	219	353,300	2/3	0	16,000
				5,583,000	

Table5 – Existing CDF Capacities

**All confined areas are estimated with 2 feet of freeboard and 1 foot for ponding or full capacity at 3 feet of total freeboard on levees.

**Sites A, B, C, F, G, J, K, 4, 5, 6, 14, 16S, 16C, 19, 20, 21 were not included due to limited capacity, land loss, cost effectiveness for expansion/future use, and/or removal of easements.

FIGURES

Gahagan & Bryant Associates, Inc.





























APPENDIX A

Shoreline Erosion

Photograph 1 – Ship Channel Shoreline of PA 16N



Picture 2 – Ship Channel View of Levee at PA 16S.



Picture 3 – Ship Channel View of PA 17.



Picture 4 – North View of PA 17 along the Ship Channel.



Picture 5 – Ship Channel View of PA 22



Picture 6 – Lakeside View of PA 22



Picture 7 - PA D Levees



Picture 8 – Ship Channel View of PA D



Picture 9 – Lakeside View of PA E



Picture 10 – Lakeside View of PA E

APPENDIX B

Dredging Template



