Computationally Efficient and Robust Approach for Multi-objective Operation of Multi-reservoir systems Subjected to Multiple Constraints

Prof. Arturo Leon, Ph.D., P.E.

Students: Elizabeth Akemi Kanashiro, Rachelle Valverde, Christopher H. Gifford-Miears, Tseganeh Gichamo, Luis Comez, Julia Rask

School of
Civil and Construction EngineeringOregon State University



Acknowledgements:

Financial support: National Science Foundation (NSF) NSF-EPSCoR Idaho Oregon State University (OSU)

Collaborators:

Faculty: Nathan Gibson (Math), David Hill (CCE) Graduate students (CCE): Rachelle Valverde,

Christopher Gifford, Julia Rask, Luis Gomez, Tseganeh Gichamo, Akemi Kanashiro (Ex-BSU)

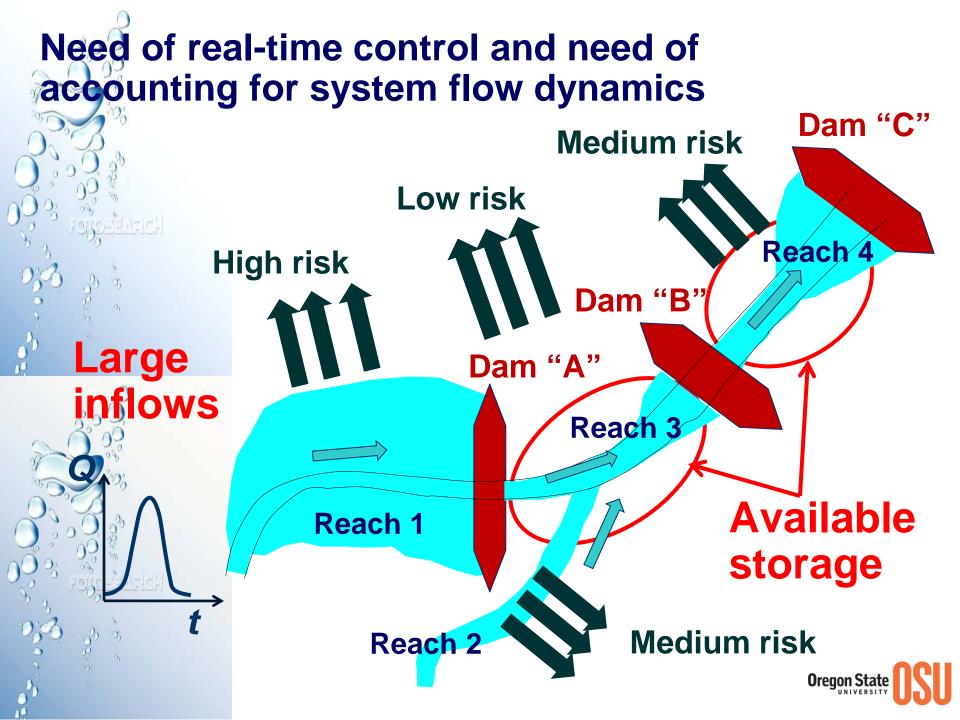


Presentation outline

Need of real-time control and need of accounting for system flow dynamics

Overview of proposed framework Applications Near-future work





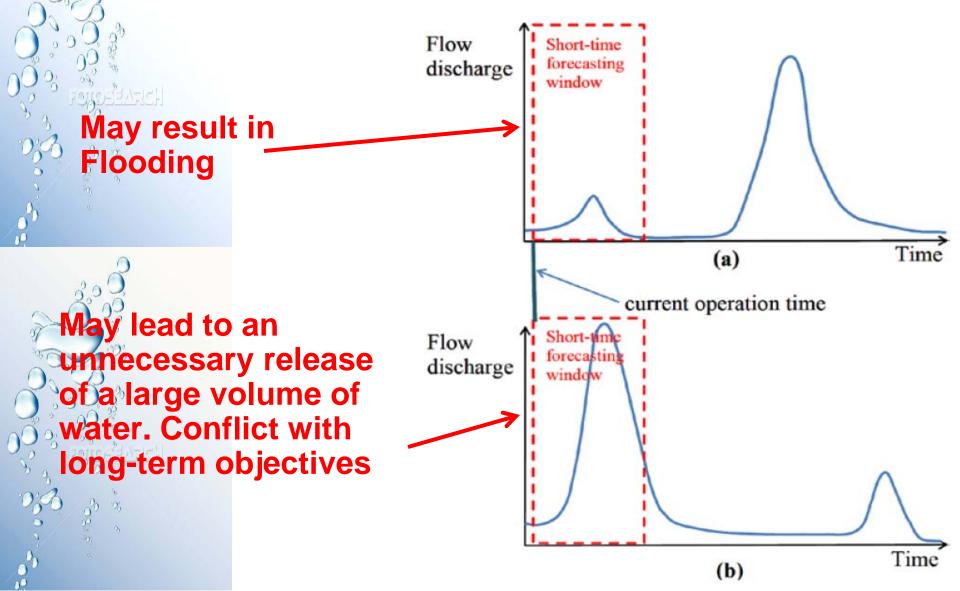
Why does current frameworks neglect system flow dynamics?

 Lack of robustness: Unsteady models typically have convergence and stability problems.

Computational burden: A framework that combines simulation and optimization may require hundreds or even thousands of simulations for each operational decision.

Oregon Sta

Need of accounting for short- and long-term forecasting



Proposed Framework (OSU Rivers)

The proposed framework couples a robust and numerically efficient hydraulic routing technique (simulation model) with a state-ofthe-art Optimization technique (Genetic Algorithm) (will add operation under uncertainty in the near-future)

Provides a system analysis and a system control in real-time conditions.



Proposed Framework (Cont.)

Two sets of objectives: Short-term and longterm (This may change depending on the user)

Long term: Maximize benefits of irrigation, ecohydrology, etc.

i=1

Constraints: Ecological flows, water rights, etc. **Short-term:** Maximize hydropower production, Avoid flooding or in the worst case allow controlled flooding

Minimize $\sum (w_{L_i} F V_{L_i} + w_{R_i} F V_{R_i})$

Proposed Framework (Cont.)

When capacity of river system is exceeded, the proposed framework allows controlled flooding based on a hierarchy of risk areas (Urban areas have highest risk)



Flow chart of Proposed Framework

Start of real-time control framework

Hydraulic routing for each **~** reach (pre-computed)

Does system should be operated to fulfill short-term or long-term objectives?

Definition of river network: nodes and river reaches Computation of River System Performance Graphs (HPG's, VPG's, LFPG's, RFPG's, RPG's) Acquire initial and boundary conditions (BCs) from real-time measurements (e.g., water stages) and forecasting Determine if system should be operated for flood forecasting No Prone to flooding? Yes Operate system to Operate system for maximize benefits flood control Generate initial population (generation = 0) River system hydraulic routing: solve a system of non-linear equations assembled based on systems' HPG's and VPG's. continuity, compatibility conditions and system BCs. population = population + 1 population > max population NSGA-II Yes NSGA-II reproduction / crossover / mutation generation = generation +1 generation > max generation No Yes Choose the optimal solution n = nHas simulation been completed? Yes End

Coupling of NSGA-II Genetic Algorithm with river system hydraulic routing. Will account for uncertainty.

Components of the proposed framework: River system flow routing

Navier Stokes equations:

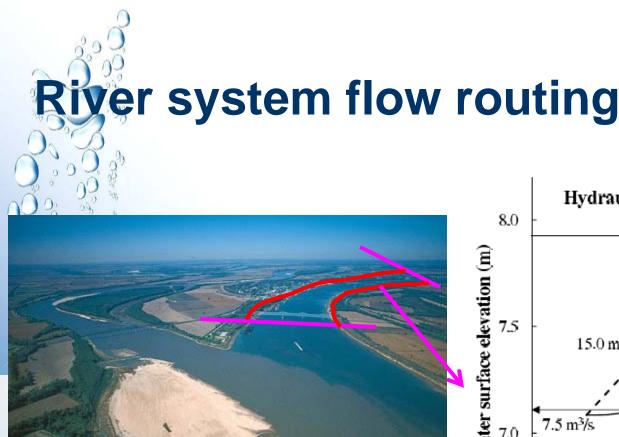
$$\frac{\partial \boldsymbol{u}_{i}}{\partial t} + \boldsymbol{u}_{j} \frac{\partial \boldsymbol{u}_{i}}{\partial \boldsymbol{x}_{j}} = \frac{-1}{\rho} \frac{\partial \boldsymbol{p}}{\partial \boldsymbol{x}_{i}} + \nu \frac{\partial^{2} \boldsymbol{u}_{i}}{\partial \boldsymbol{x}_{j} \partial \boldsymbol{x}_{j}} + \boldsymbol{g}_{i}$$
$$\frac{\partial \boldsymbol{u}_{i}}{\partial \boldsymbol{x}_{i}} = 0$$

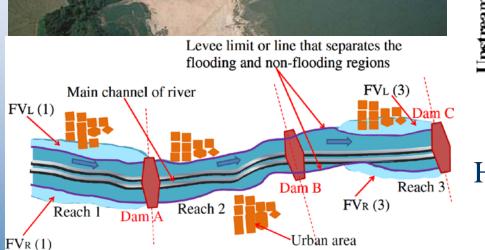
Saint-Venant equations

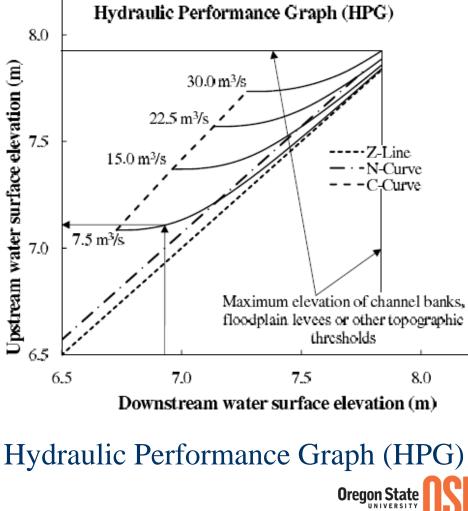
 $+U\frac{\partial U}{\partial x} + g\frac{\partial h}{\partial x} = g(S_0 - S_f)$

$$\frac{\partial A}{\partial t} + \frac{\partial Q}{\partial x} = 0$$

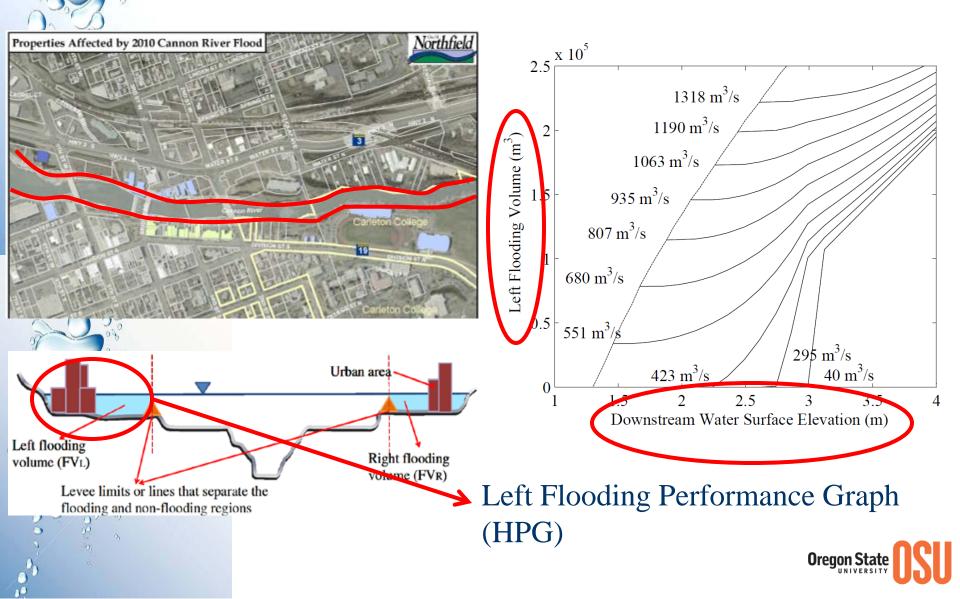




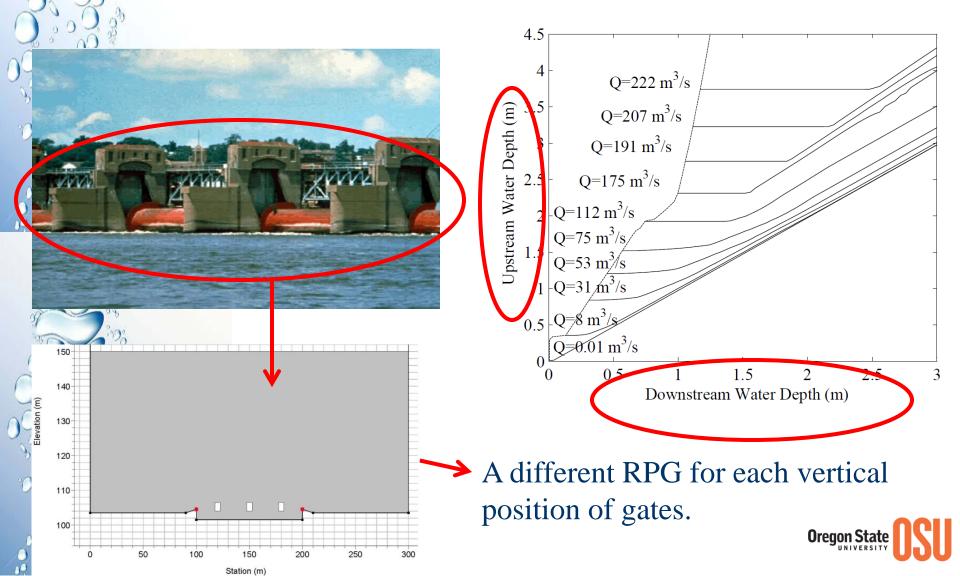


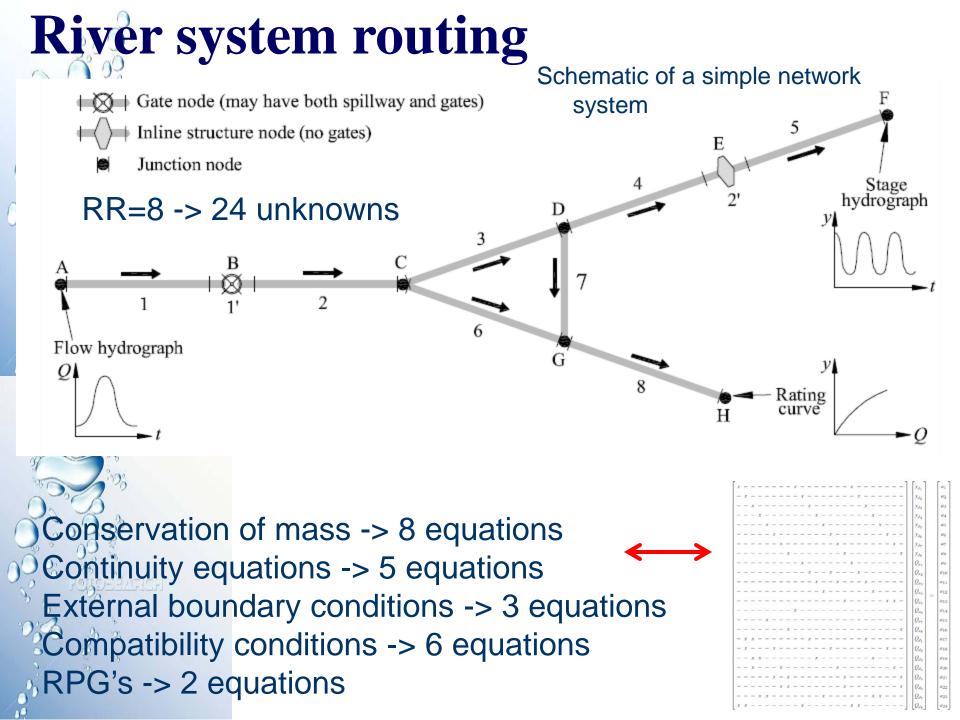


Flooding Performance Graphs (FPGs)



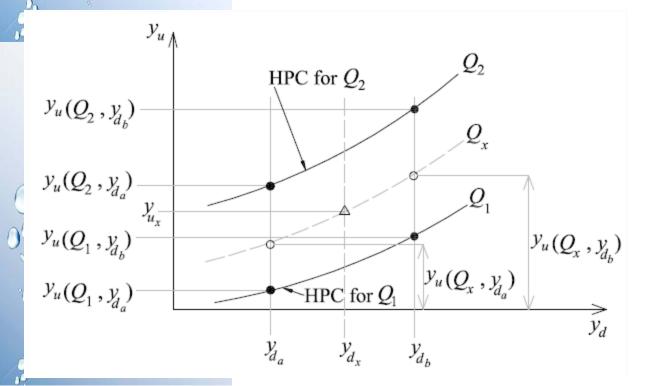
Rating Performance Graphs (RPGs)





River system hydraulic routing (Cont.)

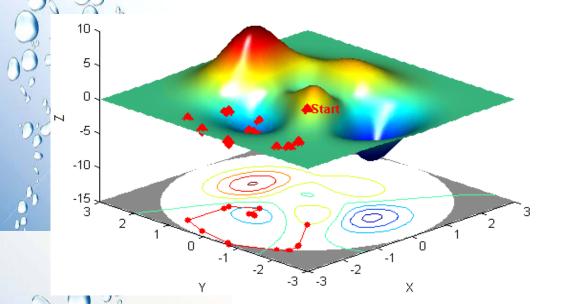
River network consisting of N reaches3N unknowns (Qu, Qd, yd of each reach)yu is known, estimated using HPG, yd and spatiallyaveraged discharge $y_u^n = HPG[y_d^n, \frac{1}{2}(\sum I^n + \sum O^n)], \forall j$

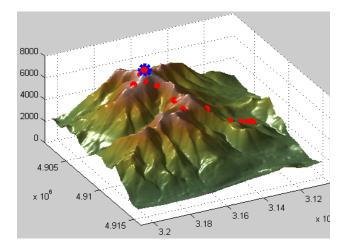


Schematic of $y_u(Q_x, y_d)$ interpolation



Optimization component: Genetic Algorithms



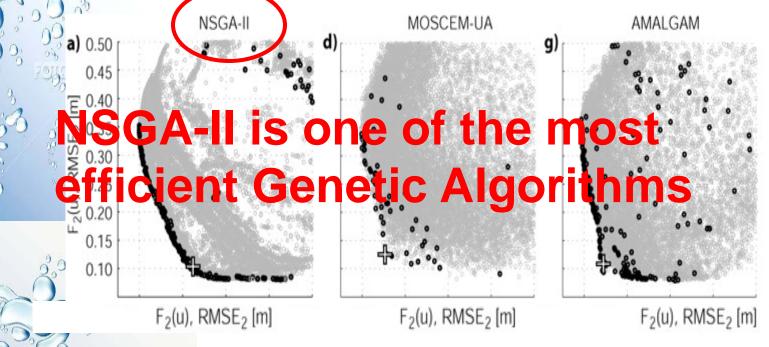


GAs is able to find the optimum set of solutions for multi-objective optimization

Handle constraints without the use of penalty functions



Optimization component: Genetic Algorithms (Cont.)



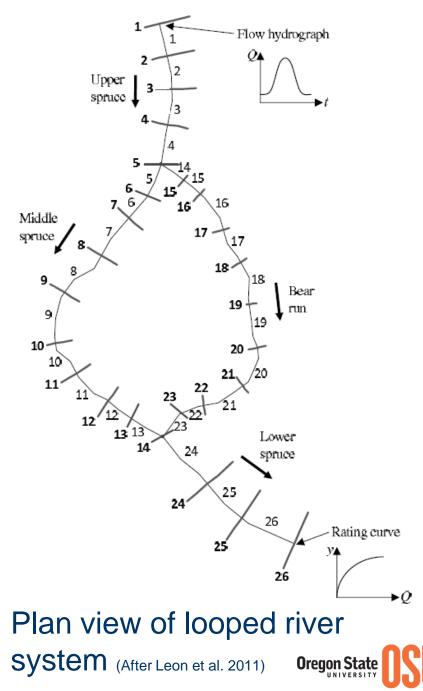
After Wöhling et al. (2007)

Combined with Newton based methods, NSGA-II may be even much better

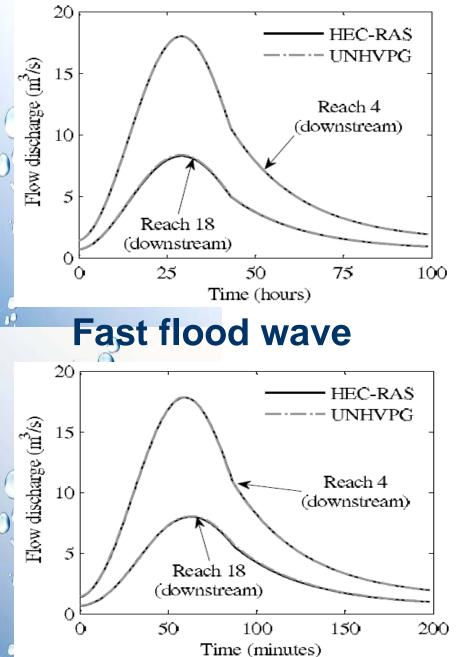


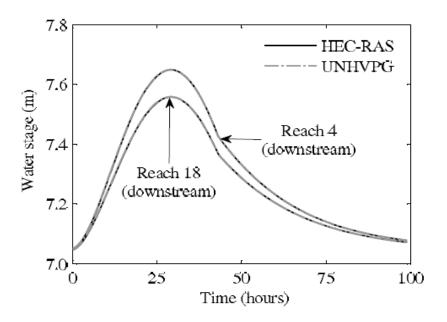
Comparison of hydraulic component of proposed framework with the Unsteady HEC-RAS model

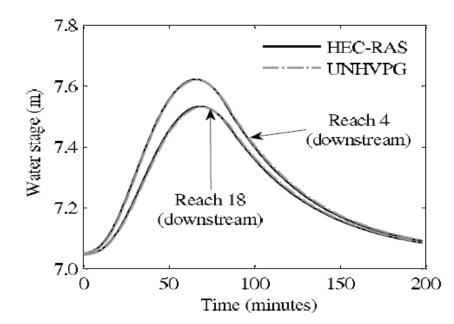
Looped river system adapted from an example in the Applications Guide of the HEC-RAS model (Hydrologic Engineering Center, 2010).



Slow flood wave









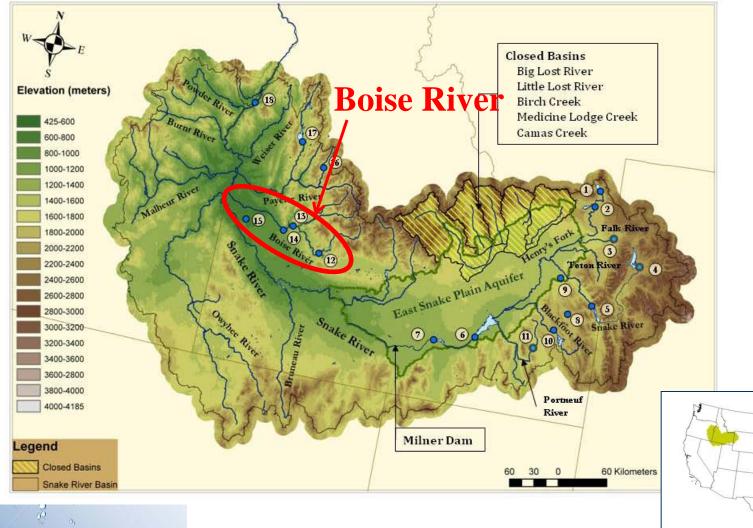
Description	UNHVPG Time (s)	HEC-RAS Time (s)
Case 1 (slow flood-wave)	218.8 ($\overline{\Delta t}$ = 9.77 s)	752.7 ($\Delta t = 10$ s)
Case 2 (fast flood-wave)	7.3 ($\overline{\Delta t}$ = 10.27 s)	52.6 ($\Delta t = 10$ s)

The results obtained with OSU Rivers (hydrodynamic portion are about 300% and 700% faster than those of the HEC-RAS model for the slow and fast flood-wave cases, respectively.

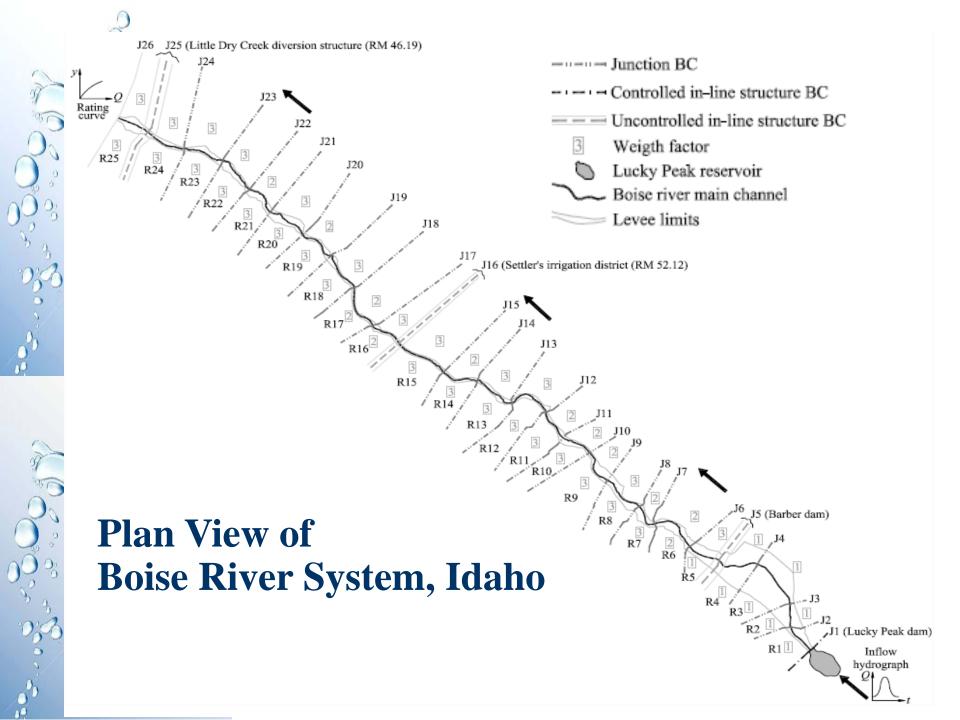
Robustness: Proposed framework is highly robust because instability issues are addressed during pre-computation of hydraulics



pplication of proposed framework to the Boise River System (Idaho)



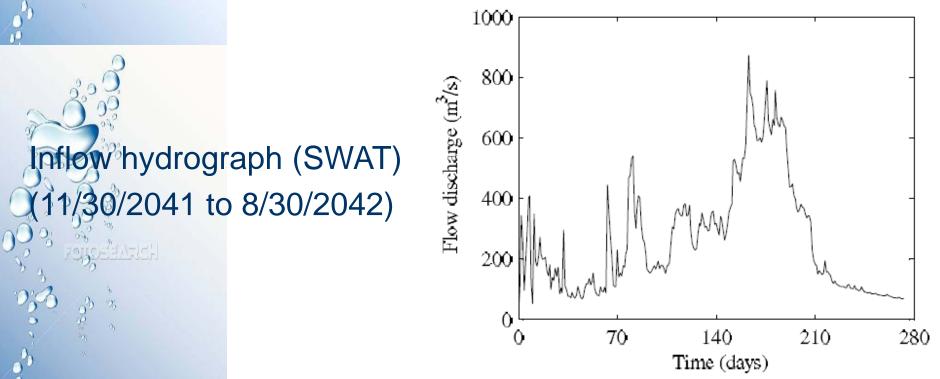




Inflow hydrographs

 Original inflow hydrograph - 50 years (from 01/01/2010 to 12/19/2059)-SWAT (Courtesy Prof. Sridhar, BSU)

- Simulation period of nine months (11/30/2041 to 8/30/2042) 274 days maximum volume of inflow.
- Original inflow hydrograph represents natural flows at the location of Lucky Peak reservoir

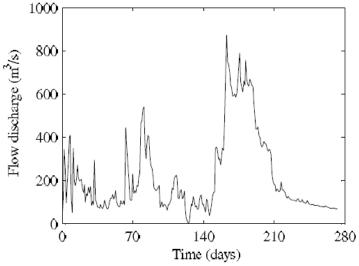


Modified inflow hydrograph

Anderson Ranch reservoir (509.6 MCM) - 9 m3/s 03/07/2042 to 05/11/2042 to fill the reservoir. Arrow Rock reservoir (335.8 MCM) - 84

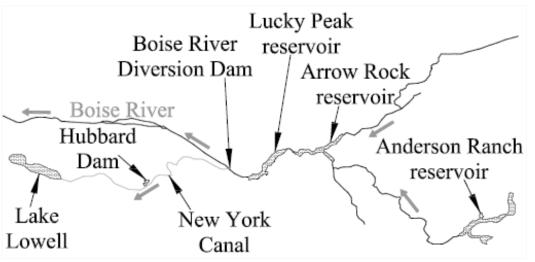
Arrow Rock reservoir (335.8 MCM) - 84 m3/s

03/25/2042 to 05/10/2042 to fill the reservoir.
Lake Lowell (196.6 MCM) - 51.5 m3/s
03/18/2042 to 04/30/2042 to fill the lake.
Hubbard Dam (4.9MCM) - 10 m3/s
05/05/2042 to 05/09/2042 to fill the reservoir.

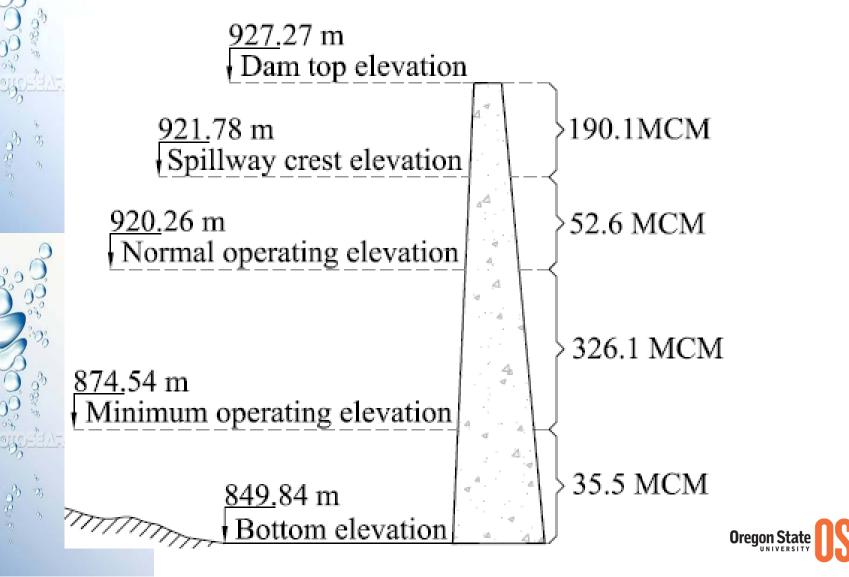


Inflow hydrograph subtracting active storage capacity of Anderson Ranch, Arrow Rock, Hubbard reservoirs and Lake Lowell.

Plan view of major storage reservoirs in the Boise river basin.



Stage-storage relationship of Lucky Peak reservoir



Optimization objective (shortterm)

Minimize $f_1 = \sum_{i=1}^{\text{RR}} (w_{L_i} F V_{L_i} + w_{R_i} F V_{R_i})$

Constraints: Q > Q minimum ecological flows:



Outlet structure of Lucky Peak

- 6.71 m diameter steel-lined pressure tunnel (upstream end) - Six sluice gates (downstream end) Gates conveyance (hydraulic capacity) was smaller than that of tunnel



View of Lucky peak reservoir and associated structures.

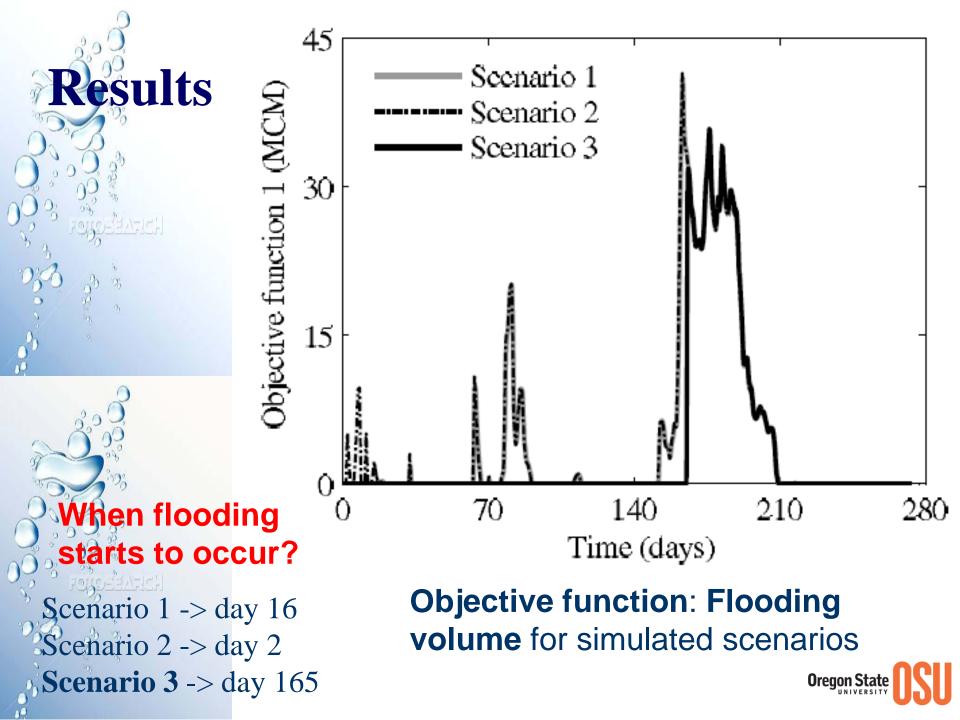
Simulated scenarios

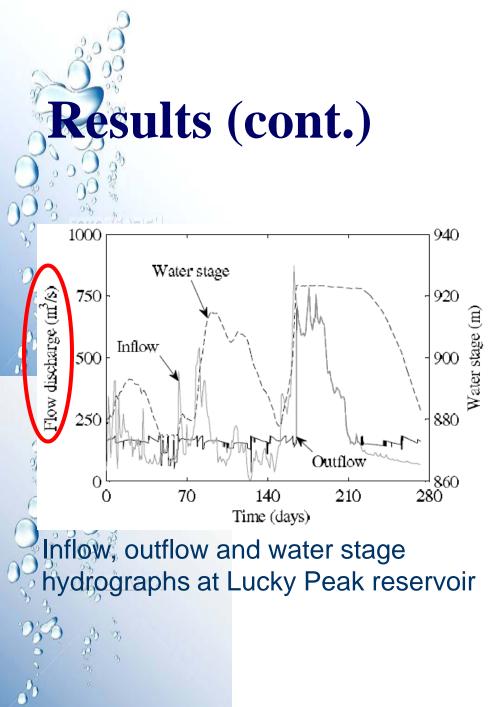
1) Without gate operation (i.e. the gates are closed)

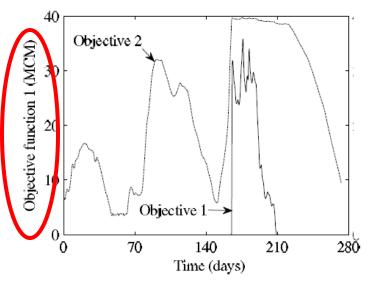
2) Assuming that Lucky Peak reservoir doesn't exist

With gate operation according to proposed framework.

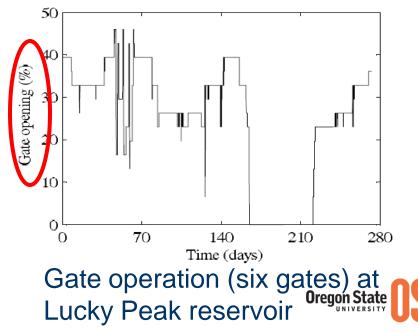








Results of objective functions for scenario 3 (proposed framework)



Optimization-simulation of reservoir operation for hydropower



Objectives:

- Maximize Hydropower benefit by producing sufficient power to satisfy demand
 - **Flood Protection**
- Ensure Adequate water levels in reservoir (for other objectives such as irrigation)

Multi-objective optimization
 Deterministic / Stochastic



A hypothetical example

Single Reservoir with simple approach for reservoir routing

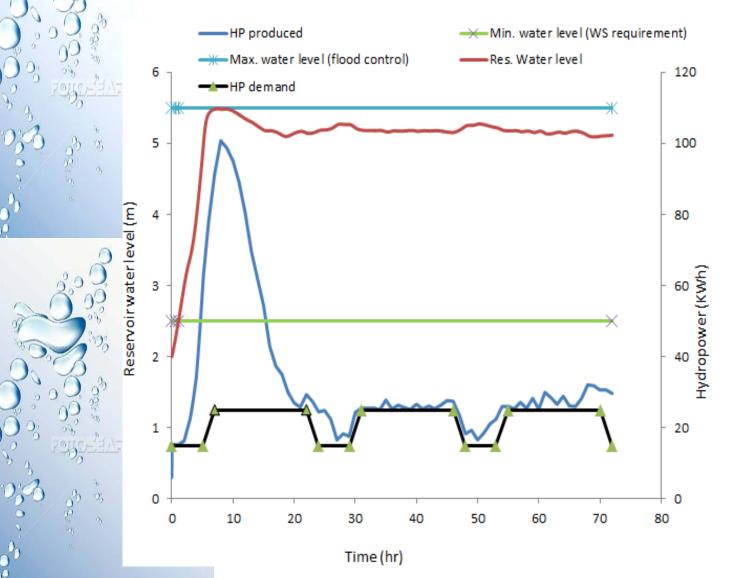
• Objective set as: Minimize (HP deficit)

Minimize (HP produced – HP demand) Flood control & Water supply demand represented as constraints of max./min. water levels

NSGA-II optimization algorithm



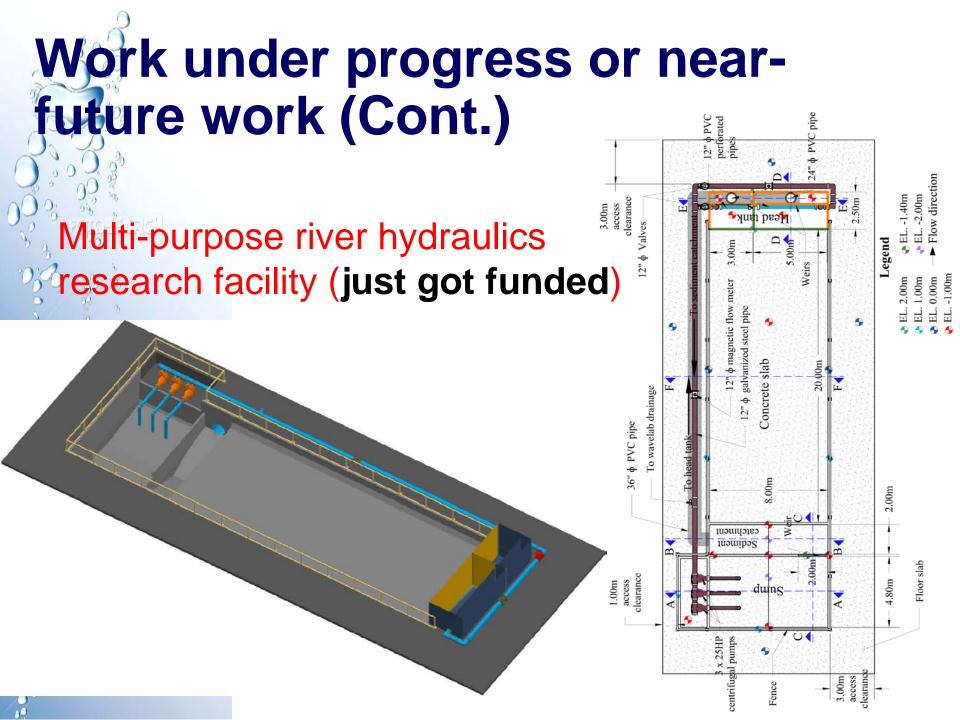
Results: 3-days ahead optimal operations



Work under progress or Nearfuture work

Physical modeling
Incorporation of uncertainty
Combining Genetic Algorithms with Newton based methods for faster convergence





Work under progress or nearfuture work (Cont.)

A physical laboratory model will be built in the OSU wave lab to validate the OSU Rivers framework

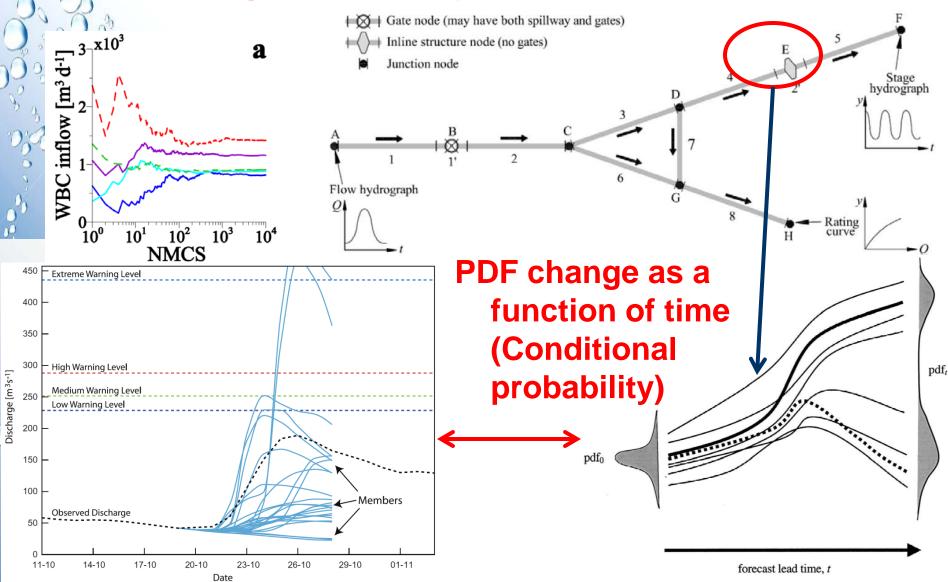
Lab View control



Proposed Framework

Work in progress

Incorporating uncertainty into reservoir operation



Work in progress (Cont.)

Qdr.

Qde

Qd-

 a_{21}

 a_{22}

 a_{23}

Incorporating Uncertainty into reservoir operation

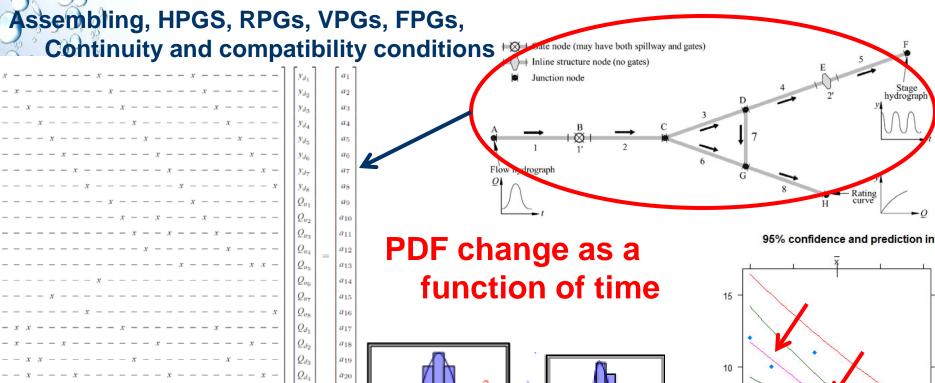
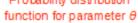


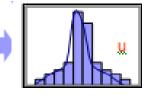
Fig. 7. Summary of equations for the simple network system in Figure 6 (After Leon et al. 2011)

Non-linear system of equations that

describe the regulated river system

Probability distribution





Reduction of uncertainty in operation of reservoirs



