



Department of Energy

Bonneville Power Administration
P.O. Box 3621
Portland, Oregon 97208-3621

PUBLIC AFFAIRS

July 5, 2012

In reply refer to: DK-7

TBH & Associates, LLC
Attn: Peter Tapio
5211 NE 88th St
Vancouver, WA 98665

FOIA #BPA-2012-01416-F

Dear Mr. Tapio:

This is a final response to your request for records that you made to the Bonneville Power Administration (BPA) under the Freedom of Information Act (FOIA), 5 U.S.C. 552.

You have requested the following:

All documents related to the John Day – McNary Transmission Line Project structure 59/4 change from drilled pier foundation to grillage.

BPA has provided the responsive documents on the enclosed CD in their entirety

You agreed to pay fees of \$5. You will be billed separately.

Pursuant to 10 CFR 1004.8, if you are dissatisfied with this determination, or the adequacy of the search, you may appeal in writing within 30 calendar days of receipt of a final response letter. The appeal should be made to the Director, Office of Hearings and Appeals, HG-1, Department of Energy, 1000 Independence Avenue, SW, Washington, DC 20585-1615. The written appeal, including the envelope, must clearly indicate that a FOIA Appeal is being made.

I appreciate the opportunity to assist you. Please contact KimWinn, Communications Specialist at 503-230-5273 with any questions about this letter.

Sincerely,

/s/Christina J. Munro

Christina J. Munro
Freedom of Information Act/Privacy Act Officer

Enclosure: CD

Winn, Kim S (BPA) - DK-7

From: Ratnathicam, Shantini (BPA) - TETQ-TPP-3
Sent: Thursday, September 09, 2010 3:46 PM
To: 'David Streetman'; 'Peveler, Mace'; 'Gerald Turner'
Cc: Cupp, Todd A (BPA) - TELM-TPP-3; Dorman, John C - TELS-TPP-3; Flansburg, Jeffrey T (BPA) - TETQ-TPP-3; Holzer, Daniel P (BPA) - TETQ-TPP-3; Kaneshiro, Julius K - NSSV-MODW
Subject: REPLY TO : MJD RFI #28 - RFI 004 for STR 59/4
Importance: High
Attachments: RFI 004.pdf

McNary - John day No.1 Project

After the telephone conference yesterday and site inspection by our geotechnical engineer today, it was decided that grillage footings will be the best option for this site. Refer to CCR # 68, that changed THE SHAFT FOOTINGS TO GRILLAGES.

From: Streetman, David [mailto:dstreetman@wilsonconst.com]
Sent: Tuesday, September 07, 2010 3:41 PM
To: Ratnathicam, Shantini - TELS-TPP-3
Cc: Holzer, Daniel P - TELS-TPP-3; Turner, Gerald; Peveler, Mace; Hesse, David M - TELD-TPP-3
Subject: MJD RFI #28 - RFI 004 for STR 59/4
Importance: High

Shantini,

Please take a look at the attached RFI from TBH. In TBH's attachment is a low density concrete mix that they are asking for approval on. Please let me know ASAP if BPA accepts this mix. TBH currently has this scheduled for tomorrow morning so we would like to know today if it is approved or not. If we have not received approval by the end of the day today we will need to push back this work. Please let me know if you have any questions. I will track this as McNary – John Day RFI #28

Thank You,
David Streetman

From: Sandy Sjoden [mailto:ssjoden@tbhdrill.com]
Sent: Tuesday, September 07, 2010 3:15 PM
To: Streetman, David
Cc: Peveler, Mace; Turner, Gerald; 'Dave Sjoden'; peter.tapio@tbhdrill.com
Subject: 0936 BPA McNary John Day Ph 2 - RFI 004 for STR 59/4
Importance: High

David,

Please find RFI 004 for structure 59/4.

THANK YOU,

SANDY SJODEN
PROJECT MANAGER

6/15/2012



5211 NE 88TH STREET
VANCOUVER, WA 98665
O.360.546.1600
F.360.546.1700
C.360.852.0705
SSJODEN@TBHDRILL.COM
WWW.TBHDRILL.COM

RFI - REQUEST FOR INFORMATION

TBH & ASSOCIATES, LLC

5211 NE 88TH STREET
VANCOUVER, WA 98665
P. 360.546.1600 F. 360.546.1700



To: Wilson Construction Company
PO Box 1190
Canby, OR 97013
David Streetman

Date: 9/7/2010 Contract No: 5376SC - TBH - 2

CRITICAL TO SCHEDULE

RFI # 4

McNary - John Day No. 1 500kV Transmission Line Project

Reply Requested by: 9/8/2010

Request for Information/Clarification:

Structure 59/4 Leg 4 started drilling on Sept 1st. Stability of the shaft prevented the completion of the drilling through 2 days of drilling. After 4 hours of drilling sloughing occurred at 13' while using a 4' core barrel. After cleaning out shaft we switched to a 2' core barrel to restart drilling. With 6 more hours of coring and augering we reached a depth of 21' and the shaft failed again at that depth. We moved to leg 1 and covered the shaft. TBH noticed additional sloughing within the hour after moving to leg 1. Arrived at structure 59/4 on Friday Sept 3rd to find leg 4 shaft had collapsed entirely. See photos attached. Wilson was informed on Friday Sept. 3rd and it was decided to backfill with 1000psi concrete. Please find attached the concrete mix design for the 1000psi mix, photos and drilled shaft report. 1000psi Concrete Mix Design is currently not an approved mix, please submit for approval. Estimated expense of this work could be as much as \$5000.00 for the material alone. To maintain schedule, TBH expects to restore shaft stability utilizing the low strength concrete tomorrow, September 8th, please advise.

By: Sandy Sjoden

Date: Wednesday, September 08, 2010

Response:

By: _____

Date: _____

Contractor Response:

- This RFI is a NO COST or IMPACT change to the project
- This RFI MAY result in ADDITIONAL COSTS OR IMPACT to the project
- This RFI WILL result in ADDITIONAL COSTS OR IMPACT to the project
- This RFI MAY add/reduce unit price items to the project

TWR 59/4 Leg #4

PHOTO #1

Sept 3rd 2010

84 inch

CMP Collar (opt)

CDF placed 9/1

<<<< VOID discovered 7:45AM

PHOTO #2

Before Collapse

TWR 59/4 Leg 4
Sept 3rd 2010

Unstable Material

The photograph shows a deep, narrow trench or excavation. The walls are composed of reddish-brown soil with visible horizontal layering and some cracking. A concrete ring is visible on the right side, partially buried in the soil. The text 'Unstable Material' is overlaid on the upper part of the trench. The text 'Edge CDF ring' is overlaid on the right side, pointing to the concrete structure. The text 'TWR 59/4', 'Leg 4', and 'Sept 3rd 2010' is overlaid on the left side. The text 'PHOTO #3' is overlaid on the bottom right corner.

TWR 59/4

Leg 4

Sept 3rd 2010

Edge

CDF

ring

PHOTO #3



Shaft CL
Blasted Rock 6 ft >>

TWR 59/4 Leg 4
Sept 3rd 2010

Photo #4

21.5

7

CRESSID MOVED TO NEXT STRUCTURE
~~LEG 4~~

MECHANIC - FUSE

SET CAV MOVED TO LEG 1

MOVED TO LEG 4

TIME	DEPTH	SOIL	TEMP	LEG	TOOL
11:25	22.5	F. Rock	49.5		6' AUGER
2:00	6.5	F. Rock		4	4' AUGER
3:00	6.5	"		4	4' AUGER
7:15	6.5	F. Rock		4	6' CORE
7:35	6.5	"		4	4' CORE
8:15	6.5	"		4	4' CORE
8:40	7.5	F. Rock		4	4' AUGER
9:05	8.0	"		4	4' AUGER
9:40	8.0	"		4	4' CORE
10:30	8.5	"		4	4' CORE
11:30	9.5	"		4	4' CORE
1:15		F. Rock	49.01-10	1	6' CORE
8:35		DIRT		1	6' CORE
8:40		F. Rock		1	4' AUGER
8:45		"		1	4' CORE
8:55	6'	DIRT		4	4' CORE
9:15		F. Rock		4	4' AUGER
9:30	6.8	"		4	4' CORE

TOWER 50/4 8-31-10

TOUR 50/4 8-31-10

TOUR 50/4 8-31-10

TOUR 50/4 8-31-10

TOUR 50/4 8-31-10

TOUR 50/4 8-31-10

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TOUR 50/4 8-31-10

TOUR 50/4 8-31-10

TIME	DEPTH	SOIL	DEPTH	LEG	TOOL
9:55	8.9	DIRT ROCK	09-01-10	4	4' CORE
10:20	10				4' ANKER
10:35	10.5				4' CORE
11:45	13				2' CORE
12:15					4' ANKER
12:25					4' CORE
12:35	11.5				4' ANKER
2:50	16.5				2' CORE
3:15	16.5				4' ANKER
09-02-10					
7:15	16.5	DIRT F. ROCK		4	4' ANKER
8:00	19.3				2' CORE
8:45	20.3				4' ANKER
9:00	21				6' CORE
9:40	19				4' ANKER
10:00					
10:30	20.4				2' CORE
11:05	20.6		04 3		4' ANKER
11:30	21.2				
11:40		DIRT F. ROCK		1	4' ANKER
3:20	13.6				
09-03-10					
7:10	13.6	DIRT F. ROCK			2' ANKER
8:30	15.0				

21.5 09-4

TOWER 59-4

~~SLUFFED OFF~~

SLUFFED OFF

TOWER 59-4

GRAVEL

SIDE SLUFFED OFF

MAID SIDE SLUFFED OFF LEG 4 AGAIN

MOVED TO LEG 1

GRAVEL

GRAVEL

Winn, Kim S (BPA) - DK-7

From: Mullen, Nathan D (BPA) - TELP-TPP-3
Sent: Friday, September 10, 2010 8:01 AM
To: Ratnathicam, Shantini (BPA) - TETQ-TPP-3; 'John Ashford (jashford@wilsonconst.com)'; Dan Fitzgerald; David Streetman; Gerald Turner; Sierra Conder; Peveler, Mace; Mark Woll
Cc: Cupp, Todd A (BPA) - TELM-TPP-3; Dorman, John C - TELS-TPP-3; Holzer, Daniel P (BPA) - TETQ-TPP-3; Kaneshiro, Julius K - NSSV-MODW; Berry, Theresa M (BPA) - TEP-TPP-3; Marquez, Tammi L (BPA) - NSSM-MODW; Roberts, Laura A (BPA) - KEP-4; Flansburg, Jeffrey T (BPA) - TETQ-TPP-3; Hesse, David M (BPA) - TELD-TPP-3
Subject: Update 1: M-JD CCR #68: REVISED Footing type at 59/4
Attachments: M-JD Site Data 59_4 (ATA 293) rev1.pdf; M-JD Tower List 59_4 rev.pdf

Just to be complete, attached are the updated Tower List and Site Data Sheet for the tower 59/4 change from shaft to grillage footings. Note that the Working Point Elevation did not change. Please let us know if there are any questions.

Thanks....
Nathan



M-JD Site Data 59_4 (ATA 293) ...
M-JD Tower List 59_4 rev.pdf (...)

From: Ratnathicam, Shantini - TELS-TPP-3
Sent: Thursday, September 09, 2010 3:29 PM
To: 'John Ashford (jashford@wilsonconst.com)'; Dan Fitzgerald; David Streetman; Gerald Turner; Sierra Conder; Peveler, Mace; Mark Woll
Cc: Cupp, Todd A - TELS-TPP-3; Dorman, John C - TELS-TPP-3; Holzer, Daniel P - TELS-TPP-3; Kaneshiro, Julius K - NSSV-MODW; Berry, Theresa M - TEP-TPP-3; Marquez, Tammi L - NSSM-MODW; Mullen, Nathan D - TELP-TPP-3; Roberts, Laura A - KEP-4; Flansburg, Jeffrey T - TELS-TPP-3; Hesse, David M - TELD-TPP-3
Subject: M-JD CCR #68: REVISED Footing type at 59/4

McNary - John day No.1 Project- Footings at 59/4 (ATA 293) for 148 D tower

This is to confirm that all four footings at 59/4 have been changed to grillages after a site visit by our geotechnical engineer this morning. The fractured rock found at this location is not suitable for the 20 ft deep shafts that were planned for this location based on reported rock depth of 4-6 ft. (refer to photos submitted w/RFI #28) As-built the structure list to reflect this change. As discussed, use the extra Grillages already on site. Call me if you have any questions. Submit a price proposal within 4 weeks to make an equitable adjustment for this change.

Shantini Ratnathicam
Construction Manager
360-619-6584

OPRNG NO	SERIAL NO	STATION	SPAN AHEAD	ANGLE DEG-MIN	TOWER TYPE	BODY HT	LEG EXTENSIONS			FTG TYPE	TOP SOIL
							1 GRND WIRE	2 MWT	3 9000#		
57/2	ATA 282	3228+50.0	850.0		148M1	80	20.0	15.0	17.5	22.5	PLATE E - R
57/3	ATA 283	3237+00.0	832.1		148M1	60	32.5	27.5	27.5	32.5	PLATE E - R
57/4	ATA 284	3245+32.1BK 3246+21.8AH	1153.2	20 45RT	148D	53	27.5	22.5	25.0	27.5	SHAFT E - R
57/5	ATA 285	3257+75.0	1225.0		148M1	60	25.0	22.5	22.5	25.0	PLATE E - R
58/1	ATA 286	3270+00.0	2023.5		148A1	80	22.5	17.5	17.5	22.5	PLATE E - R
58/2	ATA 287	3290+23.5BK 3289+17.5AH	1082.5	23 46LT	148D	53	35.0	32.5	32.5	35.0	SHAFT E - R
							END GW	MWT	9000#		
58/3	ATA 288	3300+00.0	1100.0		148M1	60	25.0	25.0	25.0	25.0	PLATE E - R
58/4	ATA 289	3311+00.0	800.0		148M1	60	7.5	7.5	7.5	7.5	PLATE E - R
59/1	ATA 290	3319+00.0	1000.0		148M1	60	17.5	17.5	20.0	17.5	PLATE E - R
59/2	ATA 291	3329+00.0	750.0		148M1	60	15.0	15.0	17.5	17.5	PLATE E - R
59/3	ATA 292	3336+50.0	819.4		148M1	60	12.5	12.5	12.5	12.5	PLATE E - R
59/4	ATA 293	3344+69.4BK 3345+72.6AH	1152.4	23 34RT	148D	53	7.5	7.5	7.5	7.5	<u>GRILL</u> E - R
59/5	ATA 294	3357+25.0	900.0		148M1	60	37.5	27.5	30.0	40.0	PLATE E - R
59/6	ATA 295	3366+25.0	925.0		148M1	60	27.5	22.5	22.5	27.5	PLATE E - R

BPA 143 OC.

REVISIONS

1. Changed footing type from shaft to grillage based on soil conditions. 9/10/10 NDM

U.S. DEPARTMENT OF ENERGY
BONNEVILLE POWER ADMINISTRATION

TOWER SITE DATA

McNARY - JOHN DAY No. 1 LINE

ROLL NO. _____ PL-6 NO. 231815 SEC.

BY JS DATE 9-09 CHKD NDM DATE 10/09

Remarks:

ELEVATIONS FROM LIDAR

Scale: 1" = 10'

ATA 293

Tower No.

59/4

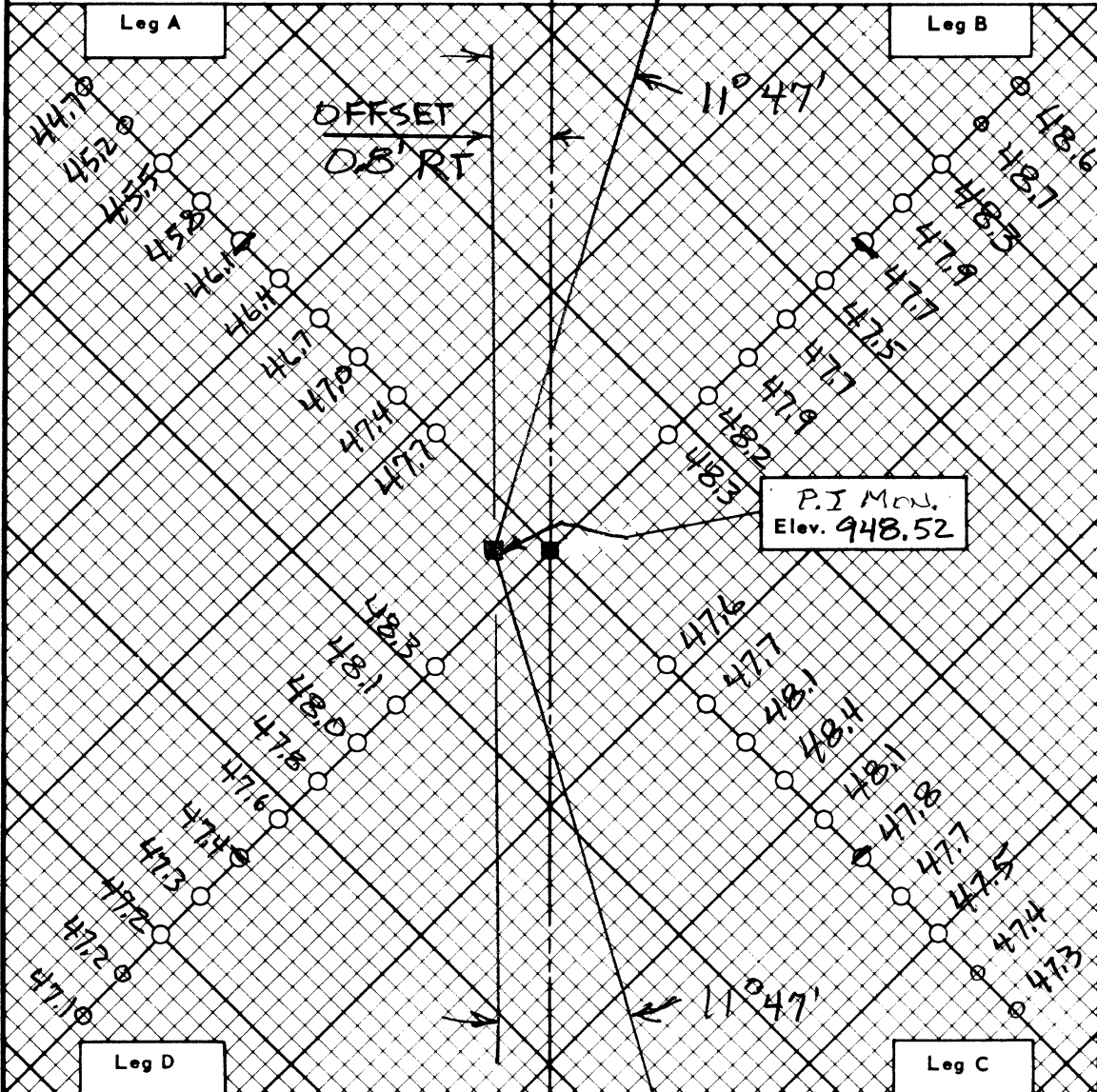
Station

3344+69.4B K

3345+72.6A H

Angle

23°34RT



Soil	EARTH & ROCK	Twr Type	148D	Body Ht	53	Cond. El.	1007.0
	Footings	Leg Ext		Gnd El.		W.P. El.	Diff.
A	GRILLAGE	7½		946.1		948.9	2.8
B	GRILLAGE	7½		47.7		48.9	1.2
C	GRILLAGE	7½		47.8		48.9	1.1
D	GRILLAGE	7½		47.4		48.9	1.5

CONCRETE MIX DESIGN SUBMITTAL
Hood River Sand, Gravel and Ready Mix, Inc.

Project	<u>Celilio Converter Station</u>	Conc. Class	<u>1000 +/-</u>
Contractor	<u>TBH</u>	Agg Size	<u>Sand</u>
Contract #	<u>BPA</u>	Mix Design	<u>30</u>
Intended Use:	<u>Backfill</u>		

MIX PROPORTIONS - QUANTITIES PER CUBIC YARD

	<u>Weight (lbs)</u>	<u>Absolute Volume</u>	<u>Brand</u>	<u>Type</u>
Cement	<u>280 #</u>	<u>1.425 ft³</u>	Lafarge - Richmond	<u>I-II</u>
Flyash	<u>#</u>	<u>0.000 ft³</u>	Boral - Boardman	<u>C</u>
Agg. Size <u>1 1/2"</u>	<u> #(SSD)</u>	<u>0.000 ft³</u>		
Agg. Size <u>3/4"</u>	<u> #(SSD)</u>	<u>0.000 ft³</u>		
Agg. Size <u>3/8"</u>	<u> #(SSD)</u>	<u>0.000 ft³</u>		
Agg. Size <u>Sand</u>	<u>2941 #(SSD)</u>	<u>17.392 ft³</u>	<u>Master Builders</u>	
			<u>Admixtures-Brand/Type/Dosage</u>	
			AEA	<u>Ae 90</u>
Mix Water	City <u>460 #</u>	<u>7.372 ft³</u>	WRA	<u>Pozz 80</u>
Entrained Air	<u>3 % +/- 1.5</u>	<u>0.810 ft³</u>	HRWRA	<u></u>
Total	<u>3681 #</u>	<u>27.00 ft³</u>	Stabilizer	<u>Delvo</u>
Unit Wt:	<u>136.3</u>	Design slump:	<u>6-10"</u>	Design W/C Ratio: <u>1.64</u>
				Spec Max W/C <u>NS</u>

AGGREGATE DATA (used in calculating the mix design)

Coarse Agg. Source Tidyman - Dallesport, WA State Source # WA-20-1
 Coarse Agg. Source Tidyman - Dallesport, WA State Source # WA-20-1
 Fine Agg. Source Tidyman - Dallesport, WA State Source # WA-20-1

<u>Size</u>	<u>Specific Gravity(SSD)</u>	<u>Absorption</u>	
<u>1 1/2" - 3/4"</u>	<u>2.78</u>	<u>1.3</u>	
<u>3/4" - #4</u>	<u>2.76</u>	<u>1.5</u>	Dry Rodded Wt. <u>110.6</u>
<u>3/8 - #4</u>	<u>2.73</u>	<u>2.0</u>	
<u>Sand</u>	<u>2.71</u>	<u>2.5</u>	Avg. Sand F.M. <u>2.75</u>

Remarks:

- Concrete proportioned in accordance with ASCI 211.1 standard practices. This information is CONFIDENTIAL to contract personnel. Approval of this mix design carries the inclusion of Hood River Sand Gravel and Ready Mix on the distribution list for all concrete test results.
- Hood River Sand and Gravel has no authority regarding the appropriate application of this mix. Therefore, it is the responsibility of the project architect, engineer, and/or contractor to insure that the above mix parameters are appropriate for the anticipated use and environmental conditions for the intended placement of this mix.
- The mix will meet the stated strength, when test specimens are sampled, fabricated, transported, cured (initial & final), and tested in strict compliance with current ASTM Standards, and evaluated for acceptance per ACI standards and practices. Deviations from ASTM standard methods, unless expressly authorized on this mix design, invalidate test results. Hood River Sand Gravel and Ready Mix reserve the right to conduct third part testing by an accredited independent laboratory.
- Design mix cementitious content is stated as a minimum and Hood River Sand Gravel and Ready Mix reserve the right to increase cementitious content. Chemical admixtures are added in accordance with the manufacture's recommendations, and may be adjusted to maintain mix properties. Aggregate weights may be adjusted to maintain yield and design gradations.

McNary - John Day No. 1, 500-KV Transmission Line
 Construction Contract No. 28333 Release 19
Phase 2 Shaft Footing Summary

Structure No.	Serial No.	Tower Type	Footing Type	Reported Rock Depths at each Leg (ft)				Minimum Concrete Shaft Depth at Each Leg (ft)				Comments
				1	2	3	4	1	2	3	4	
45/3	ATA 223	148D	SHAFT	1	2	3	4	1	2	3	4	
46/2	ATA 228	148A1	Chng to PLATE									Fig type changed in CCR 46
46/3	ATA 229	148A1	Chng to PLATE	>24	>24	>24	>24					Fig type changed in CCR 46
49/4	ATA 245	148D	SHAFT	4	4	4	4	20	20	20	20	
57/4	ATA 284	148D	SHAFT	6	3	3	3	20	20	20	20	
58/2	ATA 287	148D	SHAFT	1	1	1	1	20	20	20	20	
59/4	ATA 293	148D	Chng to GRILL	6	4.5	6	5					Fig type changed in CCR 68
62/3	ATA 306	148D	SHAFT	2	2	2	2	20	20	20	20	
64/1	ATA 315	148D	SHAFT	4	0	0	4	20	20	20	20	
66/1	ATA 325	148M1	SHAFT	>24	8	>24	>24	14	14	14	14	Increase Dia. to 6ft. CCR 69
66/2	ATA 326	48BB1	SHAFT	>24	>24	>24	>24	14	14	14	14	Increase Dia. to 6ft. CCR 72
68/1	ATA 333	148D	SHAFT	8	8	8	8	20	20	20	20	
68/2	ATA 334	148M1	SHAFT	0-3	0-3	0-3	0-3	14	14	14	14	
68/3	ATA 335	148M1	SHAFT	0-3	0-3	0-3	0-3	14	14	14	14	
68/4	ATA 336	148M1	SHAFT	0-3	0-3	0-3	0-3	14	14	14	14	Rock near surface
73/4	ATA 361	148D	SHAFT	0	4	4	0	20	20	20	20	
78/1	ATA 378	148D	Chng to GRILL	1	0	5	5					Fig type changed in CCR 84
79/1	ATA 379	48B1	SHAFT	>20	>20	>20	>20	14	14	14	14	Increase all leg Dia to 6ft CCR 83
79/2	ATA 380	148M1	SHAFT	4	6	>20	0	14	14	14	14	Increase #3 leg Dia to 6ft CCR 83

Jlf 12/23/10