



U.S. Department  
of Transportation  
**Federal Highway  
Administration**

1200 New Jersey Ave., SE  
Washington, D.C. 20590

December 22, 2010

In Reply Refer To:  
HSST/B-213

Mr. Daren Copeland  
Varley and Gulliver Limited  
Alfred Street, Sparkbrook  
Birmingham, UK B12 8JR

Dear Mr. Copeland:

This letter is in response to your request for Federal Highway Administration (FHWA) acceptance of a roadside safety system for use on the National Highway System (NHS).

Name of system: Proprietary VGAN 300 Aluminum Permanent Bridge Barrier  
Type of system: Post and Tube Railing Mounted on Reinforced Concrete Curb  
Test Level: NCHRP Report 350 Test Level 4 (TL-4)  
Testing conducted by: Texas Transportation Institute (TTI)  
Date of Request: September 30, 2010  
Drawing Designator: SBA07d

You requested that we find this system acceptable for use on the NHS under the provisions of National Cooperative Highway Research Program (NCHRP) Report 350 "Recommended Procedures for the Safety Performance Evaluation of Highway Features." (NCHRP Report 350)

### **Requirements**

Roadside safety systems should meet the guidelines contained in the NCHRP Report 350. FHWA memorandum "**ACTION**: Identifying Acceptable Highway Safety Features" of July 25, 1997, provides further guidance on crash testing requirements of longitudinal barriers.

### **Decision**

The following device was found acceptable, with details provided below:

- VGAN 300 Aluminum Permanent Bridge Barrier

### **Description**

The Varley & Gulliver proprietary aluminum bridge railing system was mounted on a reinforced concrete curb. Overall length of the entire system was 29.3 m (96.0 ft) with posts spaced at 2.44 m (8.0 ft) for a total of 12 bays, 13 posts. The posts were cast A444.0 T4 aluminum and were anchored with four M20 stainless steel bolts. Extruded 6082 T6 aluminum tubes were used



FHWA:HSSI:WLongstreet:ms:x60087:12/21/10

File: s://directory folder/HSSI/Longstreet/B213\_Description\_122110FINAL.dotx

cc: HSSI (WLongstreet; JDewar)

for the railing. They were a flattened elliptical shape. The two lower rail elements were 152 mm (6.0 inches) in the long direction and 98 mm (3.8 inches) in the short direction and 5 mm (0.2 inch) wall thickness. The upper or pedestrian rail was 114 mm (4.5 inches) in the long direction and 85 mm (3.3 inches) in the short direction with 3 mm (0.12 inch) wall thickness. Splices were achieved with 6 mm (0.24 inch) wall thickness internal tubular sections in the main rails and 4 mm (0.16 inch) wall thickness internal tubular sections in the upper or pedestrian rail. The extruded rail elements had key way section on the lower back side that allow bolts to be placed in the key way and provides longitudinal adjustment.

The concrete foundation was specified to be 5800 psi and was 6318 psi at the time of the test and was anchored to the apron with “L” shaped bars welded to existing rebar. All rebar was specified to be 60 ksi. Steel reinforcement bar stirrups, 16 mm (0.6 inch) diameter, were placed at 150 mm (6.0 inches) on center with eight spaces under the post location and four spaces at 310 mm (12.2 inches) on center between the posts. There were 12 evenly spaced 16 mm (0.6 inch) diameter longitudinal bars in the foundation. Threaded inserts were placed in the concrete with templates. After the posts were bolted into the inserts, an epoxy grout pad was cast at each post support location. The four anchor bolts on one post upstream of impact, the impact post, and one post downstream of impact were instrumented with strain gages to measure force transmitted to the bolts.

Details of the VGAN 300 bridge rail test article are enclosed within this correspondence.

### **Crash Testing**

The Proprietary VGAN 300 Aluminum Permanent Bridge Barrier was crash tested at the test facilities at TTI Proving Grounds Riverside Campus according to the following NCHRP Report 350 TL-4 tests for the evaluation of longitudinal barriers as described below.

**NCHRP Report 350 Test Designation 4-10** with an 820 kg small passenger vehicle impacting the critical impact point (CIP) of the length-of-need (LON) of the bridge rail while traveling at an impact speed and angle of 100 km/h and 20 degrees. The purpose of this test is to evaluate the overall performance of the LON section, in general, and occupant risks, in particular.

**NCHRP Report 350 Test Designation 4-11** with a 2000 kg pickup truck impacting the CIP of the LON while traveling at an impact speed and angle of 100 km/h and 25 degrees. The test is intended to evaluate strength of the section in containing and redirecting the 2000P vehicle.

**NCHRP Report 350 Test Designation 4-12** with an 8000 kg single-unit box-van truck impacting the CIP of the LON while traveling at an impact speed and angle of 80 km/h and 15 degrees. This test is intended to evaluate the strength of the LON in containing and redirecting the heavy test vehicle.

The target CIP for each of the aforementioned tests was determined according to the information provided in NCHRP Report 350. For the test with the small car and the pickup, the CIP was determined to be at post 4. Post 4 is upstream of the first splice in the system. The CIP for the test with the single-unit box-van truck was determined to be 1 ft downstream of post 4. Crash Test summaries of each of these tests are enclosed within this correspondence.

**Findings**

The FHWA concurs to the submitted physical crash testing of the Proprietary VGAN 300 Aluminum Permanent Bridge Barrier to the proposed TL-4 designation. Therefore, the Proprietary VGAN 300 Aluminum Permanent Bridge Barrier meets the TL-4 impact conditions and evaluation criteria for a NCHRP 350, and is acceptable for use on the NHS when requested by a highway agency.

Please note the following standard provisions that apply to FHWA letters of acceptance:

- This acceptance is limited to the crashworthiness characteristics of the system and does not cover their structural features, nor conformity with the Manual on Uniform Traffic Control Devices.
- Any changes that may adversely influence the crashworthiness of the system will require a new acceptance letter.
- Should the FHWA discover that the qualification testing was flawed, that in-service performance reveals unacceptable safety problems, or that the system being marketed is significantly different from the version that was crash tested, we reserve the right to modify or revoke our acceptance.
- You will be expected to supply potential users with sufficient information on design and installation requirements to ensure proper performance.
- You will be expected to certify to potential users that the hardware furnished has essentially the same chemistry, mechanical properties, and geometry as that submitted for acceptance, and that it will meet the crashworthiness requirements of the FHWA and the NCHRP Report 350.
- To prevent misunderstanding by others, this letter of acceptance is designated as number B-213 and shall not be reproduced except in full. This letter and the test documentation upon which it is based are public information. All such letters and documentation may be reviewed at our office upon request.
- This acceptance letter shall not be construed as authorization or consent by the FHWA to use, manufacture, or sell any patented system for which the applicant is not the patent holder. The acceptance letter is limited to the crashworthiness characteristics of the candidate system, and the FHWA is neither prepared nor required to become involved in issues concerning patent law. Patent issues, if any, are to be resolved by the applicant.

Sincerely yours,

Michael S. Griffith  
Director, Office of Safety Technologies  
Office of Safety

Enclosures



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Sincerely yours,



Michael S. Griffith  
Director, Office of Safety Technologies  
Office of Safety

Enclosures

# APPENDIX A. DETAILS OF THE TEST ARTICLE

**END DETAILS**

**OPTION 1.**

**OPTION 2.**

**TYPICAL JOINT LOCATIONS**

LENGTH OF END RAILS AND RAILS JOINTS TO EXPOSE JOINTS SHOWN TO EACH CONTRACT.

RAILS TO BE LOCATED TO CONTRACT.

RAIL JOINTS TO BE LOCATED TO CONTRACT.

**SECTION THROUGH PARAPET.**

SEE ANCHORAGE TIE-RODS.

36" MINIMUM TIE ROD SPACING.

50 MINIMUM TIE ROD SPACING.

TRAFFIC FACE.

27' MINIMUM.

50 MINIMUM TIE ROD SPACING.

SEE ANCHORAGE TIE-RODS.

CHECKED BY: <i>[Signature]</i>	DESIGNED BY: <i>[Signature]</i>	SCALE: 1/4" = 1'-0"	DRAWING NUMBER: VGAN 300 - 01 A
DATE: 11/15/2013	DESIGN DATE: 11/15/2013	CONTRACT: 1307 WOKMAN	

GENERAL NOTES: 1. ALL DIMENSIONS ARE TO FACE UNLESS OTHERWISE NOTED. 2. ALL DIMENSIONS ARE TO CENTER UNLESS OTHERWISE NOTED. 3. ALL DIMENSIONS ARE TO FACE UNLESS OTHERWISE NOTED. 4. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO FACE UNLESS OTHERWISE NOTED. 5. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO CENTER UNLESS OTHERWISE NOTED. 6. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO FACE UNLESS OTHERWISE NOTED. 7. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO FACE UNLESS OTHERWISE NOTED. 8. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO FACE UNLESS OTHERWISE NOTED. 9. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO FACE UNLESS OTHERWISE NOTED. 10. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO FACE UNLESS OTHERWISE NOTED.	SPECIFICATIONS: 1. ALL DIMENSIONS ARE TO FACE UNLESS OTHERWISE NOTED. 2. ALL DIMENSIONS ARE TO CENTER UNLESS OTHERWISE NOTED. 3. ALL DIMENSIONS ARE TO FACE UNLESS OTHERWISE NOTED. 4. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO FACE UNLESS OTHERWISE NOTED. 5. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO CENTER UNLESS OTHERWISE NOTED. 6. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO FACE UNLESS OTHERWISE NOTED. 7. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO FACE UNLESS OTHERWISE NOTED. 8. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO FACE UNLESS OTHERWISE NOTED. 9. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO FACE UNLESS OTHERWISE NOTED. 10. DIMENSIONS TO BE SHOWN WITH DIMENSION LINES TO FACE UNLESS OTHERWISE NOTED.
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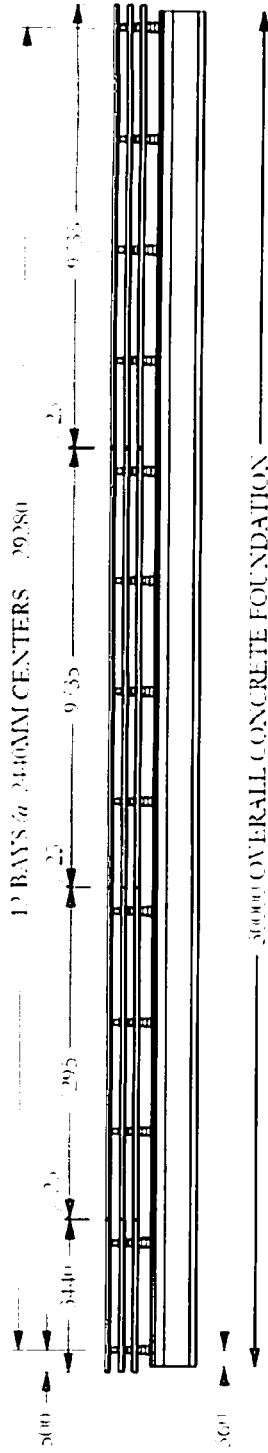
  

SECTION	SECTION NUMBER	SECTION TITLE
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SECTION 2	2	SECTION THROUGH PARAPET
SECTION 3	3	SECTION THROUGH PARAPET
SECTION 4	4	SECTION THROUGH PARAPET
SECTION 5	5	SECTION THROUGH PARAPET
SECTION 6	6	SECTION THROUGH PARAPET
SECTION 7	7	SECTION THROUGH PARAPET

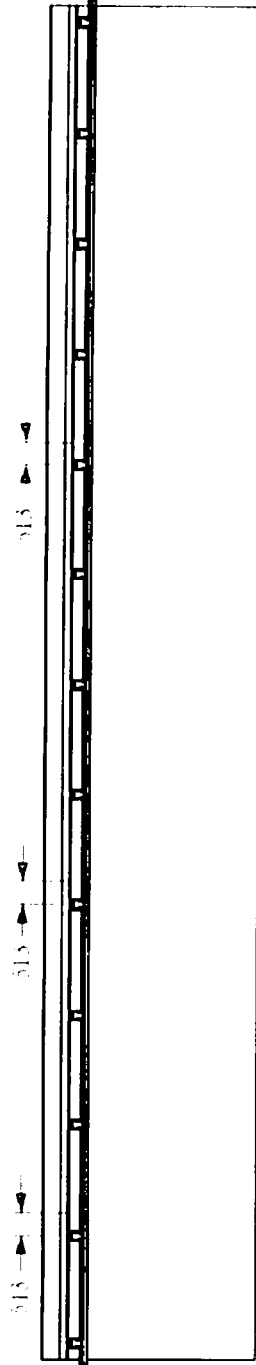
  

V&G  
Valley and Gilbreth Ltd.  
1000 11th Ave NW  
Portland, OR 97227  
Tel: (503) 224-5800  
Fax: (503) 224-5801  
www.vandg.com

# FRONT ELEVATION



# PLAN VIEW



The Texas A&M University System

Texas Transportation Institute  
College Station, Texas 77843

Revisions:	No.	Date	By	Chk	Date	Drawn By	Scale	Sheet No.
	1.				2010-2-22	JLH	1:130	1 of 10
	2.							
	3.							
	4.							
	5.							

Project No. 401761-VGL  
VGL Bridge Rail  
*Signature*

Approved: Dean Alberson: Date: 2010-05-03



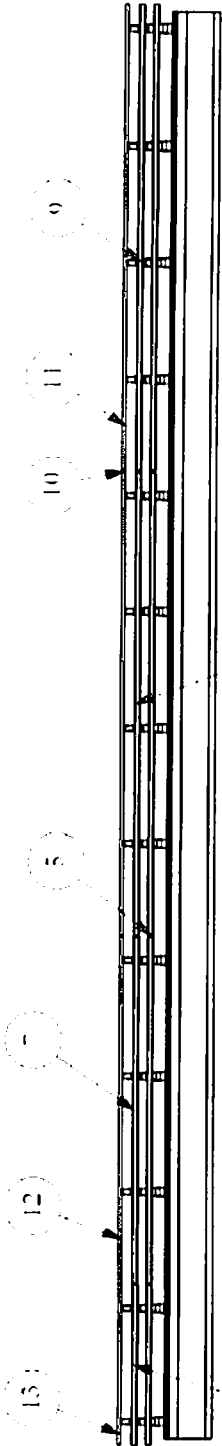
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3	Rebar, 16	1
4	Rebar, 12	1
5	Main rail joint	6
6	Main rail 9735	4
7	Main rail 7295	2
8	Main rail 3-4-40	2
9	Rail post	13
10	Pedestrian rail joint	3
11	Pedestrian rail 9735	2

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13	Pedestrian rail 3-4-40	1
14	Rail nut	54
15	Hex bolt M16 x 45	52
16	Nylon washer, M16	78
17	Stainless steel washer, M16	78
18	Spring washer, M16 for rail bolt	78
19	Stainless steel washer, M20 for anchor bolts	52
20	Nylon washer, M20 for anchor bolt	52
21	Hex bolt M20 x 100	52
22	Hex bolt M16 x 35	26
26	Hex bolt M20 x 100	52
27	B18.2.3.6M - Heavy hex bolt M20 x 2.5 x 80 --16N	52

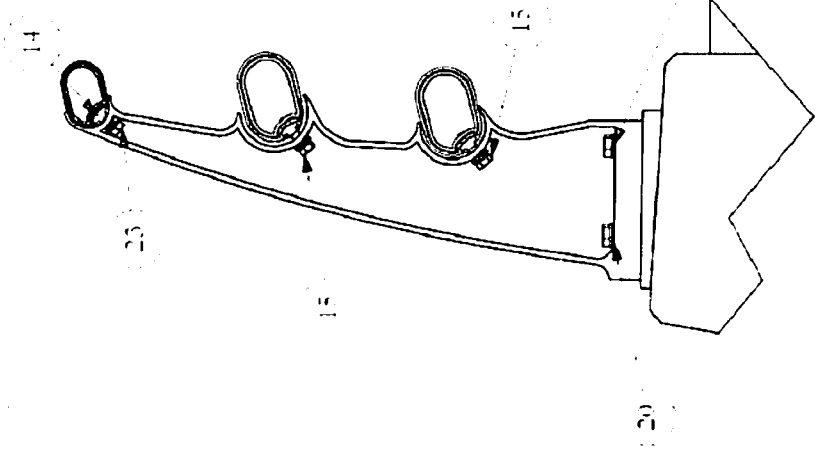
## The Texas A&amp;M University System

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4.				401761-VGL			
5.				VGL Bridge Rail			



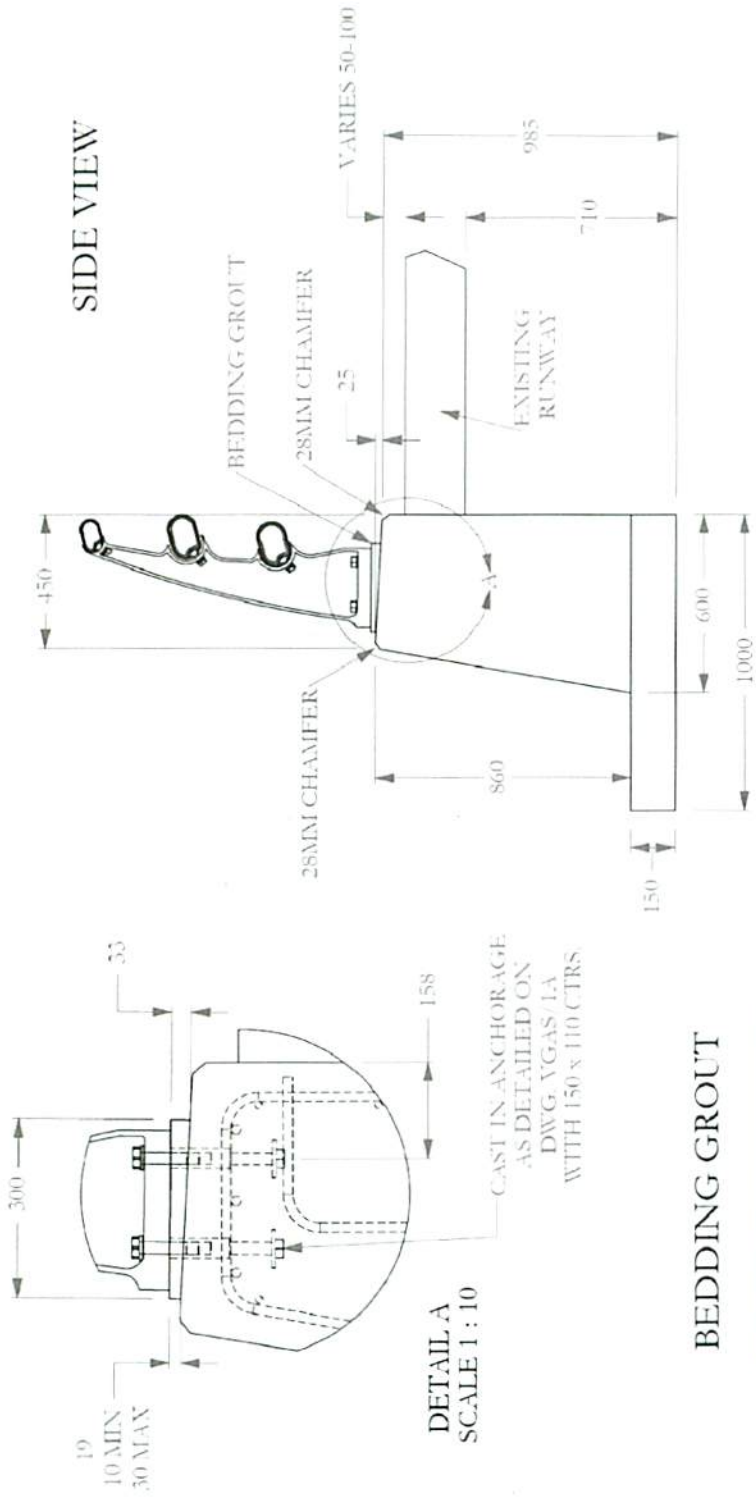
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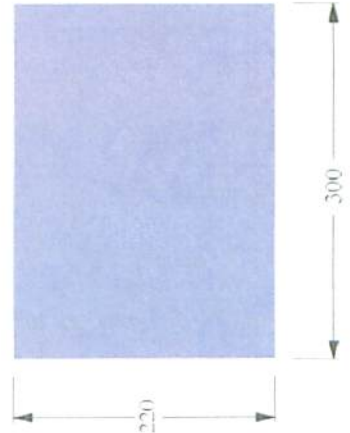
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College Station, Texas 77843		2010-02-22	JLH	1:120	3 of 10
		Project No.			Parts
		401761-VGL			
		VGL Bridge Rail			



**BEDDING GROUT**



The Texas A&M University System

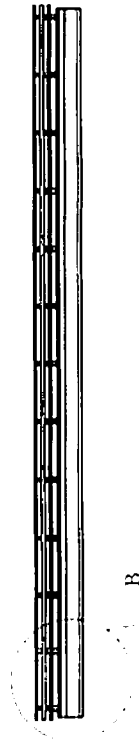
Texas Transportation Institute  
College Station, Texas 77843

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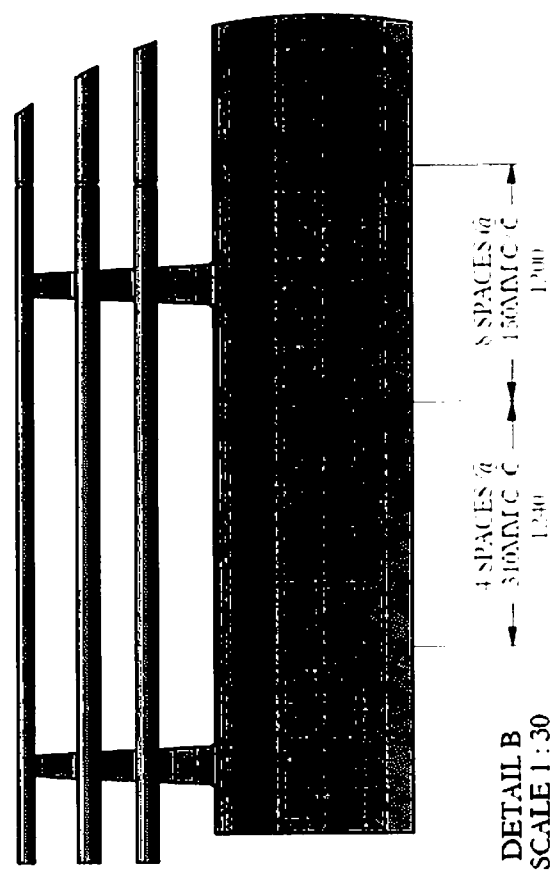
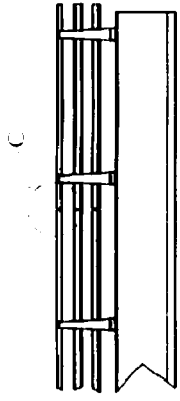
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Project No. 401761-VGL  
Concrete  
VGL Bridge Rail

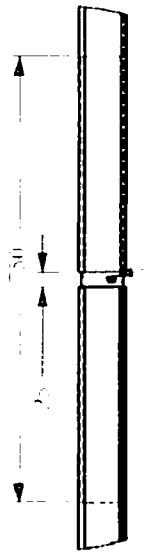
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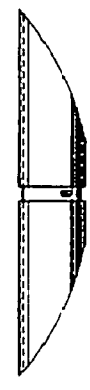
BACK VIEW



DETAIL B  
SCALE 1:30



Stirrups PIN



DETAIL C  
SCALE 1:10

STIRRUP SPACING TYPICAL - FULL INSTALLATION

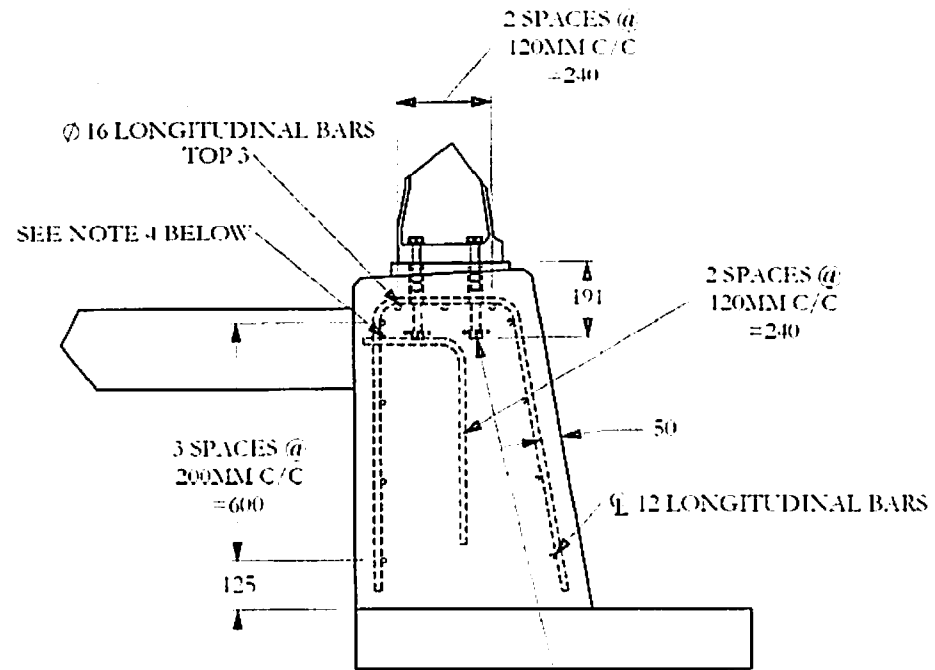
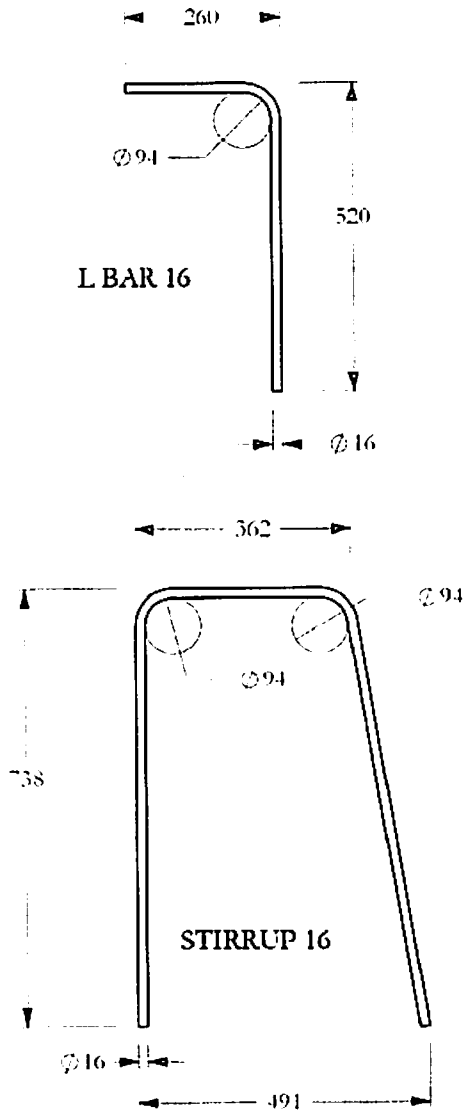
Revisions:

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The Texas A&M University System

Texas Transportation Institute  
College Station, Texas 77843

Date: 2010-02-22  
 Drawn By: JLH  
 Scale: 1:20  
 Sheet No.: 5 of 10  
 Project No.: 401761-VGL  
 Stirrup Spacing: VGL Bridge Rail



1. CONCRETE STRENGTH : 5800
2. REBAR LAP LENGTH: 815
3. REBAR : GRADE 60
4. WELDED TO EXISTING REBAR (NOT SHOWN)

CAST-IN ANCHORAGE  
AS DETAILED ON  
DWG. VGAS/1A  
WITH 150 x 110 CTRS.

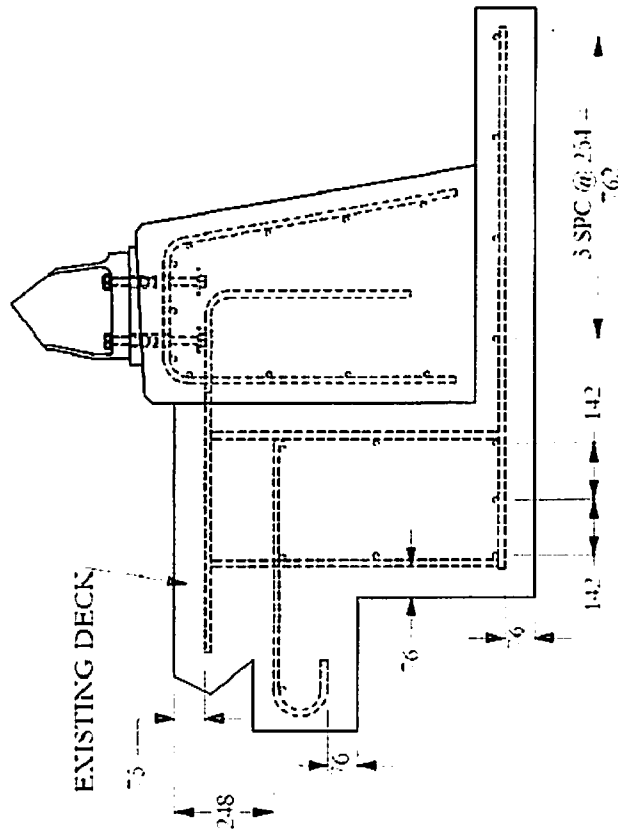
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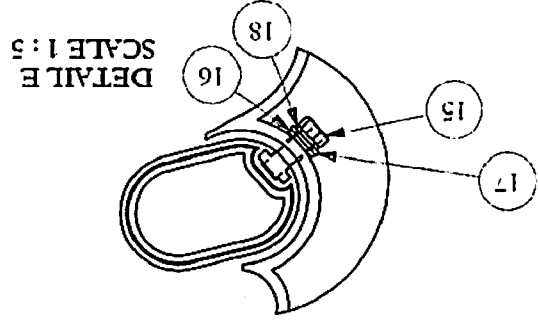
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Project No. Rebar Details  
401761-VGL  
VGL Bridge Rail



The Texas A&M University System  
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Revisions:

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3.							Existing rebar
4.							401761-VGL
5.							VGL Bridge Rail



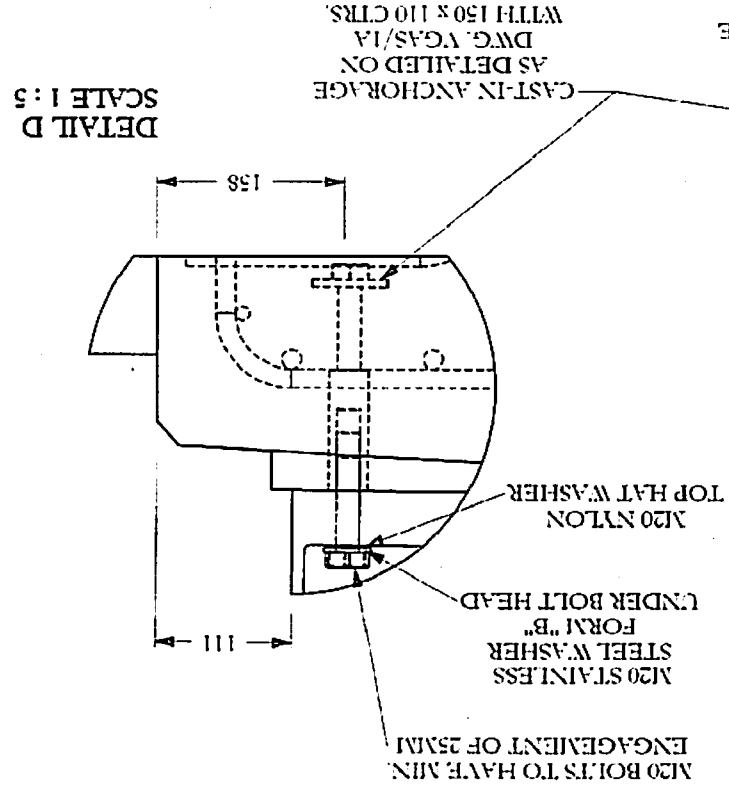
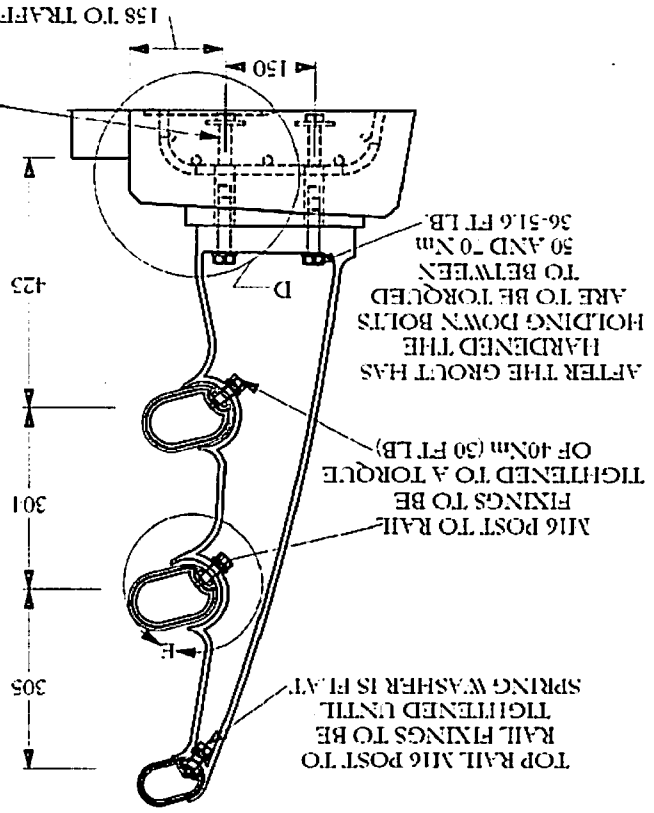
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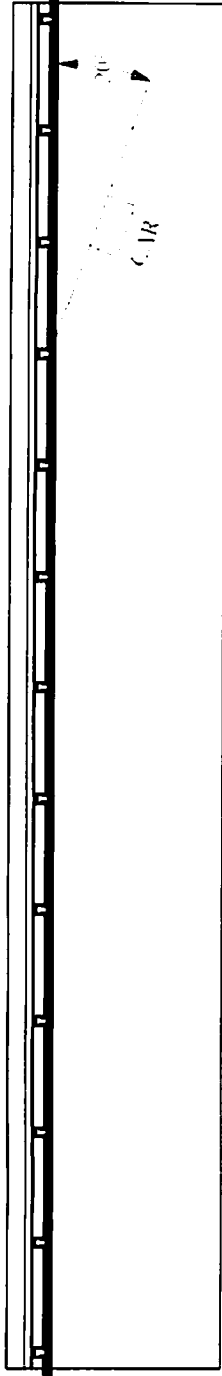
Project No.  
 401761-VGL

Anchor Bolt Details  
 VGL Bridge Rail



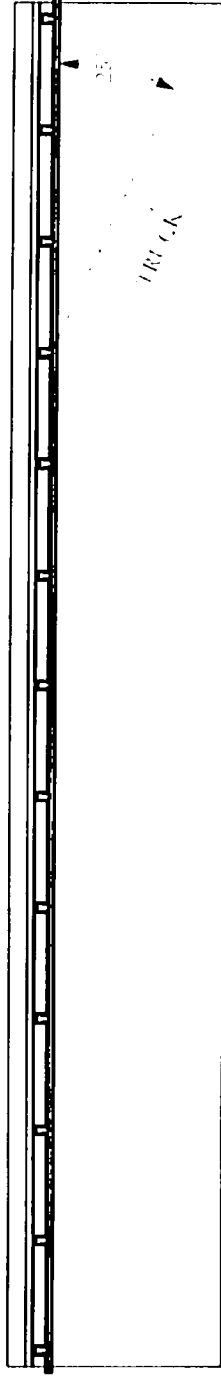
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13 12 11 10 9 8 7 6 5 4 3 2 1



PLAN VIEW OF CAR IMPACT

13 12 11 10 9 8 7 6 5 4 3 2 1



PLAN VIEW OF TRUCK IMPACT

The Texas A&M University System

Texas Transportation Institute  
College Station, Texas 77843

Revisions:

No.	Date	By	Chk	Date	Drawn By	Scale	Sheet No.
1.				2010-2-22	JLH	1:130	9 of 10
2.							
3.							
4.							
5.							

Project No. Impact 1 & 2

401761-VGL

VGL Bridge Rail

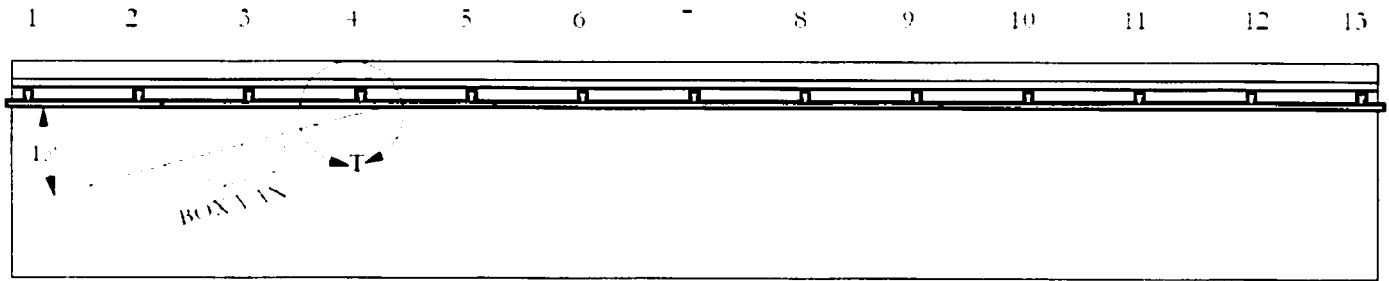
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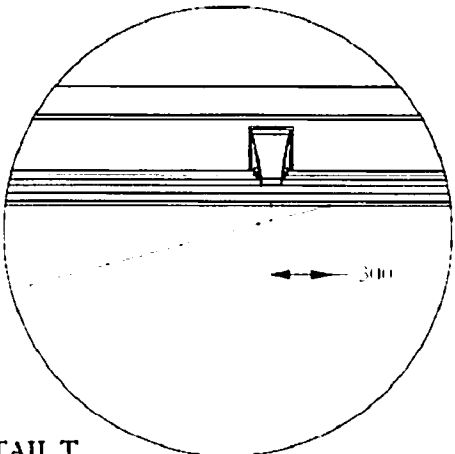
Approved:

Dean Alberson:





PLAN VIEW OF BOX VAN IMPACT



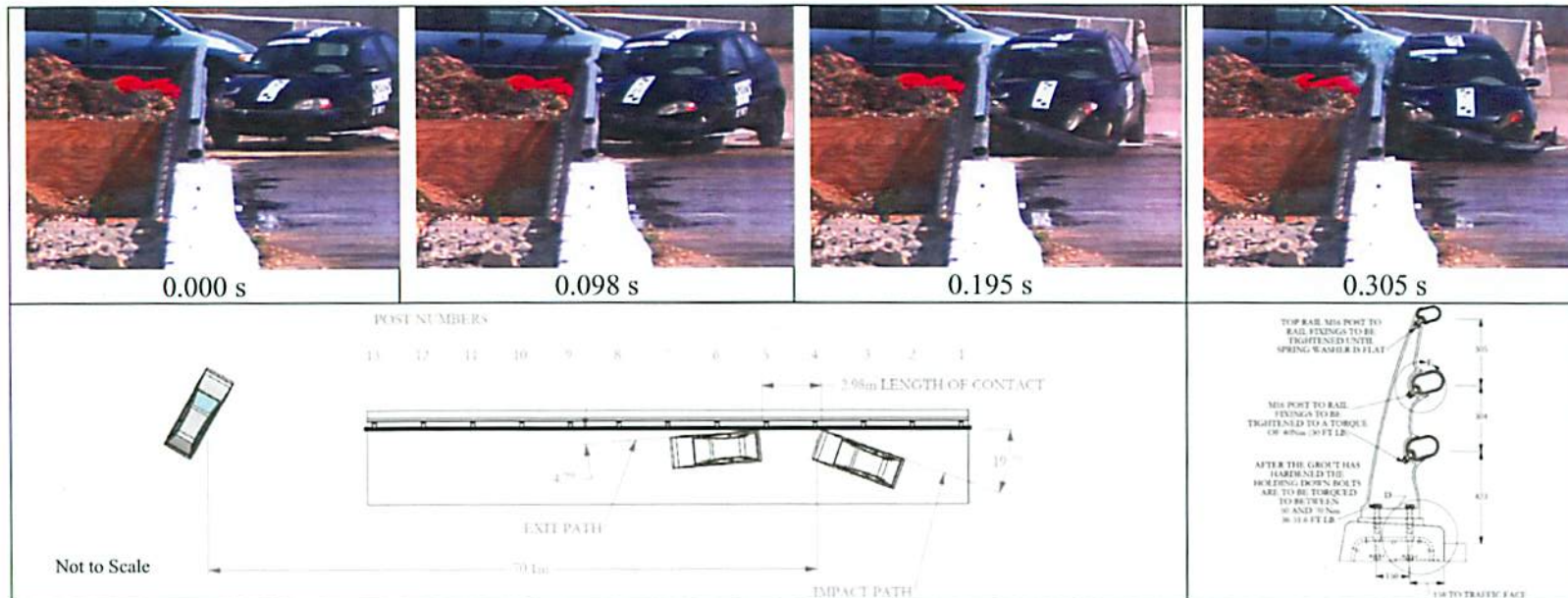
DETAIL T  
SCALE 1 : 30

The Texas A&M University System  
 Texas Transportation Institute  
 College Station, Texas 77843

Revisions:	No.	Date	By	Chk	Date	Drawn By	Scale	Sheet No.
	1.				2010-2-22	JLH	1:130	10 of 10
	2.							
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Project No. 401761-VGL  
 Impact 3  
 VGL Bridge Rail

Approved: \_\_\_\_\_ Signature: \_\_\_\_\_ Date: \_\_\_\_\_  
 Dean Alberson:



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**General Information**

Test Agency..... Texas Transportation Institute  
 Testing Standard Test No..... NCHRP Report 350 4-10  
 Test No. .... 401761-VGL1  
 Date ..... 2010-04-27

**Test Article**

Type..... Bridge Rail  
 Name ..... VGAN 300 Aluminum Bridge Parapet  
 Installation Length ..... 30.3 m  
 Material or Key Elements ..... 3 horizontal extruded 6082 T6 aluminum tubes sections on A444.0 T4 aluminum posts spaced at 2.44 m

**Soil Type and Condition**

..... Concrete Bridge Deck, Dry

**Test Vehicle**

Type/Designation..... 820C  
 Make and Model ..... 1995 Geo Metro  
 Curb ..... 837 kg  
 Test Inertial ..... 845 kg  
 Dummy ..... 75 kg  
 Gross Static ..... 920 kg

**Impact Conditions**

Speed .....101.9 km/h  
 Angle .....19.7 degrees  
 Location/Orientation .....At post 4

**Exit Conditions**

Speed .....88.4 km/h  
 Angle .....4.7 degrees

**Occupant Risk Values**

Impact Velocity  
 Longitudinal .....3.7 m/s  
 Lateral .....8.1 m/s  
 Ridedown Accelerations  
 Longitudinal ..... -6.1 G  
 Lateral ..... -9.1 G  
 THIV .....32.5 km/h  
 PHD ..... 9.2 G  
 ASI ..... 1.72  
 Max. 0.050-s Average  
 Longitudinal ..... -6.8 G  
 Lateral ..... -14.6 G  
 Vertical ..... 2.2 G

**Post-Impact Trajectory**

Stopping Distance ..... 70.1 m

**Vehicle Stability**

Maximum Yaw Angle .....432 degrees  
 Maximum Pitch Angle ..... 9 degrees  
 Maximum Roll Angle ..... 15 degrees  
 Vehicle Snagging ..... No  
 Vehicle Pocketing ..... No

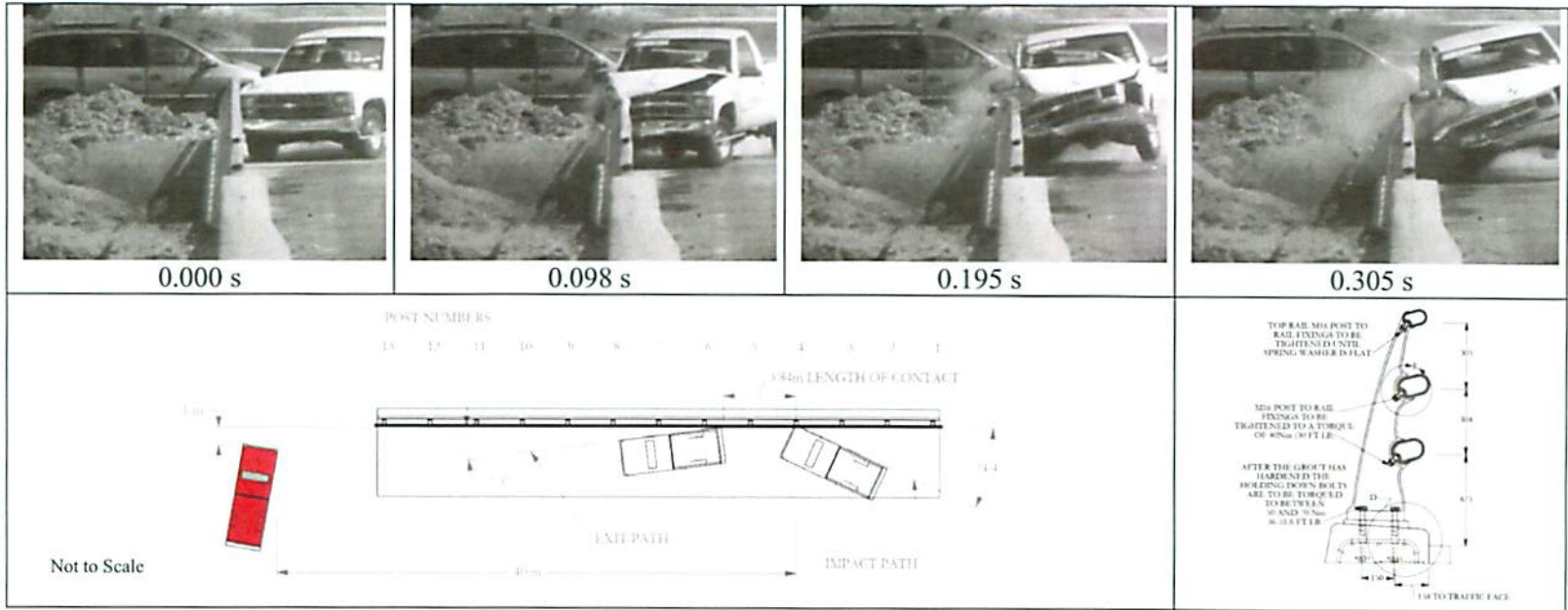
**Test Article Deflections**

Dynamic ..... 155 mm  
 Permanent ..... 5 mm  
 Working Width ..... 159 mm

**Vehicle Damage**

VDS ..... 01RFQ4  
 CDC ..... 01FREW3  
 Max. Exterior Deformation ..... 220 mm  
 Max. Occupant Compartment Deformation ..... 30 mm

Figure 10. Summary of results for NCHRP Report 350 test 4-10 on the VGAN 300 aluminum bridge rail.



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**General Information**

Test Agency..... Texas Transportation Institute  
 Testing Standard Test No..... *NCHRP Report 350 4-11*  
 Test No. .... 401761-VGL2  
 Date ..... 2010-04-28

**Test Article**

Type..... Bridge Rail  
 Name..... VGAN 300 Aluminum Bridge Parapet  
 Installation Length ..... 30.3 m  
 Material or Key Elements ..... 3 horizontal extruded 6082 T6 aluminum tubes sections on A444.0 T4 aluminum posts spaced at 2.44 m  
 Soil Type and Condition..... Concrete Deck, Dry

**Test Vehicle**

Type/Designation..... 2000P  
 Make and Model..... 1997 Chevrolet C2500 Pickup  
 Curb ..... 2174 kg  
 Test Inertial..... 2083 kg  
 Dummy ..... No dummy  
 Gross Static ..... 2083 kg

**Impact Conditions**

Speed .....100.7 km/h  
 Angle .....24.4 degrees  
 Location/Orientation ..... At post 4

**Exit Conditions**

Speed .....68.5 km/h  
 Angle .....7.2 degrees

**Occupant Risk Values**

Impact Velocity  
 Longitudinal.....6.6 m/s  
 Lateral .....7.5 m/s  
 Ridedown Accelerations  
 Longitudinal.....-18.2 G  
 Lateral .....-12.6 G  
 THIV ..... 35.7 km/h  
 PHD ..... 9.9 G  
 ASI ..... 1.48  
 Max. 0.050-s Average  
 Longitudinal.....-10.7 G  
 Lateral .....-11.7 G  
 Vertical ..... 5.7 G

**Post-Impact Trajectory**

Stopping Distance ..... 40.2 m downstrm  
 1 m twd traffic

**Vehicle Stability**

Maximum Yaw Angle..... 114 degrees  
 Maximum Pitch Angle..... 19 degrees  
 Maximum Roll Angle..... -27 degrees  
 Vehicle Snagging..... No  
 Vehicle Pocketing ..... No

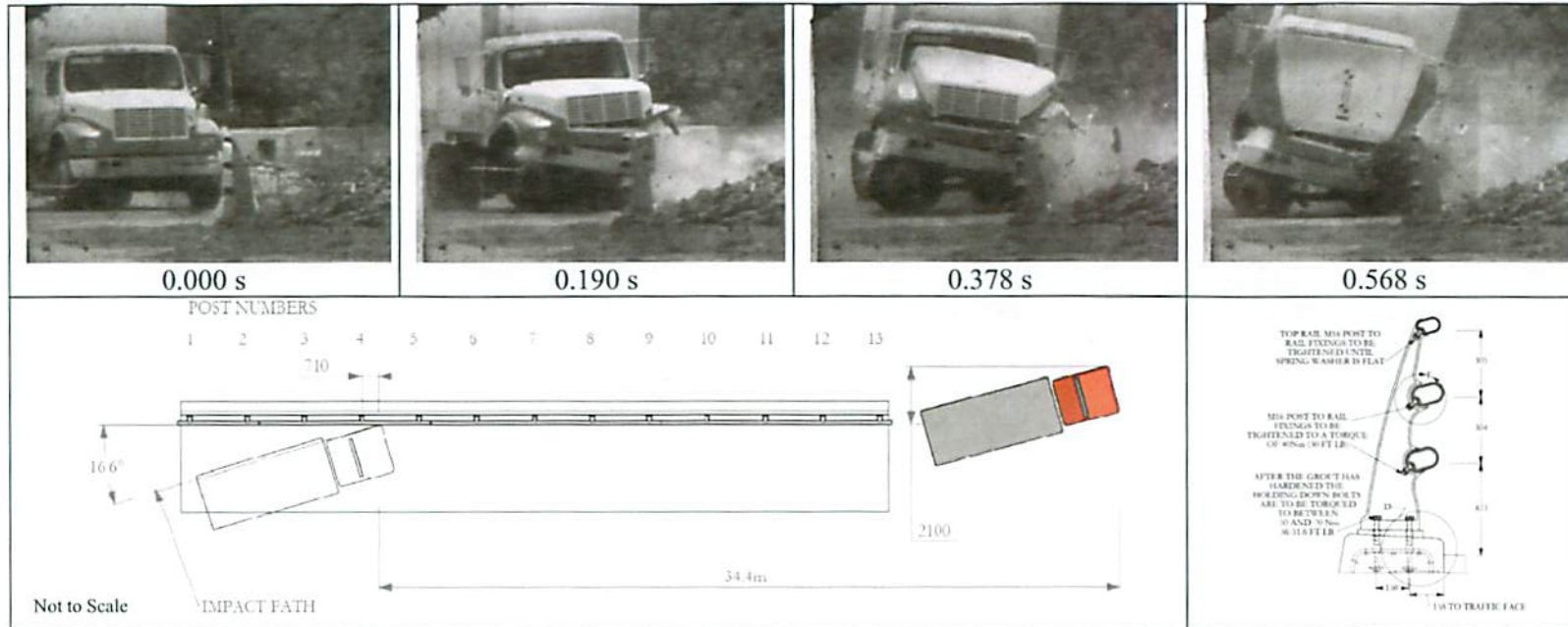
**Test Article Deflections**

Dynamic..... 360 mm  
 Permanent ..... 190 mm  
 Working Width ..... 520 mm

**Vehicle Damage**

VDS ..... 01RFQ4  
 CDC ..... 01RFEW3  
 Max. Exterior Deformation..... 600 mm  
 Max. Occupant Compartment Deformation..... 100 mm

Figure 17. Summary of results for *NCHRP Report 350* test 4-11 on the VGAN 300 aluminum bridge rail.



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**General Information**

Test Agency..... Texas Transportation Institute  
 Testing Standard Test No..... *NCHRP Report 350* 4-12  
 Test No. .... 401761-VGL3  
 Date ..... 2010-04-29

**Test Article**

Type..... Bridge Rail  
 Name ..... VGAN 300 Aluminum Bridge Parapet  
 Installation Length ..... 30.3 m  
 Material or Key Elements ..... 3 horizontal extruded 6082 T6 aluminum tubes sections on A444.0 T4 aluminum posts spaced at 2.44 m

**Soil Type and Condition**

Concrete Deck, Dry

**Test Vehicle**

Type/Designation..... 8000S  
 Make and Model..... 1999 International 4700  
 Curb..... 5647 kg  
 Test Inertial..... 7951 kg  
 Dummy ..... No dummy  
 Gross Static..... 7951 kg

**Impact Conditions**

Speed .....82.1 km/h  
 Angle .....16.6 degrees  
 Location/Orientation .....710 mm dwn

**Exit Conditions**

Speed .....Not obtainable  
 Angle .....Not obtainable

**Occupant Risk Values**

Impact Velocity  
 Longitudinal .....3.9 m/s  
 Lateral .....3.4 m/s  
 Ridedown Accelerations  
 Longitudinal .....-4.4 G  
 Lateral .....6.5 G  
 THIV .....19.8 km/h  
 PHD .....7.2 G  
 ASI.....0.42  
 Max. 0.050-s Average  
 Longitudinal .....-3.7 G  
 Lateral .....3.6 G  
 Vertical .....2.7 G

**Post-Impact Trajectory**

Stopping Distance .....40.2 m dwnstrm  
 1 m twd traffic

**Vehicle Stability**

Maximum Yaw Angle.....-17 degrees  
 Maximum Pitch Angle.....14 degrees  
 Maximum Roll Angle.....-18 degrees  
 Vehicle Snagging.....No  
 Vehicle Pocketing.....No

**Test Article Deflections**

Dynamic..... Not obtainable  
 Permanent.....300 mm  
 Working Width .....787 mm

**Vehicle Damage**

VDS .....01LFQ4  
 CDC.....01LFEW3  
 Max. Exterior Deformation.....50 mm  
 Max. Occupant Compartment Deformation.....0 mm

Figure 23. Summary of results for *NCHRP Report 350* test 4-12 on the VGAN 300 aluminum bridge rail.