

January 28, 2010

In Reply Refer To: HSSD/B-200

Mr. Ronald K. Faller, Ph.D. Research Assistant Professor Midwest Roadside Safety Facility University of Nebraska-Lincoln 527 Nebraska Hall Lincoln, NE 68588-0529

Dear Dr. Faller:

This letter is in response to your request for the Federal Highway Administration (FHWA) acceptance of a roadside safety device for use on the National Highway System (NHS).

Name of device:	West Virginia Steel Bridge Railing for use on Transverse,
	Nail-Laminated, Timber Bridges
Type of device:	Permanent Steel Barrier
Test Level:	NCHRP Report 350 TL-2
Testing conducted by:	Midwest Roadside Safety Facility (MwRSF)
Date of request:	September 13, 2009
Date of completed package:	September 13, 2009
Task Force 13 Designator:	SBT11b

You requested that we find this device acceptable for use on the NHS under the provisions of National Cooperative Highway Research Program (NCHRP) Report 350 "Recommended Procedures for the Safety Performance Evaluation of Highway Features."

Requirements

Roadside safety devices should meet the guidelines contained in the NCHRP Report 350 or the American Association of State Highway and Transportation Officials' Manual for Assessing Safety Hardware (MASH). The FHWA Memorandum "Identifying Acceptable Highway Safety Features" of July 25, 1997, provides further guidance on crash testing requirements of longitudinal barriers.



Description

For this project, the research objectives included the modification of existing test level 2 (TL-2) steel thrie beam bridge railing as per FHWA Acceptance Letter HSA-10/B-138 dated August 4, 2005. This thrie beam steel post bridge barrier was connected to a transverse, glue-laminated, timber bridge deck system. Eight 7/8-in. (22.2-mm) diameter by 7 3/4-in. (197-mm) long, ASTM A307 (Grade 2 equivalent) hex head bolts with timber shear connectors were used to anchor the posts and deck plates to the glulam timber deck panels. For this research, crash testing used the same barrier and connection plate system for use on a transverse, nail laminated, timber bridge deck supported by steel wide-flange beams. The Steel Bridge Railing for use on Transverse, Nail-Laminated Timber Bridge Decks was evaluated using dynamic bogie testing on the steel bridge posts attached to nail-laminated timber deck. The dynamic component testing program was then used to verify that the post-to-deck attachment hardware as well as the timber deck would remain intact under peak impact loading deemed representative of a pickup truck crash test conducted under the TL-2 impact safety standards of NCHRP Report 350. In addition, the testing was used to demonstrate that the peak impact loading would not result in significant deck damage. Dynamic component testing was used to verify use of the previously crash-tested bridge railing system on transverse, nail-laminated, timber deck bridges. In addition, testing also evaluated the benefits for utilizing timber shear plates within the post-to-deck connection.

The steel bridge posts were 42 3/4-in. (1,086-mm) long, W6x12 (W152x17.9) beams made from ASTM A992 or ASTM A572 Grade 50 steel, as shown in Figures 7 through 9. Near the top of the post, four 3/4-in. (19-mm) diameter bolt holes were placed within the front flange. The blockouts were bolted to the posts using these bolt holes. Slots were cut into the front flange near the bottom of each post and used to fasten the bottom deck plate to the post.

In addition to the fabricated holes and slots, a steel post plate was welded to the front flange 9 in. (229 mm) from the bottom of the post. Each post plate measured 10 3/8 in. x 4 in. x 1/2 in. (264 mm x 102 mm x 13 mm). Two slots were cut into the post plate and used to bolt the top deck plate to the post. To provide stiffness and resistance to buckling, gusset plates and stiffeners were also welded to the posts.

Gussets were placed on both sides of the web at the bottom of the post and directly behind the top of the post plate, while the post wing stiffeners were located along the top of the post plate and adjacent to the gusset plates. These gussets and stiffeners were designed to provide additional stiffness to the post and to prevent localized buckling near the deck plate attachments.

Deck plate assemblies were utilized to attach the bridge posts to the bridge deck. The top deck plate was 1/2 in. (13 mm) thick, while the bottom deck plate was 3/8 in. (10 mm) thick. The deck plates are fabricated from ASTM A36 steel and contained eight 1-in. (25-mm) diameter holes. Eight 7/8-in. (22.2-mm) diameter by 7 3/4-in. (197-mm) long, Grade 5 bolts were to be used to fasten the deck plates to the edge of the timber bridge deck. Since this detail produced minor bearing deformations around some of the vertical holes, the following alternative attachment options to reduce the incidence of deformations may be specified:

- a. Eight 7/8-in. (22.2-mm) diameter ASTM A307 (Grade 2 equivalent) bolts in combination with 4-in. (102-mm) diameter timber shear connectors.
- b. Eight 7/8-in. (22.2-mm) diameter ASTM A325 (Grade 5 equivalent) bolts in combination with 4-in. (102-mm) diameter timber shear connectors

Steel rectangular end plates were welded to the back side of the deck plates and provided the locations where the bridge post bolted to the plates. The end plates were welded to the deck plates using triangular-shaped plate stiffeners. Two 7/8-in. (22.2-mm) diameter ASTM A325 hex head bolts were used to fasten the top deck plate to each post, while two 5/8-in. (15.9-mm) diameter ASTM A325 hex head bolts were used to fasten the bottom deck plate to each post.

Post blockouts were configured with ASTM A992 or ASTM A572 Grade 50, W6x12 (W152x17.9) steel sections that attached to the front face of the bridge posts. Eight 3/4-in. (19-mm) holes, four in the front flange and four in the back flange, were placed into each blockout. Four 5/8-in. (15.9-mm) diameter by 2-in. (51-mm) long, ASTM A307 heavy hex head bolts were used to secure each blockout to each post.

A transverse, nail-laminated, timber bridge deck was constructed at MwRSF's outdoor test facility for this research project. The bridge deck was constructed from 14-ft (4.3-m) long, 2-in. x 6-in. (51-mm x 152-mm) treated, dimensional lumber and covered by a 2-in. (51-mm) thick concrete wearing surface. The timber boards were manufactured from Grade No. 1 Southern Yellow Pine and treated with ACQ-D to a minimum net retention of 0.40 lbs/ ft^3 (6.41 kg/m3) satisfying AWPA U1, UC4A. However for actual bridge installations, the research recommends that the dimensional lumber boards be treated to a net retention of 0.60 lbs/ft^3 (9.61 kg/m₃) satisfying AWPA U1, UC4B. The boards were placed on end and nailed together through and perpendicular to the wide face of the board using 20d or 20 penny "common" nails. A specific nail pattern, which repeated every four boards, was used to ensure that a nail did not contact a previously driven nail. Special care was given to the nail pattern near the deck edge to ensure the nails did not occupy space where the vertical bolt holes for the bridge rail would later be drilled. During deck assembly, two beads of Liquid Nails Heavy Duty Construction Adhesive were applied to the sides of the boards and over the outer 3 ft (0.9 m) of deck. The adhesive was used to provide additional punching shear resistance in the deck as well as improved load transfer between boards.

West Virginia TL-2 Steel Bridge Railing for use on Transverse Nail-Laminated, Timber Bridge Barrier drawings for the construction of the test installation are included with this correspondence. In addition, transition details for this bridge barrier can also be found in FHWA Acceptance Letter HSA-10/B-138 Revised.

Findings

We concur with your request that the West Virginia Steel Bridge Railing for use on Transverse Nail-Laminated, Timber Bridges be granted equivalence to existing successfully crash tested bridge rail meeting TL-2 conditions as per NCHRP Report 350 and will be considered acceptable for use on the NHS. For further information on the crash test, the Test Data Summary Sheet is included with this correspondence.

Please note the following standard provisions that apply to the FHWA letters of acceptance:

- This acceptance is limited to the crashworthiness characteristics of the devices and does not cover their structural features, nor conformity with the Manual on Uniform Traffic Control Devices.
- Any changes that may adversely influence the crashworthiness of the device will require a new acceptance letter.
- Should the FHWA discover that the qualification testing was flawed, that in-service performance reveals unacceptable safety problems, or that the device being marketed is significantly different from the version that was crash tested, we reserve the right to modify or revoke our acceptance.
- You will be expected to supply potential users with sufficient information on design and installation requirements to ensure proper performance.
- You will be expected to certify to potential users that the hardware furnished has essentially the same chemistry, mechanical properties, and geometry as that submitted for acceptance, and that it will meet the crashworthiness requirements of the FHWA and the MASH.
- To prevent misunderstanding by others, this letter of acceptance is designated as number B-200 and shall not be reproduced except in full. This letter and the test documentation upon which it is based are public information. All such letters and documentation may be reviewed at our office upon request.
- The West Virginia Steel Bridge Railing for use on Transverse, Nail-Laminated, Timber Bridges is a generic system and not considered proprietary.
- This acceptance letter shall not be construed as authorization or consent by the FHWA to use, manufacture, or sell any patented device for which the applicant is not the patent holder. The acceptance letter is limited to the crashworthiness characteristics of the candidate device, and the FHWA is neither prepared nor required to become involved in issues concerning patent law. Patent issues, if any, are to be resolved by the applicant.

Sincerely yours,

David A. Nicol, P.E. Director, Office of Safety Design Office of Safety

FHWA:HSSD:WLongstreet:tb:x60087:1/8/10

- File: s://directory folder/wlongstreet/ B200_WV steel post for nail laminated bridge deck 010710 RKF.doc
- cc: HSSD (Reader, HSA; Chron File, HSSD; W.Longstreet, HSSD; NArtimovich, HSSD; MMcDonough, HSSD)



January 28, 2010

In Reply Refer To: HSSD/B-200

Mr. Ronald K. Faller, Ph.D. Research Assistant Professor Midwest Roadside Safety Facility University of Nebraska-Lincoln 527 Nebraska Hall Lincoln, NE 68588-0529

Dear Dr. Faller:

This letter is in response to your request for the Federal Highway Administration (FHWA) acceptance of a roadside safety device for use on the National Highway System (NHS).

Name of device:	West Virginia Steel Bridge Railing for use on Transverse,
	Nail-Laminated, Timber Bridges
Type of device:	Permanent Steel Barrier
Test Level:	NCHRP Report 350 TL-2
Testing conducted by:	Midwest Roadside Safety Facility (MwRSF)
Date of request:	September 13, 2009
Date of completed package:	September 13, 2009
Task Force 13 Designator:	SBT11b

You requested that we find this device acceptable for use on the NHS under the provisions of National Cooperative Highway Research Program (NCHRP) Report 350 "Recommended Procedures for the Safety Performance Evaluation of Highway Features."

Requirements

Roadside safety devices should meet the guidelines contained in the NCHRP Report 350 or the American Association of State Highway and Transportation Officials' Manual for Assessing Safety Hardware (MASH). The FHWA Memorandum "Identifying Acceptable Highway Safety Features" of July 25, 1997, provides further guidance on crash testing requirements of longitudinal barriers.



Description

For this project, the research objectives included the modification of existing test level 2 (TL-2) steel thrie beam bridge railing as per FHWA Acceptance Letter HSA-10/B-138 dated August 4, 2005. This thrie beam steel post bridge barrier was connected to a transverse, glue-laminated, timber bridge deck system. Eight 7/8-in. (22.2-mm) diameter by 7 3/4-in. (197-mm) long, ASTM A307 (Grade 2 equivalent) hex head bolts with timber shear connectors were used to anchor the posts and deck plates to the glulam timber deck panels. For this research, crash testing used the same barrier and connection plate system for use on a transverse, nail laminated, timber bridge deck supported by steel wide-flange beams. The Steel Bridge Railing for use on Transverse, Nail-Laminated Timber Bridge Decks was evaluated using dynamic bogie testing on the steel bridge posts attached to nail-laminated timber deck. The dynamic component testing program was then used to verify that the post-to-deck attachment hardware as well as the timber deck would remain intact under peak impact loading deemed representative of a pickup truck crash test conducted under the TL-2 impact safety standards of NCHRP Report 350. In addition, the testing was used to demonstrate that the peak impact loading would not result in significant deck damage. Dynamic component testing was used to verify use of the previously crash-tested bridge railing system on transverse, nail-laminated, timber deck bridges. In addition, testing also evaluated the benefits for utilizing timber shear plates within the post-to-deck connection.

The steel bridge posts were 42 3/4-in. (1,086-mm) long, W6x12 (W152x17.9) beams made from ASTM A992 or ASTM A572 Grade 50 steel, as shown in Figures 7 through 9. Near the top of the post, four 3/4-in. (19-mm) diameter bolt holes were placed within the front flange. The blockouts were bolted to the posts using these bolt holes. Slots were cut into the front flange near the bottom of each post and used to fasten the bottom deck plate to the post.

In addition to the fabricated holes and slots, a steel post plate was welded to the front flange 9 in. (229 mm) from the bottom of the post. Each post plate measured 10 3/8 in. x 4 in. x 1/2 in. (264 mm x 102 mm x 13 mm). Two slots were cut into the post plate and used to bolt the top deck plate to the post. To provide stiffness and resistance to buckling, gusset plates and stiffeners were also welded to the posts.

Gussets were placed on both sides of the web at the bottom of the post and directly behind the top of the post plate, while the post wing stiffeners were located along the top of the post plate and adjacent to the gusset plates. These gussets and stiffeners were designed to provide additional stiffness to the post and to prevent localized buckling near the deck plate attachments.

Deck plate assemblies were utilized to attach the bridge posts to the bridge deck. The top deck plate was 1/2 in. (13 mm) thick, while the bottom deck plate was 3/8 in. (10 mm) thick. The deck plates are fabricated from ASTM A36 steel and contained eight 1-in. (25-mm) diameter holes. Eight 7/8-in. (22.2-mm) diameter by 7 3/4-in. (197-mm) long, Grade 5 bolts were to be used to fasten the deck plates to the edge of the timber bridge deck. Since this detail produced minor bearing deformations around some of the vertical holes, the following alternative attachment options to reduce the incidence of deformations may be specified:

- a. Eight 7/8-in. (22.2-mm) diameter ASTM A307 (Grade 2 equivalent) bolts in combination with 4-in. (102-mm) diameter timber shear connectors.
- b. Eight 7/8-in. (22.2-mm) diameter ASTM A325 (Grade 5 equivalent) bolts in combination with 4-in. (102-mm) diameter timber shear connectors

Steel rectangular end plates were welded to the back side of the deck plates and provided the locations where the bridge post bolted to the plates. The end plates were welded to the deck plates using triangular-shaped plate stiffeners. Two 7/8-in. (22.2-mm) diameter ASTM A325 hex head bolts were used to fasten the top deck plate to each post, while two 5/8-in. (15.9-mm) diameter ASTM A325 hex head bolts were used to fasten the bottom deck plate to each post.

Post blockouts were configured with ASTM A992 or ASTM A572 Grade 50, W6x12 (W152x17.9) steel sections that attached to the front face of the bridge posts. Eight 3/4-in. (19-mm) holes, four in the front flange and four in the back flange, were placed into each blockout. Four 5/8-in. (15.9-mm) diameter by 2-in. (51-mm) long, ASTM A307 heavy hex head bolts were used to secure each blockout to each post.

A transverse, nail-laminated, timber bridge deck was constructed at MwRSF's outdoor test facility for this research project. The bridge deck was constructed from 14-ft (4.3-m) long, 2-in. x 6-in. (51-mm x 152-mm) treated, dimensional lumber and covered by a 2-in. (51-mm) thick concrete wearing surface. The timber boards were manufactured from Grade No. 1 Southern Yellow Pine and treated with ACQ-D to a minimum net retention of 0.40 lbs/ ft^3 (6.41 kg/m3) satisfying AWPA U1, UC4A. However for actual bridge installations, the research recommends that the dimensional lumber boards be treated to a net retention of 0.60 lbs/ft^3 (9.61 kg/m₃) satisfying AWPA U1, UC4B. The boards were placed on end and nailed together through and perpendicular to the wide face of the board using 20d or 20 penny "common" nails. A specific nail pattern, which repeated every four boards, was used to ensure that a nail did not contact a previously driven nail. Special care was given to the nail pattern near the deck edge to ensure the nails did not occupy space where the vertical bolt holes for the bridge rail would later be drilled. During deck assembly, two beads of Liquid Nails Heavy Duty Construction Adhesive were applied to the sides of the boards and over the outer 3 ft (0.9 m) of deck. The adhesive was used to provide additional punching shear resistance in the deck as well as improved load transfer between boards.

West Virginia TL-2 Steel Bridge Railing for use on Transverse Nail-Laminated, Timber Bridge Barrier drawings for the construction of the test installation are included with this correspondence. In addition, transition details for this bridge barrier can also be found in FHWA Acceptance Letter HSA-10/B-138 Revised.

Findings

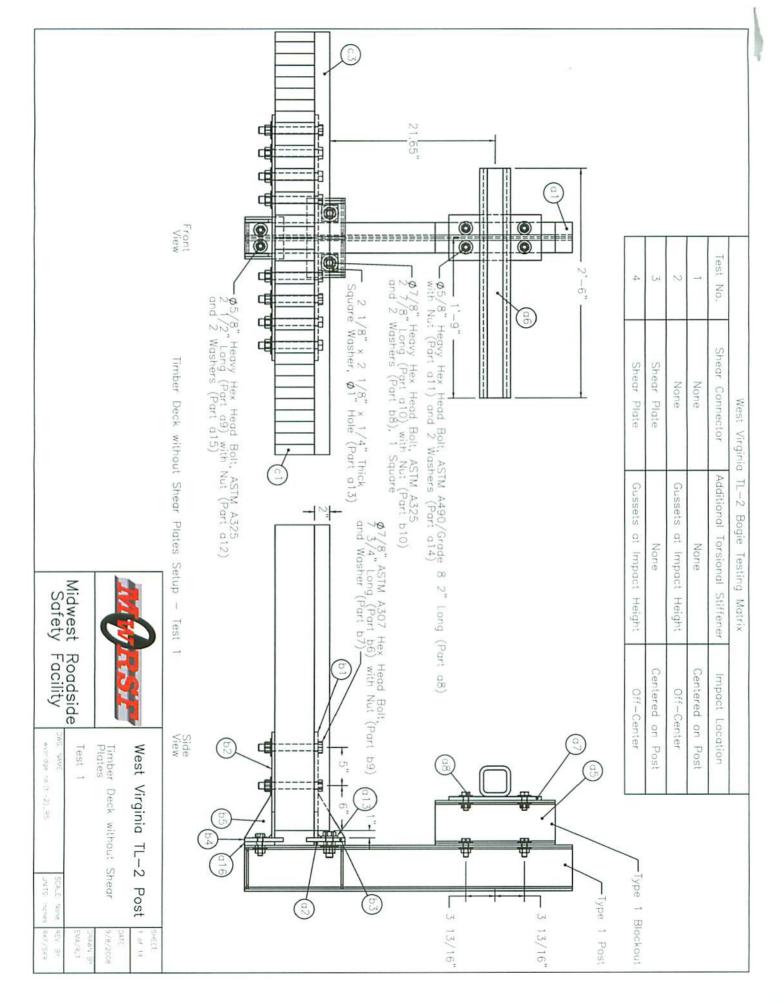
We concur with your request that the West Virginia Steel Bridge Railing for use on Transverse Nail-Laminated, Timber Bridges be granted equivalence to existing successfully crash tested bridge rail meeting TL-2 conditions as per NCHRP Report 350 and will be considered acceptable for use on the NHS. For further information on the crash test, the Test Data Summary Sheet is included with this correspondence.

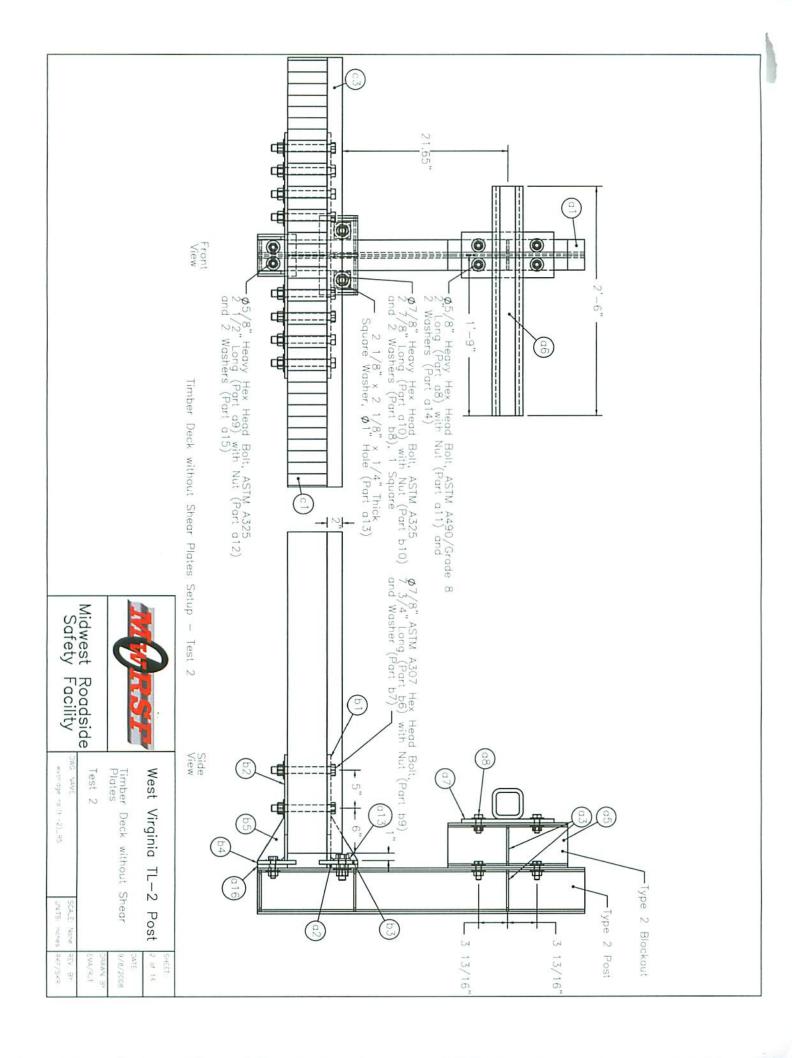
Please note the following standard provisions that apply to the FHWA letters of acceptance:

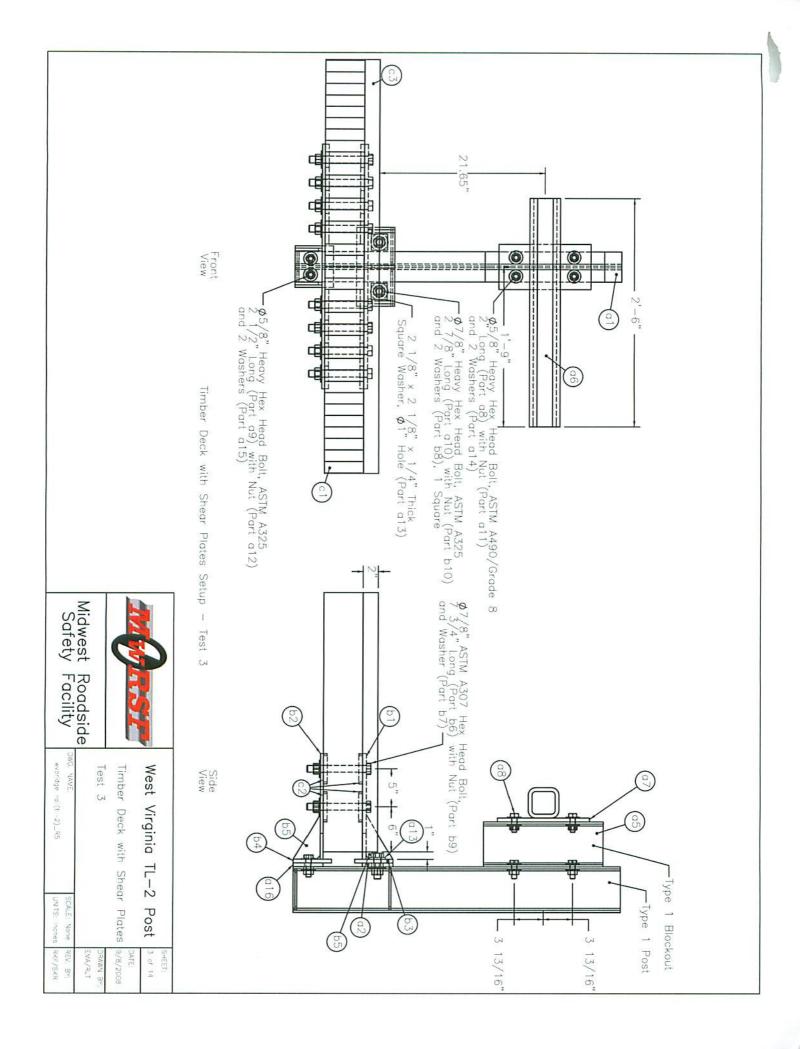
- This acceptance is limited to the crashworthiness characteristics of the devices and does not cover their structural features, nor conformity with the Manual on Uniform Traffic Control Devices.
- Any changes that may adversely influence the crashworthiness of the device will require a new acceptance letter.
- Should the FHWA discover that the qualification testing was flawed, that in-service performance reveals unacceptable safety problems, or that the device being marketed is significantly different from the version that was crash tested, we reserve the right to modify or revoke our acceptance.
- You will be expected to supply potential users with sufficient information on design and installation requirements to ensure proper performance.
- You will be expected to certify to potential users that the hardware furnished has essentially the same chemistry, mechanical properties, and geometry as that submitted for acceptance, and that it will meet the crashworthiness requirements of the FHWA and the MASH.
- To prevent misunderstanding by others, this letter of acceptance is designated as number B-200 and shall not be reproduced except in full. This letter and the test documentation upon which it is based are public information. All such letters and documentation may be reviewed at our office upon request.
- The West Virginia Steel Bridge Railing for use on Transverse, Nail-Laminated, Timber Bridges is a generic system and not considered proprietary.
- This acceptance letter shall not be construed as authorization or consent by the FHWA to use, manufacture, or sell any patented device for which the applicant is not the patent holder. The acceptance letter is limited to the crashworthiness characteristics of the candidate device, and the FHWA is neither prepared nor required to become involved in issues concerning patent law. Patent issues, if any, are to be resolved by the applicant.

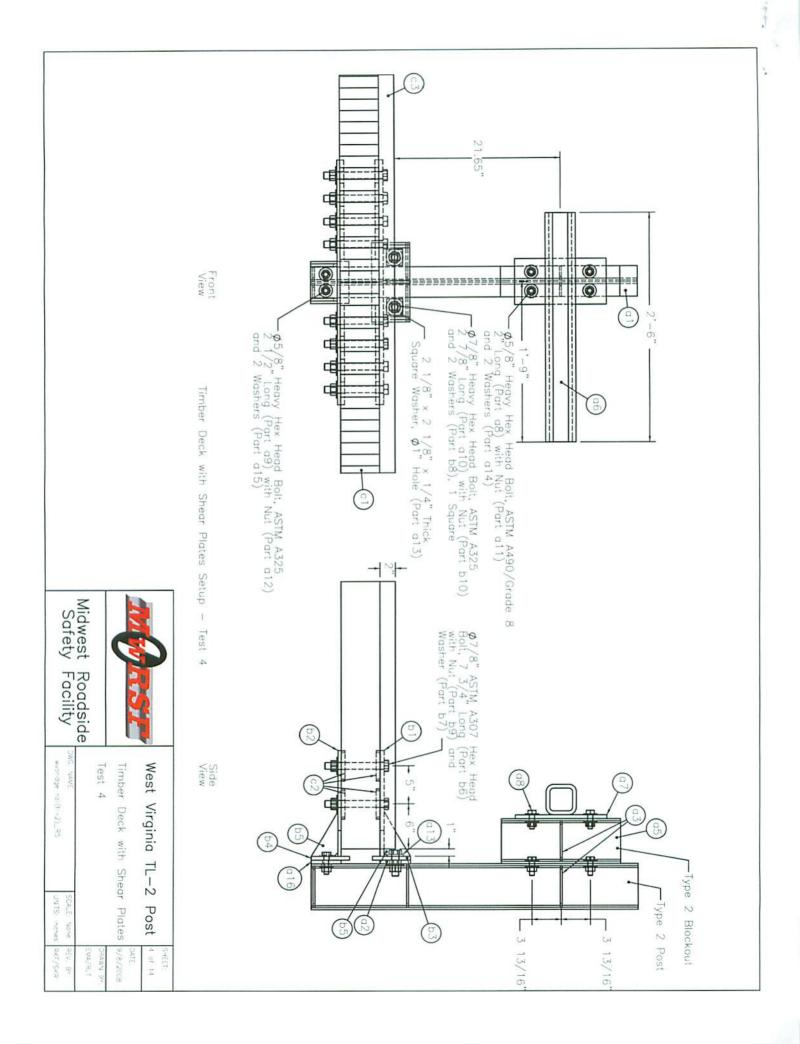
Sincerely yours,

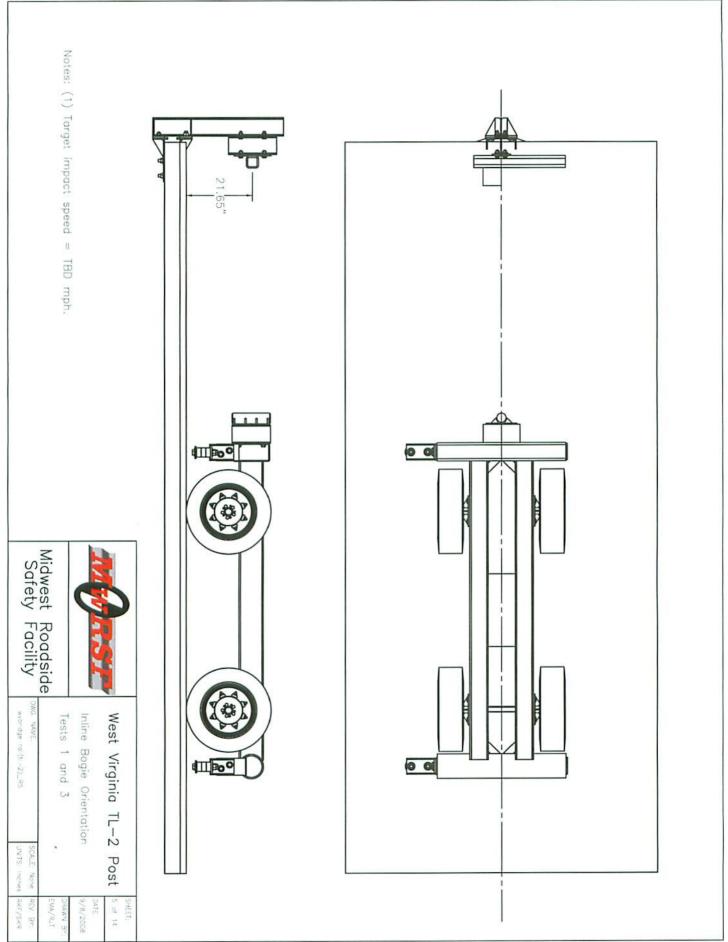
David A. Nicol, P.E. Director, Office of Safety Design Office of Safety

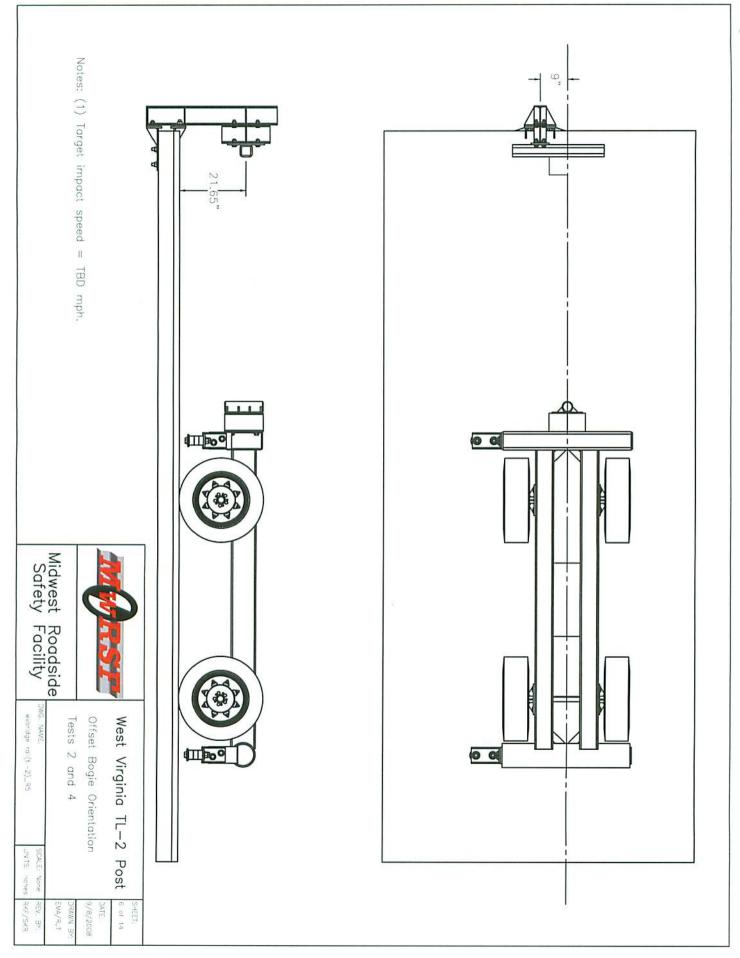


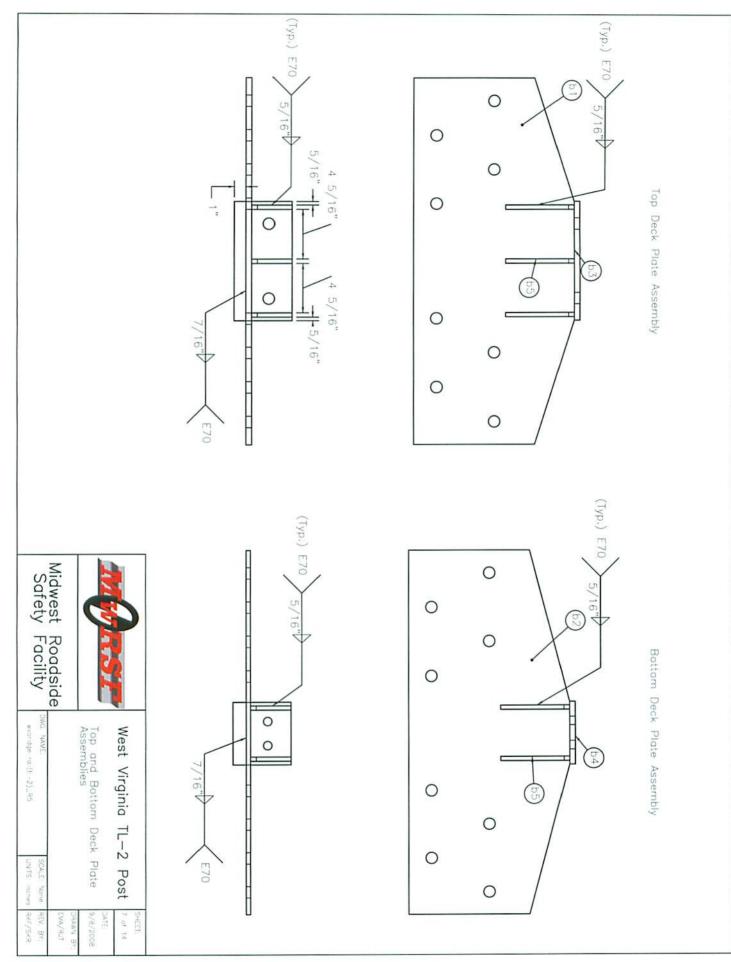




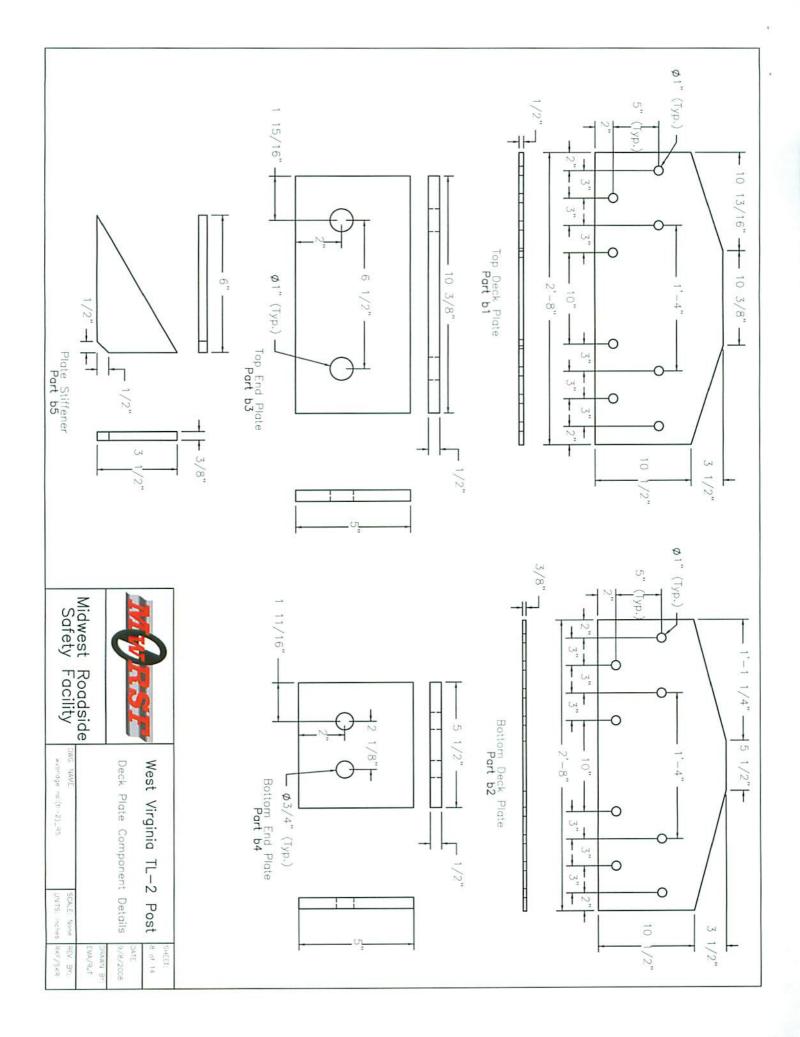


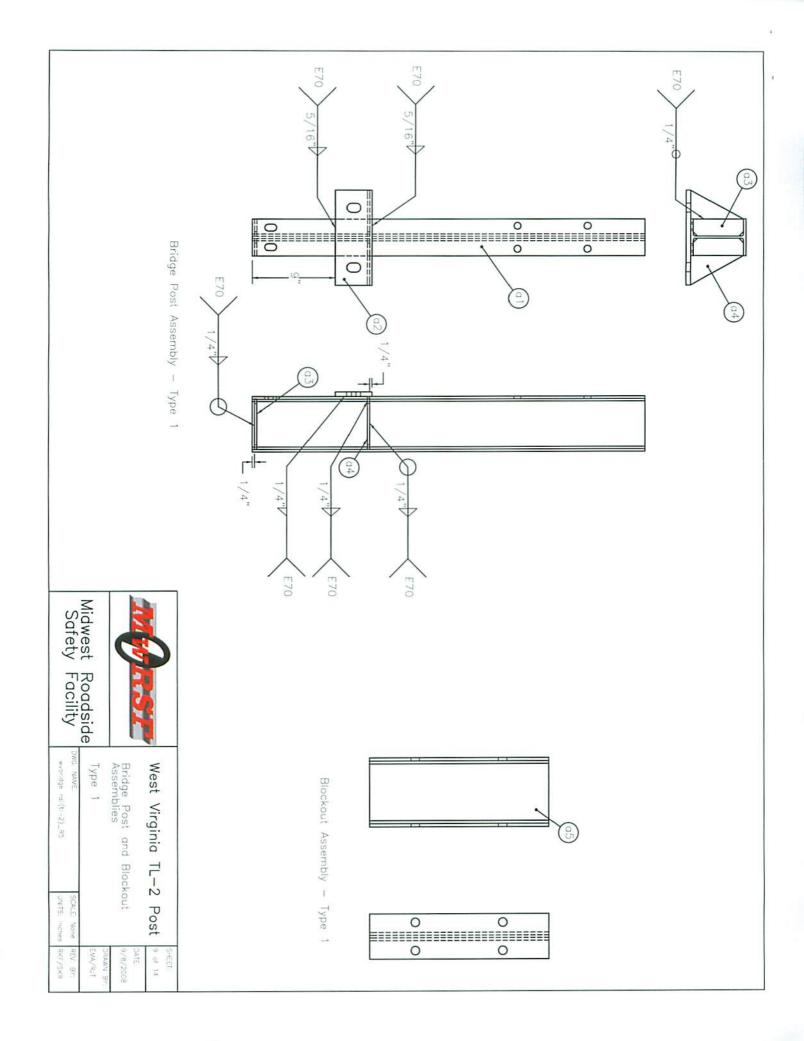


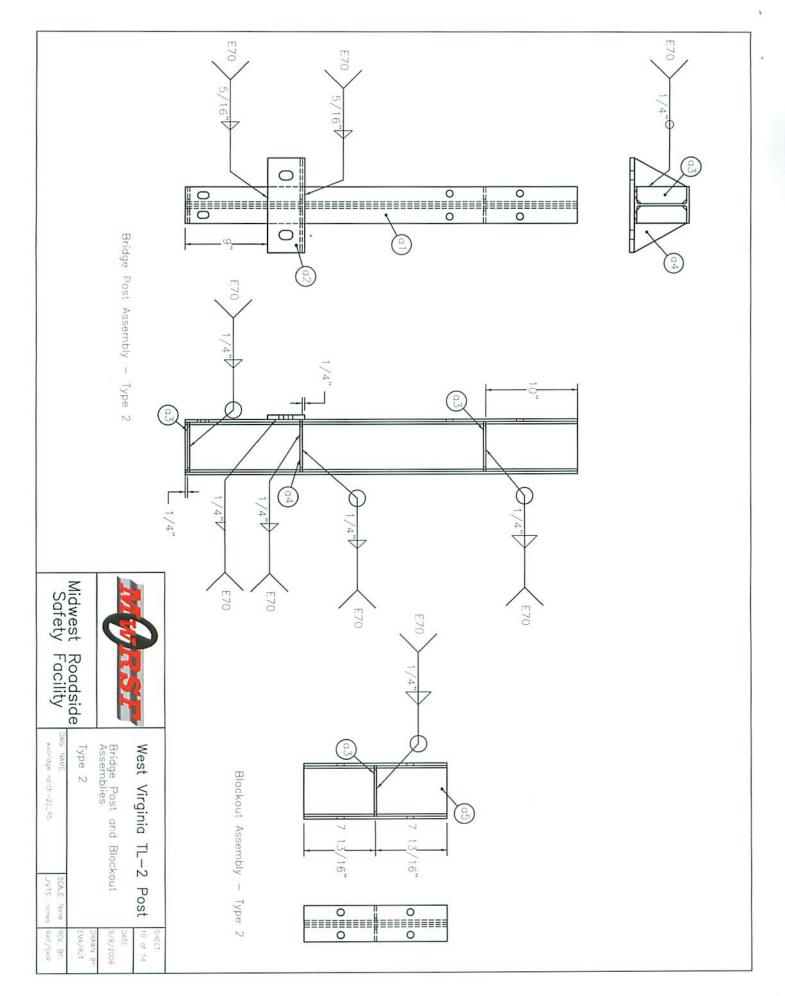


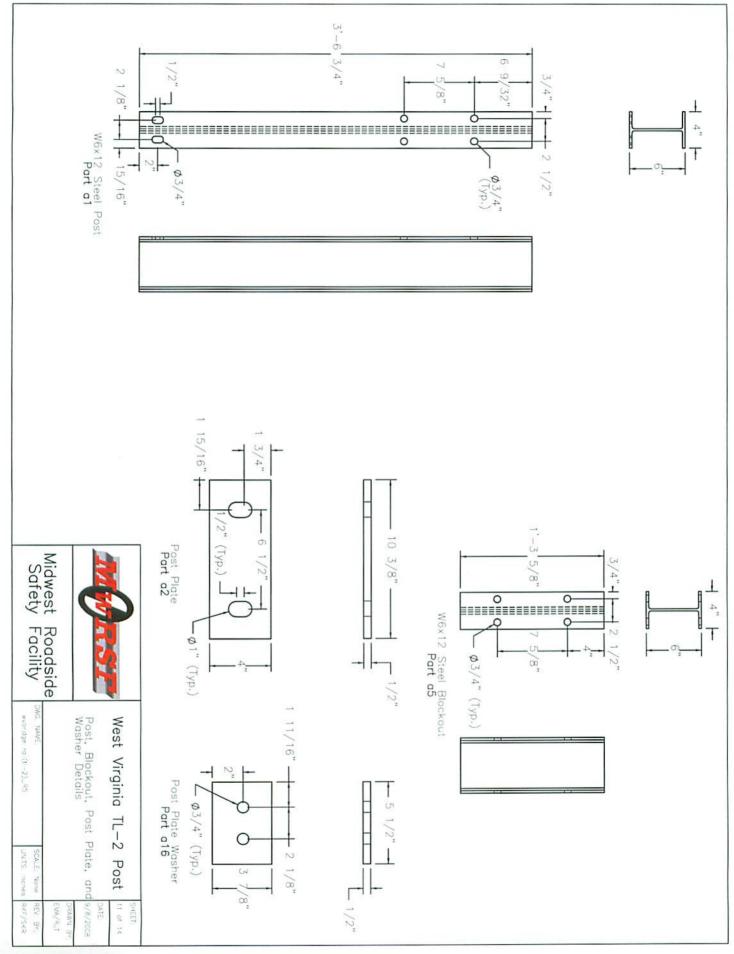


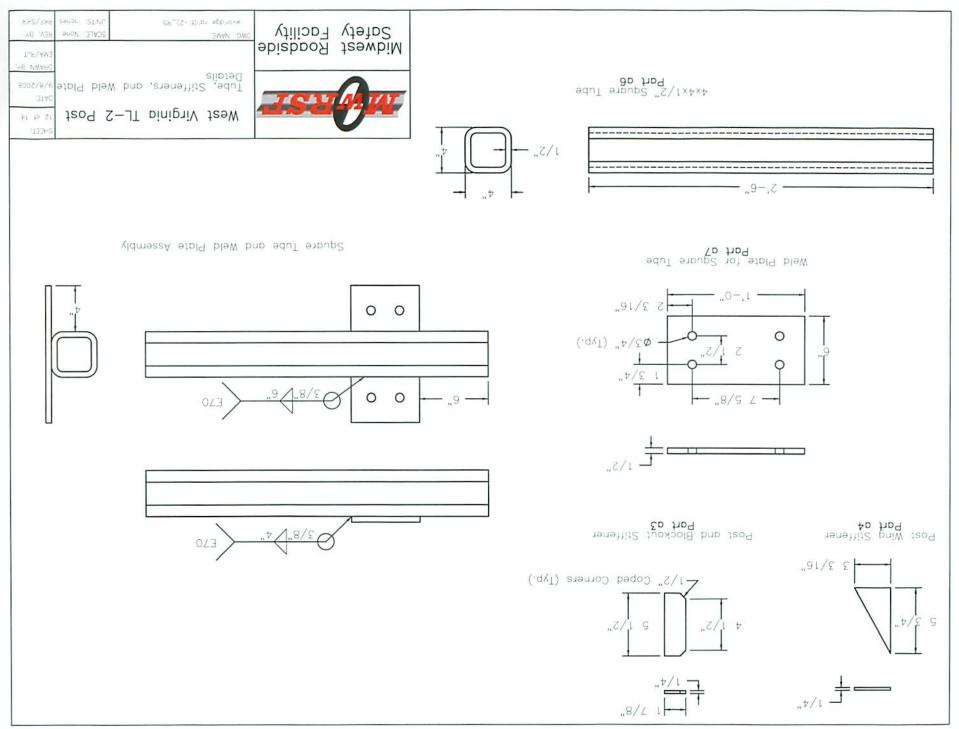
.

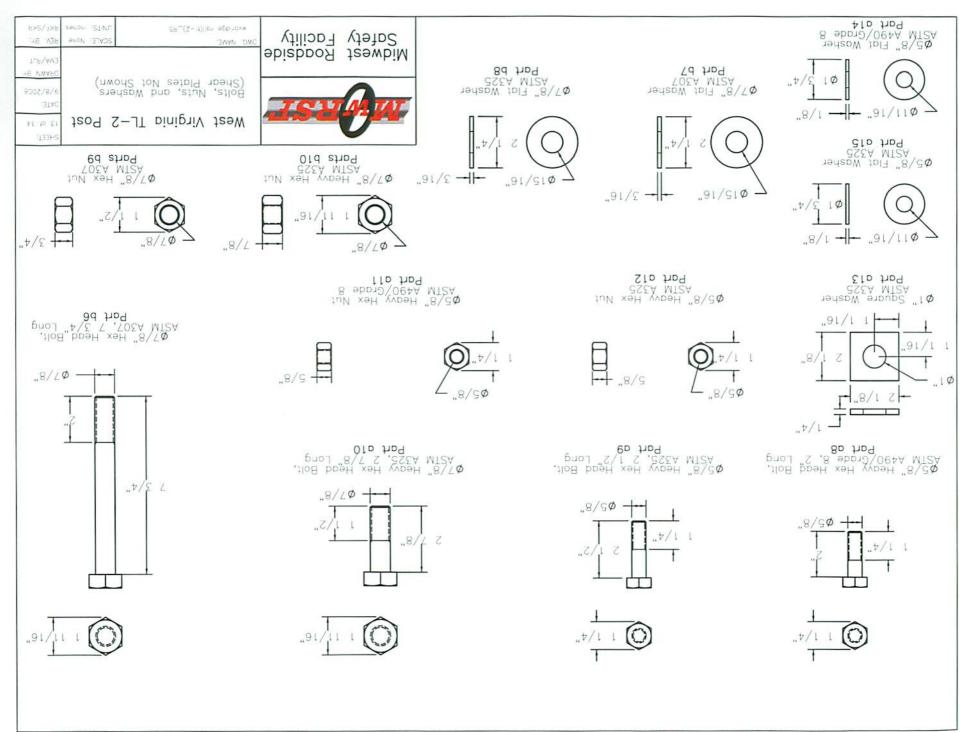












Mest Virginia TL-2 Post Midwest Roadside Safety Facility Materials for Four Post Midwest Roadside Midwest Pacility Midwest Pacility			
:133HS	tipriqeA	L	ξp
Galvanized Steel	ø4" Shear Plate for Ø7/8" bolts	32	2 C2
Southern Yellow Pine No. 1	2"x6"x14" Long Treated, Dimensional Lumber (0.60 lbs retention)	096	to.
SSEA	τυν xeh (van "8/ς	8	019
ΓΟΣΑ	JuN xeH "8\7	32	69
ZSEA	7/8" Flat Washer	8	89
ζοξά	7/8" Flat Washer	32	Zq
ΓΟΣΑ	7/8" Hex Head Bolt 7 3/4" Long	32	99
92A	Plate Stiffener	50	Gd
9ΣA	Bottom End Plate	4	t q
θ£A	Top End Plate	4	£d
θΣΑ	Bottom Deck Plate	7	P5
θ£A	Top Deck Plate	4	١٩
θ£A	Post Plate Washer	4	910
S22A	5/8" Flat Washer	91	GID
8 90/Octade 8	5/8" Flat Washer	49	⊅†D
92A	Square Washer	8	ςţD
SSEA	5/8" Heavy Hex Nut	8	als
8 9PDJO/06+4	5/8" Heavy Hex Nut	32	L L D
ZSZA	7/8" Heavy Hex Head Bolt 2 7/8" Long	8	OlD
ZSZA	5/8" Heavy Hex Head Bolt 2 1/2" Long	8	60
8 9bor0/064A	5/8" Heavy Hex Head Bolt 2" Long	32	ag
92A	Weld Plate for Square Tube	5	LD
A500 Grade B or C	4x4x0.5" Square Tube 30" Long (One per test series)	Z	90
02 STEA to SeeA	M6x12x15 5/8" Steel Blockout	4	gp
92A	Post Wing Stiffener	8	4D
92A	Post Stiffener	24	ξD
θΣΑ	Post Plate	4	QZ
02 or 5572 Grade 50	Teog "4, 52 3/4" Post	4	1 D
Material Specification	Description	.YTQ	.oN meil

.

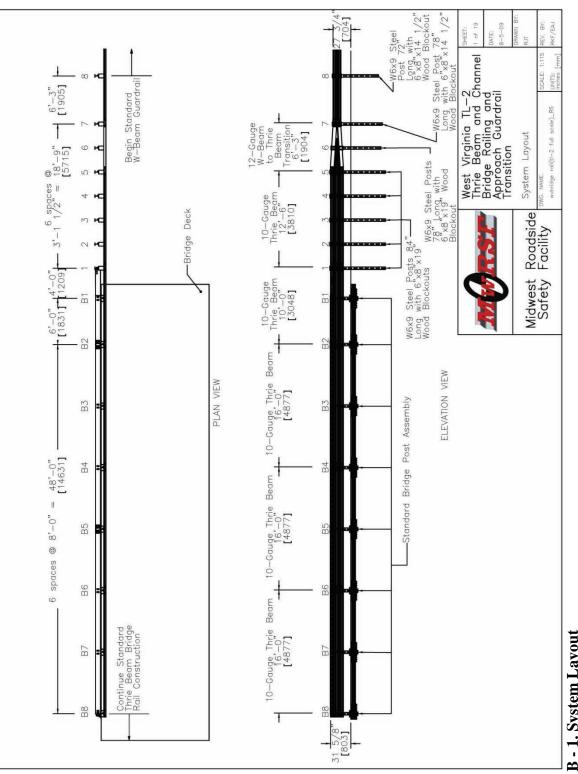


Figure B - 1. System Layout

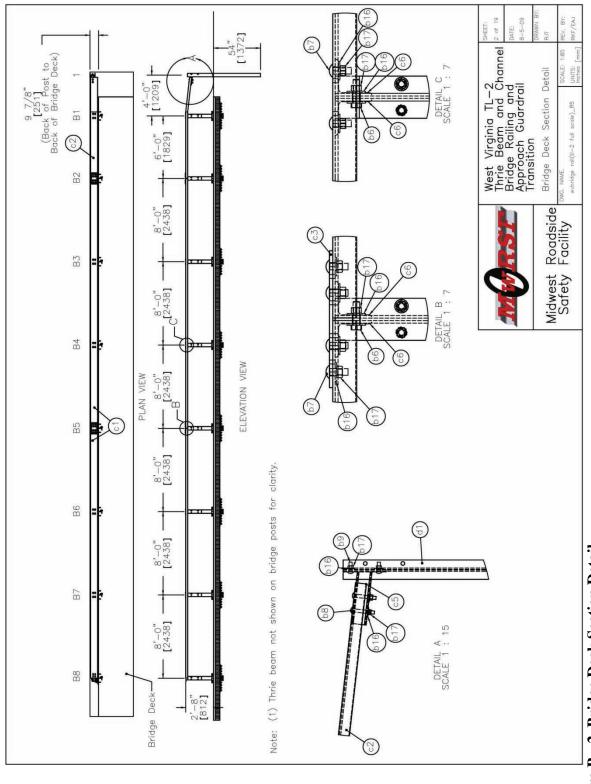
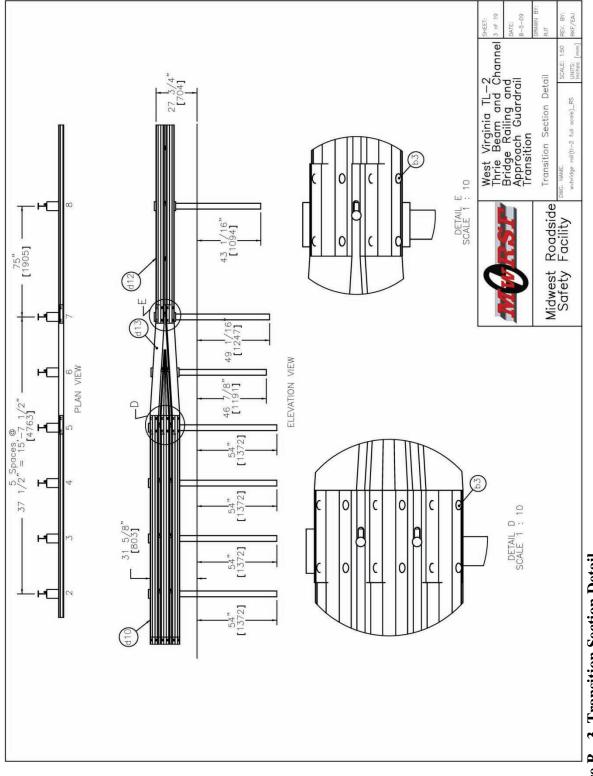


Figure B - 2. Bridge Deck Section Detail





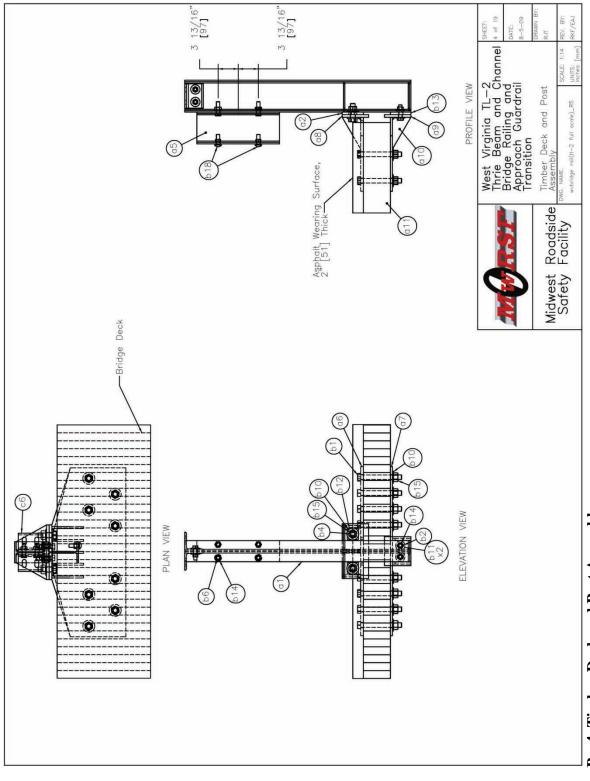
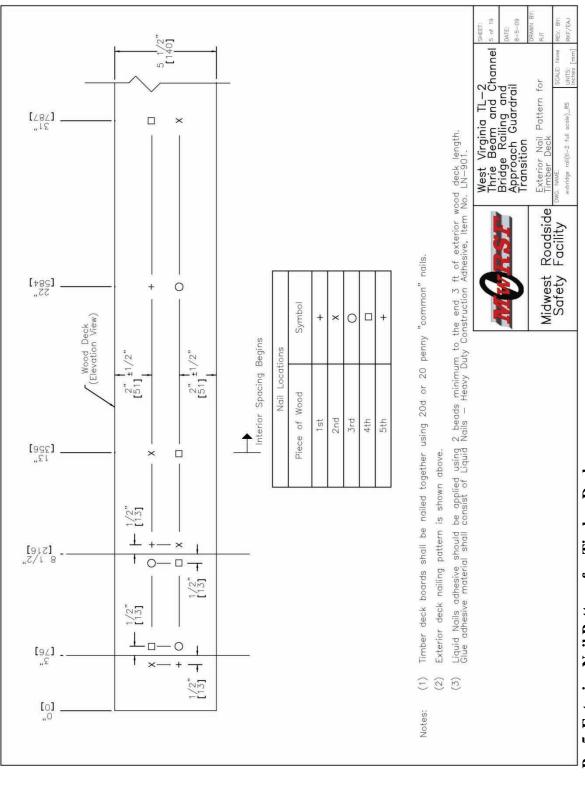
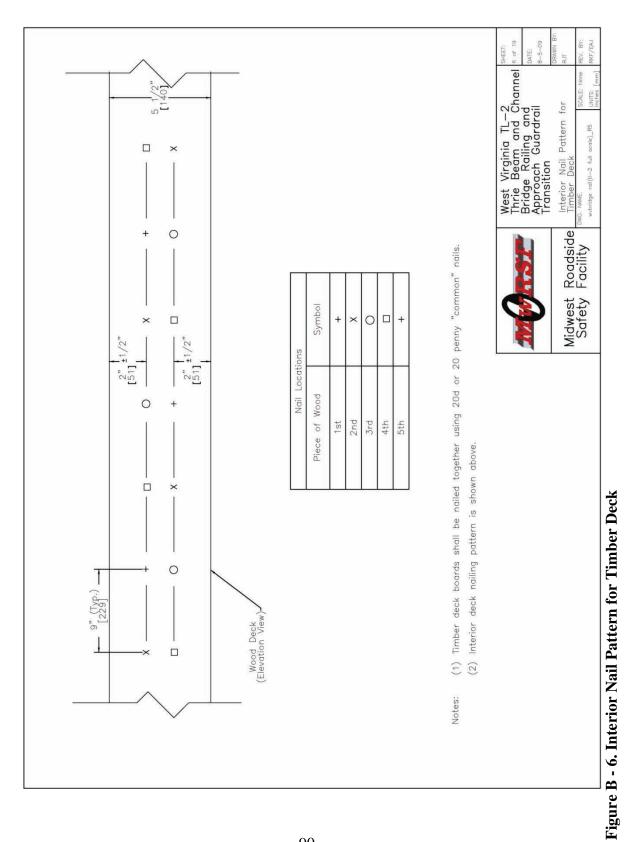


Figure B - 4. Timber Deck and Post Assembly

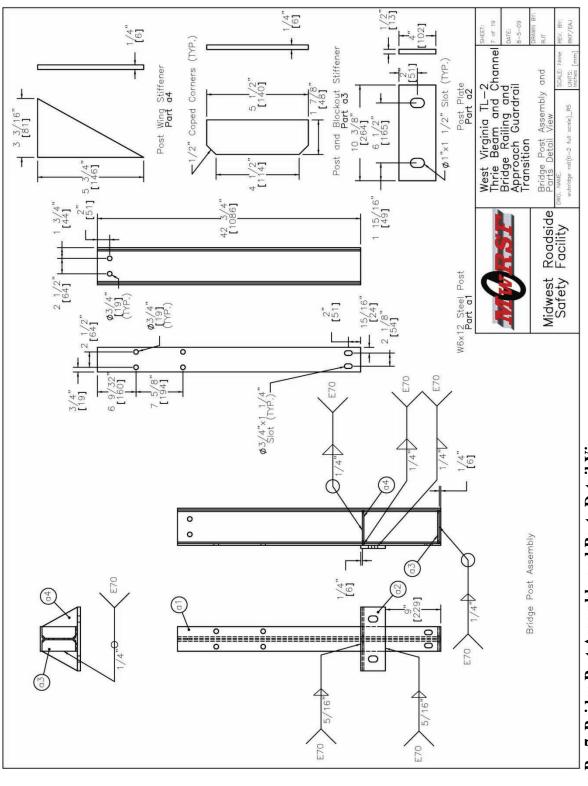




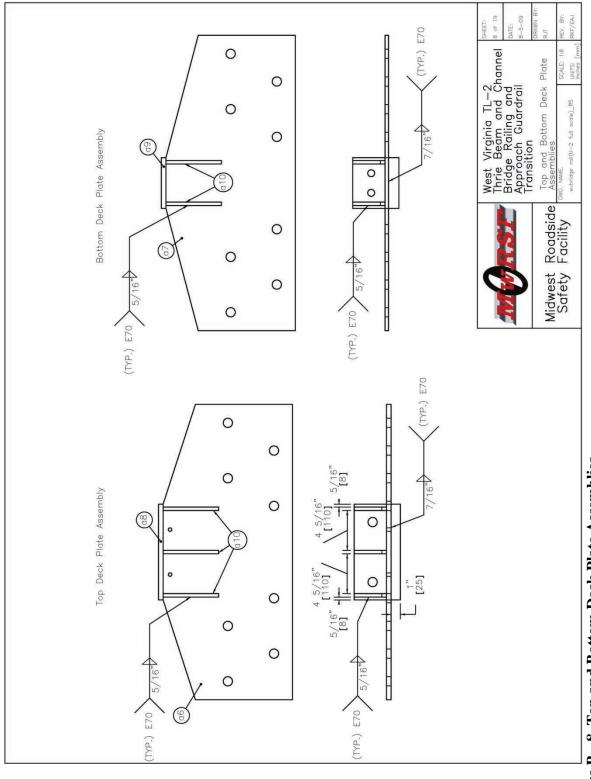
MwRSF Report No. TRP-03-212-09 August 13, 2009



90









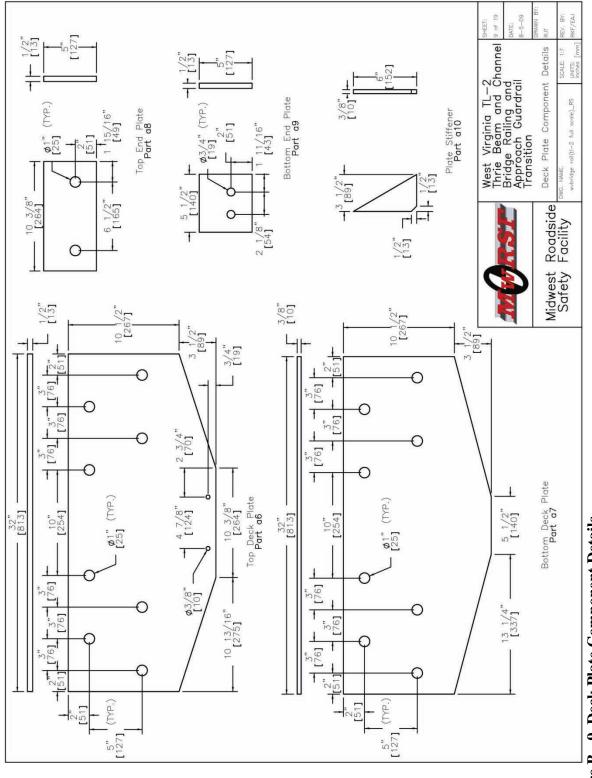
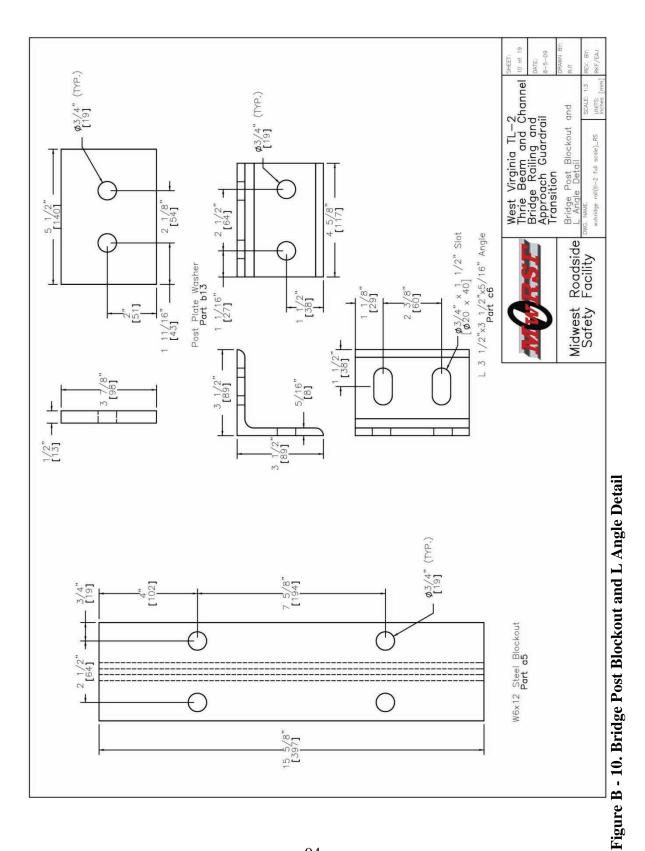


Figure B - 9. Deck Plate Component Details



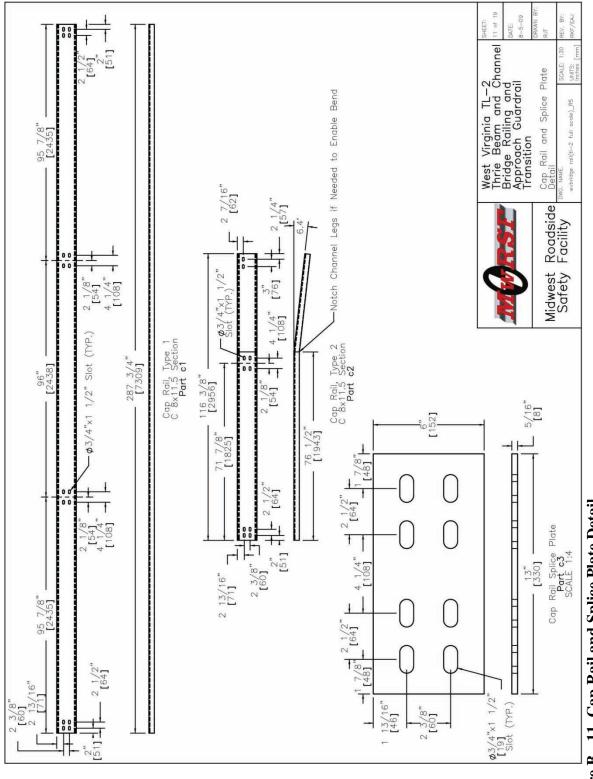
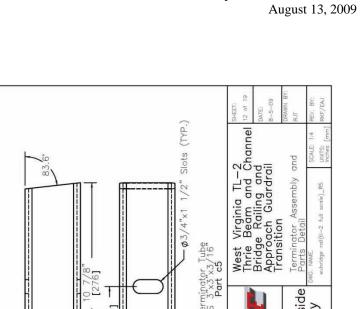
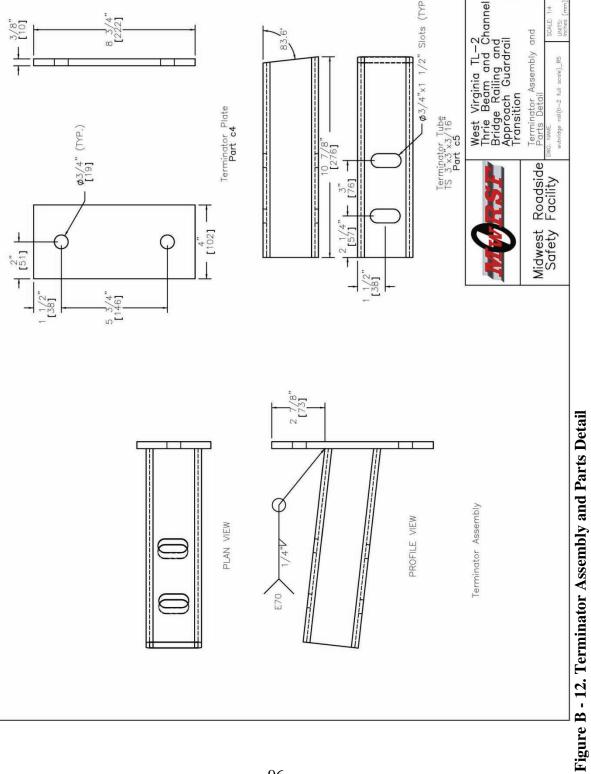
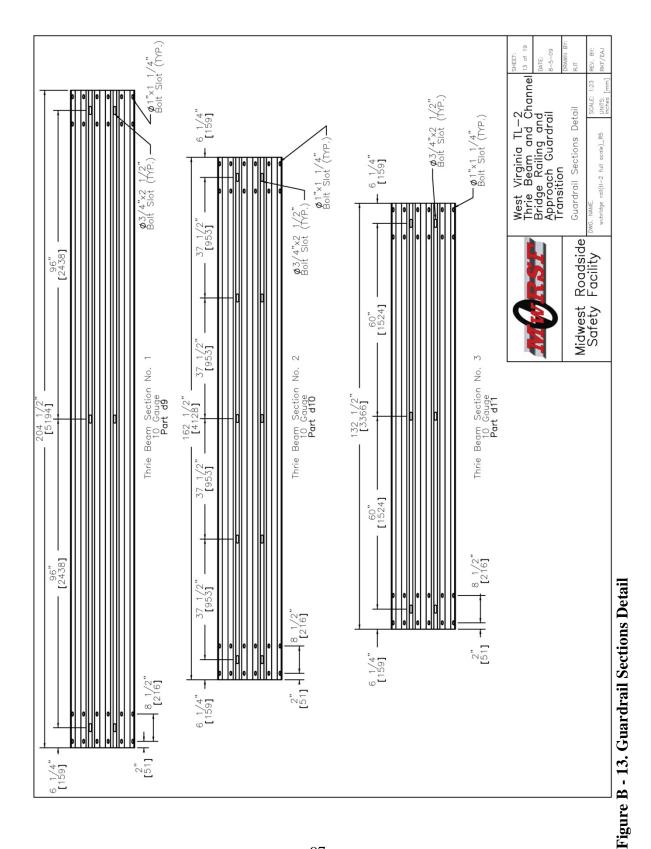
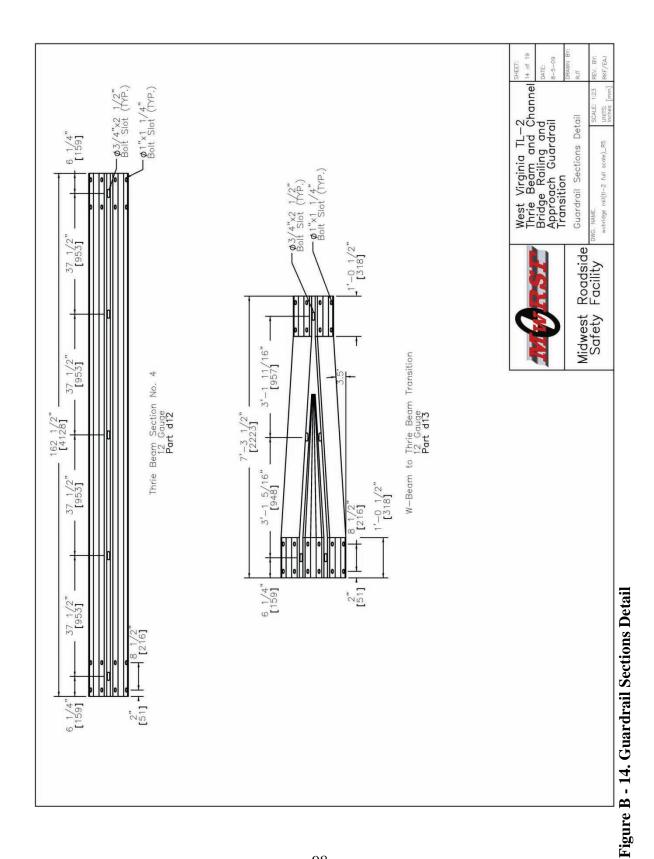


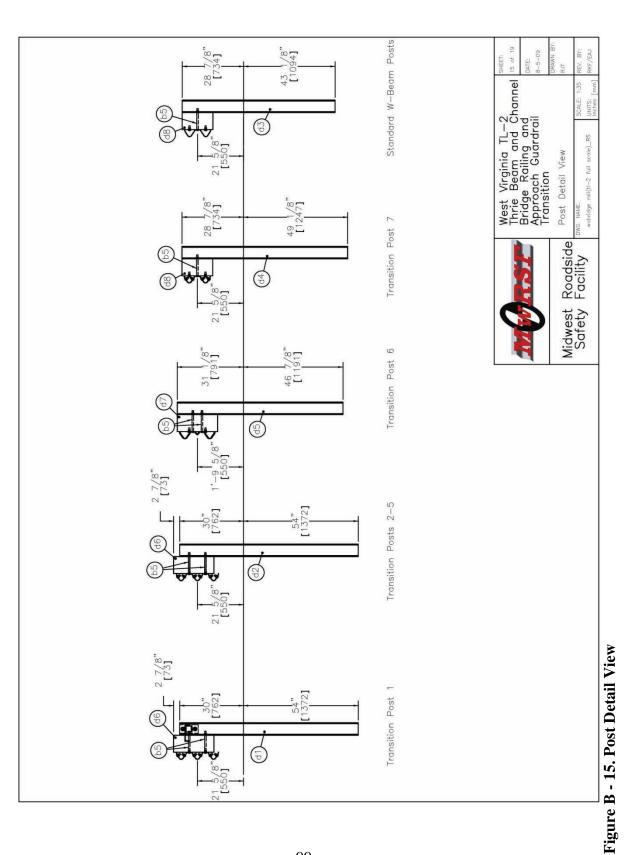
Figure B - 11. Cap Rail and Splice Plate Detail



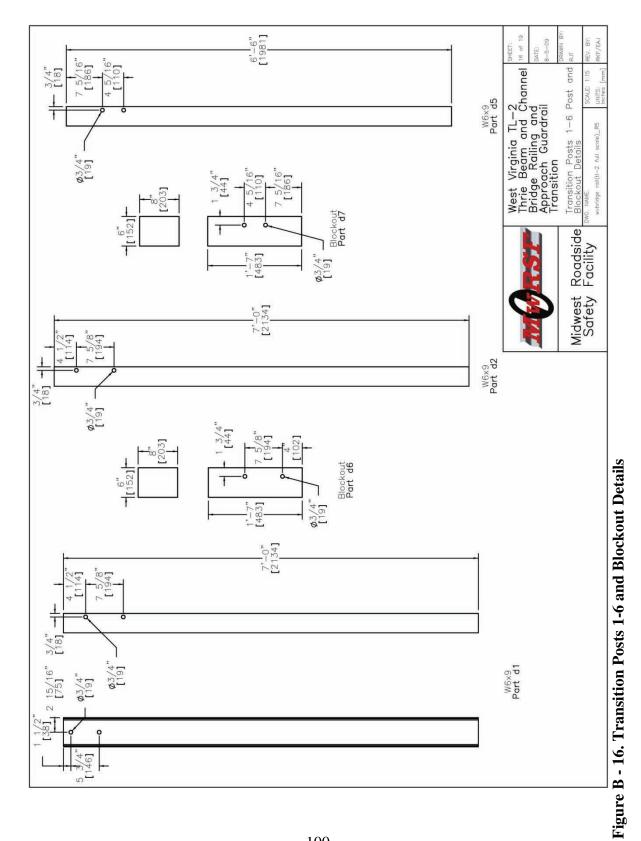


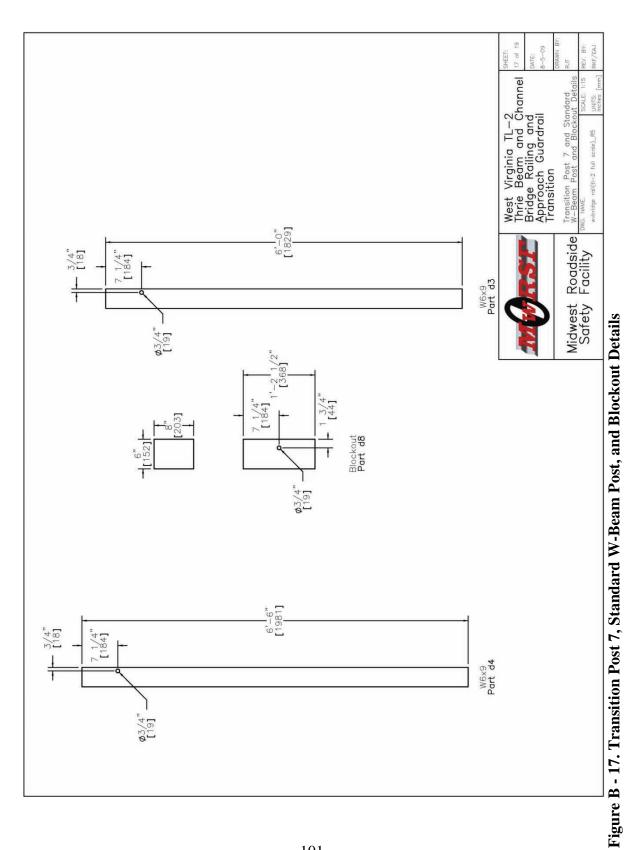






99





101

Lett NO.	QTY.	Description	Material Specification	tion Hardware Guide
a1	œ	W6x12x42 3/4" Post	A992 or A572 Grade 50	de 50 -
a2	ß	Post Plate	A36	a
a3	32	Post Stiffener	A36	I
a4	16	Post Wing Stiffener	A36	1
a5	80	W6x12x15 5/8" Steel Blockout	A992 or A572 Grade	de 50 -
a6	8	Top Deck Plate	A36	
a7	80	Bottom Deck Plate	A36	
a8	00	Top End Plate	A36	E
a9	80	Bottom End Plate	A36	E
a10	40	Plate Stiffener	A36	1
a11	1000	g Treated, Dime	Southern Yellow Pine	: No. 1 –
b1	64	\$7/8" Heavy Hex Head Bolt 7 3/4" Long	A325 Type 1	I?
b2	16	Ø5/8" Heavy Hex Head Bolt 2 1/2" Long	A325 Type 1	Т
b3	80	1 1/2" Guardrail Bolt and Nu	A307 Grade A	FBB01
b4	16	Ø7/8" Heavy Hex Head Bolt 2 7/8" Long	A325 Type 1	1
b5		Ø5/8"×10" Guardrail Bolt and Nut	A307 Grade A	FBB03
b6	48	Ø5/8"x2 1/4" Long Hex Bolt	A307 Grade A	1
b7		Ø5/8"×1 3/4" Round Head Bolt	A307 Grade A	-
b8		Ø5/8"x4 1/2" Round Head Bolt	A307 Grade A	-
b9		Ø5/8"x2" Round Head Bolt	A307 Grade A	-
b10		Ø7/8" Flat Washer	F436 Gr. 1	FWC22a
b11	32	Ø5/8" Flat Washer	F436 Gr. 1	FWC16a
b12	16	Ø1" Square Washer	A36	1
b13	80	Post Plate Washer	A36	1
b14	16	Ø5/8" Hex Nut	A563DH	
b15	80	Ø7/8" Hex Nut	A563DH	E
b16	72	Ø5/8" Flat Washer	F844	3
b17	88	Ø5/8" Hex Nut	A563A	1
b18	16	Ø5/8"x2" Guardrail Bolt and Nut	A307 Grade A	FBB02
c1	2	1000	A36	
c2	L		A36	1
c3	2	Cap Rail Splice Plate	A36	
c4	-	Terminator Plate	A36	1
c5	1	Terminator Tube	A500 Grade B	-
c6	16	3 1/2"x3 1/2"x5/16" L Angle	A36	ан Т
				West Virginia TL-2 Thrie Beam and Channel Bridge Railing and Approach Guardrail
			Rondside	Transition Bill of Materials
			Safety Facility	DWG. NAME. SCALE: None SCALE: None wobridge rail(t)-2 full scale)_R5 UNITS: f1

	Hardware Guide	ŋ	1	1	1	j	T	J.	PDB09	RTM02b	RTM04b	RTM02b	RWM04a	RWT01a	West Virginia TL-2 Thrie Beam and Channel Bridge Railing and Approach Guardrail Transition Bill Of Materials (Continued) Bill Of Materials (Continued) Bill Of Materials (Continued) Motor Materials (Continued) Materials (Continued) Rar
ach Transition	Material Specification	A36	A36	A36	A36	A36	Southern Yellow Pine No. 1	Southern Yellow Pine No. 1	Southern Yellow Pine No. 1	10 gauge AASHTO M180	10 gauge AASHTO M180	10 gauge AASHTO M180	12 gauge AASHTO M180	12 gauge AASHTO M180	Midwest Roadside Bill Of West
West Virginia TL-2 Bridge Railing and Approach Transition	Description	W6x9 84" Long, Post 1	W6x9 84" Long, Posts 2-5	W6x9 72" Long, Post 8	W6x9 78" Long, Post 7	W6x9 78" Long, Post 6	6x8x19" Blockout	6x8x19" Blockout	6x8x14 1/2" Blockout	12'-6" Thrie Beam Section	12'-6" Thrie Beam Section - 1/2 Post Spacing	10' Thrie Beam Section	12'-6" W-Beam Section - 1/2 Post Spacing	6'-3" W-Beam to Thrie Beam Transition Section	
	ατγ.	-	4	-	~	~	5	-	2	3	-	577	~		
	Item No.	d1	d2	d3	d4	d5	d6	ζþ	dð	6р	d10	d11	d12	d13	



MwRSF Report No. TRP-03-212-09 August 13, 2009