

January 12, 2010

In Reply Refer To: HSSD/B-198

Mr. Ronald K. Faller, Ph.D Research Assistant Professor Midwest Roadside Safety Facility University of Nebraska-Lincoln 527 Nebraska Hall Lincoln, NE 68588-0529

Dear Dr. Faller:

You requested that we formally find this device acceptable for use on the National Highway System (NHS) under the provisions of American Association of State Highway and Transportation Officials, Manual for Assessing Safety Hardware (MASH). This letter is a response to your request for the Federal Highway Administration (FHWA) acceptance of a roadside safety device for use on the NHS.

Name of device: West Virginia TL-1 Timber Curb-Type Bridge Barrier Type of device: Permanent Timber Barrier Bridge Railing Test Level: AASHTO Manual for Assessing Safety Hardware (MASH) TL-1 Testing conducted by: Midwest Roadside Safety Facility (MwRSF) Date of request: September 13, 2009 Date of completed package: September 13, 2009 Task Force 13 Designator: SBD05a

# Requirements

Roadside safety devices should meet the guidelines contained in the MASH.

# Description

For this project, the research objective was to adapt an existing, crashworthy, Test Level 1 (TL-1) curb-type bridge barrier for use on transverse, timber, nail-laminated bridge decks supported by steel wide-flange beams. The barrier system was redesigned to meet the TL-1 impact safety standards set forth by MASH. The existing MwRSF curb-type timber bridge barrier system was successfully crash tested to the TL-1 safety performance criteria as per



NCHRP Report 350. This timber bridge barrier system served as the basis for the new West Virginia TL-1 Timber Curb-Type Bridge Barrier design. However, the railing for this project was required to meet the TL-1 safety performance criteria provided in MASH. Therefore, modifications were made to the previous system in order to accommodate the increased impact severity and increased vehicle height resulting from the 2270P vehicle as per current MASH guidelines. These changes are described as follows.

The test installation consisted of three major sub-systems: (1) a 120-ft (36.6-m) long, nail-laminated, timber bridge deck placed on wide-flange, steel girders; (2) a curb-type, timber bridge railing system; and (3) a 35-ft (10.7-m) long, sloped, safety treatment located on the upstream end of the bridge barrier. The total length of the test installation was 155 ft (47.2 m).

The bridge barrier system consisted of three major structural components: (1) a longitudinal, glulam timber rail; (2) steel H-splice plates; and (3) post assemblies consisting of sawn lumber scupper blocks.

The glulam timber rail consisted of 19.9-ft (6.08-m) long, glulam rail segments with a 6-3/4 in. x 12 3/8 in. (171 mm x 314 mm) cross section, as shown in Figure 36. The glulam rails were manufactured from Combination No. 48 Southern Yellow Pine and were treated with pentachlorophenol in heavy oil to a minimum net retention of 0.60 lbs/ft<sup>3</sup> (9.61 kg/m<sup>3</sup>) conforming to the American Wood Preserver's Association (AWPA) use category UC4A [8]. The ends of each rail segment were narrowed to a width of 11 5/8 in. (295 mm) in order to accept the steel H-splice plates and allow the outer plate surface to be flush with the gross rail section.

The steel H-Splice plates were fabricated from ASTM A656 Grade 50 Type 7 material. The two side plates were 34 3/4 in. long by 6 3/4 in. wide by 3/8 in. thick (883 mm x 171 mm x 9.5 mm) with twelve 1 1/8-in. (29-mm) diameter holes. For the H-splice, the center connecting plate was installed orthogonal to the outer two plates and measured 11 5/8 in. long by 6 3/4 in. wide by 3/8 in. thick (295 mm x 171 mm x 9.5 mm). The H-splice plates connected adjacent glulam rail segments end to end using six 1-in. (25.4-mm) diameter by 14-in. (356-mm) long, ASTM A307 galvanized dome-head bolts in each rail end.

The bridge rail post assemblies consisted of two timber scupper blocks stacked on top of each other. Each scupper block was fabricated from Grade No. 1 Southern Yellow Pine sawn lumber and measured 23 in. long by 9 1/2 in. wide, and 7 1/2 in. tall (584 mm x 241 mm x 191mm). Four 13/16-in. (21-mm) diameter bolt holes were drilled in the scuppers at 5-in. (127-mm) spacing intervals.

The West Virginia TL-1 Timber Curb-Type Bridge Barrier drawings for the construction of the test installation are included with this correspondence.

### Findings

We concur with your request that the West Virginia TL-1 Timber Curb-Type Bridge Barrier that has been successfully crash tested to AASHTO MASH TL-1 conditions will be considered

acceptable for use on the NHS, assuming all other factors such as timber specifications and component details are identical to the tested version. For further information on the crash test, the Test Data Summary Sheet is included with this correspondence.

Please note the following standard provisions that apply to the FHWA letters of acceptance:

- This acceptance is limited to the crashworthiness characteristics of the devices and does not cover their structural features, nor conformity with the Manual on Uniform Traffic Control Devices.
- Any changes that may adversely influence the crashworthiness of the device will require a new acceptance letter.
- Should the FHWA discover that the qualification testing was flawed, that in-service performance reveals unacceptable safety problems, or that the device being marketed is significantly different from the version that was crash tested, we reserve the right to modify or revoke our acceptance.
- You will be expected to supply potential users with sufficient information on design and installation requirements to ensure proper performance.
- You will be expected to certify to potential users that the hardware furnished has essentially the same chemistry, mechanical properties, and geometry as that submitted for acceptance, and that it will meet the crashworthiness requirements of the FHWA and the MASH.
- To prevent misunderstanding by others, this letter of acceptance is designated as number B-198 and shall not be reproduced except in full. This letter and the test documentation upon which it is based are public information. All such letters and documentation may be reviewed at our office upon request.
- The Timber Curb-Type Bridge Barrier system is a generic system and not considered proprietary.
- This acceptance letter shall not be construed as authorization or consent by the FHWA to use, manufacture, or sell any patented device for which the applicant is not the patent holder. The acceptance letter is limited to the crashworthiness characteristics of the candidate device, and the FHWA is neither prepared nor required to become involved in issues concerning patent law. Patent issues, if any, are to be resolved by the applicant.

Sincerely yours,

David A. Nicol, P.E. Director, Office of Safety Design Office of Safety

### Enclosures

FHWA:HSSD:WLongstreet:tb:x60087:12/16/09

- File: s://directory folder/WLongstreet/B198-FDOT Low Profile Barrier Space Restricitive Applications.doc
- cc: HSSD (Reader, HSA; Chron File, HSSD; W.Longstreet, HSSD; NArtimovich, HSSD; MMcDonough, HSSD)



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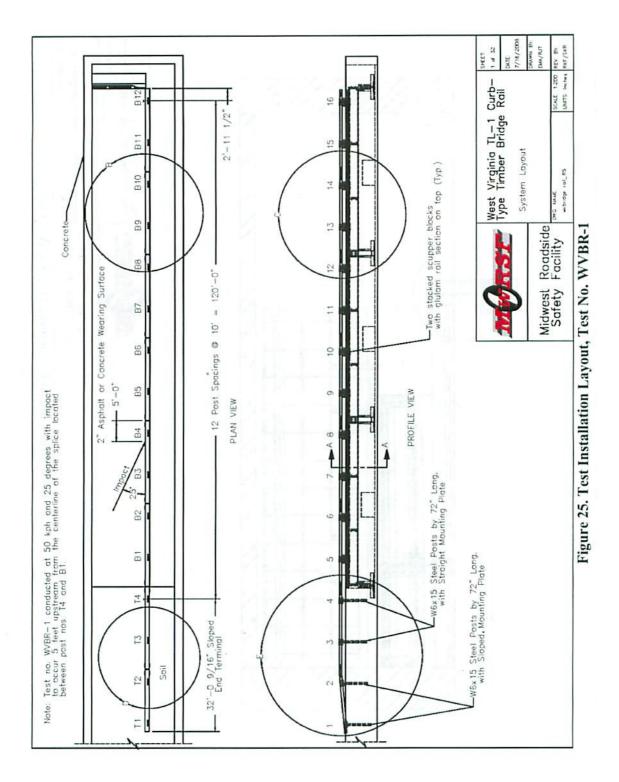
David A. Nicol, P.E. Director, Office of Safety Design Office of Safety

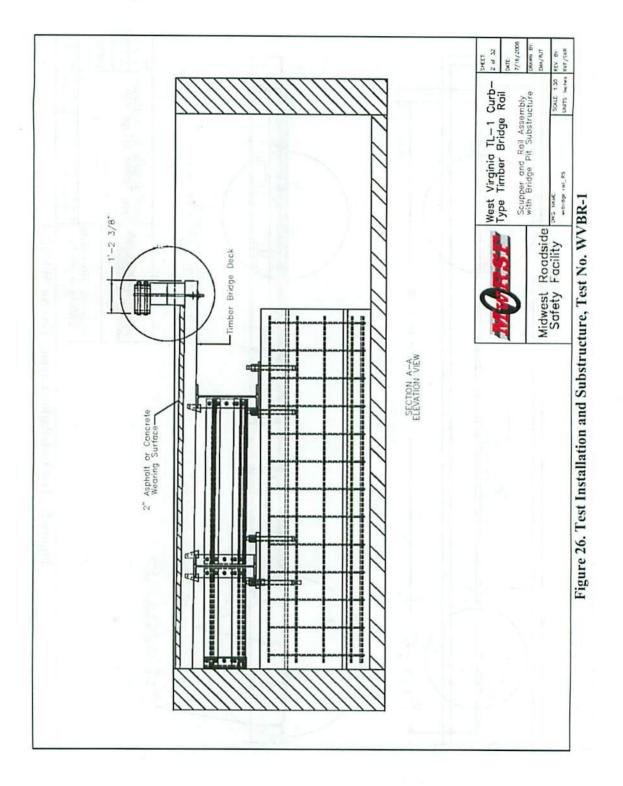
Enclosures

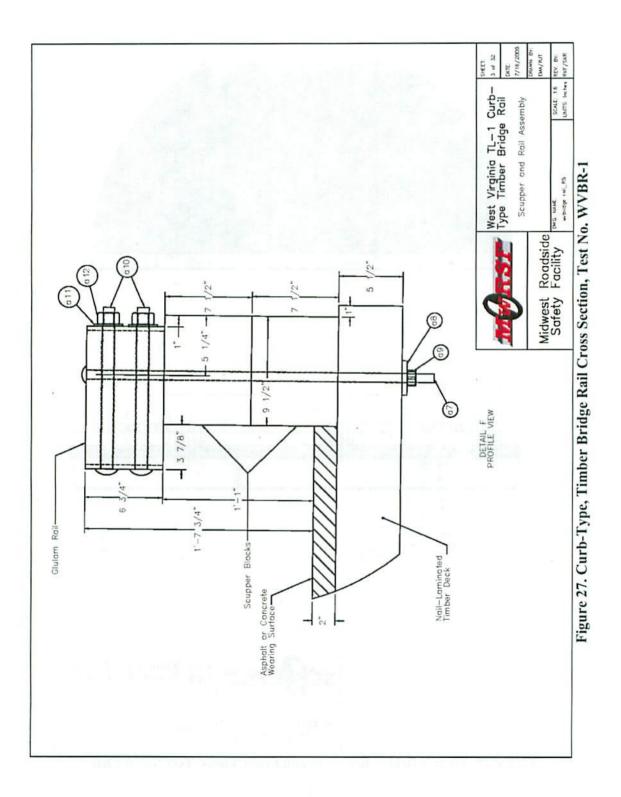
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1 2 3 4 5 6 7 8 9	10 11 12 13 14 15 16 10 10 10 10 10 10 10 10 10 10 10 10 10		(2) 9.5° 17.5° Scopper Blacks	- 11 - 11 - 12-54
			Auphan	
_			<u>+</u>	
	ation1-1		Nail Leminted Timber Deck	s
	Low-Profile, Curb-Type, Timber Bridge Rai	1		
			upant Ridedown Acceleration (DTS	
	Spacing		Longitudinal	
<ul> <li>Key Component - Glula</li> </ul>	am Bridge Rail Segment		Lateral	3.23 g's < 20.49 g's
Length		1 • Occ	upant Impact Velocity (DTS)	
9 Width			Longitudinal	
	12 ½ in		Lateral	
<ul> <li>Key Component - Scup</li> </ul>	per Block	• Ucc	upant Ridedown Acceleration (EDF	,
Length		I.	Longitudinal	
Width			Lateral	2.69 g s < 20.49 g s
		•	upant Impact Velocity (EDR-3)	
			Longitudinal	
	5,119 lb		Lateral	
			kimum Roll Angle	
	5,179 lb		V	
<ul> <li>Impact Conditions</li> </ul>			D	
-		-	icle Damage	Minimal
			TAD <sup>11</sup>	1-RFQ-3
		4	SAE <sup>12</sup>	
<ul> <li>Exit Conditions</li> </ul>			Maximum Deformation	
•			Article Damage	Minimai
		2	kimum Rail Deflections	<b>A</b> 4 1
	Satisfactor		Permanent Set	
<ul> <li>Vehicle Stopping Distant</li> </ul>	nce 120 ft Downstream from Impac	t	Dynamic	
		• Wo	rking Width	

Figure 65. Summary of Test Results and Sequential Photographs, Test No. WVBR-1

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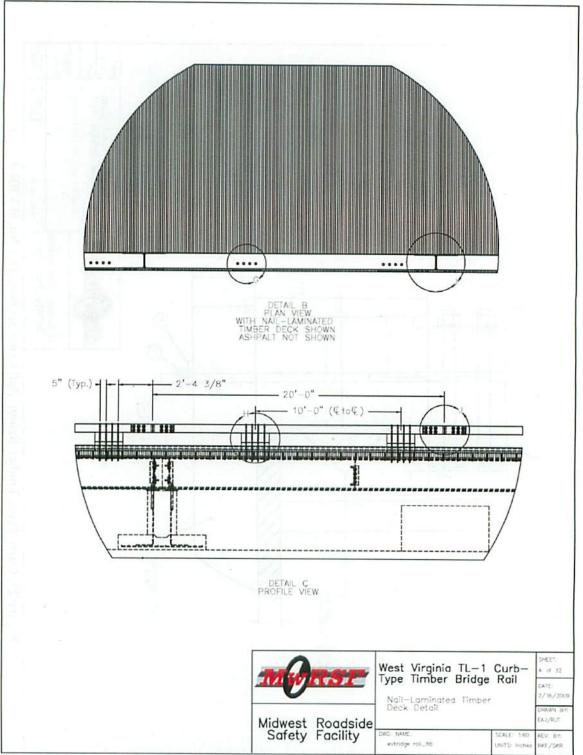
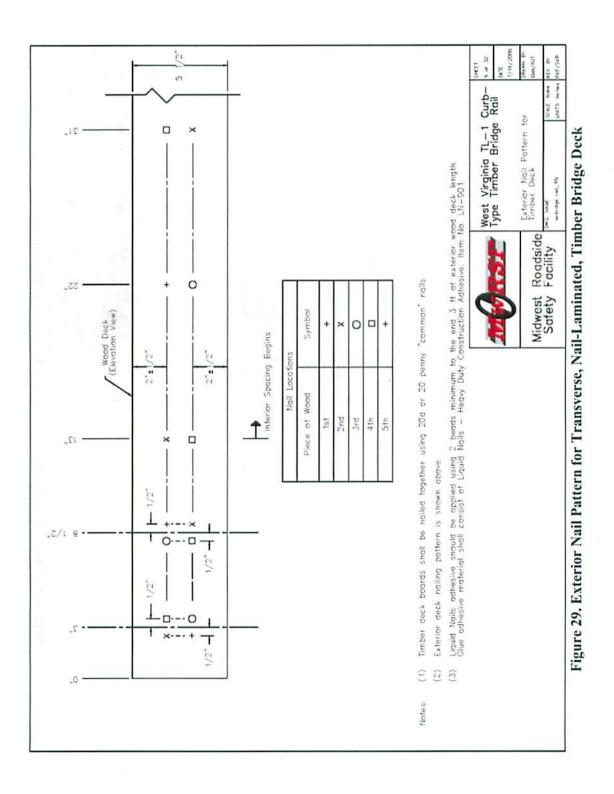
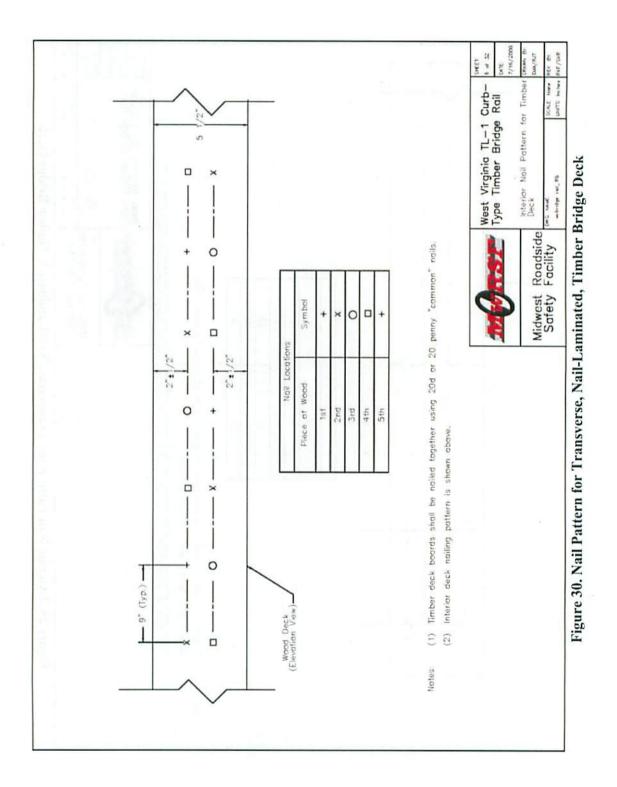


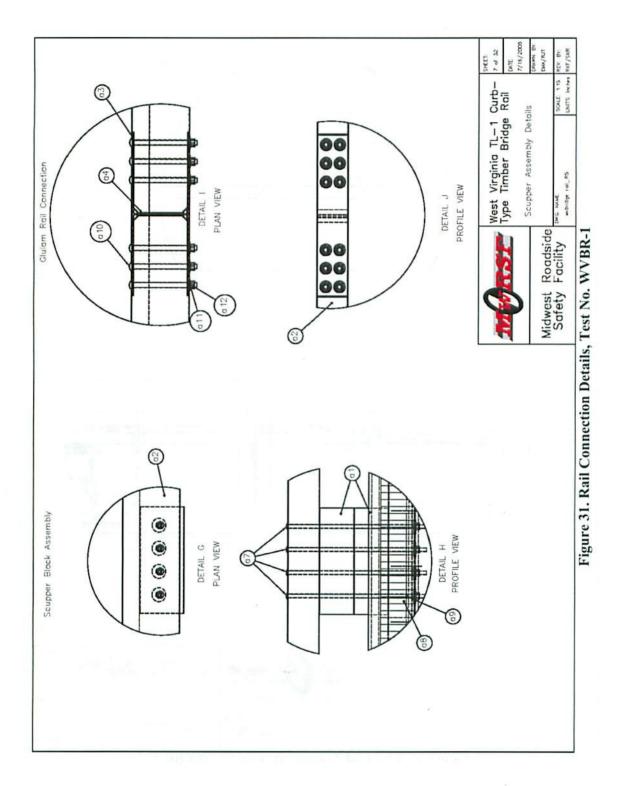
Figure 28. Timber Bridge Rail, Top and Front Views, Test No. WVBR-1





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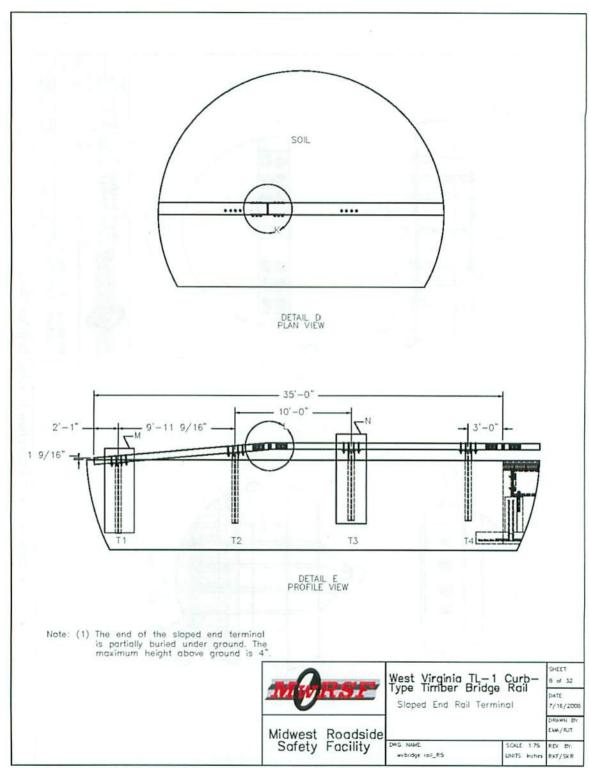


Figure 32. Rail End Treatment, Test No. WVBR-1

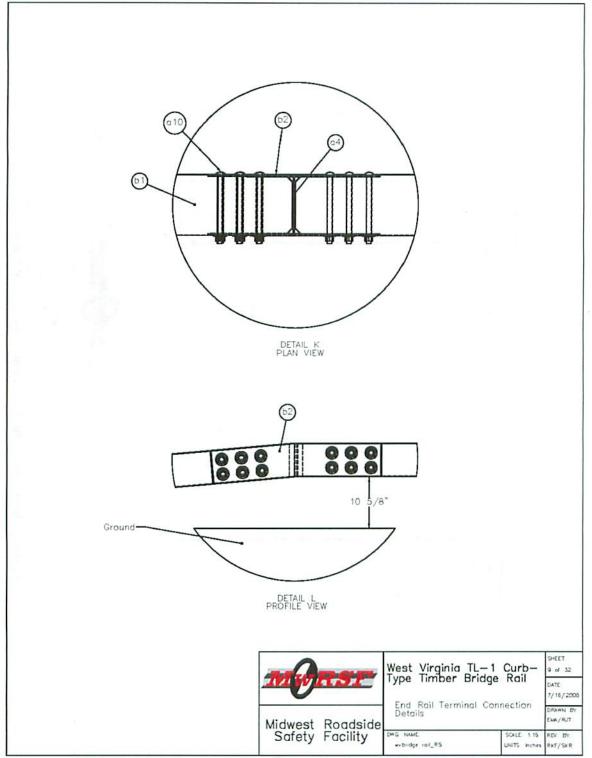
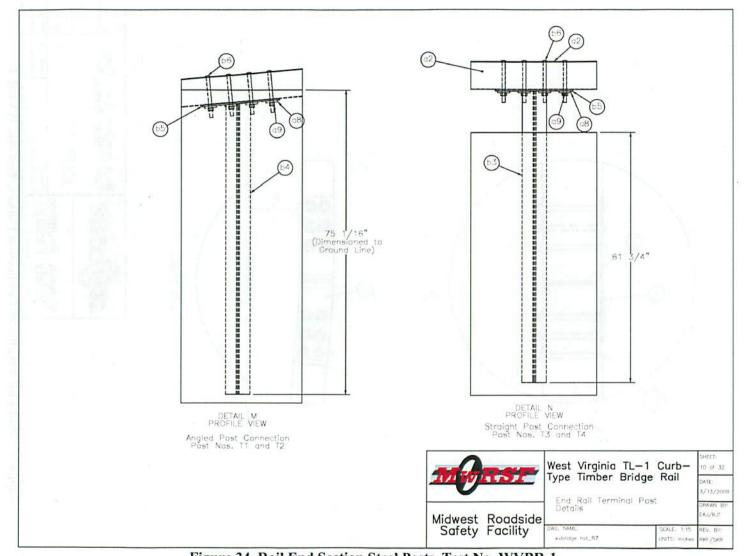
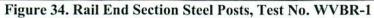
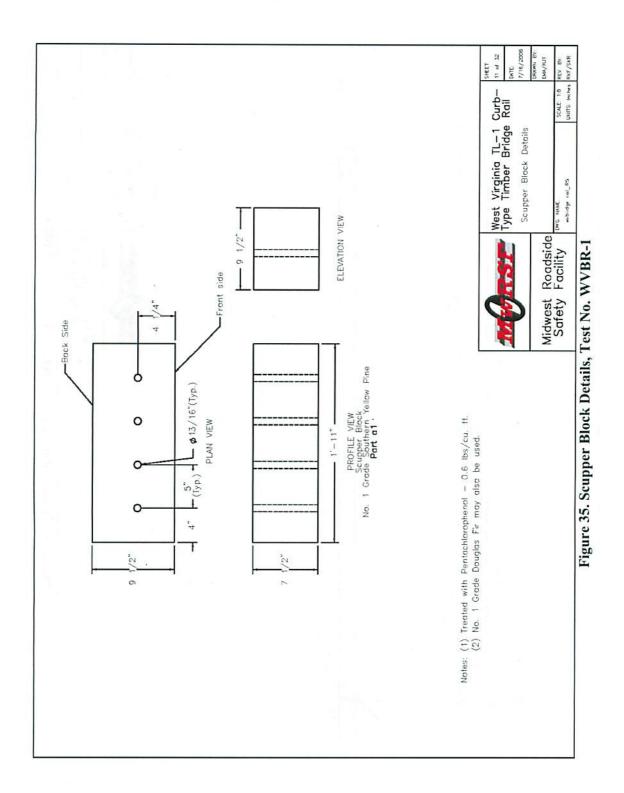


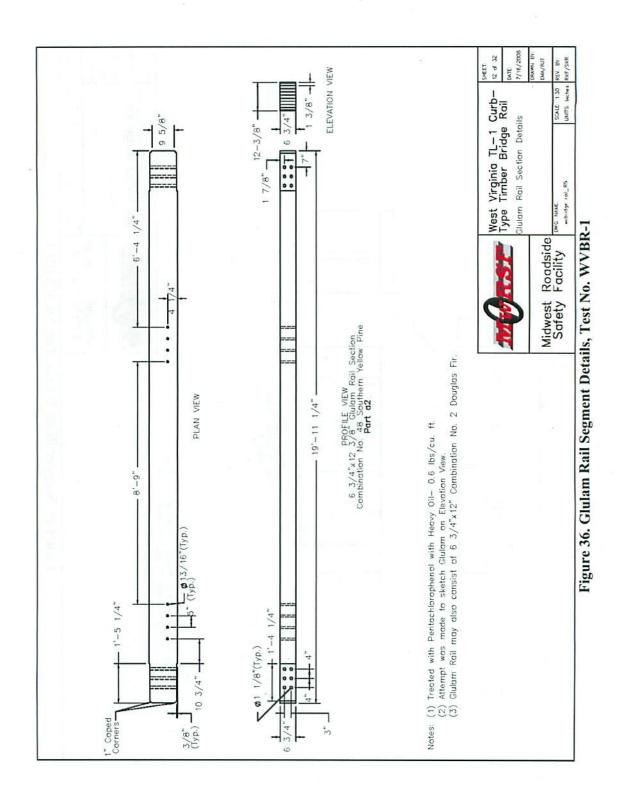
Figure 33. Rail End Treatment Connection Details, Test No. WVBR-1

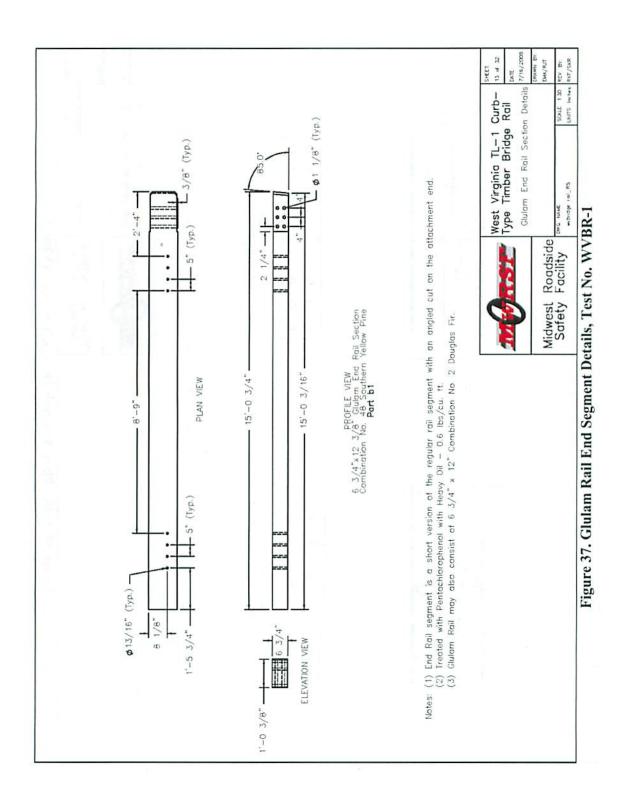


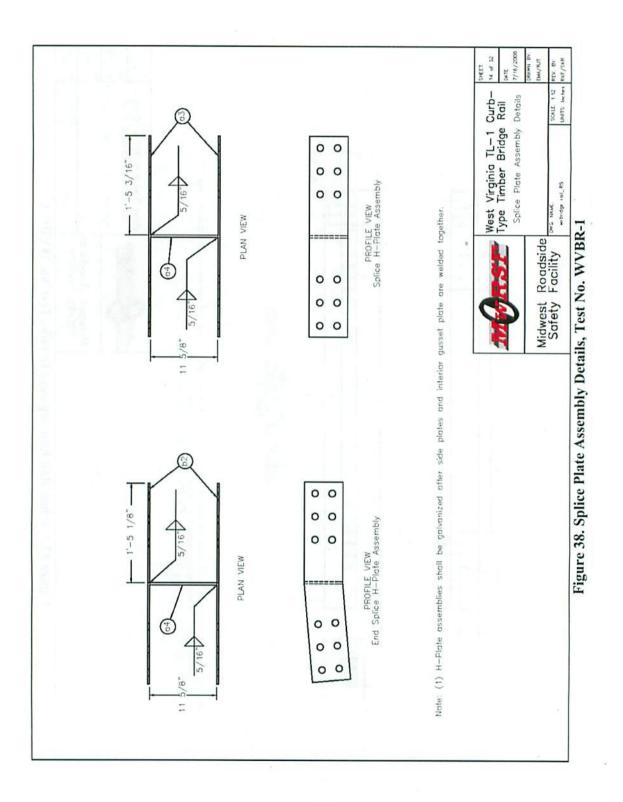




May 6, 2009 MwRSF Report No. TRP-03-211-09



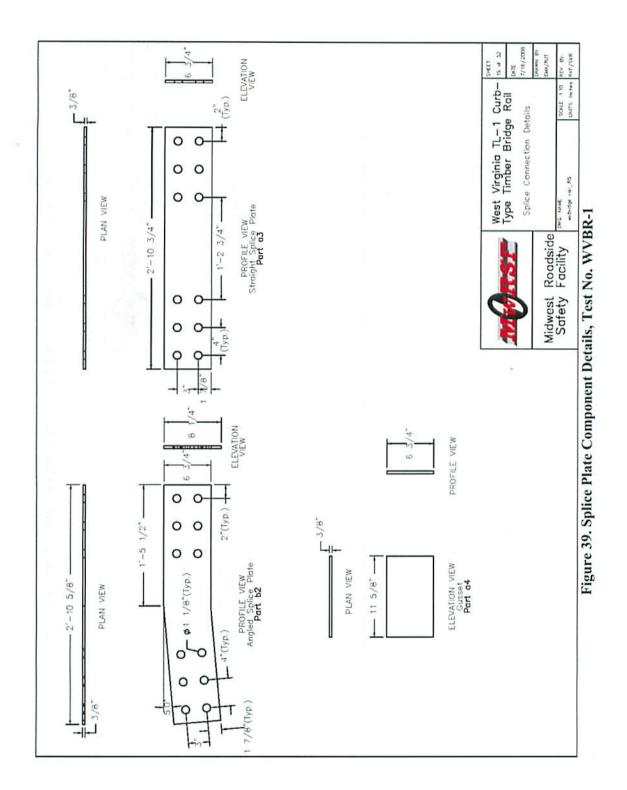




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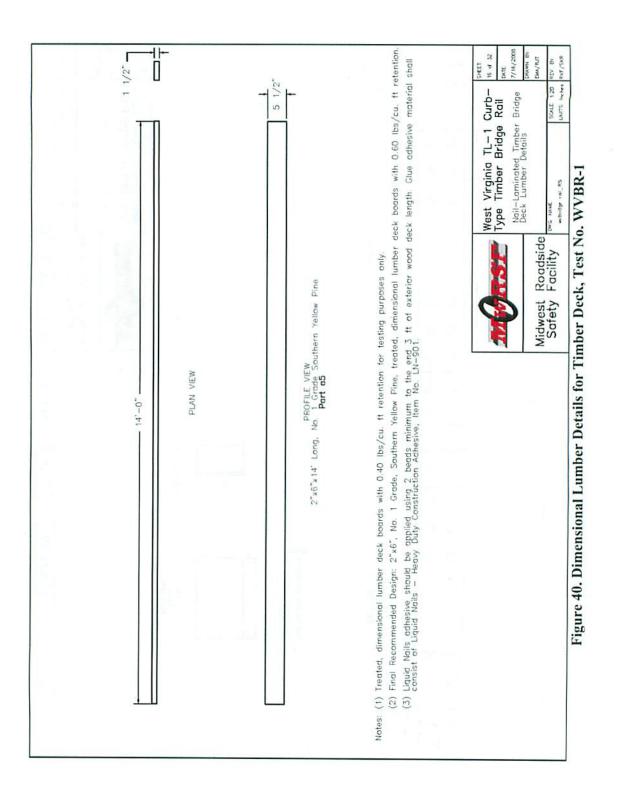
#### May 6, 2009 MwRSF Report No. TRP-03-211-09

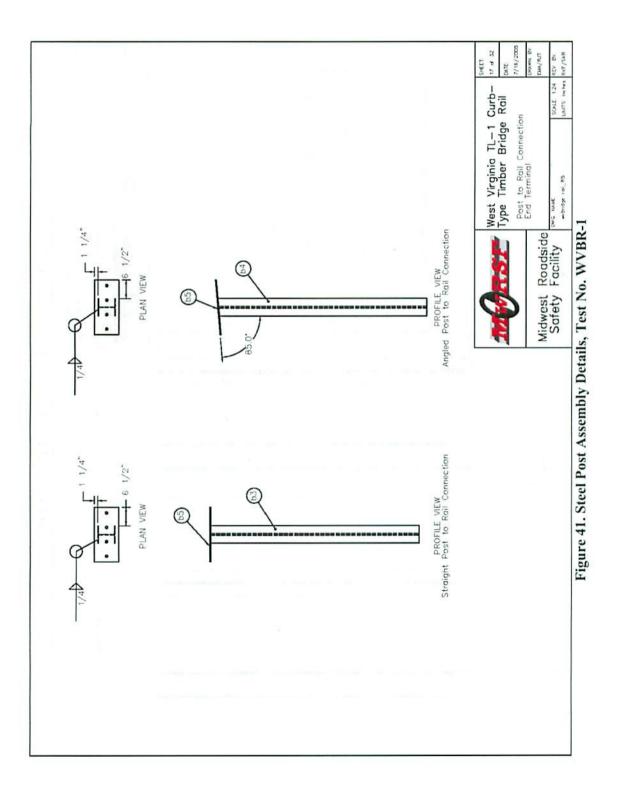
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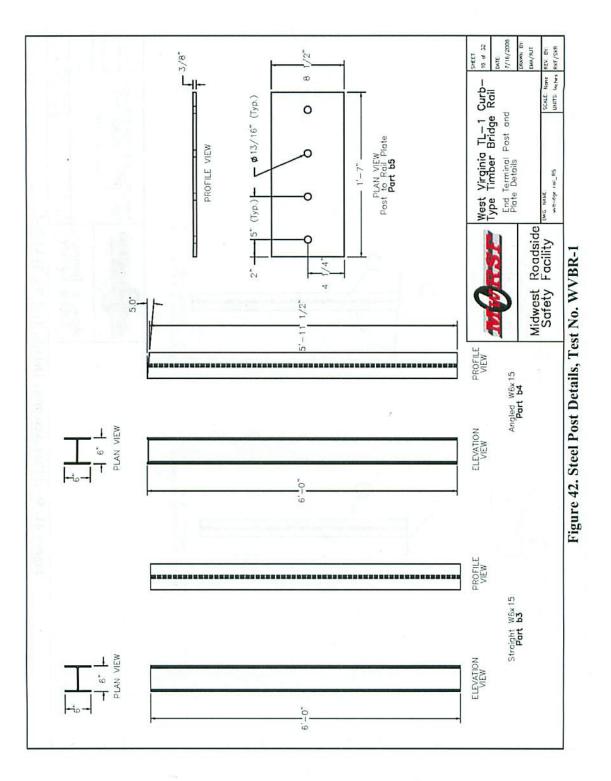


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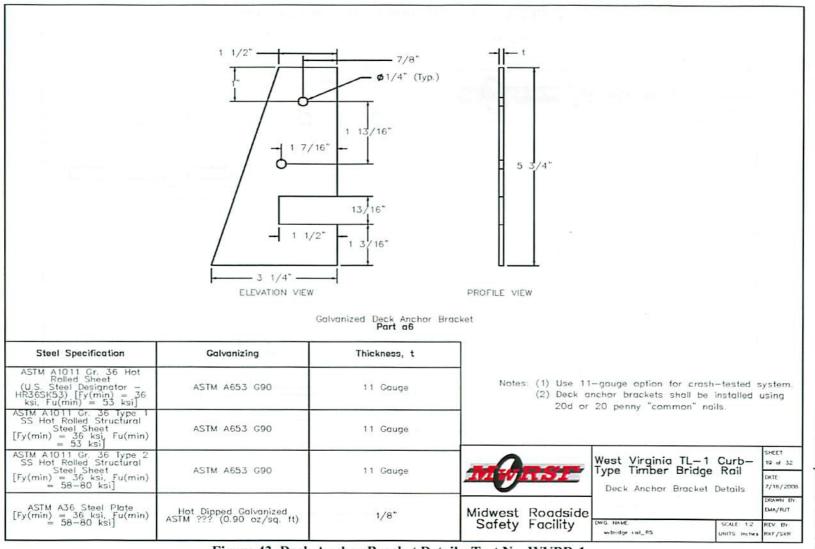


Figure 43. Deck Anchor Bracket Details, Test No. WVBR-1

May 6, 2009 MwRSF Report No. TRP-03-211-09

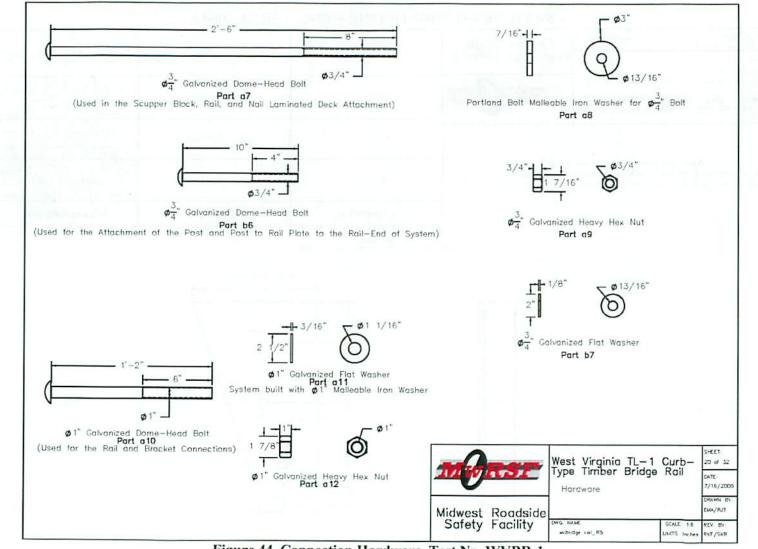


Figure 44. Connection Hardware, Test No. WVBR-1

Terrs NO.			
	110	Description	Material Specifications
a1	24	Scupper Block	Southern Yellow Pine No. 1
92	2	Glularn Rail Section	Southern Yellow Pine Combination No. 48
a3	12	Straight Splice Plate	Design-A572 Gr. 42; Used-A656 Gr. 50 Ty.
40	2	Splice Gusset	Design-A572 Gr. 42; Used-A656 Gr. 50 Ty.
đđ	960	2"x6"x14" Long Treated, Dimensional Lumber (0.60 1bs retention)	Southern Yellow Pine No. 1
90	240	Deck Anchar Plate	See Page 18 of 31
07	48	3/4" Dome Head Balt 30" Long	Galvanized A307
98	64	3/4" Malleable Iron Washer	4
60	99	3/4" Heavy Hex Nut	Galvanized A307
a10	84	1" Dame Head Bolt 14" Long	Calvanized A307
a†1	84	1" Flat Washer (as built-Malleable Iron)	Galvanized A307
012	84	1" Heavy Hex Nut	Galvanizea A307
b1	F	Glutarn End Rati Section	Southern Yellow Pine Combination No. 48
b2	2	Angled End Splice Plate	Design-A572 Gr. 42; Used-A656 Gr. 50 Ty.
b.3	2	Straight W6x15	Galvanized A36
p.4	2	Angled W5x15	Galvanized A36
b5	4	Post to Rall Plate	Calvanizea A36
.9q	16	3/4" Dome Head Bolt 10" Long	Galvanized A307
b7	16	3/4" Flat Washer	Galvanized A307
c11	18	0.625×4.75×25.375 Plote	A36
c12	18	0.375×4.75×25.375 Plate	A36.0
0.14	12	WT3x10x66.6" Long	A36
c16	œ	C15x33.9x66.5" Long	A36
C17	æ	Sole Plate 1 1/2" Thick	A36
c18	e.	Sale Plate 3/4" Thick	
c19	e.	W2 7×94	
			West Virginia TL-1 Curb- Type Timber Bridge Rail
			Midwest Roadside Safety Facility
			A MUTUAL AND

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