

Petty, Harold L.

From: Smith, Daniel R on behalf of Smith, Daniel R.
Sent: Wednesday, June 16, 2004 8:42 PM
To: Powell, Ronald D.; Petty, Harold L.
Subject: FW: Gypsum Interface

Some nice information from Jack Boschuk. This one of the primary reasons why we ran the test. There was no migration of gypsum into the "filter layer" the 60:40 mix of gypsum and flyash. He refers to it as 60:40 because this is ratio by weight, rather than volume, I think.

Anyway, this is very good news for the design. This will save TVA millions of \$\$.

Dan

-----Original Message-----

From: John Boschuk, Jr. [mailto:jboschuk@jltlabs.com]
Sent: Wednesday, June 16, 2004 4:18 PM
To: Shah, Yogesh; Greg McNulty; Smith, Daniel R; Anundson, Wade
Subject: Gypsum Interface

To All:

Attached is closeup photo of the interface between the Gypsum and 60:40 mix.
Absolutely no particle movement from the Gypsum into the 60:40 mix.

Jack Boschuk
JLT Laboratories, Inc.
938 S. Central Ave
Canonsburg, Pa. 15317
Tel: 724-746-4441
Fax: 724-745-4261
e-mail: jboschuk@jltlabs.com

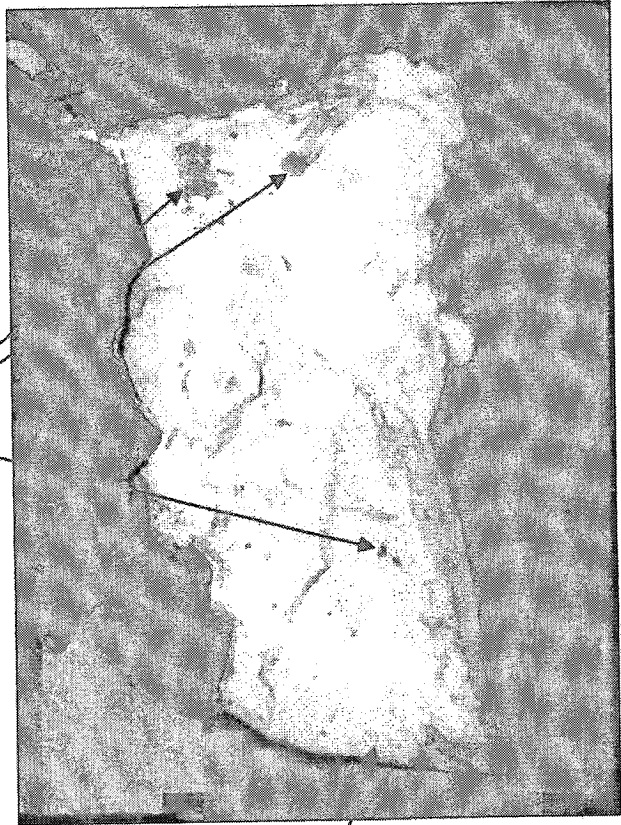
08/20/2004

TVA-00017164

Interface of Gypsum with the 60:40 mix



The 60:40 material did float up into the Gypsum slurry as the Gypsum was deposited



Note: There is no movement of Gypsum into the 60:40 layer

JLT Laboratories, Inc.

Petty, Harold L.

From: Smith, Daniel R.
Sent: Monday, April 28, 2003 9:40 AM
To: Hedgecoth, Missy
Cc: Stammler, Ted; Petty, Harold L.
Subject: Free water volume at KIF Ash Pond

A couple of weeks ago, you and I discussed the free water volume at KIF. Attached is a summary of our conversation, and the conclusions we arrived at after discussion. We met last Wednesday to discuss the presentation for gypsum disposal options, and someone made the comment that additional free water volume would have to be included if the gypsum were sluiced, and wet ash disposal continues. Based on our conversation (see attachment), there is some extra volume based on our assumptions. I would need to know what the additional volume for free water would be to include the gypsum sluicing, in order to include in the total. Someone explained that the additional volume would relate to the volume of water used for sluicing gypsum.

If you have any questions or comments on the attachment, please let me know.



Free Water Volume
at KIF Ash P...

Daniel R. (Dan) Smith, P.E.
Parsons E & C Phone: (423) 757-8088
633 Chestnut St, Suite 400 Fax: (423) 266-0922
Chattanooga, TN 37932 Cell: (423) 364-1679

Email: Daniel.R.Smith@parsons.com

Petty, Harold L.

Subject: Free Water Volume at KIF Ash Pond Discussion with Missy Hedgecoth
Entry Type: Phone call

Start: Fri 04/11/2003 2:00 PM
End: Fri 04/11/2003 2:00 PM
Duration: 0 hours

Dan Smith (Parsons) had prepared an estimate of free water volume and faxed this to Missy for review. After some discussion, it became apparent that some revision was needed. The following details the basis for developing a minimum free water volume for KIF in the event a gypsum stack is considered for the existing ash pond. There are a number of ways to accomplish this, and the following methodology is considered feasible for concept development. It may or may not become the basis for final design. Information used to determine free water volume: December 1998, 2002 letter from E.L. Deskins (KIF Plant Mgr) to Mr. Sims Crownover, Manager TDEC Division of Water Pollution Control, Enforcement and Compliance Division.

Free water volume needed for permit: 505,050.5 cy
Existing elevation of weirs in stilling basin: 754.37. Raise weir in stilling basin to 759.5 (leaving 4 ft of freeboard). This provides 430,750 cy. $505,050.5 - 430,750 = 74,300.5$ cy, minimum volume needed in ash pond to meet free water volume requirements.

Assume the water elevation in ash pond = water elevation in stilling basin, and assume an average 4.5 ft depth
Assume an L shaped area with the following dimensions: $2900 \text{ ft} \times 200 \text{ ft} \times 4.5 \text{ ft}/27 = 96,667$ cy. This provides an excess of $96,667 - 74,300.5 = 22,367$ cy.

Therefore, it would be feasible to leave the stack configuration for Option 3 the same as Option 2 for purposes of the study.

Petty, Harold L.

From: Smith, Daniel R on behalf of Smith, Daniel R.
Sent: Tuesday, June 15, 2004 5:18 PM
To: Petty, Harold L.; Powell, Ronald D.
Subject: FW: Key Conclusions

FYI

Dan

-----Original Message-----

From: John Boschuk, Jr. [mailto:jboschuk@jltlabs.com]
Sent: Tuesday, June 15, 2004 2:48 PM
To: Shah, Yogesh; Greg McNulty; Smith, Daniel R; Anundson, Wade
Subject: Key Conclusions

To All:

Thought you should be aware of the key conclusions that will be discussed, in detail, in the final report.

Basically, your suggested design with these waste materials worked exceptionally well.

Jack Boschuk
JLT Laboratories, Inc.
938 S. Central Ave
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Fax: 724-745-4261
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08/20/2004

TVA-00017168

MEMORANDUM

June 14, 2004
04LR386.01

TO: PARSONS TVA DESIGN TEAM

FROM: JACK BOSCHUK

RE: SOME KEY CONCLUSIONS

Here are a few key conclusions which will be discussed, in detail, in the final report. I thought you would want this information ASAP.

- 1) The Gypsum to 60:40 layer is compatible. No movement of fines was observed.
- 2) The 60:40 to Bottom Ash layer is also compatible. No movement of fines was observed.
- 3) The data suggests (to be confirmed) that the 60:40 and Bottom Ash layers were not saturated. This is a clear indication that the permeability of these layers is greater than the Gypsum indicating the drain is functioning well even under a normal stress of 65 psi (9,360 psf).

Overall, this test confirmed that this unique design functioned well. However, it will be difficult to construct these layers due to their sensitivity to moisture content and prevailing weather conditions. I would strongly suggest that construction of test pads under varying moisture and weather conditions be constructed to determine how best to mimic the design section in this column.

The sand drainage layer we designed for this column also performed exceptionally well. You may want to consider this gradation for your pipe drainage system design.

JB/rdo
\\wp10\memo\0476



Petty, Harold L.

From: Petty, Harold L.
Sent: Friday, September 03, 2004 4:00 PM
To: Petty, Harold L.; Powell, Ronald D.
Cc: Purkey, Ronald E.; 'ndavies@geosyntec.com'
Subject: RE: Geosyntech

Ron:

Here it is again with the e-mail corrected

-----Original Message-----

From: Petty, Harold L.
Sent: Friday, September 03, 2004 12:48 PM
To: Powell, Ronald D.
Cc: Purkey, Ronald E.
Subject: Geosyntech

Ronnie:

Here is the contact name, address, and phone number.

We need to send the FedEx package this afternoon.

Neil Davies
Geosyntech Consultants
1100 Lake Hearn Drive
Suite 200
Atlanta, GA 30342

(404) 236-7286

ndavies@geosyntec.com

Thanks,
Lynn

TAO REQUEST

Company Name: Parsons E&C		Contract No.: 99998970	
TAO Number	PR-0637	Revision	5A Phase 1
PCN:		Plant: KIF	Units(s): 1-9
Project Description:	Scrubber Addition Gypsum Stack		
TVA Lead Discipline	Civil <input checked="" type="checkbox"/>	Electrical <input type="checkbox"/>	Mechanical <input type="checkbox"/> Other (specify) <input type="checkbox"/>
Work Budgeted For FY(s):	FY04 = \$173,627	Shortcode:	001BRG4
List \$ limit for first FY:		(Only if work is budgeted for multiple FYs)	
Discipline(s) requesting design support:			
Civil	<input checked="" type="checkbox"/> Yes <input type="checkbox"/> No	TVA Civil Eng. Contact:	Ted Stammer 751-6421
Civil Mgmt. Approval:			
Electrical	<input type="checkbox"/> Yes <input type="checkbox"/> No	TVA Elect. Eng. Contact:	
Electrical Mgmt. Approval:			
Mechanical	<input type="checkbox"/> Yes <input type="checkbox"/> No	TVA Mech. Eng. Contact:	
Mechanical Mgmt. Approval:			
Other (Specify)		TVA (other) Contact:	
Other Mgmt. Approval:			
Scope of Work to be performed:		See Attachments	<input type="checkbox"/>
<p>The scope of work for Ardaman and Associates will consist of the following: Provide two senior engineering consultants (Tom Ingra and Dr. Wissa) to provide consulting engineering services to Parsons E&C and TVA;</p> <ul style="list-style-type: none"> • Attend meetings in Chattanooga and at the site (two full days); • review the overall configuration of the combined ash/gypsum disposal area; • develop concepts (rough hand sketches) for various configurations for handling wet gypsum operationally (both wet sluiced from Kingston Steam Plant and dry gypsum transported from Bull Run Steam Plant); • Discuss strengths and weaknesses of each proposed approach for handling gypsum; • Discuss uncertainties with each approach; • Address any questions/comments by TVA FES, TVA Plant or operational staff, or Parsons E&C. <p>Parsons E&C will provide personnel for the two-day meeting. Reading personnel will tie-in via telecon.</p>			

TAO REQUEST

Company Name:	GEOSYNTEC Ganesh Gopalakrishnah 404 236 7286	Contract No.:	LATER
TAO Number	n/a	Revision	0
Phase	2		
PCN:		Plant:	KIF
		Units(s)	1-9
Project Description:	Engineering Peer Review of Coal Byproduct (Gypsum and Ash) disposal plans. KIF is Kingston Fossil Plant near Kingston, TN.		
TVA Lead Discipline	<input checked="" type="checkbox"/>	Civil	<input type="checkbox"/>
		Electrical	<input type="checkbox"/>
		Mechanical	<input type="checkbox"/>
		Other (specify)	
Work Budgeted For FY(s):	04 and 05		Shortcode: 001DCDM
List \$ limit for first FY:	(Only if work is budgeted for multiple FYs)		
Discipline(s) requesting design support:			
Civil	<input checked="" type="checkbox"/>	Yes	<input type="checkbox"/>
		No	
TVA Civil Eng. Contact:	Ron Purkey 423 751 4820		
Civil Mgmt. Approval:			
Electrical	<input type="checkbox"/>	Yes	<input checked="" type="checkbox"/>
		No	
TVA Elect. Eng. Contact:			
Electrical Mgmt. Approval:			
Mechanical	<input type="checkbox"/>	Yes	<input checked="" type="checkbox"/>
		No	
TVA Mech. Eng. Contact:			
Mechanical Mgmt. Approval:			
Other (Specify)			
TVA (other) Contact:			
Other Mgmt. Approval:			
Scope of Work to be performed:			See Attachments <input type="checkbox"/>
Provide Engineering Peer Review in accordance with attached scope. Geosyntec proposal will be official task scope.			
Expected Deliverables:	<ol style="list-style-type: none"> 1. An exact description of each review component. 2. A Summary of findings. 3. Recommendations for improvement (if any). 		
Requested Start Date:	September 1, 2004	Requested Completion Date:	October 22, 2004
<i>(Copy of this request must be attached to the resulting TAO(s). If requested task duration exceeds or overlaps a single Fiscal Year, multiple TAO(s) are required (one for each FY).)</i>			

Scope of work for KIF Engineering Peer Review of Coal Byproduct (Gypsum and Ash) disposal plans:

The permit application for subject disposal plan was submitted in July to the State of TN and is currently under review by TDEC. The intent of this task is to obtain an independent peer review of the approach, the theory used, the constructability and operability of the disposal plan, the drainage and seep controls, the operations plan, and any other component of the disposal plan necessary for the complete peer review. Obviously, this is an independent review and Parsons nor the State of TN should be contacted. Anticipated work is as follows as a minimum:

- 1. Read the operations plan, the Hydrogeology Report, and overview the drawings. (complete 1 week after award)**
- 2. Visit the site and familiarize yourself with the current site and future plans. (complete 2 weeks after award)**
- 3. Perform an in depth peer review of entire disposal and operation plans. (complete 4 weeks after award)**
- 4. Provide a report that includes the items listed below (complete 6 weeks after award)**
 - a. An exact description of each review component.**
 - b. A Summary of findings.**
 - c. Recommendations for improvement (if any).**
- 5. Weekly telecons (1 hour duration) will be required**
- 6. 1 week after delivery of peer review to TVA, a meeting will be held at TVA facilities to review the findings. Geosyntec will present the findings.**

Petty, Harold L.

From: Smith, Daniel R on behalf of Smith, Daniel R.
Sent: Tuesday, September 16, 2003 3:42 PM
To: 'McEntyre, Charles'
Cc: Petty, Harold L.
Subject: Ash Settlement data for KIF

Charles, Lynn suggested that I get in touch with you to see if you know of any data for ash settling characteristics at KIF. We are doing a study to determine how to stack gypsum and continue stacking ash at the existing ash pond at KIF. The attached email provides some additional information regarding what we are trying to do. The concerns are if we build a "dike within the dike", we will reduce the available area to allow ash to settle out within the existing pond. As you may be aware, the ash pond discharges into a stilling pond, and then into the river/lake. The facility has to maintain a minimum free water volume to remain in compliance with permitting requirements, not to mention discharge limits. Modifying the ash pond could cause one or both of the following:

- 1) Exceedances at the outfall;
- 2) More ash settling in the stilling basin, thus reducing the free water volume over time to below the permitted volume.

There are computer programs developed by Waterways Experiment Station (US Army COE) in Vicksburg that we could possibly use to model this (program is free of charge), but from the description, we would need site specific data (see below).

Flocculent settling test results;
15-day compression settling test;
Zone settling test (if appropriate);
disposal area information (we should have this)
Dredge information (should have this)
Physical and engineering data on the sediments (we have this).

If this data is not available we would have to conduct these tests (probably don't have time because study is due end of October).

The other way we could do this would be to apply Stokes Law (we have gradation curves, specific gravity, etc), but accuracy might be limited somewhat.

Any information you might have would be helpful. If you are aware of any other sources of this information (i.e., within TVA or EPRI) I would be interested in that as well.

Attached is an email with a couple of drawings attached to show the proposed layout of the "dike within the dike", and the proposed contours developed for the study.



TVA Kingston ash
pond study -...

Daniel R. (Dan) Smith, P.E.
Parsons E & C Phone: (423) 757-8088
633 Chestnut St, Suite 400 Fax: (423) 266-0922
Chattanooga, TN 37932 Cell: (423) 364-1679 Email: Daniel.R.Smith@parsons.com



GEOSYNTEC CONSULTANTS

1255 Roberts Boulevard, N.W., Suite 200
Kennesaw, GA 30144-3694 USA
Voice: (678)202-9500
Fax: (678)202-9501

FAX COVER SHEET

To: Ron Powell, John Albright, cc Ron Puckey, Lynn Petty
Firm: TVA
Fax No.: 423-751-7094
From: Neil Davies
Project No.: _____

Cover Page plus 7 page(s) following

Sent by: (RD) Date: 10/25 Time: 8:30

MESSAGE:

Please find attached:

- Weekly Progress
- Kingston Alternative (Memo + sketches)

Look forward to talking with you @ 1pm / 2pm

Neil.

IMPORTANT NOTICE:

This facsimile is confidential, may be legally privileged, and is for the intended recipient only. Access, disclosure, copying, distribution, or reliance on any of it by anyone else is prohibited and may be a criminal offense. Please destroy if obtained in error and fax confirmation of destruction to sender.

Weekly Progress Summary
TVA - Kingston Fossil Plant and John Sevier Fossil Plant Sevier Projects
GeoSyntec Project No. GR3471

Week Ending October 24, 2004

KINGSTON PROJECT (Peer Review)

Progress/Activities This Report Period

- Additional review of Operations Plan, drawings and other supporting materials
- Develop independent review of stability - close to complete
- Report preparation - in progress
- Review of proposed fix for existing dike - in progress
- Detailed review of drawings
 - Inconsistencies and cross references identified
- Hydrogeo report
 - Completed memo describing our findings. Memo to be issued in advance of our report.
- Alternative Layout
 - In the course of our review, we have identified a potential alternative arrangement that may be of interest to TVA.
 - a summary memo is included with this progress report for discussion on today's weekly telecon

Activities Proposed for Next Period

- Continue review of Operations Plan, drawings and other supporting materials
- Continue work on peer review report
- Obtain preliminary feedback from TVA on alternative layout

Information Needs (from TVA)

- Preliminary feedback on potential alternative layout

Schedule

This project is currently on schedule.

JOHN SEVIER PROJECT (Siting Study)

Progress/Activities This Report Period

- Additional evaluation of preliminary sites/tracts proposed by TVA
- Preliminary volume estimates prepared for each site (using simplified layout) - note these have been posted to web site
- Internal QA checks on preliminary volumes
- Evaluation of combinations of areas - complete
- Preliminary evaluation of existing dry stack expansion alternatives - in progress
- Additional GIS maps prepared and posted to website
- Obtained summary details of permitted landfill (Brooks Site) from TDEC.

Activities Proposed for Next Period

- Refine alternatives based on actual property lines (if available from TVA)
- Refinement of volume estimates
- Complete evaluation of existing dry stack (potential for volume expansion)
- Development of site ranking - in progress

Information Needs (from TVA)

- Need feedback from TVA on ownership for tracts under consideration (Critical Path Item)
- Electronic copy of areal photo (provided by TVA (if available))

Schedule

This project is currently on schedule.

Draft - For Discussion Purposes Only

**Introduction of Alternative Ash and Gypsum Disposal Strategy
Kingston Fossil Fuel (KFF) Plant
GeoSyntec Consultants
22 October, 2004**

Description of Alternative On-Site Disposal Strategy

- **Relevant Figures:** Please reference Figures 1 (Plan View), Figure 2 (E-W Cross-Section), and Figure 3 (N-S Cross-Section), each one displaying currently proposed option at the top of the page and the alternative option at the bottom of the page.
- **Vertically Expand Existing Dredge Ash Disposal Area:** Increase disposal capacity of existing dredge cells by controlling water levels within ash using toe drain network, to allow vertical expansion of cells at current 4H:1V side slope geometry to achieve approximately 3M yd³ (minimum) of additional ash disposal capacity.
- **Develop Gypsum Storage Basins:** Develop southern end of existing ash pond into a conventional two-pond gypsum disposal area using rim ditch and sequential filling techniques, while maintaining "valley" between this gypsum disposal area and the existing dredge cells as a conventional ash pond for as long as possible, possibly using eastern side for a dredge ash staging area as needed.
- **Convert from Wet Ash to Dry Ash Management:** When wet ash disposal capacity is impacted, convert operations to a dry ash management technique and fill remainder of ash pond and eventually close entire ash disposal area using dry ash.

Assessment of Technical Components

- **Stability of Existing Dredge Ash Disposal Areas:** It appears that the recent pond instability was caused by locally high water levels in the dredge cells. If the water levels can be controlled, then it appears that the height of the existing cells can be increased significantly. The assumed properties of the ash used in stability analyses are reasonable. Based on these shear strength properties for the fly ash and the currently constructed side slopes, adequate short- and long-term stability can be controlled by the selective use of a network of toe drains that is incorporated into the design.
- **Stability of Gypsum Basins:** Under the alternative plan, the gypsum ponds will be constructed, filled, and managed using conventional gypsum disposal techniques and will incorporate side slopes no steeper than those currently proposed. Since the gypsum is inherently stronger than fly ash, the gypsum monocell design will be inherently more stable than the current proposed design.
- **Site Operations - Gypsum:** The alternative strategy incorporates a "conventional" gypsum disposal technique including construction via two sequentially operated, large surface area monocells. Gypsum distribution and sedimentation and the subsequent dike construction will incorporate perimeter rim ditches. From an operational perspective, this technique is significantly easier than the currently proposed technique.

Draft - For Discussion Purposes Only

- **Site Operations - Ash:** Ash disposal will utilize the current wet disposal technique for as long as possible. Therefore, operations regarding ash disposal will be largely unaffected in the short-term. It is acknowledged that the current ash pond will decrease in surface area to accommodate gypsum storage, so ash disposal operations regarding dredging and placement in the dredge cells will be impacted. The currently disposal strategy realizes these same wet ash disposal impacts, but because of the increased disposal quantities included in the alternative strategy, the impacts are not expected to be as severe and major operational impacts involving the transition to dry ash management can be delayed for several years.
- **Site Operations - General O&M:** The alternative strategy introduces a network of toe drains and potentially an increase in overall height of the dredge cells. It will be necessary to develop an inspection and monitoring plan for the side slopes and the toe drains. The increased height necessitates the development of haul roads to and from the dredge cells. It is anticipated that the net impact of these components will be relatively minor.

Design Considerations

- **Stability of Dredge Ash Cells:** A detailed evaluation of the slope stability, including the appropriate use of benches and toe drains needs to be completed.
- **Volume of Ash and Gypsum Disposal Areas:** The attached figures are conceptual renderings of the alternative strategy. The footprint of the gypsum basins needs to be established based on design disposal volumes. Similarly, the size of the wet ash pond during the life of the facility needs to be assessed.
- **Surface Water and Process Water Management:** As with all gypsum and ash disposal strategies, the design of the surface water management ditches and retention ponds needs to be considered, as does the detention times materials within the wet ash pond.

Closure

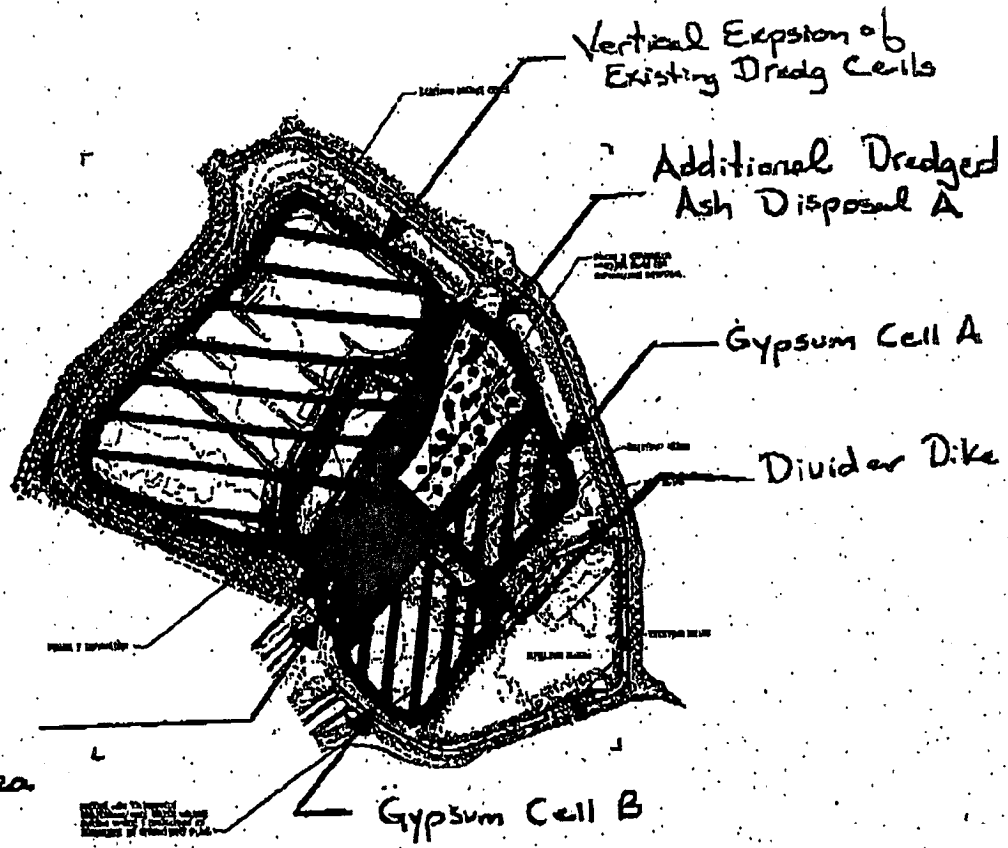
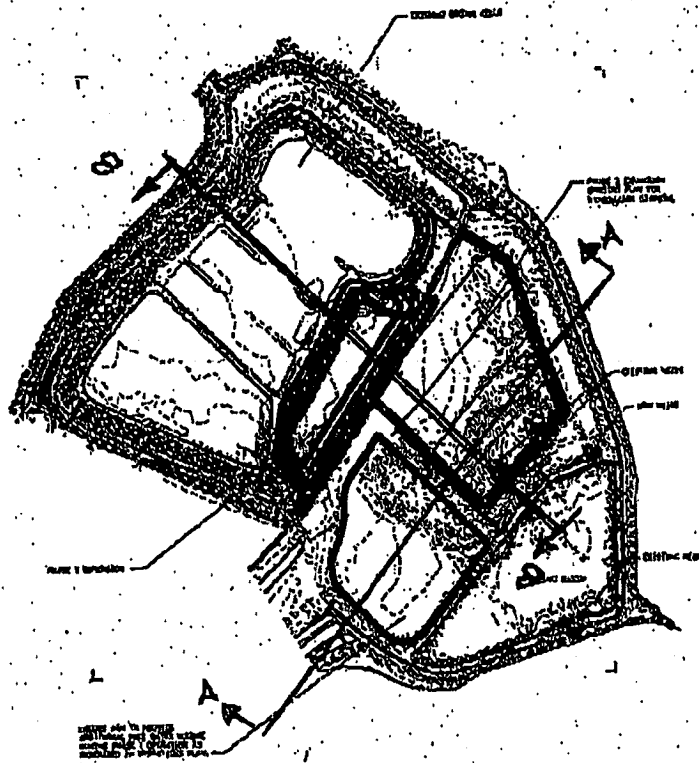
The concepts presented herein are offered for consideration, comment, and critique by TVA. We are particularly interested in identifying any site-specific considerations that have been overlooked in the development of this alternative and those that are considered problematic by TVA. Based on GeoSyntec's preliminary evaluation, however, the concept introduced here appears to be a technically viable strategy for significantly increasing design capacity while minimizing impacts to site and plant operations. We look forward to discussing these ideas with the TVA team.

Oct. 21. 2004 11:09AM

BFI MODEL FILL LANDFILL

No. 1949 P. 2

Figure 1.
Plan View



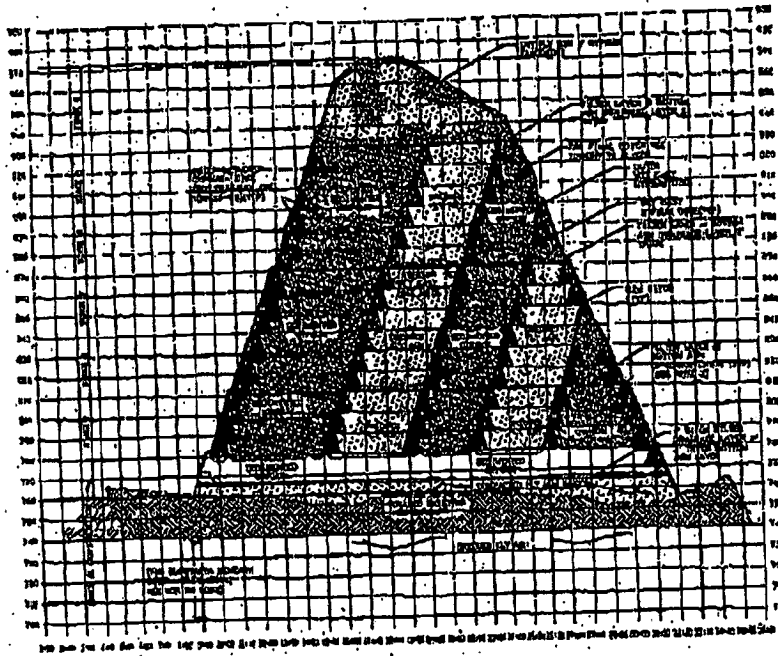
Ash Pond F
Eventual Ash
Disposal Area
(Dry Ash)

Oct. 21. 2004 11:10AM

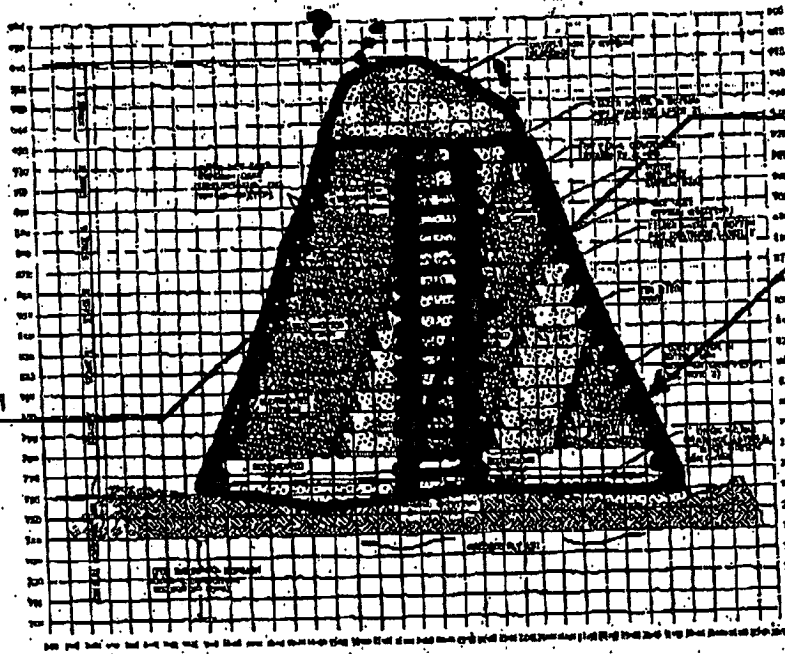
BFI MODELFILL LANDFILL

No. 1949 P. 3

Figure 2.
Section A-A
Across
Gypsum
Ponds



Gypsum
Cell B

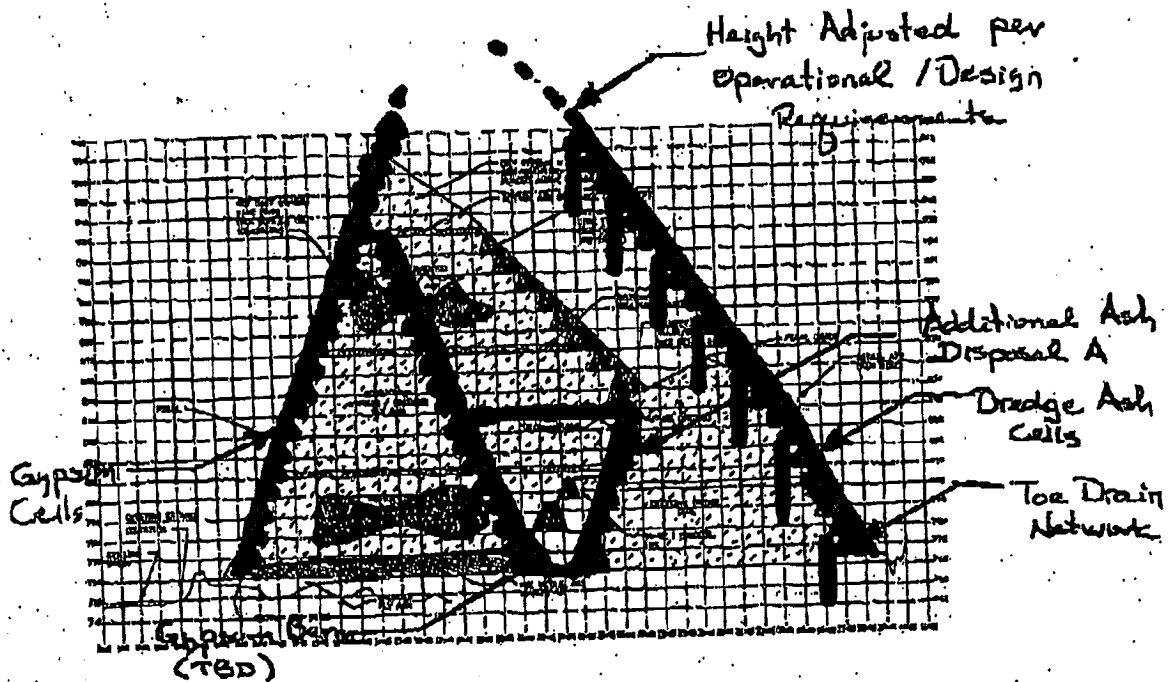
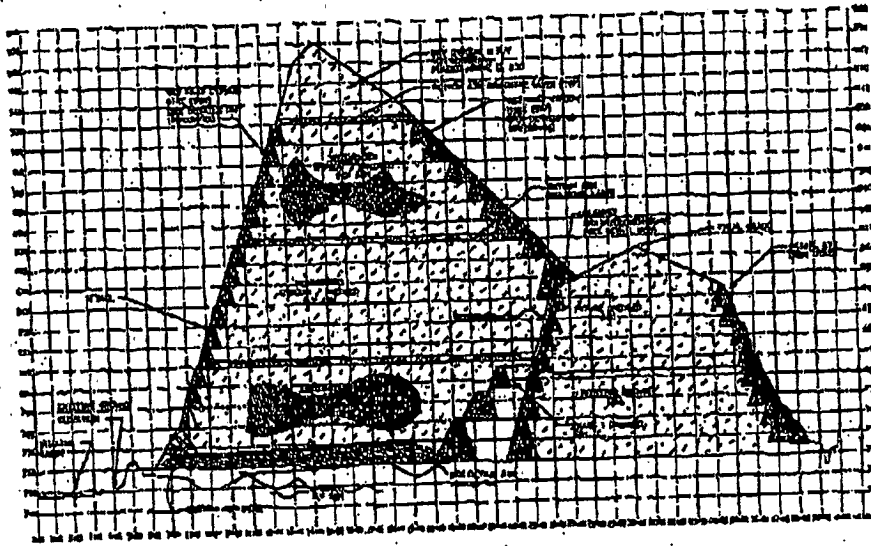


Divider Dike
(Inevitably
Constructed)

Gypsum
Cell A

Oct. 21. 2004 11:10AM BFI MODEL FILL LANDFILL

Figure 3.
Section B-i-3
Across
Dredge
Pond and
Gypsum
Pond



Pore water pressures defined by piezometric line
Number of the piezometric line used = 1
Negative pore pressures set to zero

DATA FOR MATERIAL TYPE 6
FA + BA, Loose

Unit weight of material = 103.000

CONVENTIONAL (ISOTROPIC) SHEAR STRENGTHS
Cohesion - - - - - .000
Friction angle - - - - - 32.100 degrees

Pore water pressures defined by piezometric line
Number of the piezometric line used = 1
Negative pore pressures set to zero

DATA FOR MATERIAL TYPE 7
FA Loose

Unit weight of material = 100.000

CONVENTIONAL (ISOTROPIC) SHEAR STRENGTHS
Cohesion - - - - - 100.000
Friction angle - - - - - 14.500 degrees

Pore water pressures defined by piezometric line
Number of the piezometric line used = 1
Negative pore pressures set to zero

DATA FOR MATERIAL TYPE 8
Natural Clay CL (Alluvium)

Unit weight of material = 126.400

CONVENTIONAL (ISOTROPIC) SHEAR STRENGTHS
Cohesion - - - - - 400.000
Friction angle - - - - - 23.000 degrees

Pore water pressures defined by piezometric line
Number of the piezometric line used = 1
Negative pore pressures set to zero

DATA FOR MATERIAL TYPE 9
Residuum SC-SM

Unit weight of material = 130.400

CONVENTIONAL (ISOTROPIC) SHEAR STRENGTHS
Cohesion - - - - - 600.000
Friction angle - - - - - 30.000 degrees

Pore water pressures defined by piezometric line
Number of the piezometric line used = 1
Negative pore pressures set to zero

DATA FOR MATERIAL TYPE 10
Bedrock Limestone/Shale

Unit weight of material = 150.000

CONVENTIONAL (ISOTROPIC) SHEAR STRENGTHS
Cohesion - - - - - 735.000
Friction angle - - - - - 29.900 degrees

Pore water pressures defined by piezometric line
Number of the piezometric line used = 1
Negative pore pressures set to zero

All new material properties defined - No old data retained
UTEXAS3 - VER. 1.209 - 2/28/98 - (C) 1985-1998 S. G. WRIGHT
One (1) copy licensed to Parsons I & T, Cincinnati, OH
Date: 12: 4:2004 Time: 1:14:13 Input file: kfgyye3.dat

Negative pore pressures set to zero

DATA FOR MATERIAL TYPE 2
Sedimented Gypsum + FA with .8 reduction

Unit weight of material = 116.400

---- 2-STAGE STRENGTHS FOR SECOND STAGE OF COMPUTATIONS ----

UNDRAINED ENVELOPE (= Tau-ff vs. sigma-fc
strength envelope from R tests):

Intercept of envelope (d-sub-R) - - - - - 4214.400
Slope of envelope (psi-sub-R) (degrees) - - 23.400

DRAINED/EFFECTIVE STRESS (S) ENVELOPE:

Cohesion (d-sub-S) - - - - - .000
Friction angle (psi-sub-S) (degrees) - - - - 37.000

Pore water pressures defined by piezometric line

Number of the piezometric line used = 1

Negative pore pressures set to zero

DATA FOR MATERIAL TYPE 3
Sedimented Gypsum with .8 reduction

Unit weight of material = 113.400

---- 2-STAGE STRENGTHS FOR SECOND STAGE OF COMPUTATIONS ----

UNDRAINED ENVELOPE (= Tau-ff vs. sigma-fc
strength envelope from R tests):

Intercept of envelope (d-sub-R) - - - - - 4214.400
Slope of envelope (psi-sub-R) (degrees) - - 23.400

DRAINED/EFFECTIVE STRESS (S) ENVELOPE:

Cohesion (d-sub-S) - - - - - .000
Friction angle (psi-sub-S) (degrees) - - - - 37.000

Pore water pressures defined by piezometric line

Number of the piezometric line used = 1

Negative pore pressures set to zero

DATA FOR MATERIAL TYPE 4
Bottom Ash Dike(new) with .8 reduction

Unit weight of material = 116.400

---- 2-STAGE STRENGTHS FOR SECOND STAGE OF COMPUTATIONS ----

UNDRAINED ENVELOPE (= Tau-ff vs. sigma-fc
strength envelope from R tests):

Intercept of envelope (d-sub-R) - - - - - 1460.800
Slope of envelope (psi-sub-R) (degrees) - - 42.700

DRAINED/EFFECTIVE STRESS (S) ENVELOPE:

Cohesion (d-sub-S) - - - - - .000
Friction angle (psi-sub-S) (degrees) - - - - 36.000

Pore water pressures defined by piezometric line

Number of the piezometric line used = 1

Negative pore pressures set to zero

DATA FOR MATERIAL TYPE 5
Rolled Compacted Fly Ash(New) with .8 reduction

Unit weight of material = 113.400

---- 2-STAGE STRENGTHS FOR SECOND STAGE OF COMPUTATIONS ----

UNDRAINED ENVELOPE (= Tau-ff vs. sigma-fc
strength envelope from R tests):

Intercept of envelope (d-sub-R) - - - - - 3463.200
Slope of envelope (psi-sub-R) (degrees) - - 36.300

DRAINED/EFFECTIVE STRESS (S) ENVELOPE:

Cohesion (d-sub-S) - - - - - 180.000
Friction angle (psi-sub-S) (degrees) - - - - 36.000

Pore water pressures defined by piezometric line
Number of the piezometric line used = 1
Negative pore pressures set to zero

DATA FOR MATERIAL TYPE 6
FA + BA, Loose with .8 reduction

Unit weight of material = 108.400

---- 2-STAGE STRENGTHS FOR SECOND STAGE OF COMPUTATIONS ----

UNDRAINED ENVELOPE (= Tau-ff vs. sigma-fc
strength envelope from R tests):

Intercept of envelope (d-sub-R) - - - - -	5318.900
Slope of envelope (psi-sub-R) (degrees) - -	26.400

DRAINED/EFFECTIVE STRESS (S) ENVELOPE:

Cohesion (d-sub-S) - - - - -	.000
Friction angle (psi-sub-S) (degrees) - - - -	32.100

Pore water pressures defined by piezometric line
Number of the piezometric line used = 1
Negative pore pressures set to zero

DATA FOR MATERIAL TYPE 7
FA Loose with .8 reduction

Unit weight of material = 100.000

---- 2-STAGE STRENGTHS FOR SECOND STAGE OF COMPUTATIONS ----

UNDRAINED ENVELOPE (= Tau-ff vs. sigma-fc
strength envelope from R tests):

Intercept of envelope (d-sub-R) - - - - -	851.000
Slope of envelope (psi-sub-R) (degrees) - -	.000

DRAINED/EFFECTIVE STRESS (S) ENVELOPE:

Cohesion (d-sub-S) - - - - -	100.000
Friction angle (psi-sub-S) (degrees) - - - -	14.500

Pore water pressures defined by piezometric line
Number of the piezometric line used = 1
Negative pore pressures set to zero

DATA FOR MATERIAL TYPE 8
Natural Clay CL (Alluvium) with .8 reduction

Unit weight of material = 126.400

---- 2-STAGE STRENGTHS FOR SECOND STAGE OF COMPUTATIONS ----

UNDRAINED ENVELOPE (= Tau-ff vs. sigma-fc
strength envelope from R tests):

Intercept of envelope (d-sub-R) - - - - -	823.500
Slope of envelope (psi-sub-R) (degrees) - -	.000

DRAINED/EFFECTIVE STRESS (S) ENVELOPE:

Cohesion (d-sub-S) - - - - -	100.000
Friction angle (psi-sub-S) (degrees) - - - -	14.500

Pore water pressures defined by piezometric line
Number of the piezometric line used = 1
Negative pore pressures set to zero

DATA FOR MATERIAL TYPE 9
Residuum SC-SM with .8 reduction

Unit weight of material = 125.000

---- 2-STAGE STRENGTHS FOR SECOND STAGE OF COMPUTATIONS ----

UNDRAINED ENVELOPE (= Tau-ff vs. sigma-fc
strength envelope from R tests):

Intercept of envelope (d-sub-R) - - - - -	234.500
Slope of envelope (psi-sub-R) (degrees) - -	16.100

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Date: 12: 4:2004 Time: 1:14:13 Input file: kfgyye3.dat
Kingston 6 Section Y-Y, Multi-Stage Run With UTEXAS3
GYPSUM STACK Option 2, Earthquake = 0.11g
KIF Wet with Pond Surcharge Included

TABLE NO. 15

* NEW ANALYSIS/COMPUTATION DATA *

Circular Shear Surface(s)

Automatic Search Performed

Starting Center Coordinate for Search at -

X = 573.000
Y = 1011.000

Required accuracy for critical center (= minimum spacing between grid points) = 10.000

Critical shear surface not allowed to pass below Y = 720.000

For the initial mode of search
all circles are tangent to horizontal line at -
Y = 720.000

Maximum number of iterations allowed for calculating the factor of safety = 1000

Seismic coefficient = .110

Procedure used to compute the factor of safety: SPENCER

Depth of crack = 5.000

THE FOLLOWING REPRESENT EITHER DEFAULT OR PREVIOUSLY DEFINED VALUES:

Initial trial estimate for the factor of safety = 3.000

Initial trial estimate for side force inclination = 15.000 degrees
(Applicable to Spencer's procedure only)

Allowed force imbalance for convergence = 100.000

Allowed moment imbalance for convergence = 100.000

Initial trial values for factor of safety (and side force inclination for Spencer's procedure) will be kept constant during search

Maximum subtended angle to be used for subdivision of the circle into slices = 3.00 degrees

Search will be continued to locate a more critical shear surface (if one exists) after the initial mode is complete

Depth of water in crack = .000

Unit weight of water in crack = 62.400

Conventional (single-stage) computations to be performed
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Kingston 6 Section Y-Y, Multi-Stage Run With UTEXAS3
GYPSUM STACK Option 2, Earthquake = 0.11g
KIF Wet with Pond Surcharge Included

TABLE NO. 18

INFORMATION FOR CURRENT MODE OF SEARCH - All Circles Are Tangent to a Horizontal Line at Y = 720.000

Center Coordinates			1-Stage Factor	Side Force of Inclination	Iterations
X	Y	Radius	Safety	(degrees)	
273.00	711.00	-9.00	Center of circle is below lowest point of slope - CIRCLE REJECTED		
573.00	711.00	-9.00	Center of circle is below lowest point of slope - CIRCLE REJECTED		
873.00	711.00	-9.00	Center of circle is below lowest point of slope - CIRCLE REJECTED		
273.00	1011.00	291.00	2.967	4.16	4
573.00	1011.00	291.00	1.306	16.60	7
873.00	1011.00	291.00	2.542	11.27	4
273.00	1311.00	591.00	2.793	4.41	4
573.00	1311.00	591.00	1.400	20.08	8
873.00	1311.00	591.00	2.299	14.50	6
523.00	961.00	241.00	1.387	11.33	7
573.00	961.00	241.00	1.300	15.90	7
623.00	961.00	241.00	1.313	18.88	8
523.00	1011.00	291.00	1.413	11.81	7
623.00	1011.00	291.00	1.310	19.71	8
523.00	1061.00	341.00	1.420	12.44	7
573.00	1061.00	341.00	1.318	17.22	7
623.00	1061.00	341.00	1.319	20.36	8
523.00	911.00	191.00	1.395	10.63	7
573.00	911.00	191.00	1.302	15.27	7
623.00	911.00	191.00	1.363	17.32	7
543.00	931.00	211.00	1.337	12.85	7
573.00	931.00	211.00	1.300	15.50	7
603.00	931.00	211.00	1.299	17.75	8
543.00	961.00	241.00	1.346	13.12	7
603.00	961.00	241.00	1.291	18.22	8
543.00	991.00	271.00	1.354	13.45	7
573.00	991.00	271.00	1.303	16.32	7
603.00	991.00	271.00	1.291	18.60	8
633.00	931.00	211.00	1.363	17.87	7
633.00	961.00	241.00	1.341	18.66	8
633.00	991.00	271.00	1.329	19.44	8
593.00	951.00	231.00	1.292	17.38	7
603.00	951.00	231.00	1.293	18.07	8
613.00	951.00	231.00	1.300	18.58	8
593.00	961.00	241.00	1.291	17.53	7
613.00	961.00	241.00	1.296	18.76	8
593.00	971.00	251.00	1.291	17.67	8
603.00	971.00	251.00	1.290	18.37	8
613.00	971.00	251.00	1.295	18.89	8
593.00	981.00	261.00	1.291	17.81	8
603.00	981.00	261.00	1.290	18.50	8
613.00	981.00	261.00	1.296	18.99	8
593.00	991.00	271.00	1.291	17.96	8
613.00	991.00	271.00	1.298	19.08	8

At the end of the current mode of search the most critical circle which was found has the following values -
 X-center = 603.00 Y-center = 981.00 Radius = 261.00
 Factor of Safety = 1.290 Side Force Inclination = 18.50
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 Date: 12: 4:2004 Time: 1:14:13 Input file: kfgyye3.dat
 Kingston 6 Section Y-Y, Multi-Stage Run With UTEXAS3
 GYPSUM STACK Option 2, Earthquake = 0.11g

KIF Wet with Pond Surcharge Included

TABLE NO. 19

INFORMATION FOR CURRENT MODE OF SEARCH - All Circles Have the Same Radius - Radius = 261.000

Center Coordinates			1-Stage		Iterations
X	Y	Radius	Factor of Safety	Side Force of Inclination (degrees)	
303.00	681.00	261.00	Center of circle is below lowest point of slope - CIRCLE REJECTED		
603.00	681.00	261.00	Center of circle is below lowest point of slope - CIRCLE REJECTED		
903.00	681.00	261.00	Center of circle is below lowest point of slope - CIRCLE REJECTED		
303.00	981.00	261.00	3.006	4.10	5
903.00	981.00	261.00	2.980	10.21	5
303.00	1281.00	261.00	See Message on Next Line(s)		
CIRCLE DOES NOT INTERSECT SLOPE					
603.00	1281.00	261.00	See Message on Next Line(s)		
CIRCLE DOES NOT INTERSECT SLOPE					
903.00	1281.00	261.00	See Message on Next Line(s)		
CIRCLE DOES NOT INTERSECT SLOPE					
553.00	931.00	261.00	Bottom of circle exceeds allowable depth - CIRCLE REJECTED		
603.00	931.00	261.00	Bottom of circle exceeds allowable depth - CIRCLE REJECTED		
653.00	931.00	261.00	Bottom of circle exceeds allowable depth - CIRCLE REJECTED		
553.00	981.00	261.00	1.332	14.30	7
653.00	981.00	261.00	1.380	18.88	7
553.00	1031.00	261.00	See Message on Next Line(s)		
DEPTH OF CRACK IS GREATER THAN DEPTH OF CIRCLE					
603.00	1031.00	261.00	2.528	21.26	5
653.00	1031.00	261.00	2.554	21.27	5
573.00	951.00	261.00	Bottom of circle exceeds allowable depth - CIRCLE REJECTED		
603.00	951.00	261.00	Bottom of circle exceeds allowable depth - CIRCLE REJECTED		
633.00	951.00	261.00	Bottom of circle exceeds allowable depth - CIRCLE REJECTED		
573.00	981.00	261.00	1.302	16.18	7
633.00	981.00	261.00	1.332	19.20	8
573.00	1011.00	261.00	1.941	23.16	6
603.00	1011.00	261.00	1.951	22.88	6
633.00	1011.00	261.00	1.999	21.04	6
593.00	971.00	261.00	Bottom of circle exceeds allowable depth - CIRCLE REJECTED		
603.00	971.00	261.00	Bottom of circle exceeds allowable depth - CIRCLE REJECTED		
613.00	971.00	261.00	Bottom of circle exceeds allowable depth - CIRCLE REJECTED		
593.00	981.00	261.00	1.291	17.81	8
613.00	981.00	261.00	1.296	18.99	8
593.00	991.00	261.00	1.204	21.58	9
603.00	991.00	261.00	1.213	21.61	9
613.00	991.00	261.00	1.226	21.23	8
583.00	981.00	261.00	1.295	17.04	7
583.00	991.00	261.00	1.204	20.91	9
583.00	1001.00	261.00	1.700	21.75	7
593.00	1001.00	261.00	1.715	21.59	7
603.00	1001.00	261.00	1.735	21.47	7

At the end of the current mode of search the most critical circle which was found has the following values -

X-center = 593.00 Y-center = 991.00 Radius = 261.00

**KINGSTON FOSSIL PLANT OPTION 1 - WET ASH IN POND GYPSUM ON PENINSULA
(WITHOUT POND BUFFER)
PRESENT WORTH**

ESTIMATE NUMBER: 09000001

Item No.	Description	Units	Unit Cost 2007 Dollars	Number of Cycles	2006 Cost	2007	2008	2009	2010	2011	2012	2013	2014	2015	2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	Present Worth 2007 Dollars		
	CAPITAL COSTS																															
1	Fixed Costs For Basin Buffer	Units	\$1,987,631	1		\$1,987,631																									\$1,987,631	
2	Wet Ash Pond	Units	\$665,446	1		\$665,446																									\$665,446	
3A	Wet Ash Pond Construction (Wet Ash Pond)	Units	\$2,713,854	1		\$2,713,854																									\$2,713,854	
3B	Wet Ash Pond Construction (Wet Ash Pond)	Units	\$8,071,569	1		\$8,071,569																									\$8,071,569	
4	Wet Ash Pond Construction (Wet Ash Pond)	Units	\$450,000	1		\$450,000																									\$450,000	
5	Wet Ash Pond Construction (Wet Ash Pond)	Units	\$550,000	1		\$550,000																									\$550,000	
	Total Capital Costs					\$4,358,500																									\$4,358,500	
	OPERATING COSTS																															
6	Wet Ash Pond	Units	\$11,846,461	10		\$118,464,610																									\$118,464,610	
7A	Wet Ash Pond Construction (Wet Ash Pond)	Units	\$3,442,000	20		\$68,840,000																									\$68,840,000	
7B	Wet Ash Pond Construction (Wet Ash Pond)	Units	\$30,483,000	10		\$304,830,000																										\$304,830,000
8	Wet Ash Pond Construction (Wet Ash Pond)	Units	\$41,754,000	20		\$835,080,000																										\$835,080,000
	Total Operating Costs					\$4,358,500																									\$4,358,500	
	Total Costs					\$8,717,000																									\$8,717,000	
	Present Worth of this Option					\$8,717,000																									\$8,717,000	

09000001

KINGSTON FOSSIL PLANT OPTION 1 - WET ASH IN POND GYPSUM ON PENINSULA
(WITHOUT POND BUFFER)

PRESENT WORTH

ESTIMATE NUMBER: 08060101

ITEM No.	DESCRIPTION	UNITS	Total Cost 2009 Dollars	Number of Days	2009 Public Rate Cost	2009	2010	2011	2012	2013	2014	2015	2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	Present Worth of Total Capital Costs	
CAPITAL COSTS																													
1	Wet Ash Drying For Better Power Prod.	Lump Sum	\$1,997,629	1	\$1,997,629																							\$1,997,629	
2	Pen St. Pond	Lump Sum	\$482,486	1	\$482,486																							\$482,486	
3A	Water Meter Construction (Water Meter)	Lump Sum	\$1,287,565	2	\$1,287,565																							\$1,287,565	
3	System Oil Treatment	Lump Sum	\$81,000	1	\$81,000																							\$81,000	
4	Microbiocides	Lump Sum	\$49,000	4	\$49,000																							\$196,000	
5	Watermeter / Damers	Lump Sum	\$88,000	1	\$88,000																							\$88,000	
Total Capital Costs																													
6																													
7																													
OPERATING COSTS																													
8	Shutdown Cost Phase 1	Lump Sum	\$66,219	12	\$66,219	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442	\$1,104,442
14	Shutdown Oil Production Beyond Cost	Lump Sum	\$58,488	2	\$58,488																							\$58,488	
20	Shutdown Wet Ash Drying Pen St. Pond	Lump Sum	\$18,284	1	\$18,284																							\$18,284	
21	Shutdown Wet Ash Drying Pen St. Pond	Lump Sum	\$52,892	1	\$52,892																							\$52,892	
Total Operating Costs																													
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08060101

ESTIMATE NUMBER: 60820089

KINGSTON FOSSIL PLANT OPTION 5 - WET ASH IN POND GYPSUM ON PENINSULA (WITH BUFFER)

PRESENT WORTH

ITEM No.	DESCRIPTION	UNITS	Total Cost 2008 Dollars	Number of Cycles	2008 Dollars per Cycle	2008	2009	2010	2011	2012	2013	2014	2015	2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	2030	Present Worth of Capital Dollars											
1	Installation		\$1,887,628			1,887,628																																		
2	Wet Ash in Pond	Units	\$24,929	1	\$24,929																																			
3A	Wet Ash in Pond (with Buffer)	Units	\$5,121,971	2	\$2,560,985.50																																			
3	Operation and Maintenance	Units	\$8,073,081	1	\$8,073,081																																			
4	Microbiology	Units	\$5,457,035	2	\$2,728,517.50																																			
5	Replacement / Overhaul	Units	\$28,192,541	1	\$28,192,541																																			
Total Capital Costs						\$47,227,220																																		
6	Development Phase 1	Units	\$11,066,416	12	\$922,201																																			
7	Strengthen On Peninsula Structural Deck	Units	\$1,424,072	20	\$71,204																																			
8	Strengthen On Peninsula (with Buffer)	Units	\$18,433,379	20	\$921,669																																			
9	Operations for Construction of Dike	Units	\$471,247	20	\$23,562																																			
Total Operating Costs						\$28,192,541																																		
Total Costs						\$75,419,761																																		
Present Worth of this Option						\$51,458,844																																		

03/20/08

KINGSTON FOSSIL PLANT OPTION 6 - DRY ASH IN POND GYPSUM ON PENINSULA
(WITH BUFFER)

PRESENT WORTH

ITEM No.	DESCRIPTION	UNIT	2009 Cost (2009 Dollars)	Number of Years	2009 Cost	2010	2011	2012	2013	2014	2015	2016	2017	2018	2019	2020	2021	2022	2023	2024	2025	2026	2027	2028	2029	Revised Subtotal	Present Worth of the Capital Costs																								
Total Capital Costs																																																			
1	Asph In Pond	Lump Sum	\$332,698	1	\$332,698																						\$332,698	\$358,458																							
2A	Phase 2 Dry Ash Comminution (Bank Layout)	Lump Sum	\$4,645,153	2	\$9,290,306																						\$9,290,306	\$17,884,371																							
3	Operation Cost (Maintenance)	Lump Sum	\$22,524,927	1	\$22,524,927																						\$22,524,927	\$4,874,733																							
4A	Phase 2 Dry Ash Comminution (Three Stages 2)	Lump Sum	\$2,533,000	1	\$2,533,000																						\$2,533,000	\$1,002,855																							
5	Embarkment / Ditchwork	Lump Sum	\$824,000	1	\$824,000																						\$824,000	\$824,000																							
Total Capital Costs																																																			
OPERATION COSTS																																																			
1	Underpool Pumps	Lump Sum	\$20,011,810	12	\$240,141,720																						\$240,141,720	\$3,107,674																							
2	Gypsum On Peninsula Disposal Cost	Lump Sum	\$3,244,078	20	\$64,881,556																						\$64,881,556	\$1,101,000																							
3	Phase 2 Dry Ash (Under Three Stages 2)	Lump Sum	\$1,000,100	12	\$12,001,200																						\$12,001,200	\$151,000																							
4	GRAND FOR CONSTRUCTION OF DISPOSAL	Lump Sum	\$472,347	24	\$11,336,328																						\$11,336,328	\$183,000																							
Total Operation Costs																																																			
Present Worth of this Option																																																			
																								\$45,930,710	\$4,874,733	\$4,874,733	\$5,209,443	\$5,567,839	\$5,958,611	\$6,384,821	\$6,851,061	\$7,363,518	\$7,919,471	\$8,527,210	\$9,194,722	\$9,923,298	\$10,714,411	\$11,578,461	\$12,528,531	\$13,568,198	\$14,703,151	\$15,939,077	\$17,282,571	\$18,740,225	\$20,320,661	\$22,033,491	\$23,889,421	\$25,899,253	\$28,073,795		
Total Present Worth of this Option																																																			
																								\$86,029,468	\$86,029,468	\$86,029,468	\$91,248,911	\$96,816,750	\$102,745,191	\$109,044,412	\$115,824,473	\$123,106,484	\$130,910,455	\$139,258,386	\$148,181,197	\$157,701,868	\$167,854,359	\$178,674,670	\$190,199,721	\$203,577,292	\$217,856,481	\$233,100,982	\$249,381,909	\$266,771,986	\$285,354,122	\$305,220,427	\$326,474,802	\$349,223,455	\$373,585,498	\$400,671,023	\$430,594,718

ITEM	DESCRIPTION	UNITS	QUANTITY
	Pumps & Pipe Phase 1		
17.00	100 hp pump for Phase 1 Stormwater Pond	ea	1
17.01	18 in dia discharge HDPE pipe SDR 11	lf	2000
17.02	Platform steel	ton	5
17.03	Pumps & Pipe Full buildout		
17.04	50 hp pump for Phase 2 stormwater Pond	ea	1
17.05	12 in dia discharge HDPE Pipe SDR 11	lf	4500
17.06	Platform steel	ton	5
18.00	Electrical - Phase 1		
18.01	Phase 1 Transformer 200kva	LS	\$8,000
18.02	4 / 0 600 volt Cable	lf	200
18.03	3 in dia conduit	lf	200
18.04	13 kV 15 amp pole mount fuse disconnect	ea	1
18.05	Electrical - Full buildout		
18.06	Phase 1 Transformer 200kva	LS	\$8,000
18.07	4 / 0 600 volt Cable	lf	200
18.08	3 in dia conduit	lf	200
18.09	13 kV 15 amp pole mount fuse disconnect	ea	1
18.10	No 6 15 kvolt overhead cable	lf	4500
18.11	Poles between ponds	ea	15
18.12			
18.13			
18.14			

**KIF Gypsum on Peninsula
Phase 2 - Gypsum Long Term Rim Ditch Stack**

PRELIMINARY

ITEM	DESCRIPTION	UNITS	QUANTITY	T-1 Spec	Comments/Assumptions
7.00	Gypsum Stack Peninsula (7.00 through 18.00)				
7.01	Clear and Grub				See Note 1 at end of table
7.02	Clear and grub	ac	35		C - Tree line only
7.03	Strip 1 ft vegetation and topsoil - spoil at stockpile	bcy	193,600		C - 120 ac entire site, place soil on site approximately 1 mile roundtrip
8.00	Erosion Controls / Ponds				
8.01	Erect silt fence	lf	9,250		C - 6 ft post spacing, trench bottom of fence, 10% hay bales
8.02	Cut for stormwater runoff ponds	bcy	170,970		C - 2 ponds, Phase 1 Pond: 6 ac 17' deep, Phase 2 Pond: 6 ac 10' deep. Place as fill in gypsum stack area.
8.03	Fill for stormwater runoff ponds	bcy	16,425		C - Fill for dikes around Phase 2 pond
8.04	Riprap for stormwater runoff pond	ton	2,713		C - Combined - 1 ft deep along inside slope
8.05	72 in dia cmp for outlet structure	lf	12		C
8.06	48 in dia cmp for riser for outlet structure	lf	14		C
8.07	Cut holes in riser	ea	6		C
8.08	48 in dia cmp outlet pipe (principle spillway)	lf	300		C
8.09	(2) Concrete for riser base (assume 7ft x 7 ft x 2 ft)	cy	9		C
8.10	Anti-seep collars (assume concrete)	ea	14		C - Assume 1.5 cy concrete per collar
8.11	Pipe bedding - #57 stone	ton	40		C
8.12	Clean out stormwater runoff pond	bcy	7,150		C - Assume 2 ft sediment, 67 cy/ac depth x 110 ac
9.00	Access Roads				Assume 1.3 miles of roadway (7000 ft); road is 20 ft wide
9.01	Bottom Ash	bcy	2593		C - Assume 6 in bottom ash
9.02	Crushed stone base	ton	2541		C - Assume 4 inch stone (1032), 110 pcf
9.03	Crushed stone base - parking lot	ton	275		C - Assume 100 x 100 ft w/ 6 in crushed stone (110 pcf)
10.00	Fencing (Exclude De-watering Facility)				
10.01	New fencing (including grounding)	lf	200		C - Assume chain link fence to block road only - no perimeter fence
10.02	Gates, swinging	ea	1		C - personnel
10.03	Gates, sliding, w/ motorized operator	ea	1		C - 20 ft wide
11.00	Seed/Mulch				
11.01	Seed/Mulch disturbed areas	ac	25		C - Areas outside dike

12.00	Borrow Area Development					
12.01	Clear and grub	ac	33			C
12.02	Erect silt fence	lf	3,000			C
12.03	Temporary perimeter ditch	bcy	2,900			C - 15 acres of runoff
12.04	Cut for temporary stormwater runoff ponds	bcy	4,422			C - 33 acres*134 cy/ac
12.05	Fill for temporary stormwater runoff ponds	bcy	884			C
12.06	72 in dia cmp for outlet structure	lf	12			C
12.07	48 in dia cmp for riser for outlet structure	lf	20			C
12.08	Cut holes in riser	ea	5			C
12.09	48 in dia cmp outlet pipe (principle spillway)	lf	200			C
12.10	(2) Concrete for riser base (assume 7ft x 7 ft x 2 ft)	cy	9			C
12.11	Anti-seep collars (assume concrete)	ea	20			C - Assume 1.5 cy per collar
12.12	Pipe bedding - #57 stone	ton	25			C - Assumed Unit wt = 125 pcf
12.13	Clean out stormwater runoff pond	bcy	4,422			C - Assume 2ft sediment, 67 cy/ac @ 33 ac
13.00	Gypsum Disposal Facility					
13.01	Disposal Facility Construction					
13.01	Earthwork cut	bcy	357,647			C - to shape base in gypsum stack area
13.02	Earthwork fill from borrow area	bcy	899,679			C - Remaining fill from onsite borrow area. 1428296-(357647+170970)=899679 Haul Length = 2 miles roundtrip
13.03	Additional spoil material	bcy	0			
13.04	Proofroll subgrade	ac	100			C
13.05	Cut and recompact liner	bcy	445,944			C - Over excavate 3' and recompact to build liner
13.06	Perimeter underdrain pipe	lf	8,220			C - 8 in dia HDPE, SDR = 17, perforated
13.07	Cut for Underdrain system	cy	2,667			C
13.08	Geotextile 8oz nonwoven for underdrain	sy	20,000			C - Wrapped around pipe bedding
13.09	6" depth 1081 crushed stone (110 pcf)	ton	1,320			C - Initial bedding for underdrain piping
13.10	Underdrain piping (8" dia HDPE)	lf	24,000			C
13.11	HDPE Fittings for underdrain piping	ea	65			
13.12	2.5" thick bottom ash drainage layer	bcy	371,620			C
13.13	6" Fly ash layer	bcy	74,324			C - Mix
13.14	Perimeter road surfacing - bottom ash	cy	3130			C - 6 in bottom ash topped w/ 4 in stone - 1.6 mi of roadway 20 ft wide
13.15	Perimeter road surfacing - crushed stone	ton	3067			C - Assumed Unit wt = 110pcf - 1.6 miles of roadway
13.16	Riprap Ditch (D50 = 6 in)	ton	64,682			C - Assume riprap 2 ft depth, 8055 lf ditch, Unit wt = 110 pcf
13.17	8 oz nonwoven geotextile (if riprap is used)	sy	65,335			C - Assume ditch has 9 ft bottom width, 10 ft depth and 3:1 side slopes
14.00	Gypsum on Peninsula Disposal Cost					Spread the O&M Cost Over the Life of the Stack

14.01	Cut for Underdrain system	cy	7,359	O&M
14.02	8 oz nonwoven geotextile for underdrain	sy	55,193	O&M - Wrapped around stone envelope
14.03	6" depth 1081 crushed stone (110 pcf)	ton	3,643	O&M - Initial bedding for underdrain piping
14.04	6" dia perforated HDPE SDR 17 underdrains	lf	66,232	O&M - Inside Wet Cast Gypsum Dike, Elevations 775 to 850 (20 yrs)
14.05	Fill for Underdrain system	cy	5,039	O&M - Bottom ash
14.06	Cut for Lateral outlet pipes	cy	2,392	O&M
14.07	6" dia non-perforated HDPE SDR 17 Lateral outlet pipes	lf	21,525	O&M - Lateral pipe located every 200' on center, avg length 65 ft
14.08	Fill for Lateral outlet pipes	cy	2,235	O&M - Bottom ash
14.09	6" depth 1081 crushed stone (110 pcf)	ton	1,184	O&M
14.10	Gypsum Disposal Stack (wet sluice)	cy	7918000	O&M
14.11	Wet cast Gyp Dike	cy	1324640	O&M - Elevations 775 to 850 (20 yrs)
14.12	Cut Rim Ditches	cy	127558	O&M
14.13	Life of Gypsum Disposal Stack	yr	20.0	O&M - Assume 395900 cubic yards per year (@ 84 pcf)
15.00	Construction parking			
15.01	Earthwork cut	bcy	833	C
15.02	Earthwork fill	bcy	300	C
15.03	Crushed stone base	ton	675	C - Assume 150 x 150 ft x 6 in thick crushed stone base
16.00	Engineering			
16.01	Engineering	ls		C - Take 10% of construction cost (see Dan Smith to confirm engineering cost).
17.00	Pump Station			
17.01	Phase 1 Pond			Later
17.02	Phase 2 Pond			Later
18.00	Electrical			See Electrical takeoffs

Notes:

- Item #7 corresponds to Gypsum Disposal at the Peninsula. This numbering system matched the one used for the comparative study done in December '04.

C - Capital cost

O&M - Operation and Maintenance cost

ITEM	DESCRIPTION	UNITS	QUANTITY
	Pumps & Pipe Phase 1		
17.00	100 hp pump for Phase 1 Stormwater Pond	ea	1
17.01	18 in dia discharge HDPE pipe SDR 11	lf	2000
17.02	Platform steel	ton	5
17.03	Pumps & Pipe Full buildout		
17.04	50 hp pump for Phase 2 stormwater Pond	ea	1
17.05	12 in dia discharge HDPE Pipe SDR 11	lf	4500
17.06	Platform steel	ton	5
18.00	Electrical - Phase 1		
18.01	Phase 1 Transformer 200kva	LS	\$8,000
18.02	4 /0 600 volt Cable	lf	200
18.03	3 in dia conduit	lf	200
18.04	13 kV 15 amp pole mount fuse disconnect	ea	1
18.05	Electrical - Full buildout		
18.06	Phase 1 Transformer 200kva	LS	\$8,000
18.07	4 /0 600 volt Cable	lf	200
18.08	3 in dia conduit	lf	200
18.09	13 kV 15 amp pole mount fuse disconnect	ea	1
18.10	No 6 15 kvolt overhead cable	lf	4500
18.11	Poles between ponds	ea	15
18.12			
18.13			
18.14			

Comments/Assumptions
Bury 3.5 ft deep with bottom ash bedding
Bury 3.5 ft deep with bottom ash bedding
13.8 kV to 480 v
underground
underground
13.8 kV to 480 v
underground
underground
aboveground - see below
Assume 300 ft spacing - aboveground on poles

KIF450 U6-9 FGD ADDITION
Phase A Preliminary Engineering

Activity ID	Activity Description	Forecast Start	Forecast Finish	Finish Target	Total Resp Float	Prtn Engr	RE	Res ID	Bdgt Mhrs	Frcst Mhrs	Actual Mhrs
A	DIFAGYP20 Req TAO For Parsons - Mike Hughes (HLP)		01APR05A			HLP	MSH		0.00	0.00	0.00
A	DIFAGYP22 Scoping Discussion		01APR05A			HLP	MSH		0.00	0.00	0.00
A	DIFAGYP18 Map of Survey - Scope to Surveying (HLP)		08APR05A			HLP	MSH		0.00	0.00	0.00
A	DIFAGYP17 Scope of Drilling to MATEC (HLP)		11APR05A			HLP	MSH		0.00	0.00	0.00
A	DIFAGYP12 RSO&E Survey of Holes- Predrill (Phy Layout HLP)		18APR05A			HLP	MSH		0.00	0.00	0.00
A	DIFAGYP31 CEC for Drilling (Generic #5259) (HLP)		20APR05A			HLP	MSH		0.00	0.00	0.00
A	DIFAGYP15 Issue MATEC TAO (HLP)		25APR05A			HLP	MSH		0.00	0.00	0.00
A	DIFAGYP16 Receive MATEC Proposal (HLP)		25APR05A			HLP	MSH		0.00	0.00	0.00
A	DIFAGYP13 RSO&E Prepare Hydrogeo Workplan (Hank Julian)		26APR05A			HLP	MSH		0.00	0.00	0.00
A	DIFAGYP11 Env Aff Meet /TDEC - Hydrogeo Wkpin-(Amos/Larry)		27APR05A			HLP	MSH		0.00	0.00	0.00
A	DIFAGYP14 MACTEC Start Drilling (HLP)		28APR05A			HLP	MSH		0.00	0.00	0.00
A	DIFAGYP07 RSO&E Begin Hydrogeo Report (Draft) (H Julian)		29APR05A			HLP	MSH		0.00	0.00	0.00
A	DIFAGYP21 LOA for RSO&E (HLP)		29APR05A			HLP	MSH		0.00	0.00	0.00
A	DIFAGYP09 LOA for GeoProbe (HLP)		03MAY05A			HLP	MSH		0.00	0.00	0.00
A	35 DIFAGYP06 RSO&E Receive Prelim MACTEC Rept (TVA) (HLP)		20JUN05A			HLP	MSH		0.00	0.00	0.00
A	35 DIFAGYP19 Phase 1B: Issue TAO to Parsons (HLP)		15JUL05A			HLP	MSH		0.00	0.00	0.00
A	35 DIFAGYP25 Carrier Site Visit	20JUL05A	21JUL05A			MSH	HLP	MSH	0.00	0.00	0.00
A	30 DIFAGYP04 RSO&E Complete Draft to Environ Aff -Hank Julian		23AUG05A			MSH	HLP	MSH	0.00	0.00	0.00
A	30 DIFAGYP05 RSO&E Complete Draft to Parsons -Hank Julian		23AUG05A			MSH	HLP	MSH	0.00	0.00	0.00
A	35 DIFAGYP10 Receive Final Mactec Lab Results		26AUG05A			MSH	HLP	MSH	0.00	0.00	0.00
A	50 DIFAGYP03 Hydro Geo Final Internal Review Enviro Aff		29AUG05A			ALS	HLP	MSH	0.00	0.00	0.00
A	35 DIFAGYP35 Part 1 Application (Larry Bowers-Begin Package)		01SEP05A			MSH	HLP	MSH	0.00	0.00	0.00
A	35 DIFAGYP30 Nuyt update meeting		23SEP05A			MSH	HLP	MSH	0.00	0.00	0.00
A	35 DIFAGYP37 NOD Response by Environmental Affairs		31MAR06		28	MSH	HLP	MSH	0.00	0.00	0.00
A	AM DIFAGYP41 Site Meeting to Review MATEC Report -Steve Baugh	05OCT05A	05OCT05A			BSL	JSB	MSH	0.00	0.00	0.00
A	35 DIFAGYP32 Preliminary Footprint Drawing to JPT		10OCT05A			MSH	HLP	MSH	0.00	0.00	0.00
A	35 DIFAGYP33 JPT Meeting - Status		19OCT05A			MSH	HLP	MSH	0.00	0.00	0.00

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S E T C	Activity ID	Activity Description	Forecast Start	Forecast Finish	Finish Target	Total Resp Float Engr	Prin Engr	RE	Res ID	Bdgt Mhrs	Frstc Mhrs	Actual Mhrs	Year			
													2005	2006	2007	2008
A	35	DIFAGYP02	Final Hydrogeo Report to Envir Aff (Hank Julian)	28OCT05A			MSH HLP	MSH		0.00	0.00	0.00				
A	35	DIFAGYP34	Quantity Takeoffs to Radford/Toney	28OCT05A			MSH HLP	MSH		0.00	0.00	0.00				
A	15	DIFAGYPES	Complete Cost Estimate for Phase 3 (GYP POND)	14NOV05A			CLT JLH	HLP		0.00	0.00	0.00				
A	35	DIFAGYP36	Final Estimate back from Radford/Toney	18NOV05A			MSH HLP	MSH		0.00	0.00	0.00				
A	35	DIFAGYP38	Final Footprint Study Drawing	18NOV05A			MSH HLP	MSH		0.00	0.00	0.00				
A	35	DIFAGYP01	State Receives Hydro Geo Report	21NOV05A			MSH HLP	MSH		0.00	0.00	0.00				
A	35	DIFAGYP39	PPD Preliminary to JPT	28NOV05A			MSH HLP	MSH		0.00	0.00	0.00				
A	35	DIFAGYP40	Final PPD	02DEC05*			MSH HLP	MSH		0.00	0.00	0.00				
A		DIFAGYP08	State Approves Hydro Geo Report	15APR06*		13	HLP	MSH		0.00	0.00	0.00				
A	30	DIFAGYPPC	Preliminary Engineering Complete (Phase 1B)	15APR06		13	SMH HLP	MSH		0.00	0.00	0.00				
Phase B Final Engineering																
A	50	DIFBGYP10	Part I Application to TDEC	21OCT05A			MSH HLP	MSH		0.00	0.00	0.00				
A	35	DIFBGYP05	10% Design Review	12DEC05*		4	MSH HLP	MSH		0.00	0.00	0.00				
A	35	DIFBHP15	Send Drawings to Plant 1 Week Prior to 50% Mtg	06MAR06*		649	SMH HLP	MSH		0.00	0.00	0.00				
A	35	DIFBGYP20	50% Design Review Meeting	15MAR06*		640	SMH HLP	MSH		0.00	0.00	0.00				
A	50	DIFBGYP28	Part II Application to TDEC	16MAY06*		168	SMH HLP	MSH		0.00	0.00	0.00				
A	50	DIFBGYP30	ARAP/404 Permit Application	16MAY06*		168	SMH HLP	MSH		0.00	0.00	0.00				
A		DIFBGYP35	ARAP/404 Permit Issued	01MAY07*		228	SMH HLP	MSH		0.00	0.00	0.00				
A		DIFBGYP40	TDEC Issues Solid Waste Permit	15OCT07*		61	SMH HLP	MSH		0.00	0.00	0.00				
A	35	DIFBGYP45	100% Design Review Meeting	15DEC07*		0	SMH HLP	MSH		0.00	0.00	0.00				
Phase C																
A	39	VRRESKIF69	FY 05 AG&WS SUPPORT - KINGSTON 6- 9 FGD ADDITION	17JAN05A	30SEP05A		GTM REJ	TJM	FSAG,	500.00	38.00	38.00				
A	35	VRRESKIFC0	FY 05 CIVIL SUPPORT - KINGSTON 6- 9 FGD ADDITION	04MAR05A	30SEP05A		VJD HLP	TJM	FDCEA	50.00	42.00	42.00				
A	31	VRRESKIFZ1	FY 05 MECH ENG SUPP - KINGSTON 6- 9 FGD ADDITION	16MAY05A	30SEP05A		MDL DLD	TJM	FDMEA	200.00	200.00	200.00				
A	33	VRRESKIFZ2	FY 05 I&C ENG SUPP - KINGSTON 6- 9 FGD ADDITION	06JUN05A	23SEP05A		RDG RCO	TJM	FDEEZD	200.00	4.00	4.00				
A	31	VRRESKIFZ4	FY 06 MECH ENG SUPP - KINGSTON 6- 9 FGD ADDITION	03OCT05A	30SEP06	-1	MDL VVD	TJM	FDME	800.00	800.00	89.00				
A	39	VRRESSYSAG	FY 06 FIRE PROT SUPP-KINGSTON 6- 9 FGD ADDITION	03OCT05A	30SEP06	0	NCR REJ	TJM	FSAG,	200.00	208.00	8.00				
A	39	VRRESSYSAH	FY 06 AGW&S SUPP-KINGSTON 6- 9 FGD ADDITION	03OCT05A	30SEP06	0	GTM REJ	TJM	FSAG	2,000.00	2,000.00	46.00				
A	35	VRRESSYSAI	FY 06 CIVIL SUPP-KINGSTON 6- 9 FGD ADDITION	03OCT05A	30SEP06	0	HLP HLP	TJM	FDCEA	1,000.00	1,000.00	8.00				

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