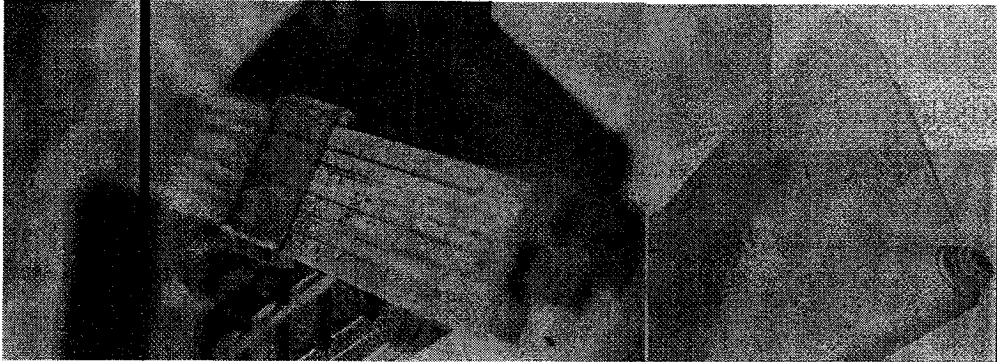


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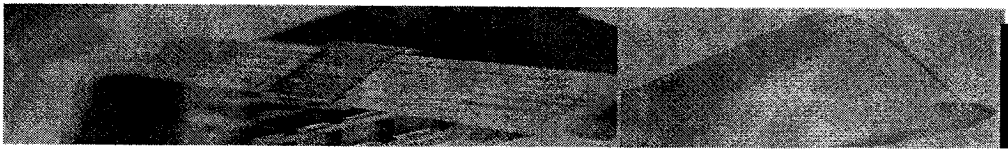
TVA Kingston Fossil Plant

Gypsum Disposal Pond Sizing and Development of Design



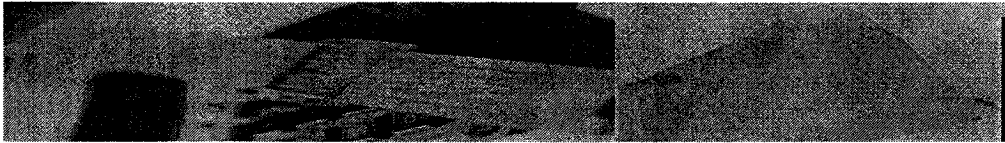
Comments on Hyrdogeology

- Surface is evident of Karst features, but no cavities evident in overburden beneath these features, only clay
- Some solutioning of the bedrock along bedding planes and formation contacts; some cavities noted in the rock with the largest in borehole N66 which had a cavity of about 8 ft
- Does not appear to be an “active” karst system even though there is some openings in the rock ;these solution openings are probably in the 10’s of millions of years old
- Water table follows topography and water levels monitored since 2003 with continuous monitoring
- Groundwater flows to the southeast toward the river
- Seasonal high water levels are between 767ft (sideslope) and 749 ft (site area) MSL
- River stage is about 741 ft MSL
- 100-year flood is mapped at 747.6 ft MSL



Starting Point and Site Constraints

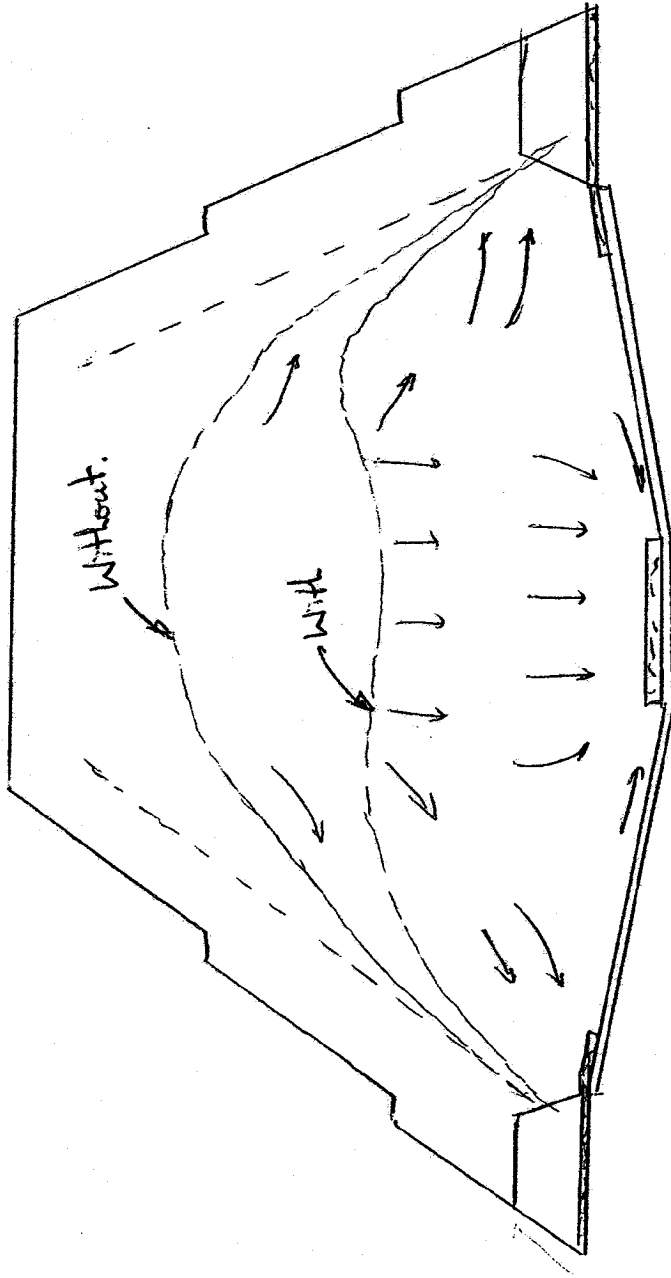
- July 2005 potentiometric elevation in area of disposal area is 741
- Statistical high is 749
- Based on need to provide a 5' min separation to groundwater, lowest subgrade elevation of 752' ~ 754' EL was selected for preliminary layouts



Need for Interior Drainage

- Blanket drain is recommended beneath initial soil starter dikes; however, a continuous blanket is not likely needed to address stability issues
- In subsequent lifts, outer gypsum dikes are expected to be permeable providing adequate drainage (note: as a safeguard, lateral drainage pipes will also be integrated into gypsum dikes)
- Central Drainage Corridor is recommended for the following reasons:
 - Provides relief to hydrostatic pressure on clay layer (minimizing the risk of blowout and activation of karst)
 - Eliminates need for second stormwater pond by providing a conduit for Phase II decant
 - Will likely require a sump/lift station at outlet





Probable Influence of Interior Drainage.

SEEPAGE RUNS TO CONFIRM.



GEO SYNTEC CONSULTANTS

Gypsum Properties

- Based on Cumberland Fossil Plant Gypsum and Bull Run calculations
- Three groups of Gypsum properties:
 - Cast ($\gamma = 120$ pcf; $\phi' = 43^\circ$)
 - Sedimented ($\gamma = 116$ pcf; $\phi' = 40^\circ$)
 - Dry placed ($\gamma = 107$ pcf; $\phi' = 35^\circ$)
- Sedimented Gypsum will be modeled as undrained at end of construction



Natural/Alluvial Clay Soil

- 30 to 50 ft of natural soil above bedrock
- Primarily silty clay (SPT N~10)
- 5 to 15 ft of soft sandy to sandy/clayey silt residuum exists above the bedrock with SPT blowcount equivalent to WOH to 2 bpf
- Triaxial test data available?
- Water table at 741' MSL to 781' MSL; increase toward project north



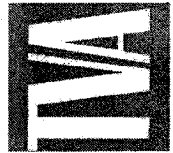
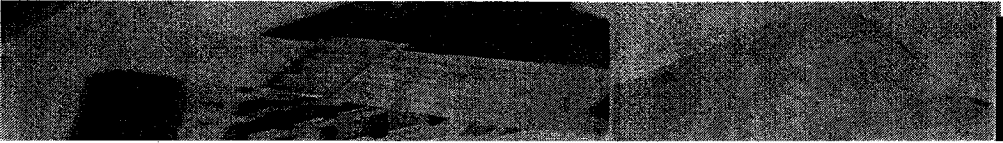
Natural/Alluvial Clay Soil (cont.)

- Undrained shear strength of N~10 silty clay, $s_u = 1950$ psf (Ref. Parsons calculation package and draft triaxial tests)
- Additional estimate of undrained shear strength to be obtained from CPT borings on-site



Critical Cross Sections

- Cross-sections cut at maximum grade (average maximum grade of 1V:3.3H) through maximum height of stack
- Similar to Parsons critical cross-section A-A' including information from borings B-22, NB-45, NB-47, NB-59, NB-65, and NB-73 (etc.)
- Similar to Parsons cross-section B-B' through Borings B-12, B-13, NB-39 and NB-44 (etc.)



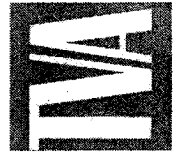
Basis for Size (re-work of Parsons calculation)

- Parsons assumed 385,000 ton/yr production (based on 2.5# coal)
- Using a design coal of 3.2#, assumed production becomes 492,800 ton/yr
- Assume operation as a wet pond for first 2 yrs; base capacity on full by-pass
- Assume 67 lb/cf density for settled gypsum (0.90ton/cy)
- 547,500 cy/yr of settled gypsum
- 1,095,100 cy for 2 years = 680 ac-ft
- Approximate interior footprint of Phase I is 45 ac
- Approximate depth required (not incl. freeboard or 5 ft deep settling zone) = 680 ac-ft / 45 ac = 15 ft

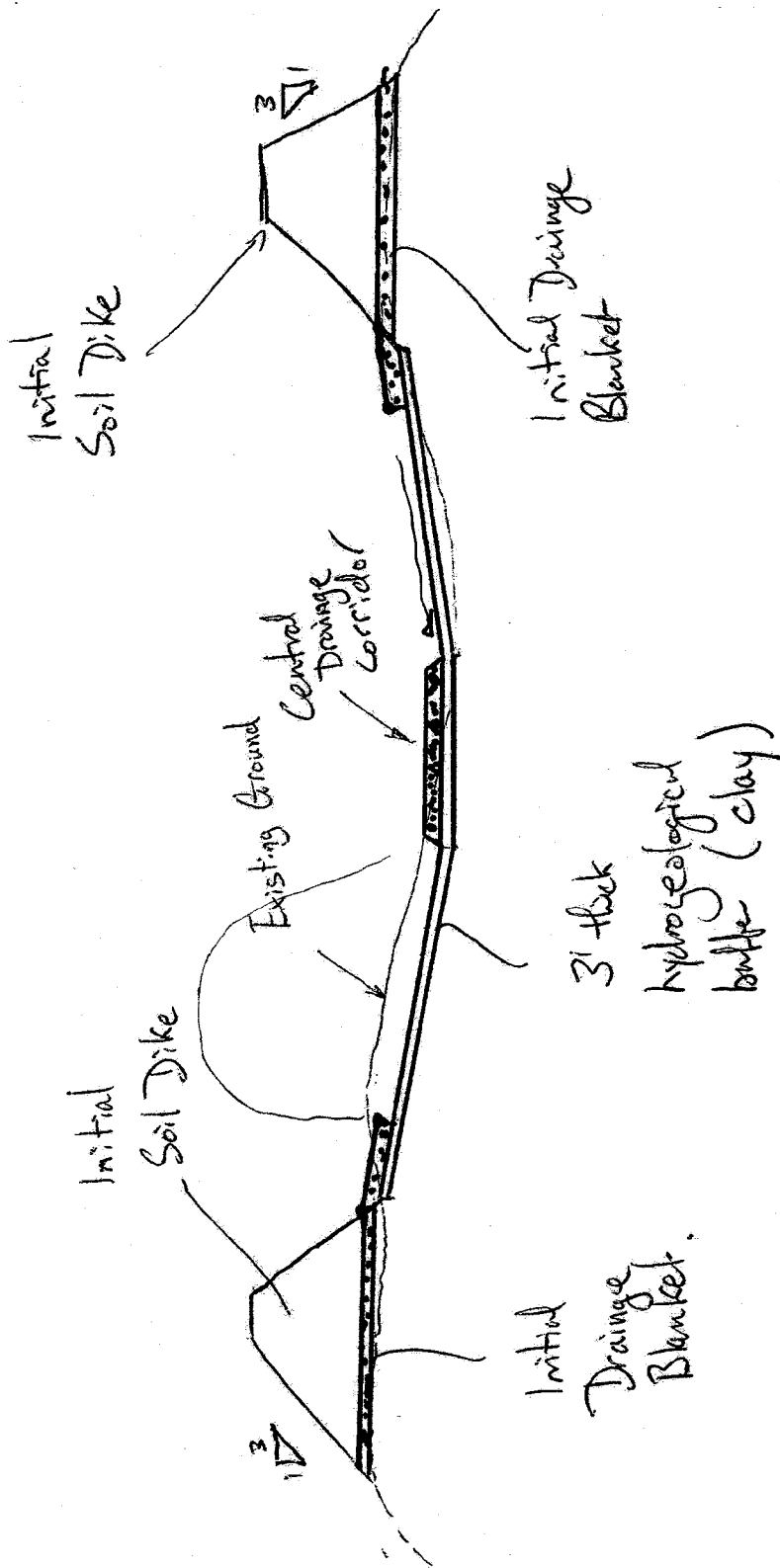


GeoSyntec – Initial Layout

- GeoSyntec developed an initial layout incorporating:
 - Initial soil starter dike with max height around 20 ft
 - 1% slope around top perimeter (for future use as surface water drainage ditch)
 - Sloping base with approximately 0.3% slope
 - Central drainage gallery
- Operational Question
 - At this configuration, initial storage volume is only 212,000 cy (say 2 months) [note: accounts for site topography and disposal area orientation]
 - Is it feasible to start wet cast operations at low end to raise dikes rather than constructing massive starter dikes?
 - Options
 - Commence wet cast operations at low end early
 - Raise elevation of initial soil starter dikes (expensive, eats into airspace)
 - Compromise (i.e., size initial pond for less than 2 years)



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Stage 1 - Initial Starter Dikes.

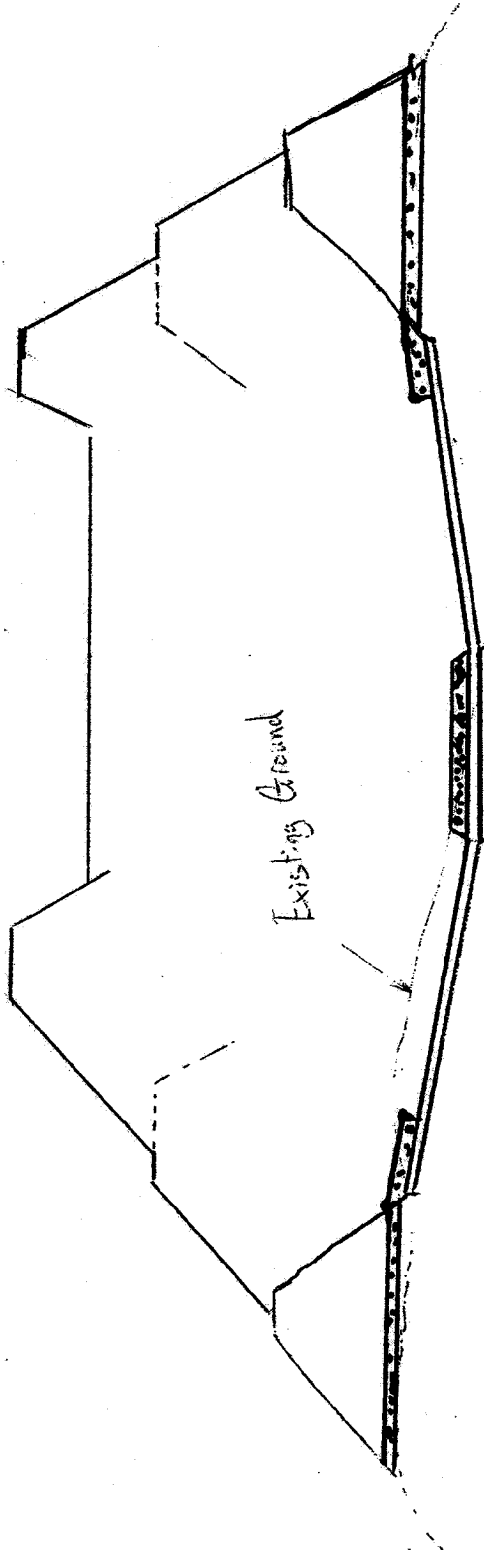


[Note]

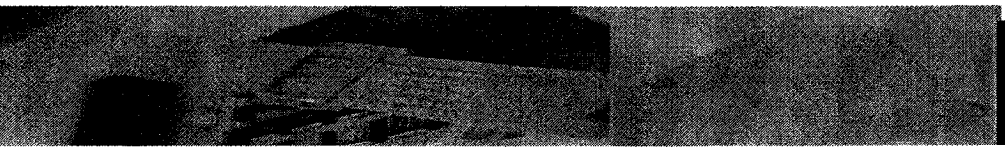
- {Developing a series of slides to illustrate filling operations and transition to Rim Ditch operation}



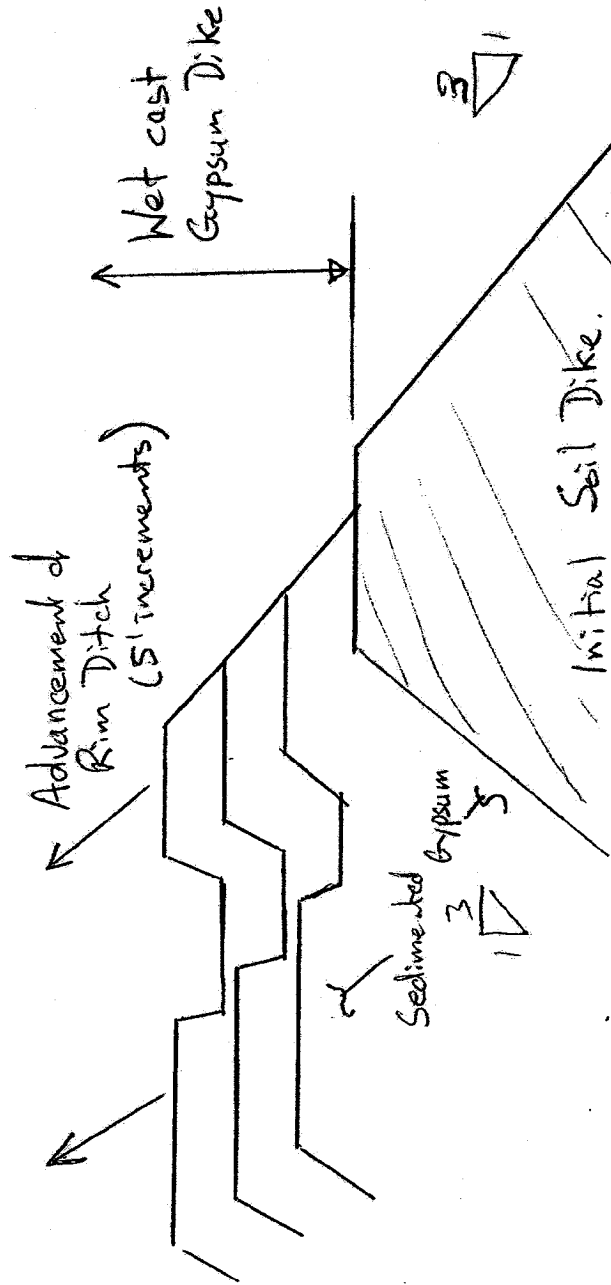
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Stage 2 - Transition To Wet Cast Dike.



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Surface Water Pond Sizing (approx.)

Purpose

- Takes run-off from design storm event
- Takes decant from gypsum slurry operations

Stormwater (sizing)

- Estimated runoff:
 - 100 yr, 24 hr storm gives 5.25 in over 91 acres
 - 6 acres at 7 in
 - Needed capacity = 44 ac-ft.

Decant

- Assume 5# coal; gives 1190 gpm flow (from Parsons, seems reasonable if approximately a 30% slurry)
- Requires approx. 5.25 ac-ft

Min Capacity

- $44 + 5.25 = 50$ ac-ft
- For a berm EL of 768' and base EL of 754' (14 ft)
- With 3 ft freeboard, gives available capacity of 6 ac x 11 ft = 66 ac-ft

