

Simulation of Daily Streamflow

April 1968

REPORT DOCUMENTATION PAGE Form Approved OMB No. 0704			Form Approved OMB No. 0704-0188			
The public reporting burden for this collection of information is estimated to average 1 hour per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. Send comments regarding this burden estimate or any other aspect of this collection of information, including suggestions for reducing this burden, to the Department of Defense, Executive Services and Communications Directorate (0704-0188). Respondents should be aware that notwithstanding any other provision of law, no person shall be subject to any penalty for failing to comply with a collection of information if it does not display a currently valid OMB control number.						
1. REPORT DATE (DD-	-ММ-ҮҮҮҮ)	2. REPORT TYPE		3. DATES CO	OVERED (From - To)	
April 1968		Technical Paper				
4. TITLE AND SUBTIT			5a	. CONTRACT N	UMBER	
Simulation of Dail	ly Sueanniow		54	. GRANT NUME		
			JL	JD. GRANT NOMBER		
			50	. PROGRAM EL	EMENT NUMBER	
6. AUTHOR(S) Leo R. Beard			50	5d. PROJECT NUMBER		
			5e	5e. TASK NUMBER		
			5F	. WORK UNIT N	IUMBER	
7. PERFORMING ORC US Army Corps of Institute for Water Hydrologic Engine 609 Second Street Davis, CA 95616	f Engineers Resources eering Center (HI			TP-6	IING ORGANIZATION REPORT NUMBER	
9. SPONSORING/MON	ITORING AGENCY N	AME(S) AND ADDRES	SS(ES)	10. SPONSO	PR/ MONITOR'S ACRONYM(S)	
				11. SPONSO	PR/ MONITOR'S REPORT NUMBER(S)	
12. DISTRIBUTION / A Approved for publ	-					
13. SUPPLEMENTAR						
Presented at the In	ternational Hydro	ology Symposium	, 6-8 September 196	7, Colorado S	State University, Fort Collins.	
element of serial c	orrelation and a r teristics between	andom componen high flows and low	t in the generation of w flows are evaluate	f a daily strea	l model including more than one mflow. Differences in statistical and A generalized equation for	
15. SUBJECT TERMS synthetic hydrolog	y, statistical mod	els, stochastic pro	cesses, streamflow f	orecasting, M	larkov processes, statistical methods,	
correlation analysis, regression analysis, simulation analysis						
16. SECURITY CLASSIFICATION OF: 17. LIMITATIO			17. LIMITATION	18. NUMBER	19a. NAME OF RESPONSIBLE PERSON	
a. REPORT	b. ABSTRACT	c. THIS PAGE	OF ABSTRACT	OF PAGES		
U	U	U	UU	22	19b. TELEPHONE NUMBER	
					Ctenderd Farm 000 (Day 0/00)	
					Standard Form 298 (Rev. 8/98) Prescribed by ANSI Std. Z39-18	

Simulation of Daily Streamflow

April 1968

US Army Corps of Engineers Institute for Water Resources Hydrologic Engineering Center 609 Second Street Davis, CA 95616

(530) 756-1104 (530) 756-8250 FAX www.hec.usace.army.mil Papers in this series have resulted from technical activities of the Hydrologic Engineering Center. Versions of some of these have been published in technical journals or in conference proceedings. The purpose of this series is to make the information available for use in the Center's training program and for distribution with the Corps of Engineers.

The findings in this report are not to be construed as an official Department of the Army position unless so designated by other authorized documents.

The contents of this report are not to be used for advertising, publication, or promotional purposes. Citation of trade names does not constitute an official endorsement or approval of the use of such commercial products.

SIMULATION OF DAILY STREAMFLOW

by

Leo R. Beard¹

INTRODUCTION

Considerable work has been done by individuals and organizations in developing procedures for simulating annual streamflows and monthly streamflows for the purpose of increasing the reliability of reservoir yield estimates. Some of this work is described in references 1, 2, and 4. It has been observed that relatively minor changes in the sequence of recorded droughts can have major influence on yield obtainable and storage space required at a reservoir project. The purpose of simulating streamflows is generally to produce long series of values that could as likely occur in the future as could repetition of historical values, thus providing a more complete sampling of potential runoff than is represented in flows as they occurred historically. The mathematical model generally used for simulating these streamflows consists of a Markov chain, usually of the first order.

Although fluctuations of flows within a month usually have minor influence on reservoir storage required for conservation purposes, such fluctuations are ordinarily crucial in the determination of reservoir space requirements for flood control. Because of this, and since there are many uncertainties in present procedures and criteria for the design of reservoirs for flood control, simulation of daily streamflows could constitute a major contribution toward improving flood-control

Director, The Hydrologic Engineering Center, US Army Corps of Engineers, Davis, California design. This possibility results primarily from the fact that virtually all available streamflow data at a station would be used in calibrating a simulation model for that location, in contrast with the rather limited use of recorded data in present flood-control design procedures. It is considered that the process would be useful primarily for generating daily flows during flood months, rather than during average or dry months.

The model described herein was based on adequate representation of daily streamflow variations at 7 selected stations listed in table 1.

Table 1

Stations Used in Study

No.

Location

l	Blue R at Dillon, Colorado
2	Apple R nr Somerset, Wisconsin
3	Kern R nr Bakersfield, California
4	Tule R at Porterville, California
5	Kaweah R nr Three Rivers, California
6	Kings R at Piedra, California
7	M. B. Westfield R nr Goss Heights. Mass.

Data used for tests of the model were also limited to these 7 stations, and other data will eventually be used for further testing and possible refinement of the model. It will be noted that some of the results of this study are similar to results of concurrent work reported in reference 3.

BASIC GENERATION MODEL

Inasmuch as most water resources studies must be based on many years of recorded or generated flows (preferably 50 to 1,000 or more), it will usually not be feasible to generate and use daily flows for the entire period of study. Accordingly, the procedure described herein has been based on generation of monthly streamflows and subsequent allocation of the monthly total amounts to each day. Thus, daily flows would only be generated for those months when flow fluctuations within a month are important. The monthly streamflow generator used is that developed in the Hydrologic Engineering Center of the Corps of Engineers and described in reference 1, with the exception that the antecedent 6-month variable is no longer used in the model. For a single station, it consists of a simple Markov chain, with each flow consisting of a random component and a component correlated with the preceding month's flow. Other model characteristics are similar to the ones described herein for daily flow generation.

The mathematical model used for daily streamflow simulation consists of a second order Markov chain, using standardized variates. Daily flows are first classified by calendar month, and their frequency characteristics (logarithmic mean, standard deviation and skew coefficient) for each calendar month are determined from observed data. One statistic would be, for example, the mean logarithm of all flows recorded in January of all years. In order to avoid the logarithm of zero flows, a slight increment equal to .OL times the average monthly flow is added to each daily flow before taking the logarithm. The same amount would subsequently be subtracted from generated flows.

Standardized variates are obtained by subtracting the mean logarithm from the daily-flow logarithm, dividing by the standard deviation, and

transforming to normal by use of the following equation, which approximates the Pearson Type III distribution:

$$t = \frac{6}{g} \left[\left(\frac{g}{2} k + 1 \right)^{1/3} - 1 \right] + \frac{g}{6}$$
 (1)

in which t = a normal standard deviate, k = a Pearson Type III standard deviate, and g = the skew coefficient. Standardizing the variates (transforming to zero mean, unit variance and zero skew) is necessary in order to retain the desired degree of skew in the daily-flow logarithms for each calendar month. Combining random and correlated components would change non-zero skew coefficients by an amount that is difficult to determine mathematically.

To determine the order of the Markov chain required for realistic representation of daily flows recorded at the 7 streamflow stations selected for study, correlation studies were made between the standardized variates for each day in three ways: the first correlated with the preceding day only, the second with two preceding days independently, and the third with three preceding days independently. Results for the seven stations are summarized in table 2, which shows that there is a slight average increase in determination when the second antecedent day is considered, but no increase when the third antecedent day is also considered.

Table 2

Required Order of Markov Chain

Station	Years of	Avg. De	etermination Coef	ficients
No.	Record	lst order	2nd order	3rd order
l	4 6	.9637	•9654	.9656
2	4 6	•7569	.7860	.7860
3	60	.9581	.9598	.9598
4	50	.9322	.9322	.9322
5	52	.9168	.9169	.9163
6	54	.9162	.9178	.9178
7	52	.8106	.8121	.8121

Table 3 shows that the regression coefficient for the second antecedent day is negative, which would be expected from the fact that an upward trend or downward trend would tend to continue from day to day.

Table 3

Average Coefficients for Twelve Months

Station	lst antecedent day	2nd antecedent day	Determination
1	1.145	166	.966
2	•595	.318	.786
3	1.194	219	.960
4	.996	031	.932
5	1.006	050	.917
6	1.089	136	.918
7	1.067	092	.812

Regression coefficient

(The coefficient for station 2 is indicated to be positive, but this is believed to be accidental, in view of the low determination for that station.)

TEST OF CORRELATION VARIATION

There is some question as to whether the frequency and serial correlation characteristics of daily streamflows will differ for the same calendar month in wet and dry years. In order to examine the variation of serial correlation for the same calendar month in different years, serial determination coefficients (squares of correlation coefficients) were computed for each calendar month and for each year of record at the seven stations used in this study. For each calendar month, this serial determination coefficient was correlated with the total runoff for the month, using values determined for that calendar month in each year as a separate observation. Results are summarized in table 4, which shows that there is generally some small correlation, but the correlation is not highly significant. Inasmuch as application of different serial determination coefficients would considerably complicate the model from a mathematical standpoint, and in view of the weak indication of variation with monthly streamflow, the serial determination coefficient is assumed to be independent of the amount of runoff during the month.

VARIATION OF STANDARD DEVIATION

The possible variation of standard deviation with monthly runoff for each calendar month among the various years was tested in the same manner as was the serial correlation discussed in the preceding section. Results are shown in table 4. Here the indication is very consistent and statistically significant. The positive regression coefficients indicate that the standard deviation of logarithms in wet months is greater

than in dry months (for the same calendar months in different years) which is the reverse of what one might expect. However, the results are considered to be realistic, as well as statistically significant. Accordingly, the standard deviation of flow logarithms within each month in the model is computed as a linear function of the total monthly flow. Such linear functions are computed from recorded streamflows as a different relation for each calendar month.

MONTH-END DISCONTINUITIES

Inasmuch as the monthly streamflows are generated before entering the daily streamflow generator, total monthly amounts are pre-determined insofar as the daily streamflow generator is concerned. Daily flows for each month are first generated by generating correlated standardized variates and transforming these to logarithms by use of the following approximate transform equation for the Pearson Type III distribution:

$$k = \frac{2}{g} \left[\frac{g}{6} \left(t - \frac{g}{6} \right) + 1 \right]^3 - \frac{2}{g}$$
(2)

which is simply another form of equation 1. These logarithms are converted to flows, which are added to obtain a monthly total. This sum includes random components and is therefore expected to be somewhat different from the desired total entered from the monthly streamflow model. A ratio of these totals could be applied to each daily streamflow, but this would ordinarily result in discontinuities at the end of each month. In order to avoid this, the random numbers used for that month are preserved in the computer, and new daily values are generated on the basis

of a temporary monthly total obtained by multiplying the desired monthly total by the ratio of the desired total to the generated total for the first pass.

Table 4

	<u>S</u> v	rs <u>M</u> (1)	₹ ² v	s <u>M</u> (2)
Station	<u>C(3)</u>	<u>R</u> 2(4)	_C (3)	$\overline{\mathbb{R}^{2(4)}}$
1	042	.181	346	.029
2	.021	.116	.322	.127
3	.026	.144	.022	.001
4	.000	.162	105	.074
5	.076	.205	043	.043
6	.062	.191	041	.016
7	.065	.052	- .076	.020

Relation of Standard Deviation and Serial Correlation to Average Monthly Streamflow

- (1) Average of 12 monthly correlations of standard deviation of daily flow logarithms to logarithm of monthly flow.
- (2) Average of 12 monthly correlations of serial determination coefficients of daily-flow logarithms with logarithm of monthly flow.
- (3) Linear regression coefficient
- (4) Determination coefficient

The flows resulting from the second-pass generation will very nearly total the desired monthly amount. These can therefore be multiplied by a ratio to obtain the exact monthly total without creating a serious discontinuity at the end of each month.

SUMMARY OF PROCEDURE

Generation of daily streamflows for a location where daily flow records exist and where monthly total flows for the periods of generation are given is accomplished by two elaborate computer programs usable in high-speed computers. The analysis program computes for each of twelve calendar months:

a. The mean, serial correlation coefficient, and skew coefficient of the logarithms of all recorded daily flows for that calendar month, and the correlation coefficient between flow logarithms of each day and the second antecedent day.

b. The linear regression coefficients of the standard deviation of daily-flow logarithms within each month of record to the logarithm of total flow for the month.

The synthesis program accepts the total flow for each month, and proceeds as follows:

a. Generates for each day standardized variates conforming to the serial correlation observed in the recorded data for that calendar month, using the following equation:

$$X_{i+2} = b_1 X_{i+1} + b_2 X_i + (1-\bar{R}^2) X_r$$
(3)

in which X_i , X_{i-1} and X_{i-2} are standardized variates for successive days, X_r is a random standardized variate, b_1 and b_2 are regression coefficients derived from correlation coefficients obtained by the analysis program, and \overline{R}^2 is the determination coefficient for the regression equation.

b. Obtains the logarithm of the monthly mean flow as a first estimate of the mean logarithm of daily flows to be generated.

c. Computes the standard deviation of flow logarithms using the regression equation derived in the analysis program for the calendar month.

d. By means of equation 2, transforms the standardized variates to flows using the above mean and standard deviation and the skew coefficient derived in the analysis program.

e. Adds the difference between the logarithm of the given monthly total flow and the logarithm of the total generated flow for the month to the logarithm of the given monthly total flow and repeats steps a, c and d (second pass).

f. Multiplies each transformed second-pass flow by the ratio of given total monthly flow to total generated monthly flow.

TEST OF OVER-ALL RESULTS

Since the daily streamflow generation model is considered to be needed for generating streamflows during flood periods and of little practical application in generating streamflows during drought periods, over-all tests that have been made to date were restricted to high streamflows. They consist essentially of obtaining the maximum one-day, 3-day and 10-day flows for each year of record and comparing their frequency distributions with corresponding quantities from the generated flows. Results of these comparisons are not complete, but partial results illustrated in table 5 indicate that the generator is producing reasonable results insofar as high flows are concerned.

Table 5

Duration	Event	Recorded	Generated
l-day	Max	15,500	12,200
	Min	890	968
	Median	3,360	2,620
3-day	Max	11,000	11,700
	Min	881	948
	Median	3,250	2,580
10-day	Max	9,180	11,000
	Min	836	930
	Median	2,870	2,370

Comparison of Recorded and Generated Maximum Flows Kern River at Bakersfield, 60 Years

CONCLUSIONS

It is believed that a daily streamflow generator can contribute considerably to water resources studies that are sensitive to variation of high streamflows. Since most water resources studies must be based on long periods of recorded or generated streamflows, it ordinarily is not practical to generate and use daily streamflows for the entire period of study. Accordingly, a daily streamflow generator that will accept monthly streamflows generated by another model is highly desirable. A reservoir study could then be based on monthly streamflows except for those relatively few months when variations of flow within the month are important.

The daily streamflow generator described herein consists of a 2-pass generation by use of a second-order Markov chain applied to standardized variates derived from a logarithmic Pearson Type III distribution. It is considered to give reasonably good results, as shown herein, but will warrant further testing and possible refinement.

ACKNOWLEDGMENT

The daily streamflow generator described herein was developed in the Hydrologic Engineering Center of the Corps of Engineers. Much of the developmental work and practically all of the computer programming was done by William L. Morse. Denver L. Mills made some of the test studies required for verification of the model.

REFERENCES

- 1. Beard, Leo R., "Use of Interrelated Records to Simulate Streamflow", Journal of the Hydraulics Division, ASCE, September 1965.
- 2. Maass et al, "Design of Water Resource Systems", Harvard University Press, 1962.
- 3. Quimpo, R. G., "Stochastic Model of Daily River Flow Sequences", Hydrology Papers, Colorado State University, 1967.
- 4. Yevdjevich, V. M., "Fluctuations of Wet and Dry Years", Hydrology Papers, Colorado State University, 1964.

Technical Paper Series

- TP-1 Use of Interrelated Records to Simulate Streamflow TP-2 Optimization Techniques for Hydrologic Engineering TP-3 Methods of Determination of Safe Yield and Compensation Water from Storage Reservoirs TP-4 Functional Evaluation of a Water Resources System TP-5 Streamflow Synthesis for Ungaged Rivers TP-6 Simulation of Daily Streamflow TP-7 Pilot Study for Storage Requirements for Low Flow Augmentation TP-8 Worth of Streamflow Data for Project Design - A Pilot Study TP-9 Economic Evaluation of Reservoir System Accomplishments Hydrologic Simulation in Water-Yield Analysis **TP-10 TP-11** Survey of Programs for Water Surface Profiles **TP-12** Hypothetical Flood Computation for a Stream System **TP-13** Maximum Utilization of Scarce Data in Hydrologic Design **TP-14** Techniques for Evaluating Long-Tem Reservoir Yields **TP-15** Hydrostatistics - Principles of Application **TP-16** A Hydrologic Water Resource System Modeling Techniques Hydrologic Engineering Techniques for Regional **TP-17** Water Resources Planning **TP-18** Estimating Monthly Streamflows Within a Region **TP-19** Suspended Sediment Discharge in Streams **TP-20** Computer Determination of Flow Through Bridges TP-21 An Approach to Reservoir Temperature Analysis **TP-22** A Finite Difference Methods of Analyzing Liquid Flow in Variably Saturated Porous Media **TP-23** Uses of Simulation in River Basin Planning **TP-24** Hydroelectric Power Analysis in Reservoir Systems **TP-25** Status of Water Resource System Analysis **TP-26** System Relationships for Panama Canal Water Supply **TP-27** System Analysis of the Panama Canal Water Supply **TP-28** Digital Simulation of an Existing Water Resources System **TP-29** Computer Application in Continuing Education **TP-30** Drought Severity and Water Supply Dependability TP-31 Development of System Operation Rules for an Existing System by Simulation **TP-32** Alternative Approaches to Water Resources System Simulation **TP-33** System Simulation of Integrated Use of Hydroelectric and Thermal Power Generation **TP-34** Optimizing flood Control Allocation for a Multipurpose Reservoir **TP-35** Computer Models for Rainfall-Runoff and River Hydraulic Analysis **TP-36** Evaluation of Drought Effects at Lake Atitlan **TP-37** Downstream Effects of the Levee Overtopping at Wilkes-Barre, PA, During Tropical Storm Agnes **TP-38** Water Quality Evaluation of Aquatic Systems
- TP-39 A Method for Analyzing Effects of Dam Failures in Design Studies
- TP-40 Storm Drainage and Urban Region Flood Control Planning
- TP-41 HEC-5C, A Simulation Model for System Formulation and Evaluation
- TP-42 Optimal Sizing of Urban Flood Control Systems
- TP-43 Hydrologic and Economic Simulation of Flood Control Aspects of Water Resources Systems
- TP-44 Sizing Flood Control Reservoir Systems by System Analysis
- TP-45 Techniques for Real-Time Operation of Flood Control Reservoirs in the Merrimack River Basin
- TP-46 Spatial Data Analysis of Nonstructural Measures
- TP-47 Comprehensive Flood Plain Studies Using Spatial Data Management Techniques
- TP-48 Direct Runoff Hydrograph Parameters Versus Urbanization
- TP-49 Experience of HEC in Disseminating Information on Hydrological Models
- TP-50 Effects of Dam Removal: An Approach to Sedimentation
- TP-51 Design of Flood Control Improvements by Systems Analysis: A Case Study
- TP-52 Potential Use of Digital Computer Ground Water Models
- TP-53 Development of Generalized Free Surface Flow Models Using Finite Element Techniques
- TP-54 Adjustment of Peak Discharge Rates for Urbanization
- TP-55 The Development and Servicing of Spatial Data Management Techniques in the Corps of Engineers
- TP-56 Experiences of the Hydrologic Engineering Center in Maintaining Widely Used Hydrologic and Water Resource Computer Models
- TP-57 Flood Damage Assessments Using Spatial Data Management Techniques
- TP-58 A Model for Evaluating Runoff-Quality in Metropolitan Master Planning
- TP-59 Testing of Several Runoff Models on an Urban Watershed
- TP-60 Operational Simulation of a Reservoir System with Pumped Storage
- TP-61 Technical Factors in Small Hydropower Planning
- TP-62 Flood Hydrograph and Peak Flow Frequency Analysis
- TP-63 HEC Contribution to Reservoir System Operation
- TP-64 Determining Peak-Discharge Frequencies in an Urbanizing Watershed: A Case Study
- TP-65 Feasibility Analysis in Small Hydropower Planning
- TP-66 Reservoir Storage Determination by Computer Simulation of Flood Control and Conservation Systems
- TP-67 Hydrologic Land Use Classification Using LANDSAT
- TP-68 Interactive Nonstructural Flood-Control Planning
- TP-69 Critical Water Surface by Minimum Specific Energy Using the Parabolic Method

TP-70	Corps of Engineers Experience with Automatic Calibration of a Precipitation-Runoff Model
TP-71	Determination of Land Use from Satellite Imagery
	for Input to Hydrologic Models
TP-72	Application of the Finite Element Method to Vertically Stratified Hydrodynamic Flow and Water Quality
TP-73	Flood Mitigation Planning Using HEC-SAM
TP-74	Hydrographs by Single Linear Reservoir Model
TP-75	HEC Activities in Reservoir Analysis
TP-76	Institutional Support of Water Resource Models
TP-77	Investigation of Soil Conservation Service Urban Hydrology Techniques
TP-78	Potential for Increasing the Output of Existing Hydroelectric Plants
TP-79	Potential Energy and Capacity Gains from Flood
11-7)	Control Storage Reallocation at Existing U.S.
	Hydropower Reservoirs
TP-80	Use of Non-Sequential Techniques in the Analysis
11 00	of Power Potential at Storage Projects
TP-81	Data Management Systems of Water Resources
11-01	Planning
TP-82	The New HEC-1 Flood Hydrograph Package
TP-83	River and Reservoir Systems Water Quality
11 00	Modeling Capability
TP-84	Generalized Real-Time Flood Control System
	Model
TP-85	Operation Policy Analysis: Sam Rayburn
	Reservoir
TP-86	Training the Practitioner: The Hydrologic
	Engineering Center Program
TP-87	Documentation Needs for Water Resources Models
TP-88	Reservoir System Regulation for Water Quality Control
TP-89	A Software System to Aid in Making Real-Time
TD 00	Water Control Decisions
TP-90	Calibration, Verification and Application of a Two- Dimensional Flow Model
TP-91	HEC Software Development and Support
TP-91 TP-92	Hydrologic Engineering Center Planning Models
TP-92 TP-93	Flood Routing Through a Flat, Complex Flood
11-75	Plain Using a One-Dimensional Unsteady Flow
TP-94	Computer Program Dredged-Material Disposal Management Model
TP-95	Infiltration and Soil Moisture Redistribution in
11-75	HEC-1
TP-96	The Hydrologic Engineering Center Experience in
11 90	Nonstructural Planning
TP-97	Prediction of the Effects of a Flood Control Project
TP-98	on a Meandering Stream Evolution in Computer Programs Causes Evolution
11-90	in Training Needs: The Hydrologic Engineering
	Center Experience
TP-99	Reservoir System Analysis for Water Quality
TP-100	Probable Maximum Flood Estimation - Eastern
11 100	United States
TP-101	Use of Computer Program HEC-5 for Water Supply
	Analysis
TP-102	Role of Calibration in the Application of HEC-6
TP-103	Engineering and Economic Considerations in Formulating
TP-104	Modeling Water Resources Systems for Water
	Quality
	-

- TP-105 Use of a Two-Dimensional Flow Model to Quantify Aquatic Habitat
- TP-106 Flood-Runoff Forecasting with HEC-1F
- TP-107 Dredged-Material Disposal System Capacity Expansion
- TP-108 Role of Small Computers in Two-Dimensional Flow Modeling
- TP-109 One-Dimensional Model for Mud Flows
- TP-110 Subdivision Froude Number
- TP-111 HEC-5Q: System Water Quality Modeling
- TP-112 New Developments in HEC Programs for Flood Control
- TP-113 Modeling and Managing Water Resource Systems for Water Quality
- TP-114 Accuracy of Computer Water Surface Profiles -Executive Summary
- TP-115 Application of Spatial-Data Management Techniques in Corps Planning
- TP-116 The HEC's Activities in Watershed Modeling
- TP-117 HEC-1 and HEC-2 Applications on the Microcomputer
- TP-118 Real-Time Snow Simulation Model for the Monongahela River Basin
- TP-119 Multi-Purpose, Multi-Reservoir Simulation on a PC
- TP-120 Technology Transfer of Corps' Hydrologic Models
- TP-121 Development, Calibration and Application of Runoff Forecasting Models for the Allegheny River Basin
- TP-122 The Estimation of Rainfall for Flood Forecasting Using Radar and Rain Gage Data
- TP-123 Developing and Managing a Comprehensive Reservoir Analysis Model
- TP-124 Review of U.S. Army corps of Engineering Involvement With Alluvial Fan Flooding Problems
- TP-125 An Integrated Software Package for Flood Damage Analysis
- TP-126 The Value and Depreciation of Existing Facilities: The Case of Reservoirs
- TP-127 Floodplain-Management Plan Enumeration
- TP-128 Two-Dimensional Floodplain Modeling
- TP-129 Status and New Capabilities of Computer Program HEC-6: "Scour and Deposition in Rivers and Reservoirs"
- TP-130 Estimating Sediment Delivery and Yield on Alluvial Fans
- TP-131 Hydrologic Aspects of Flood Warning -Preparedness Programs
- TP-132 Twenty-five Years of Developing, Distributing, and Supporting Hydrologic Engineering Computer Programs
- TP-133 Predicting Deposition Patterns in Small Basins
- TP-134 Annual Extreme Lake Elevations by Total Probability Theorem
- TP-135 A Muskingum-Cunge Channel Flow Routing Method for Drainage Networks
- TP-136 Prescriptive Reservoir System Analysis Model -Missouri River System Application
- TP-137 A Generalized Simulation Model for Reservoir System Analysis
- TP-138 The HEC NexGen Software Development Project
- TP-139 Issues for Applications Developers
- TP-140 HEC-2 Water Surface Profiles Program
- TP-141 HEC Models for Urban Hydrologic Analysis

- TP-142 Systems Analysis Applications at the Hydrologic Engineering Center
- TP-143 Runoff Prediction Uncertainty for Ungauged Agricultural Watersheds
- TP-144 Review of GIS Applications in Hydrologic Modeling
- TP-145 Application of Rainfall-Runoff Simulation for Flood Forecasting
- TP-146 Application of the HEC Prescriptive Reservoir Model in the Columbia River Systems
- TP-147 HEC River Analysis System (HEC-RAS)
- TP-148 HEC-6: Reservoir Sediment Control Applications
- TP-149 The Hydrologic Modeling System (HEC-HMS): Design and Development Issues
- TP-150 The HEC Hydrologic Modeling System
- TP-151 Bridge Hydraulic Analysis with HEC-RAS
- TP-152 Use of Land Surface Erosion Techniques with Stream Channel Sediment Models

- TP-153 Risk-Based Analysis for Corps Flood Project Studies - A Status Report
- TP-154 Modeling Water-Resource Systems for Water Quality Management
- TP-155 Runoff simulation Using Radar Rainfall Data
- TP-156 Status of HEC Next Generation Software Development
- TP-157 Unsteady Flow Model for Forecasting Missouri and Mississippi Rivers
- TP-158 Corps Water Management System (CWMS)
- TP-159 Some History and Hydrology of the Panama Canal
- TP-160 Application of Risk-Based Analysis to Planning Reservoir and Levee Flood Damage Reduction Systems
- TP-161 Corps Water Management System Capabilities and Implementation Status