

Hydrologic Aspects of Flood Warning – Preparedness Programs

August 1990

Approved for Public Release. Distribution Unlimited.

TP-131

F	REPORT DO		Form Approved OMB No. 0704-0188				
The public reporting burden for this collection of information is estimated to average 1 hour per response, including the time for reviewing instructions, searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. Send comments regarding this burden estimate or any other aspect of this collection of information, including suggestions for reducing this burden, to the Department of Defense, Executive Services and Communications Directorate (0704-0188). Respondents should be aware that notwithstanding any other provision of law, no person shall be subject to any penalty for failing to comply with a collection of information if it does not display a currently valid OMB control number.							
1. REPORT DATE (DD- August 1990	ММ-ҮҮҮҮ)	2. REPORT TYPE Technical Paper		3. DATES C	COVERED (From - To)		
4. TITLE AND SUBTITLE Hydrologic Aspects of Flood Warning – Preparedness				a. CONTRACT	NUMBER		
Hydrologic Aspect	s of Flood Warni	ng – Preparedness			. GRANT NUMBER		
			4	5c. PROGRAM ELEMENT NUMBER			
6. AUTHOR(S) Harry W. Dotson,	John C. Peters		:	5d. PROJECT NU	PROJECT NUMBER		
marry w. Dotson,	John C. Teters		4	5e. TASK NUMBER			
			:	5F. WORK UNIT NUMBER			
7. PERFORMING ORG	ANIZATION NAME(S)	AND ADDRESS(ES)		8. PERFOR	MING ORGANIZATION REPORT NUMBER		
US Army Corps of		/112/1221(20(20)		TP-131			
Institute for Water							
Hydrologic Engine 609 Second Street	ering Center (HE	C)					
Davis, CA 95616-	4687						
9. SPONSORING/MON		AME(S) AND ADDRES	S(ES)	10. SPONS	10. SPONSOR/ MONITOR'S ACRONYM(S)		
			- (-)		OR/ MONITOR'S REPORT NUMBER(S)		
12. DISTRIBUTION / A	VAILABILITY STATE	MENT					
Approved for publ	-						
13. SUPPLEMENTARY							
Presented at the As August 1990.	SCE Hydraulics I	Division 1990 Nati	onal Conference of	on Hydraulic I	Engineering, San Diego, CA,		
14. ABSTRACT							
	reat recognition s	ystem is a vital co	mponent of a sour	d flood warni	ng-preparedness program.		
					system are: What warning times can		
					nd on watershed and storm		
characteristics, and reliability is illustr					ff between warning time and warning		
renability is mustr	ated, and methods	s for estimating wa	arning time are dis	cussea.			
15. SUBJECT TERMS flood threat recognition, flood warning time, warning reliability, historical storm events, forecasting ,rainfall-runoff modeling, HEC-1F program							
16. SECURITY CLASS			17. LIMITATION	18. NUMBER	19a. NAME OF RESPONSIBLE PERSON		
a. REPORT	b. ABSTRACT	c. THIS PAGE	OF ABSTRACT	OF PAGES			
U	U	U	UU	16	19b. TELEPHONE NUMBER		
L	I	1	1		Standard Form 298 (Rev. 8/98)		
					Prescribed by ANSI Std. Z39-18		

Hydrologic Aspects of Flood Warning – Preparedness Programs

August 1990

US Army Corps of Engineers Institute for Water Resources Hydrologic Engineering Center 609 Second Street Davis, CA 95616

(530) 756-1104 (530) 756-8250 FAX www.hec.usace.army.mil

TP-131

Papers in this series have resulted from technical activities of the Hydrologic Engineering Center. Versions of some of these have been published in technical journals or in conference proceedings. The purpose of this series is to make the information available for use in the Center's training program and for distribution with the Corps of Engineers.

The findings in this report are not to be construed as an official Department of the Army position unless so designated by other authorized documents.

The contents of this report are not to be used for advertising, publication, or promotional purposes. Citation of trade names does not constitute an official endorsement or approval of the use of such commercial products.

Hydrologic Aspects of Flood Warning - Preparedness Programs

Harry W. Dotson*, M.ASCE and John C. Peters*, M.ASCE

Abstract

A reliable flood-threat recognition system is a vital component of a sound flood warning-preparedness program. Fundamental questions associated with the development of a flood-threat recognition system are: what warning times can be achieved, and how reliable will the warnings be? Answers to these questions depend on watershed and storm characteristics, and the flood-threat recognition method being considered. The tradeoff between warning time and warning reliability is illustrated, and methods for estimating warning time are discussed.

Introduction

Flood warning and preparedness programs involve flood-threat recognition, warning dissemination, emergency response and post-flood recovery. The design and implementation of a sound, cost-effective program and the determination of the scope of the program depend substantially on the supporting hydrologic analyses. An important aspect of the hydrologic analyses is the development of a flood-threat recognition system. The analysis includes the evaluation of flood warning times, warning criteria, and the reliability of the warning.

Warning Time and Reliability

The concept of warning time is illustrated in Figure 1 (FIACWD, 1989). As indicated, maximum potential warning time (T_{wp}) is the time from the first indication of precipitation to the time flooding begins. Use of time (T_{wp}) as the actual warning time (T_w) would be totally unreliable because it would indicate that it floods every time it rains. There must be a flood recognition time (T_r) which is the time required for specific warning criteria to indicate flooding is imminent. The criteria could be that a specific amount of precipitation has occurred or that a stream has reached a specified stage. The longer the flood recognition time, the

^{*}Hydraulic Engineer, US Army Corps of Engineers, Hydrologic Engineering Center, 609 Second Street, Davis, CA 95616. Presented at the ASCE Hydraulics Division 1990 National Conference on Hydraulic Engineering, August 1990, San Diego, CA.

warning time. However, one must be aware of the tradeoffs between warning time and warning reliability.

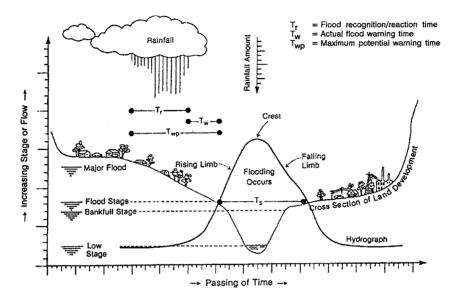


Figure 1. Illustration of Flood Warning Time

Consider Figure 2, which illustrates aspects of reliability. Sets of storm events are labeled $\{A\}, \{B\}, \{C\}$ and $\{D\}$, where:

- $\{A\}$ = storm events that cause flooding
- $\{B\}$ = storm events that do not cause flooding
- $\{C\}$ = storm events that cause flooding but for which warnings are not issued
- {D} = storm events that do not cause flooding but for which warnings are issued

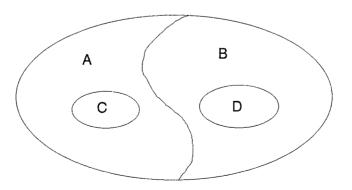


Figure 2. Reliability of Flood Warnings

The goal of a warning system is to minimize both {C} and {D}. Events from {C} can cause damage and loss of life that could possibly be prevented; events from {D} increase the likelihood that future warnings will be ignored. Alternative warning systems will be reflected by different configurations of {C} and {D}. The basis for a warning can range from measured stage at an index gage to results of a rainfall-runoff model that incorporate recent rain data and possibly estimates of future rainfall. Although the more sophisticated warning systems will tend to provide longer lead times, their reliability may not necessarily be greater than that associated with simpler systems. Both warning time and reliability should be evaluated when analyzing alternative warning systems

The tradeoff between lead time (warning time) and warning reliability can be illustrated by considering a simple threshold-stage method of warning, as illustrated in Figure 3. The warning stage is sensed at location A. The primary flood threat is downstream at location B. The problem is to choose a threshold (index) stage for location A such that when that stage is exceeded, a warning for flooding at location B is to be issued. It is desired that the lead time to prepare for the flood threat be as long as possible. The lower the index stage at A, presumably the more lead time will be afforded. However, if the threshold stage is too low, there will be too many false warnings, so that genuine warnings will not be heeded. In terms of Figure 2, as $\{C\}$ is made smaller, $\{D\}$ becomes larger.

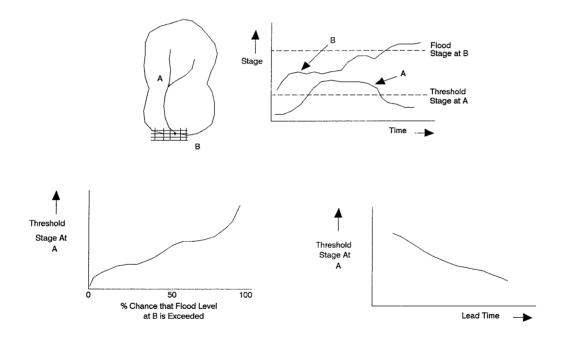




Illustration of Flood Warning and Reliability

To illustrate the tradeoff between warning time and reliability that is implicit in a flood warning system, consider a situation like that in Figure 3 in which a threshold stage at an index gage is to be used to trigger an alarm that warns of the impending exceedance of flood stage at a damage center. Although most flood warning systems are more sophisticated than this, analysis of a simple system can provide insights that have broader implications.

The basin used in this illustration is part of the Central Great Plains Experimental Watershed near Hastings, Nebraska (USDA, undated). In particular, discharge data collected over a 29-year period (1939-1967) at three gages on the west branch of Beaver Creek were used. The locations are labeled W3, W8 and W11 in Figure 4a. The drainage areas at these locations are very small and warning times will be very short. However, the intent of this analysis is to illustrate concepts rather than a practical design, and the available data is well suited to this purpose.

Assume that location W11 is the damage center for which a warning is to be issued, and that flood stage at W11 corresponds to a discharge of 300 cfs. This discharge was exceeded for 16 events during the 29-year period of record. Locations W3 and W8 will be considered individually as index locations for triggering a warning. That is, when a threshold discharge is exceeded at the index location, a warning is issued. The problem is to determine the threshold discharge to be used, and to assess associated warning time and reliability.

Period-of-record discharge data at a 15-minute interval for the three locations were acquired. The data were processed to determine events that exceed the flood discharge (300 cfs) at W11, and to determine threshold discharge exceedances at W3 and W8. Table

Table 1 Warning Time Analysis for a Threshold Q of 200 cfs at W8								
Flood discharge at W11 = 300 cfs.								
Date & Time of Flood at W11	Peak Q at W11	Time of Peak Q W11	Thresh. Q at W8 Exceeded?	Time of Exceed. Thresh. Q	Potential Warning Time (hr:min)			
12 MAY 44 0315	394	0330	yes	0115	2:00			
25 AUG 44 1045	343	1515	yes	1100	-:15			
16 JUL 45 2045	333	2100	yes	1745	3:00			
9 JUN 49 0030	374	0145	yes	2045 ²	3:45			
20 SEP 50 0115	730	0300	yes	2230	2:45			
1 JUL 51 2045	1147	2215	yes	1930	1:15			
10 JUL 51 0815	918	0900	yes	0630	1:45			
14 JUL 52 0400	1063	0430	yes	0115	2:45			
7 JUN 53 1815	680	2000	yes	1745	:30			
22 MAY 54 2315	999	0200 1	yes	2300	:15			
27 MAY 54 0330	325	0345	yes	2345	3:45			
15 JUN 57 1730	1459	2115	yes	1215	5:15			
29 AUG 57 0045	414	0130	yes	0130	-:45			
3 JUL 59 2130	838	2400	yes	2115	:15			
27 MAR 60 1645	365	1745	yes	1315	3:30			
15 MAY 60 2230	811	0115 1	yes	2230	:00			

¹ Next day.

² Previous day

16 flood events in 29 years

Number of events threshold discharge (200 cfs) was exceeded: 45 Reliability = $16/45 \times 100 = 36\%$

1 illustrates results for a threshold dischage of 200 cfs at W8. The first three columns pertain to the flood event at W11; the last three columns refer to the exceedance of the thresholddischarge at W8. In this illustration, the threshold discharge was exceeded during all 16 flood events. The potential warning time associated with the events is shown in the last column. For two of the events, the time is negative.

As noted at the bottom of Table 1, the threshold discharge was exceeded 45 times during the 29 years of record, which means that a false warning would have been generated 29 times. The realiability of the warning mechanism, that is, the percent of true warnings to total warnings, is $16/45 \times 100$, or 36 percent. As may be noted from the table, a warning time ≥ 1 hour would have been provided for 10 of the 16, or 63 percent of the flood events. A warning time ≥ 30 minutes would have been provided for 69 percent of the events. The analysis illustrated in Table 1 was also applied with threshold discharges

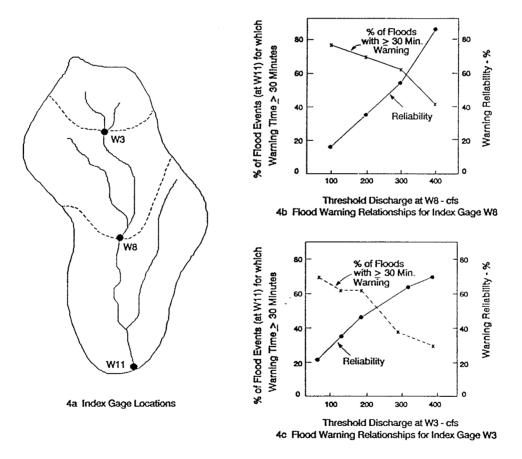


Figure 4. Beaver Creek Watershed

at W8 of 100, 300 and 400 cfs. Figure 4b shows forecast reliability and occurrence of at least a 30-minute warning time, both as a function of threshold discharge at W8. Figure 4c shows results for W3.

The inverse relationship between warning reliability and warning occurrence is readily apparent in Figures 4b and 4c. Suppose that it were desired to have a warning reliability of 70 percent, meaning that 7 out of 10 warnings would be for actual flood events. From Figure 4b, the corresponding threshold discharge at W8 is about 350 cfs and the percent

of flood events for which a warning time \geq 30 minutes is provided is 53 percent. That is, a warning time \geq 30 minutes would be provided for only about half the flood events, and 3 out of 10 warnings would be erroneous. These are not very impressive figures, and such a warning system would obviously be far less than adequate.

By comparison, Figure 4c indicates that a 70 percent reliability could be achieved with a threshold discharge of 400 cfs at W3, for which a warning time \geq 30 minutes would be provided for only 31 percent of the flood events. For this level of reliability, index location W8 is the better of the two locations.

Estimation of Flood Warning Time

Flood-threat recognition essentially involves real-time sampling of characteristics of a storm event and forecasting the probable near-term runoff response. The more variability associated with the event being sampled, the more difficulty there is in obtaining an adequate sample and the more uncertain the forecast.

Key variables upon which warning time depends include: (1) spatial variability of precipitation, (2) temporal variability of precipitation, (3) rainfall-runoff response characteristics of the watershed and (4) antecedent soil moisture conditions. Storm rainfall, and consequently warning time, typically exhibit substantial variability. To properly evaluate the potential warning time for a watershed, a set of storms should be analyzed that reflects such variability. Warning time can then be defined in terms of a median value and a standard deviation or some other measure of variability.

Warning time for a specific historical storm event can be estimated using a rainfall-runoff forecast model such as HEC-IF (Peters, 1985). The model accounts for precipitation and streamflow that has occurred up to the specified time-of-forecast and simulates streamflow into the future. Successive times-of-forecast can be evaluated until the simulated future runoff exceeds flood stage. The time between the time-of-forecast and the time when flooding begins represents an estimate of the gross warning time for the event being analyzed. An estimated time for collecting and analyzing real-time data during an actual storm would need to be estimated and subtracted from the gross warning time. If climatological forecasts had indicated a significant probability of future rainfall, such rainfall could be incorporated in the forecast and a longer warning time achieved. However, quantitative estimates of future precipitation are notoriously uncertain.

Ideally the analysis as described would be made for a number of historical events, and the median value and variability of warning determined. If there were no historical precipitation data for the basin, it would be reasonable to transpose rainfall information from within a hydrometeorologically homogeneous region. If no concurrent precipitation and streamflow data were available for a basin, there would, of course, be additional uncertainty associated with lack of data with which to calibrate the rainfall-runoff model.

References

- 1. USDA, Agricultural Research Service, The Central Great Plains Experimental Watershed, A Summary Report of 30 Years of Hydrologic Research, (undated report).
- 2. Federal Interagency Advisory Committee on Water Data (FIACWD), Hydrology Subcommittee, Guidelines on Community Local Flood Warning and Response Systems, 1985.
- 3. Peters, J., and P. Ely, Flood-Runoff Forecasting with HEC1F, Water Resources Bulletin, 21 (1), 1985.

Technical Paper Series

- TP-1 Use of Interrelated Records to Simulate Streamflow TP-2 Optimization Techniques for Hydrologic Engineering TP-3 Methods of Determination of Safe Yield and Compensation Water from Storage Reservoirs TP-4 Functional Evaluation of a Water Resources System TP-5 Streamflow Synthesis for Ungaged Rivers TP-6 Simulation of Daily Streamflow TP-7 Pilot Study for Storage Requirements for Low Flow Augmentation TP-8 Worth of Streamflow Data for Project Design - A Pilot Study TP-9 Economic Evaluation of Reservoir System Accomplishments Hydrologic Simulation in Water-Yield Analysis **TP-10 TP-11** Survey of Programs for Water Surface Profiles **TP-12** Hypothetical Flood Computation for a Stream System **TP-13** Maximum Utilization of Scarce Data in Hydrologic Design **TP-14** Techniques for Evaluating Long-Tem Reservoir Yields **TP-15** Hydrostatistics - Principles of Application **TP-16** A Hydrologic Water Resource System Modeling Techniques Hydrologic Engineering Techniques for Regional **TP-17** Water Resources Planning **TP-18** Estimating Monthly Streamflows Within a Region **TP-19** Suspended Sediment Discharge in Streams **TP-20** Computer Determination of Flow Through Bridges TP-21 An Approach to Reservoir Temperature Analysis **TP-22** A Finite Difference Methods of Analyzing Liquid Flow in Variably Saturated Porous Media **TP-23** Uses of Simulation in River Basin Planning **TP-24** Hydroelectric Power Analysis in Reservoir Systems **TP-25** Status of Water Resource System Analysis **TP-26** System Relationships for Panama Canal Water Supply **TP-27** System Analysis of the Panama Canal Water Supply **TP-28** Digital Simulation of an Existing Water Resources System **TP-29** Computer Application in Continuing Education **TP-30** Drought Severity and Water Supply Dependability TP-31 Development of System Operation Rules for an Existing System by Simulation **TP-32** Alternative Approaches to Water Resources System Simulation **TP-33** System Simulation of Integrated Use of Hydroelectric and Thermal Power Generation **TP-34** Optimizing flood Control Allocation for a Multipurpose Reservoir **TP-35** Computer Models for Rainfall-Runoff and River Hydraulic Analysis **TP-36** Evaluation of Drought Effects at Lake Atitlan **TP-37** Downstream Effects of the Levee Overtopping at Wilkes-Barre, PA, During Tropical Storm Agnes **TP-38** Water Quality Evaluation of Aquatic Systems
- TP-39 A Method for Analyzing Effects of Dam Failures in Design Studies
- TP-40 Storm Drainage and Urban Region Flood Control Planning
- TP-41 HEC-5C, A Simulation Model for System Formulation and Evaluation
- TP-42 Optimal Sizing of Urban Flood Control Systems
- TP-43 Hydrologic and Economic Simulation of Flood Control Aspects of Water Resources Systems
- TP-44 Sizing Flood Control Reservoir Systems by System Analysis
- TP-45 Techniques for Real-Time Operation of Flood Control Reservoirs in the Merrimack River Basin
- TP-46 Spatial Data Analysis of Nonstructural Measures
- TP-47 Comprehensive Flood Plain Studies Using Spatial Data Management Techniques
- TP-48 Direct Runoff Hydrograph Parameters Versus Urbanization
- TP-49 Experience of HEC in Disseminating Information on Hydrological Models
- TP-50 Effects of Dam Removal: An Approach to Sedimentation
- TP-51 Design of Flood Control Improvements by Systems Analysis: A Case Study
- TP-52 Potential Use of Digital Computer Ground Water Models
- TP-53 Development of Generalized Free Surface Flow Models Using Finite Element Techniques
- TP-54 Adjustment of Peak Discharge Rates for Urbanization
- TP-55 The Development and Servicing of Spatial Data Management Techniques in the Corps of Engineers
- TP-56 Experiences of the Hydrologic Engineering Center in Maintaining Widely Used Hydrologic and Water Resource Computer Models
- TP-57 Flood Damage Assessments Using Spatial Data Management Techniques
- TP-58 A Model for Evaluating Runoff-Quality in Metropolitan Master Planning
- TP-59 Testing of Several Runoff Models on an Urban Watershed
- TP-60 Operational Simulation of a Reservoir System with Pumped Storage
- TP-61 Technical Factors in Small Hydropower Planning
- TP-62 Flood Hydrograph and Peak Flow Frequency Analysis
- TP-63 HEC Contribution to Reservoir System Operation
- TP-64 Determining Peak-Discharge Frequencies in an Urbanizing Watershed: A Case Study
- TP-65 Feasibility Analysis in Small Hydropower Planning
- TP-66 Reservoir Storage Determination by Computer Simulation of Flood Control and Conservation Systems
- TP-67 Hydrologic Land Use Classification Using LANDSAT
- TP-68 Interactive Nonstructural Flood-Control Planning
- TP-69 Critical Water Surface by Minimum Specific Energy Using the Parabolic Method

TP-70	Corps of Engineers Experience with Automatic Calibration of a Precipitation-Runoff Model				
TP-71	Determination of Land Use from Satellite Imagery				
	for Input to Hydrologic Models				
TP-72	Application of the Finite Element Method to Vertically Stratified Hydrodynamic Flow and Water Quality				
TP-73	Flood Mitigation Planning Using HEC-SAM				
TP-74	Hydrographs by Single Linear Reservoir Model				
TP-75	HEC Activities in Reservoir Analysis				
TP-76	Institutional Support of Water Resource Models				
TP-77	Investigation of Soil Conservation Service Urban Hydrology Techniques				
TP-78	Potential for Increasing the Output of Existing Hydroelectric Plants				
TP-79	Potential Energy and Capacity Gains from Flood				
11-7)	Control Storage Reallocation at Existing U.S.				
	Hydropower Reservoirs				
TP-80	Use of Non-Sequential Techniques in the Analysis				
11 00	of Power Potential at Storage Projects				
TP-81	Data Management Systems of Water Resources				
11-01	Planning				
TP-82	The New HEC-1 Flood Hydrograph Package				
TP-83	River and Reservoir Systems Water Quality				
11 00	Modeling Capability				
TP-84	Generalized Real-Time Flood Control System				
	Model				
TP-85	Operation Policy Analysis: Sam Rayburn				
	Reservoir				
TP-86	Training the Practitioner: The Hydrologic				
	Engineering Center Program				
TP-87	Documentation Needs for Water Resources Models				
TP-88	Reservoir System Regulation for Water Quality Control				
TP-89	A Software System to Aid in Making Real-Time				
TD 00	Water Control Decisions				
TP-90	Calibration, Verification and Application of a Two- Dimensional Flow Model				
TP-91	HEC Software Development and Support				
TP-91 TP-92	Hydrologic Engineering Center Planning Models				
TP-92 TP-93	Flood Routing Through a Flat, Complex Flood				
11-75	Plain Using a One-Dimensional Unsteady Flow				
TP-94	Computer Program Dredged-Material Disposal Management Model				
TP-95	Infiltration and Soil Moisture Redistribution in				
11-75	HEC-1				
TP-96	The Hydrologic Engineering Center Experience in				
11 90	Nonstructural Planning				
TP-97	Prediction of the Effects of a Flood Control Project on a Meandering Stream				
TP-98	Evolution in Computer Programs Causes Evolution				
11-90	in Training Needs: The Hydrologic Engineering				
	Center Experience				
TP-99	Reservoir System Analysis for Water Quality				
TP-100	Probable Maximum Flood Estimation - Eastern				
11 100	United States				
TP-101	Use of Computer Program HEC-5 for Water Supply Analysis				
TP-102	Role of Calibration in the Application of HEC-6				
TP-102	Engineering and Economic Considerations in				
100	Formulating				
TP-104	Modeling Water Resources Systems for Water				
	Quality				

- TP-105 Use of a Two-Dimensional Flow Model to Quantify Aquatic Habitat
- TP-106 Flood-Runoff Forecasting with HEC-1F
- TP-107 Dredged-Material Disposal System Capacity Expansion
- TP-108 Role of Small Computers in Two-Dimensional Flow Modeling
- TP-109 One-Dimensional Model for Mud Flows
- TP-110 Subdivision Froude Number
- TP-111 HEC-5Q: System Water Quality Modeling
- TP-112 New Developments in HEC Programs for Flood Control
- TP-113 Modeling and Managing Water Resource Systems for Water Quality
- TP-114 Accuracy of Computer Water Surface Profiles -Executive Summary
- TP-115 Application of Spatial-Data Management Techniques in Corps Planning
- TP-116 The HEC's Activities in Watershed Modeling
- TP-117 HEC-1 and HEC-2 Applications on the Microcomputer
- TP-118 Real-Time Snow Simulation Model for the Monongahela River Basin
- TP-119 Multi-Purpose, Multi-Reservoir Simulation on a PC
- TP-120 Technology Transfer of Corps' Hydrologic Models
- TP-121 Development, Calibration and Application of Runoff Forecasting Models for the Allegheny River Basin
- TP-122 The Estimation of Rainfall for Flood Forecasting Using Radar and Rain Gage Data
- TP-123 Developing and Managing a Comprehensive Reservoir Analysis Model
- TP-124 Review of U.S. Army corps of Engineering Involvement With Alluvial Fan Flooding Problems
- TP-125 An Integrated Software Package for Flood Damage Analysis
- TP-126 The Value and Depreciation of Existing Facilities: The Case of Reservoirs
- TP-127 Floodplain-Management Plan Enumeration
- TP-128 Two-Dimensional Floodplain Modeling
- TP-129 Status and New Capabilities of Computer Program HEC-6: "Scour and Deposition in Rivers and Reservoirs"
- TP-130 Estimating Sediment Delivery and Yield on Alluvial Fans
- TP-131 Hydrologic Aspects of Flood Warning -Preparedness Programs
- TP-132 Twenty-five Years of Developing, Distributing, and Supporting Hydrologic Engineering Computer Programs
- TP-133 Predicting Deposition Patterns in Small Basins
- TP-134 Annual Extreme Lake Elevations by Total Probability Theorem
- TP-135 A Muskingum-Cunge Channel Flow Routing Method for Drainage Networks
- TP-136 Prescriptive Reservoir System Analysis Model -Missouri River System Application
- TP-137 A Generalized Simulation Model for Reservoir System Analysis
- TP-138 The HEC NexGen Software Development Project
- TP-139 Issues for Applications Developers
- TP-140 HEC-2 Water Surface Profiles Program
- TP-141 HEC Models for Urban Hydrologic Analysis

- TP-142 Systems Analysis Applications at the Hydrologic Engineering Center
- TP-143 Runoff Prediction Uncertainty for Ungauged Agricultural Watersheds
- TP-144 Review of GIS Applications in Hydrologic Modeling
- TP-145 Application of Rainfall-Runoff Simulation for Flood Forecasting
- TP-146 Application of the HEC Prescriptive Reservoir Model in the Columbia River Systems
- TP-147 HEC River Analysis System (HEC-RAS)
- TP-148 HEC-6: Reservoir Sediment Control Applications
- TP-149 The Hydrologic Modeling System (HEC-HMS): Design and Development Issues
- TP-150 The HEC Hydrologic Modeling System
- TP-151 Bridge Hydraulic Analysis with HEC-RAS
- TP-152 Use of Land Surface Erosion Techniques with Stream Channel Sediment Models

- TP-153 Risk-Based Analysis for Corps Flood Project Studies - A Status Report
- TP-154 Modeling Water-Resource Systems for Water Quality Management
- TP-155 Runoff simulation Using Radar Rainfall Data
- TP-156 Status of HEC Next Generation Software Development
- TP-157 Unsteady Flow Model for Forecasting Missouri and Mississippi Rivers
- TP-158 Corps Water Management System (CWMS)
- TP-159 Some History and Hydrology of the Panama Canal
- TP-160 Application of Risk-Based Analysis to Planning Reservoir and Levee Flood Damage Reduction Systems
- TP-161 Corps Water Management System Capabilities and Implementation Status