

(A000 6869)

19 Feb 75

To: Structural Design Section
Design Personnel

Attached is an explanation of the effective width of skinplate to be used in the design of vertical rib members for gates. This explanation was presented by Don Dressler, LMVD.

What can be concluded from this explanation is that we can use an effective flange width of 42 times the skinplate thickness^(for $F_y = 36 \text{ ksi}$) in the design of vertical ribs with a continuous skin plate acting as the flange. We have been using

an effective flange width of $32 \times$ the skinplate thickness for our gate designs.

The $42t$ requirement is derived from the expression $1.5 \sqrt{\frac{E}{F_y}} t$ presented in EM 1110-2-2702. $32t$ was derived from the requirements of AISC Section 1.9.1. The EM expression is for members with continuous flanges (skinplates) whereas the AISC requirement has application for flanges with free ends. (WFlanges, Plate Girders)

JJ Beghin

References:

1. Structural Steel Design, by L.S. Beedle, et. al, Ronald Press, 1964, pp. 208, 534-538.
2. Steel Structures: Design & Behavior by C. G. Salmon & J. E. Johnson, Intext, 1972, pp. 300-303, 314, 324-328.
3. Manual Of Steel Construction by AISC, 7th Edition, 1970, pp. 5-17, 5-18, 5-124, 5-125, 5-25, 5-26, 5-140, 5-141, 5-142, 5-115, 5-116, 5-117, 5-60, 5-61, 5-161, 5-165.

ie W shapes etc

Local Buckling Provisions For Flanges:

a) Attaining The Yield Moment (M_y):

$\lambda = 0.7 = \sqrt{\frac{F_y}{F_{cr}}}$ (eqn 17.21) $\frac{b}{t} \leq \frac{3600 \sqrt{k'}}{\sqrt{F_y}}$ (eqn 17.11)

$k = 0.7$ (para 17.4, 2.2) $\frac{b}{t} \leq \frac{3012}{\sqrt{F_y}}$ (elastic case)

F_y	33,000	36,000	47,000	50,000
b/t	16.6	15.9	14.7	13.5

b) Attaining The Plastic Moment (M_r):

$\lambda = 0.46 = \sqrt{\frac{F_y}{F_{cr}}}$ (Fig 17.12) $F_{cr} = k \frac{\pi^2 E_t}{12(1-\mu^2)} \left(\frac{t}{b}\right)^2$ (eqn 17.11)

so: $\frac{b}{t} \leq \frac{\pi}{\sqrt{F_{cr}}} \sqrt{\frac{k E_t}{12(1-\mu^2)}} = 0.46 \pi \sqrt{\frac{29,000,000}{12 \times 0.91}} \sqrt{\frac{k}{F_y}}$

$\frac{b}{t} \leq \frac{2354 \sqrt{k}}{\sqrt{F_y}}$

$k = 0.425$ (p. 538)

$\frac{b}{t} \leq \frac{1535}{\sqrt{F_y}}$ (plastic case)

F_y	33,000	36,000	42,000	50,000
b/t	8.4	8.1	7.5	6.9

0.5 $\frac{0.5}{\sqrt{F_y}}$ (per AISC)
 $\frac{0.43}{\sqrt{F_y}}$
 $\frac{0.46 \pi \sqrt{\frac{29,000,000}{12 \times 0.91}}}{\sqrt{F_y}} \approx \frac{42.6}{\sqrt{F_y}}$
 $\frac{b}{t} = \frac{42.6}{\sqrt{F_y}}$ (per AISC)

ref: ref 2, p. 303

ie. continuous flange

Local Buckling Provisions For Skinplates

a) Attaining The Yield Moment (M_y):

$$\lambda = 0.7 = \sqrt{\frac{F_y}{F_{cr}}} \quad \frac{b}{t} \leq \frac{3600 \sqrt{k}}{\sqrt{F_y}}$$

(eqns ref 1 & 2, 17.21, 17.22)

$k = 5.0$ (para 17.4.2.4)

$$\frac{b}{t} \leq \frac{8050}{\sqrt{F_y}}$$

or: $\frac{b}{t} \leq \frac{7\pi}{\sqrt{F_y}} \sqrt{\frac{E}{12(1-\mu^2)}} = \frac{7\pi}{\sqrt{F_y}} \sqrt{\frac{5 \times 27,000}{12 \times .91}} = \frac{253.4}{\sqrt{F_y}}$

$$\frac{b}{t} \leq \frac{253.4}{\sqrt{F_y}}$$

or: $\frac{b}{t} \leq \lambda \pi \sqrt{\frac{k}{12(1-\mu^2)}} \sqrt{\frac{E}{F_y}} = 7\pi \sqrt{\frac{5}{12 \times .91}} \sqrt{\frac{E}{F_y}}$

$$\frac{b}{t} \leq 1.47 \sqrt{\frac{E}{F_y}}$$

and $F_b \leq 0.5 F_y$
per EM 1110-2-2702

b) Attaining The Plastic Moment (M_p):

$$\lambda = 0.58 = \sqrt{\frac{F_y}{F_{cr}}} \quad F_{cr} = k \frac{\pi^2 E t}{12(1-\mu^2)} \left(\frac{t}{b}\right)^2$$

(Fig 17.12, eqn 17.12) ref 1 & 2

$k = 1.0$ (p. 538)

$$\frac{b}{t} = \frac{\lambda \pi}{\sqrt{F_y}} \sqrt{\frac{k E t}{12(1-\mu^2)}}$$

$\frac{b}{t} \leq \frac{5.9\pi}{\sqrt{F_y}} \sqrt{\frac{1 \times 27,000,000}{12(.91)}} = \frac{5938}{\sqrt{F_y}}$

$$\frac{b}{t} \leq \frac{5938}{\sqrt{F_y}}$$

$\frac{b}{t} \leq \frac{5.0\pi}{\sqrt{F_y}} \sqrt{\frac{1 \times 27,000}{12 \times .91}} = \frac{187.6}{\sqrt{F_y}}$

$$\frac{b}{t} \leq \frac{188}{\sqrt{F_y}}$$

$\frac{b}{t} \leq .58\pi \sqrt{\frac{4}{12 \times .91}} \sqrt{\frac{E}{F_y}} = 1.10 \sqrt{\frac{E}{F_y}}$

$$\frac{b}{t} \leq 1.1 \sqrt{\frac{E}{F_y}}$$

and $F_b \leq 0.55 F_y$

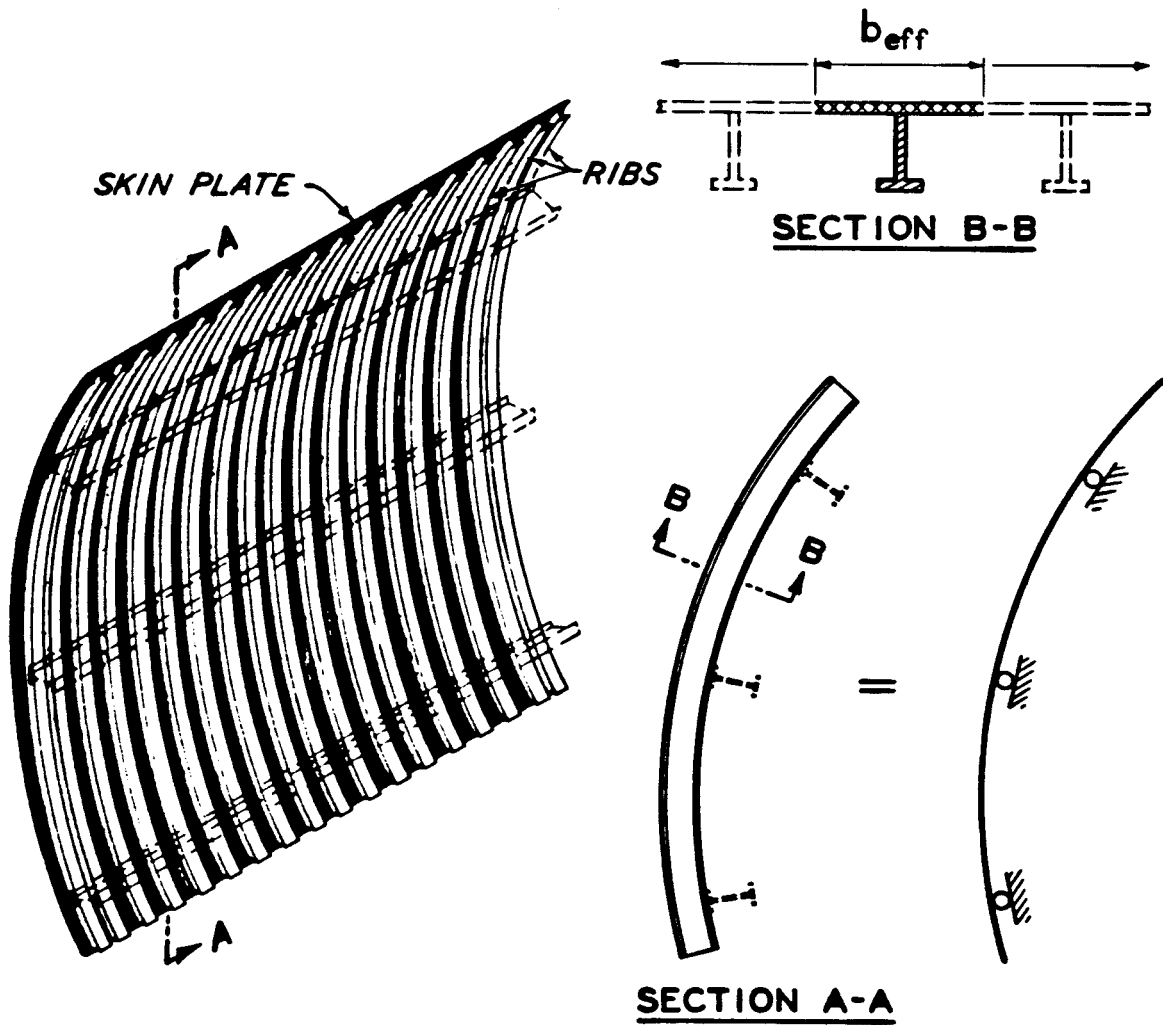
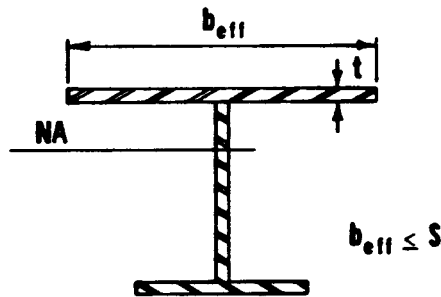
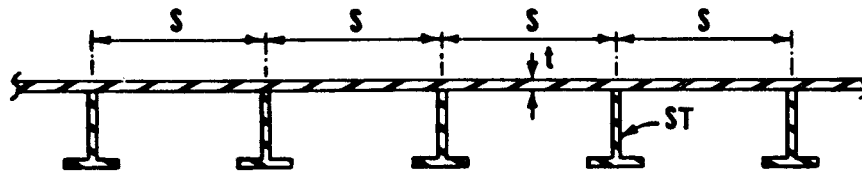


Figure B-24. Composite rib action



$$b_{eff} \leq 1.5 t \sqrt{\frac{E}{F_y}} \quad (\text{FOR } f \leq 0.5 F_y)$$

$$b_{eff} \leq 1.1 t \sqrt{\frac{E}{F_y}} \quad (\text{FOR } f = 0.55 F_y)$$

Figure B-25. EM 1110-2-2702 criterion for effective skin plate width

THE BASIC EQUATION FOR PLATE BUCKLING STRESS IS:

$$F_{cr} = k \frac{\pi^2 E_t}{12(1 - \mu^2)} \left(\frac{t}{b}\right)^2$$

PER STRUCTURAL STEEL DESIGN BY BEEDLE ET AL., P. 525

TO ENSURE THAT YIELDING WILL OCCUR PRIOR TO BUCKLING
THE FOLLOWING SAFETY FACTOR (F_s) IS REQUIRED

$$F_s = \frac{F_{cr}}{F_y} \text{ OR } \lambda^2 = \frac{1}{F_s} = \frac{F_y}{F_{cr}}$$

SO:

$$\lambda = \sqrt{\frac{F_y}{F_{cr}}} = \frac{b}{t} \sqrt{\frac{F_y}{E_t} \cdot \frac{12(1 - \mu^2)}{k\pi^2}}$$

USING:

μ = POISSON'S RATIO = 0.3

k = A PLATE BUCKLING COEFFICIENT DEPENDENT
ON THE TYPE OF SUPPORTS

$E_t = E$ = MODULUS OF ELASTICITY

Figure B-26. Basic equation for stability
of thin plates

ACCORDING TO EXPERIMENTAL RESULTS BY BEEDLE:

FOR W SHAPES



CANTILEVER FLANGES

$$\frac{b}{t} \leq \lambda \pi \sqrt{\frac{k}{12(1 - \mu^2)} \cdot \frac{E}{F_y}}$$

FOR $f_y < 0.6 F_y$ (PER AISC)

$\lambda = 0.7$ (BEEDLE, P. 533)

$k = 0.7$ (BEEDLE, P. 534)

$$\frac{b}{t} = 0.7 \pi \sqrt{\frac{0.7 \times 29,000}{12(1 - 0.3^2)} \cdot \frac{E}{F_y}}$$

$$\frac{b}{t} = \frac{95}{F_y}$$

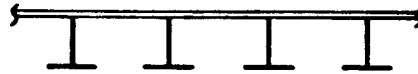
FOR $f_y = 0.66 F_y$ (PER AISC)

$\lambda = 0.46$ (BEEDLE, P. 538)

$k = 0.425$ (BEEDLE, P. 526)

$$\frac{b}{t} = \frac{52.2}{F_y}$$

FOR SKINPLATES



CONTINUOUS UPSTREAM FLANGE

$$\frac{b}{t} < \lambda \pi \sqrt{\frac{k}{12(1 - \mu^2)} \cdot \frac{E}{F_y}}$$

FOR $M < M_p$ OR $f_y < 0.5 F_y$:

$\lambda = 0.7$ (BEEDLE, P. 533)

$k = 5.0$ (BEEDLE, P. 534-6)

$$\frac{b}{t} = 0.7 \pi \sqrt{\frac{5}{12(0.91)} \cdot \frac{E}{F_y}}$$

$$\frac{b}{t} = 1.5 \sqrt{\frac{E}{F_y}}$$

FOR $M < M_p$ OR $f_y < 0.55 F_y$

$\lambda = 0.58$ (BEEDLE, P. 538)

$k = 4.0$ (BEEDLE, P. 538)

$$\frac{b}{t} = 1.1 \sqrt{\frac{E}{F_y}}$$

Figure B-27. Comparison of AISC and EM width-thickness ratios