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Performance Evaluation of New Orleans and Southeast Louisiana Hurricane Protection System

Floodwall and Levee Performance Analysis



Interagency Performance Evaluation Task Force (IPET)











Floodwall and Levee Performance **US Army Corps** Analysis of Engineers

The Performance: How did the floodwalls and levees, individually and acting as an integrated system, perform in response to Hurricane Katrina, and why?

Objective

- Analyze the levees and floodwalls performance during Hurricane Katrina
- Investigate the most likely causes of the damage and failure of \bullet the levees and floodwalls in the system
- Compare them with similar sections or reaches where the ۲ performance was satisfactory
- Understand mechanisms that led to the breaches along a reaches in order evaluate the potential performance of the similar unbreached reaches of the protective system



- Primary Factors Leading to the 17th Street Canal Breach:
 - Development of a gap between the wall and the levee fill on the canal side of the wall
 - Variation in foundation clay shear strength from levee crest to landside toe
- Except for the outfall canals, all other damage to the floodwalls and levees has been due to overtopping



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Cross-Sections & Soil Profiles for Use in Analysis





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LEGEND

Point bar

Inland swamp

Spoil

Top of Pleistocene-ft msl

 $\wedge \wedge$

-40

New Orleans Area





US Army Corps Spanish Fort: X-Section C-C'







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Pine Is Beach Ridge





US Army Corps of Engineers 17th Street Canal Breach





EAST

17th Street Canal Breach

NEST

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17th Street Canal Breach

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US Army Corps 17 Street Canal Swamp of Engineers









17th Street Slide Block

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17th Street Slide Block

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17th Street Canal I-wall Soil Strength and Stability

17th Street Canal C/L Failure Section

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1992

1.011





Station 10+00

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Undrained Shear Strength (psf)



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IPET shear strength model





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Comparison of IPET shear strength model with design shear strengths





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Clay Strengths in Breach and Adjacent Areas

- Data are sparse and scattered
- Based on five UC and one UU-1 tests from two borings in the breach area, <u>the average s_u is</u> <u>260 psf</u>
- Based on three UC, three UU, and one UU-1 tests from two borings north of the breach area, <u>the average s_u is 335 psf (30%)</u>
- Based on nine UC, two UU, and one UU-1 tests from three borings south of the breach, <u>s_u 318</u> psf (20%)



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DISTANCE IN FEET



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17th Street Canal Hydrograph

Lake Pontchartrain Canal Hydrographs



Date and Time, CDT









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- W. L. = 11.3 ft, with crack, F = 1.00
- W. L. was 7.5 ft to 9.5 ft, plus wave effects, at time of failure
- Wave effects may be + 1.0 ft
- W. L. for F = 1.0 is one to two feet higher than estimated effective water level at time of failure



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W. L. = 11.5 NGVD No crack Method of planes F = 1.30





Design Cross Section for Breach a Area

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W. L. = 11.5 NGVD No crack Spencer's method F = 1.45





Design cross section and strength

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W. L. = 13.6 NGVD, with crack for F = 1.00 using Spencer's method





Probabilities of failure

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Probabilities of failure (in yellow)				
			COV _F	
Area	WL	F_{MLV}	15%	30%
Breach	11.5 ft	0.99	56%	57%
Adjacent	11.5 ft	1.15	19%	37%



- The peat is not the weak link
- The peat is stronger than the clay beneath the peat

• The strength of the clay increases markedly with depth



- Strengths are lower beneath levee slope and beyond toe than beneath crest
- GDM 20 strengths were the same beneath the levee crest, slope and beyond the toe
- Strengths are about 20% higher to the south of the breach and 30% higher to the north
- Factor of safety are about 15% higher for adjacent areas than for the breach area



 Factors of safety are about 25% lower for the cracked condition than for uncracked condition

 Development of a crack on the canal side of the wall is an important factor in the mechanism of failure



- Water level = 11.3 ft required for F = 1.00
- These water levels are higher than the eyewitness water level at time of failure
- Differences may be due to:
 - Wave effects
 - IPET shear strengths higher than actual
 - Circular slip surfaces give factors of safety that are higher by about 3%, and water levels for F = 1.0 that are about 1.2 ft higher than noncircular surfaces



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System-Wide Assessment

Impacted Area



Hri

US Army Corps of Engineers, New Orleans District

Local Authorities

- Louisiana DOTD
- Port of New Orleans
- Lake Borgne Basin Levee District
- N.O. Sewerage and Water Board
- Orleans Levee District
- Plaquemines Parish Government
- St. Bernard Parish Government

Hurricane Protection System

- 284 miles: Federal levees/floodwalls
- 71 pump stations

Damage

- 169 miles: Federal levees/floodwalls
- 34 pump stations

Damaged No Significant Damage Non-Federal Levee



Assessment of Entire System

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Selection For Detailed Analysis

- Walls that failed (category WF)
- Walls that were close to failure, indicated by permanent deflection (WCF)
- Walls that are stable, with no permanent deflection (WS)
- Levees that overtopped and breached (LOB)
- Levees that overtopped and did not breach (LONB)
- Levee under seepage locations (LU)
- Failures at transitions between different types of flood protection structures (TF)



New Orleans East Basin

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Erosion Assessment

- Pre-Katrina and post-Katrina LIDAR surveys
 - Determine depth and surface area of erosion
 - Categorize the severity of the erosion
- Storm surge height and duration
- Wave height and duration
- Levee surface soil type
- Elevation of the levee crest





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Remaining Effort

- Additional CPTU, Vane Shear, DSS
- Soil-Structure Interaction Analysis





Remaining Effort

- London Avenue Canal
- Orleans Canal
- Inner Harbor Navigation Canal
- St. Bernard Parish
 - Mississippi River Gulf Outlet
- Plaquemines Parish