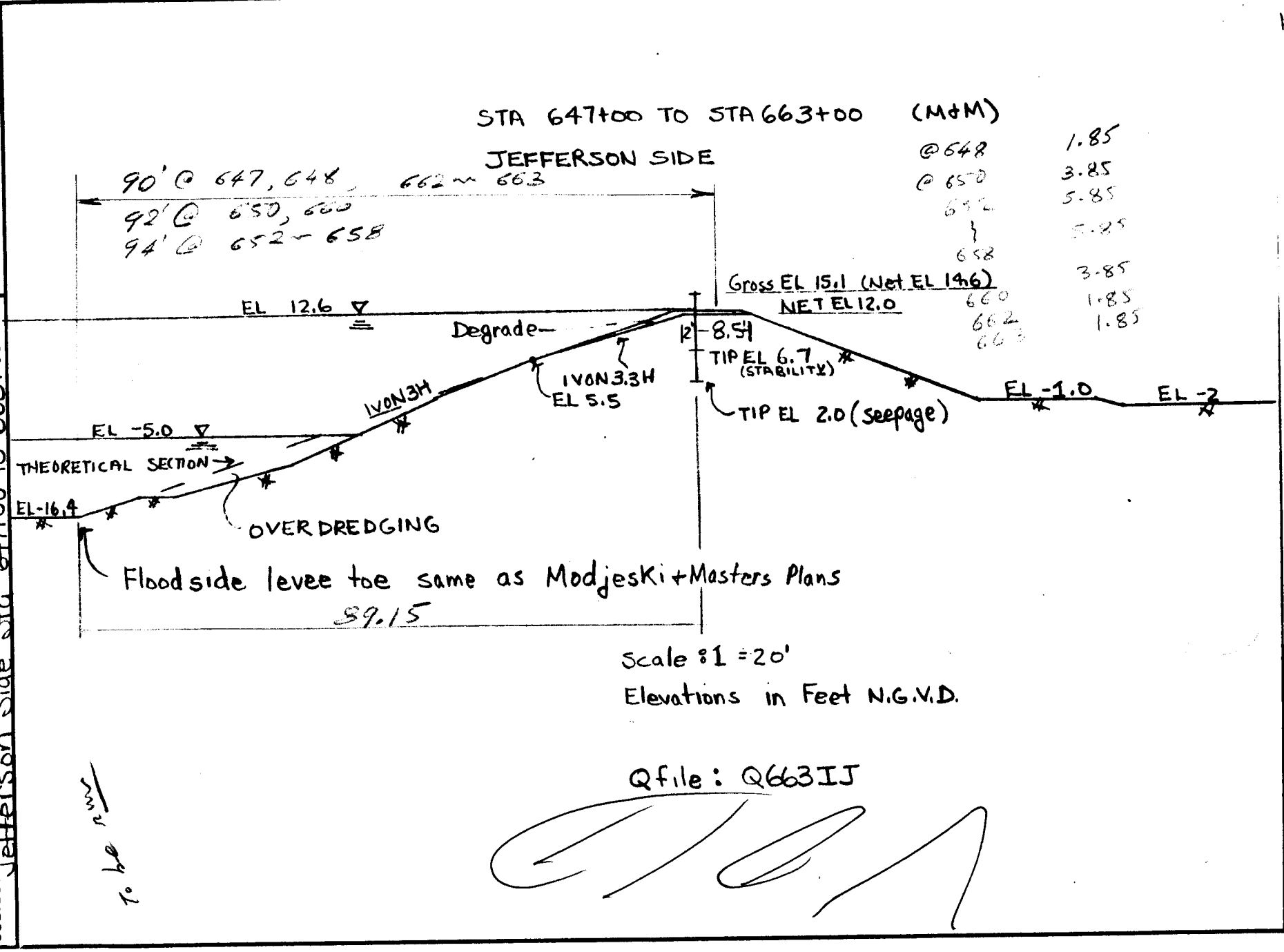


ADD 6668

COMPUTATION SHEET

PROJECT	17th St Outfall Canal
SUBJECT	Jefferson Side Sta 647+00 TO 663+00
COMPUTED BY	FJV
CHECKED BY	
DATE	4/24/87
DATE	



1 10001 47TH STREET CANAL HLP FROM STA. 647+00 TO STA. 663+00
 2 10002 I-WALL TOP EL. 14.6 BOTT. EL. 6.7 F.S.=1.5
 3 10003 3 0.14600000E+02 0.00000000E+01
 4 10004 3 0.13600000E+02 0.62500000E+02
 10005 3 0.12600000E+02 0.12500000E+03
 6 10006 3 0.12000000E+02 0.16250000E+03
 7 10007 3 0.12000000E+02 0.16250000E+03
 8 10008 3 0.11100674E+02 0.00000000E+01
 9 10009 3 0.11000000E+02 -0.18190847E+02
 10 10010 3 0.10000000E+02 -0.19888169E+03
 11 10011 3 0.90000000E+01 -0.37957254E+03
 12 10012 3 0.86254231E+01 -0.44871407E+03
 13 10013 3 0.78870802E+01 0.00000000E+01
 14 10014 3 0.66904102E+01 0.72725374E+03
 15 10015 4 0.66904102E+01 0.00000000E+01
 16 10016 0 0.66904102E+01 0.00000000E+01
 17 10017 0.66904102E+01 0.15939921E-02 0.36234752E+02
 EOT..

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH STREET CANAL HLP FROM STA. 647+00 TO STA. 663+00

WAL

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.01 INCHES AND OCCURS AT MEMBER COORDINATE
14.60 FT.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
6.69	POINT LD	6.94 LBF
6.69	COUPLE	-36.20 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
14.60	CONTN LD	0.00 LBF/SQ FT
13.60	CONTN LD	62.50 LBF/SQ FT
12.60	CONTN LD	125.00 LBF/SQ FT
12.00	CONTN LD	162.50 LBF/SQ FT
12.00	CONTN LD	162.50 LBF/SQ FT
11.10	CONTN LD	0.00 LBF/SQ FT
11.00	CONTN LD	-18.19 LBF/SQ FT
10.00	CONTN LD	-198.88 LBF/SQ FT
9.00	CONTN LD	-379.57 LBF/SQ FT
8.63	CONTN LD	-448.71 LBF/SQ FT
7.89	CONTN LD	0.00 LBF/SQ FT
6.69	CONTN LD	727.25 LBF/SQ FT
6.69	CONTN LD	0.00 LBF/SQ FT

Z-22 PROPERTIES ARE AS FOLLOWS.

MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
CROSS SECTIONAL AREA= 6.47 SQ IN.
ELASTIC MODULUS= 29000000. LBF/SQ IN.

58 DEFLECTION REFERENCE IS AT 6.700

59

60

61 THE MAXIMUM BENDING MOMENT IS 752.38 LBF-FT AND OCCURS AT 9.40
62 WHICH HAS THE SHEAR FORCE OF 21.60 LBF.

64

65

66

67	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	DEFLECTION
68	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	FROM TANG.
69	14.600	0.0	0.0	0.0	THRU DEFLE
					REFERENCE
					(INCHES)
70	14.599	0.0	0.0	0.0	0.0090
71	14.000	11.2	1.7	2.2	0.0079
72	13.000	80.0	12.4	42.7	0.0061
73	12.000	211.2	32.7	183.1	0.0043
74	11.101	284.3	44.0	416.9	0.0028
75	11.000	283.4	43.8	445.5	0.0026
76	10.000	174.9	27.0	689.7	0.0013
77	9.395	21.6	3.3	752.4	0.0007
78	9.000	-114.4	-17.7	735.0	0.0005
79	8.000	-431.3	-66.7	432.4	0.0001
80	7.888	-435.1	-67.3	383.8	0.0001
81	7.886	-435.1	-67.3	382.9	0.0001
82	7.000	-196.0	-30.3	68.1	0.0000
83	6.701	-7.7	-1.2	36.3	0.0000
84	6.700	-6.9	-1.1	36.3	0.0000

85

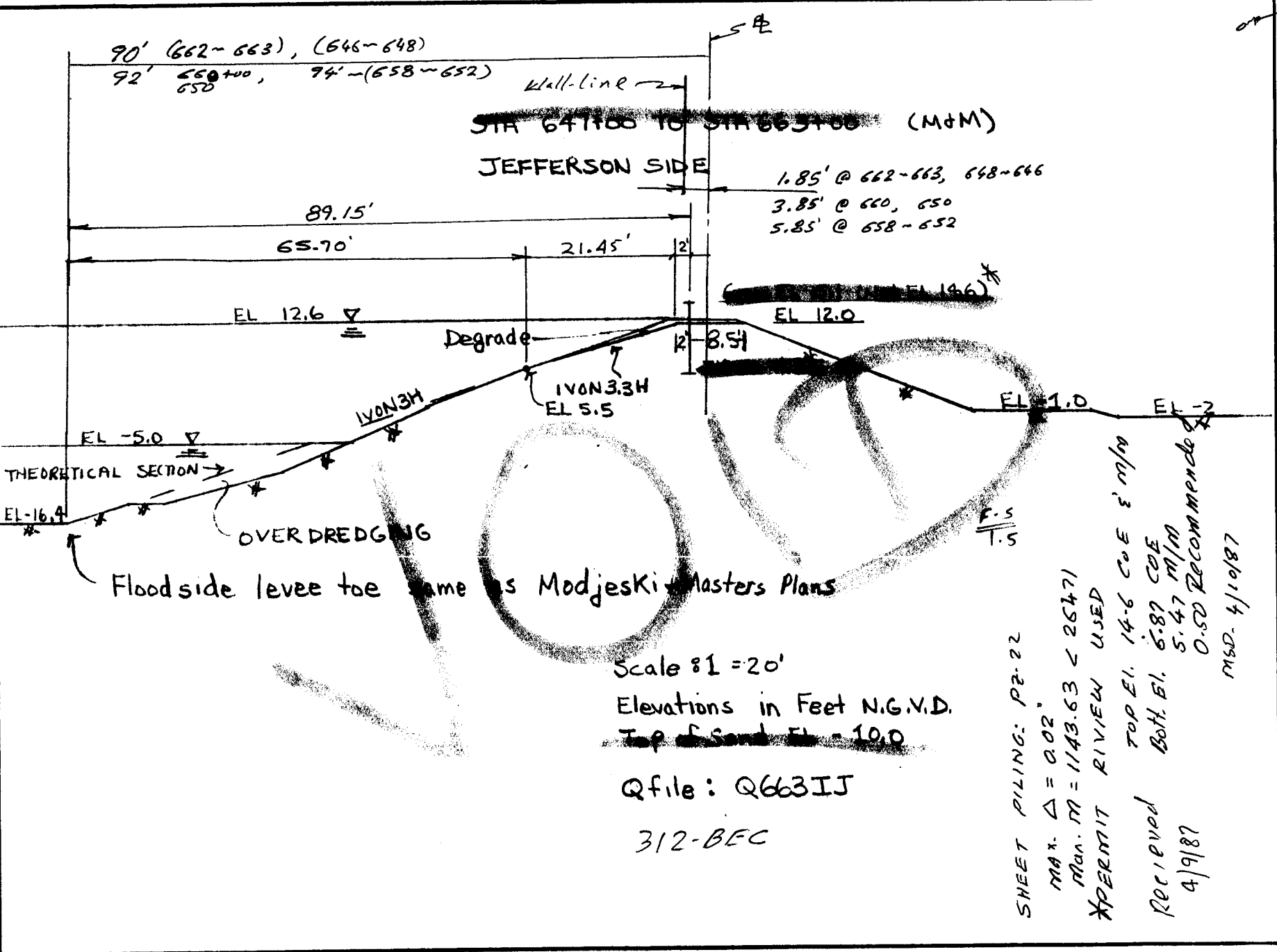
86

RUN COMPLETED

89

EOT..

PROJECT 17th St Outfall Canal	PAGE OF	COMPUTED BY FJV	DATE
SUBJECT Jefferson Side Sta 64700 TO 66310	CHECKED BY		DATE



Scale 1" = 20'
 Elevations in Feet N.G.V.D.
 Top of Sand Fl. = 10.0

Qfile: Q663IJ
 312-BEC

SHEET PILING: PE-22
 MAX. Δ = 0.02"
 Max. M = 1143.63 < 26471
 *PERMIT REVIEW USED
 TOP EL. 14.6 COE 8' M/M
 Bot. EL. 6.87 COE
 5.47 M/M
 0.50 Recommended
 M&D. 4/10/87
 Received
 4/9/87

LIST Q663IJ

1 10001 17TH ST CANAL HLF STA 647+00 TO STA 663+00
 2 10002 I-WALL TOP EL 15.1 BOTT. EL. 4.5 F.S.=1.5
 3 10003 3 0.14600000E+02 0.0000000E+01
 4 10004 3 0.13600000E+02 0.6250000E+02
 10005 3 0.12600000E+02 0.1250000E+03
 6 10006 3 0.12000000E+02 0.1625000E+03
 7 10007 3 0.12000000E+02 0.1625000E+03
 8 10008 3 0.11000000E+02 0.51615272E+02
 9 10009 3 0.10534514E+02 0.0000000E+01
 10 10010 3 0.10000000E+02 -0.59269456E+02
 11 10011 3 0.90000000E+01 -0.17221642E+03
 12 10012 3 0.80000000E+01 -0.20090214E+03
 13 10013 3 0.71116163E+01 -0.39256301E+03
 14 10014 3 0.61090023E+01 0.0000000E+01
 15 10015 3 0.46323729E+01 0.66242290E+03
 16 10016 4 0.46323729E+01 0.0000000E+01
 17 10017 0 0.46323729E+01 0.0000000E+01
 18 10018 0.46323729E+01 0.0000000E+01 0.30961010E+02

EOT..

LIST Q663IJ0

JOBCTRL ER 366 : INVALID LIST/DISPLAY ARGUMENT
LIST Q663IJ0

BEAMS (SHEAR, MOMENT, DEFLECTION)

3
4
5 17TH ST CANAL HLP STA 647+00 TO STA 663+00
6 WAL
7

8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
10 AS COUNTERCLOCKWISE.

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12
13 THE MAXIMUM DEFLECTION IS 0.02 INCHES AND OCCURS AT MEMBER COORDINATE
14 15.10 FT.

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18 Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19
20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21
22
23 CALCULATED EXTERNAL LOADS

24

25 DISTANCE FROM	26 TYPE OF	27 MAGNITUDE OF
REFERENCE (FT)	LOAD	LOAD
28 4.63	29 POINT LD	0.00 LBF
30 4.63	31 COUPLE	-30.96 LBF-FT

32 INPUTTED LOADS

34 DISTANCE FROM	35 TYPE OF	36 MAGNITUDE OF
REFERENCE (FT)	LOAD	LOAD
37 14.60	38 CONTN LD	0.00 LBF/SQ FT
39 13.60	40 CONTN LD	62.50 LBF/SQ FT
41 12.60	42 CONTN LD	125.00 LBF/SQ FT
43 12.00	44 CONTN LD	162.50 LBF/SQ FT
45 12.00	46 CONTN LD	162.50 LBF/SQ FT
47 11.00	48 CONTN LD	51.62 LBF/SQ FT
49 10.53	50 CONTN LD	0.00 LBF/SQ FT
51 10.00	52 CONTN LD	-59.27 LBF/SQ FT
53 9.00	54 CONTN LD	-172.22 LBF/SQ FT
55 8.00	56 CONTN LD	-288.90 LBF/SQ FT
57 7.11	58 CONTN LD	-392.56 LBF/SQ FT
59 6.19	60 CONTN LD	0.00 LBF/SQ FT
61 4.63	62 CONTN LD	662.42 LBF/SQ FT
63 4.63	64 CONTN LD	0.00 LBF/SQ FT

65 Z-22 PROPERTIES ARE AS FOLLOWS.

66
67 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
68 CROSS SECTIONAL AREA= 6.47 SQ IN.
69 ELASTIC MODULUS= 29000000. LBF/SQ IN.

59 DEFLECTION REFERENCE IS AT 4.500

60

61

62 THE MAXIMUM BENDING MOMENT IS 1143.63 LBF-FT AND OCCURS AT 8.14
63 WHICH HAS THE SHEAR FORCE OF 6.94 LBF.

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94 *RUN COMPLETED*

95

EOT..

DEFLECTION
FROM TANG.
THRU DEFLE
REFERENCE
(INCHES)

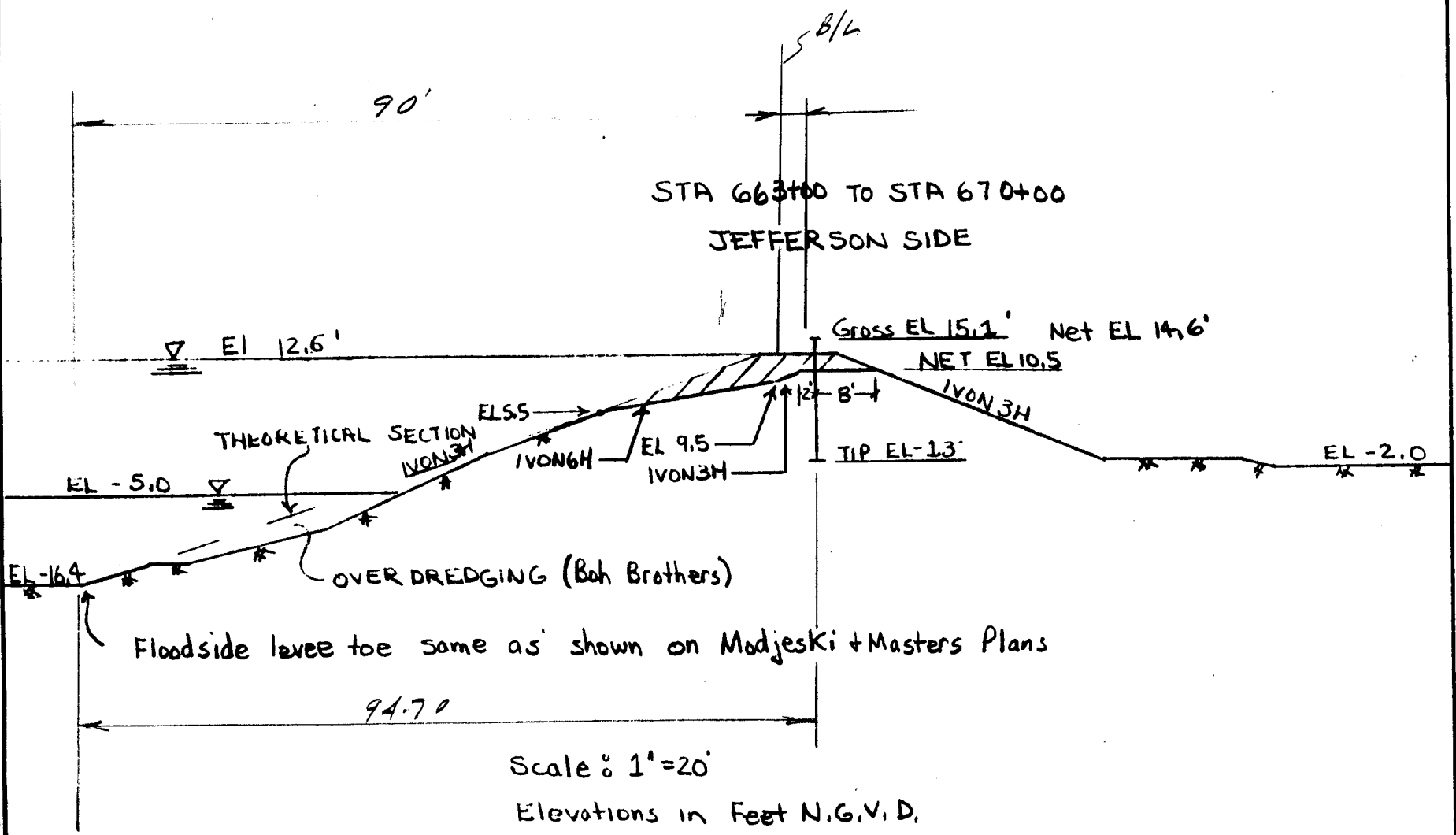
DISTANCE (FEET)	SHEAR FOR (LBF)	SHEAR STR (LBF/SQIN)	BENDING MOM (LBF-FT)	DEFLECTION FROM TANG. THRU DEFLE REFERENCE (INCHES)
15.100	0.0	0.0	0.0	0.0237
15.099	0.0	0.0	0.0	0.0237
15.000	0.0	0.0	0.0	0.0233
14.000	11.2	1.7	2.2	0.0197
13.000	00.0	12.4	42.7	0.0161
12.000	211.2	32.7	183.1	0.0125
11.000	318.3	49.2	457.1	0.0091
10.535	330.3	51.1	609.0	0.0076
10.000	314.5	48.6	782.7	0.0060
9.000	198.7	30.7	1048.8	0.0034
8.138	6.9	1.1	1143.6	0.0018
8.000	-31.8	-4.9	1141.9	0.0016
7.000	-375.7	-58.1	946.3	0.0005
6.190	-515.6	-79.7	566.6	0.0001
6.188	-515.6	-79.7	565.5	0.0001
6.000	-508.0	-78.5	469.0	0.0001
5.000	-214.8	-33.2	72.2	0.0000
4.633	-0.7	-0.1	31.0	0.0000
4.631	0.0	0.0	0.0	0.0000
4.501	0.0	0.0	0.0	0.0000
4.500	0.0	0.0	0.0	0.0000

PROJECT 17th St Outfall Canal
 SUBJECT Jefferson Side STA 663+00 To 670+00

COMPUTED BY [Signature]
 CHECKED BY [Signature]

DATE 4/24/87
 DATE

PAGE OF 670+00



Scale: 1" = 20'
 Elevations in Feet N.G.V.D.

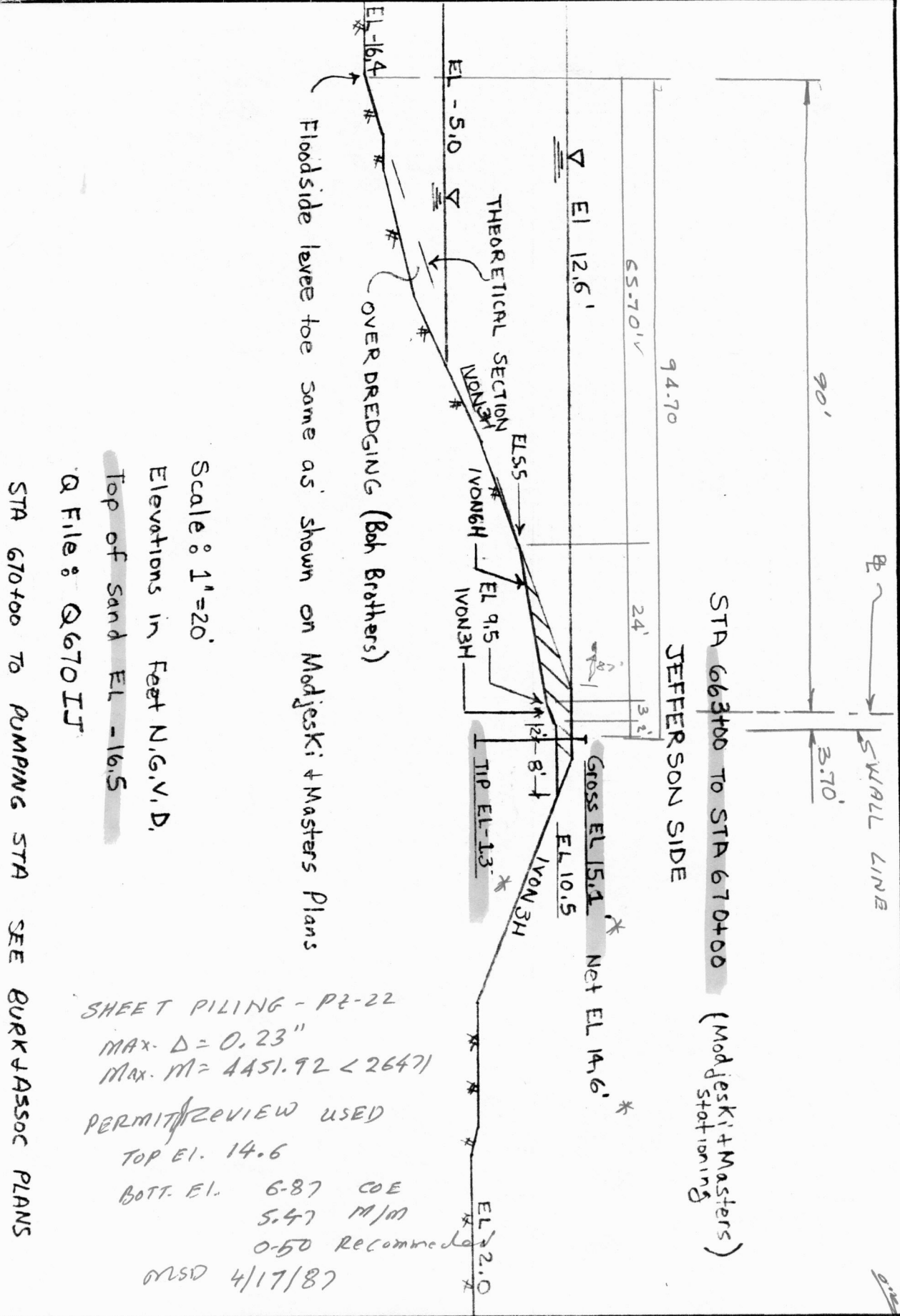
Q File: Q670IJ

[Signature]

(FOR USE WITH 10 x 10 GRID) ENCL 7
 PREVIOUS EDITIONS MAY BE USED
 LMV FORM 107e MAR 82

Encl 7

PROJECT	17th St Outfall Canal	PAGE OF	COMPUTED BY	DATE
SUBJECT	Jefferson Side Sta 663+00 To 670+00		CHECKED BY	DATE



Floodside levee toe same as shown on Modjeski + Masters Plans

Scale: 1"=20'

Elevations in Feet N.G.V.D.

Top of Sand EL -16.5

Q File: Q670IJ

STA 670+00 TO PUMPING STA SEE BURKJASSOC PLANS

SHEET PILING - P2-22
 Max. D = 0.23"
 Max. M = 4451.92 < 26471
 PERMIT REVIEW USED
 TOP EL. 14.6
 BOTT. EL. 6.87 COE
 5.47 M/M
 0.50 Recommended
 MSD 4/17/87

STA 663+00 TO STA 670+00
 JEFFERSON SIDE
 (Modjeski + Masters)
 Stationing

LIST Q670IJ

1 10001 17TH STREET HLP S.A. 663+00 TO STA 670+00
2 10002 I-WALL TOP EL. 15.1 BOTT EL. -1.3 F.S.=1.5
3 10003 3 0.14600000E+02 0.00000000E+01
4 10004 3 0.13600000E+02 0.62500000E+02
10005 3 0.12600000E+02 0.12500000E+03
6 10006 3 0.11600000E+02 0.18750000E+03
7 10007 3 0.10600000E+02 0.25000000E+03
8 10008 3 0.10500000E+02 0.25625000E+03
9 10009 3 0.10500000E+02 0.25625000E+03
10 10010 3 0.95000000E+01 0.14536527E+03
11 10011 3 0.85000000E+01 0.34480544E+02
12 10012 3 0.81951751E+01 0.00000000E+01
13 10013 3 0.75000000E+01 -0.78635341E+02
14 10014 3 0.65000000E+01 -0.19579625E+03
15 10015 3 0.55000000E+01 -0.31295715E+03
16 10016 3 0.55000000E+01 -0.31295715E+03
17 10017 3 0.45000000E+01 -0.41835424E+03
18 10018 3 0.35000000E+01 -0.46432045E+03
19 10019 3 0.25000000E+01 -0.50610077E+03
20 10020 3 0.24098109E+01 -0.50973138E+03
21 10021 3 0.11503886E+01 0.00000000E+01
22 10022 3 -0.13235832E+01 0.10109607E+04
23 10023 4 -0.13235832E+01 0.00000000E+01

24 10024 0 -0.13235832E+01 0.00000000E+01
25 10025 -0.13235832E+01 0.14901161E-07 0.31351022E+02

EOT..

LIST MSD22

1 100 1 15.1 -1.3 1 -1.3 0 -1
200 PZ-22
300 29000000 6.4691 04.3818

EOT..

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH STREET HLP STA. 663+00 TO STA 670+00

WAL

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.23 INCHES AND OCCURS AT MEMBER COORDINATE
15.10 FT.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
-1.32	POINT LD	23.73 LBF
-1.32	COUPLE	-31.07 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
14.60	CONTN LD	0.00 LBF/SQ FT
13.60	CONTN LD	62.50 LBF/SQ FT
12.60	CONTN LD	125.00 LBF/SQ FT
11.60	CONTN LD	187.50 LBF/SQ FT
10.60	CONTN LD	250.00 LBF/SQ FT
10.50	CONTN LD	256.25 LBF/SQ FT
10.50	CONTN LD	256.25 LBF/SQ FT
9.50	CONTN LD	145.37 LBF/SQ FT
8.50	CONTN LD	34.48 LBF/SQ FT
8.20	CONTN LD	0.00 LBF/SQ FT
7.50	CONTN LD	-78.64 LBF/SQ FT
6.50	CONTN LD	-195.80 LBF/SQ FT
5.50	CONTN LD	-312.96 LBF/SQ FT
5.50	CONTN LD	-312.96 LBF/SQ FT
4.50	CONTN LD	-418.35 LBF/SQ FT
3.50	CONTN LD	-464.32 LBF/SQ FT
2.50	CONTN LD	-506.11 LBF/SQ FT
2.41	CONTN LD	-509.73 LBF/SQ FT
1.16	CONTN LD	0.00 LBF/SQ FT
-1.32	CONTN LD	1010.96 LBF/SQ FT
-1.32	CONTN LD	0.00 LBF/SQ FT

Z-22 PROPERTIES ARE AS FOLLOWS.

62
 63 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
 64 CROSS SECTIONAL AREA 6.47 SQ IN.
 65 ELASTIC MODULUS= 29000000. LBF/SQ IN.
 66 DEFLECTION REFERENCE IS AT -1.300
 67

68
 69 THE MAXIMUM BENDING MOMENT IS 4451.92 LBF-FT AND OCCURS AT 4.43
 70 WHICH HAS THE SHEAR FORCE OF 8.87 LBF.
 71
 72

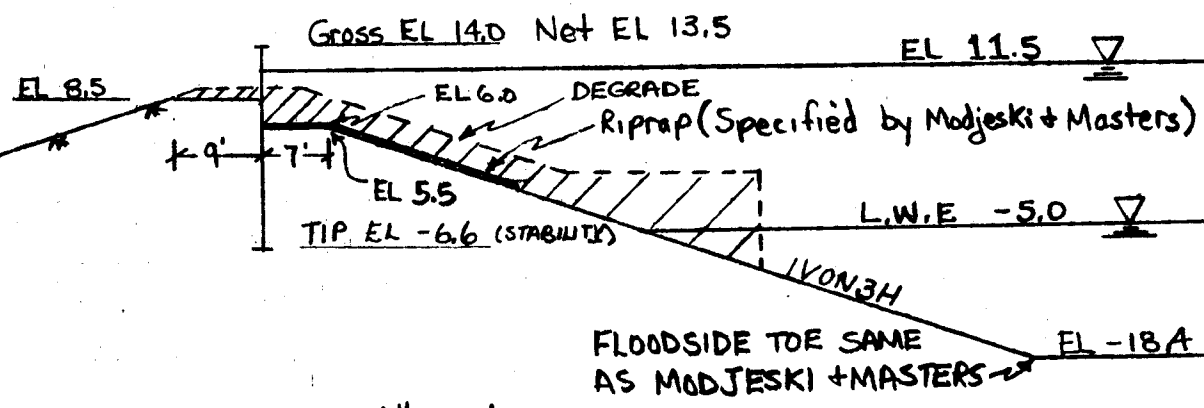
73	DEFLECTION FROM TANG. THRU DEFLE				
74	REFERENCE				
75	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	DEFLECTION
76	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	(INCHES)
77	15.100	0.0	0.0	0.0	0.2273
78	15.099	0.0	0.0	0.0	0.2273
79	15.000	0.0	0.0	0.0	0.2251
80	14.000	11.2	1.7	2.2	0.2025
81	13.000	80.0	12.4	42.7	0.1799
82	12.000	211.2	32.7	183.1	0.1573
83	11.000	405.0	62.6	486.0	0.1349
84	10.000	639.6	98.9	1010.3	0.1129
85	9.000	784.9	121.3	1731.8	0.0916
86	8.195	821.3	127.0	2383.0	0.0754
87	8.000	819.1	126.6	2543.2	0.0716
88	7.000	740.0	114.4	3332.3	0.0533
89	6.000	544.2	84.1	3984.2	0.0374
90	5.000	232.7	36.0	4382.0	0.0242
91	4.434	8.9	1.4	4451.9	0.0181
92	4.000	-178.2	-27.5	4415.5	0.0141
93	3.000	-642.0	-99.2	4009.1	0.0071
94	2.000	-1110.3	-171.6	3122.7	0.0029
95	1.159	-1254.6	-193.9	2108.5	0.0010
96	1.157	-1254.6	-193.9	2106.0	0.0010
97	1.000	-1249.5	-193.1	1908.8	0.0008
98	0.000	-981.3	-151.7	759.5	0.0001
99	-1.000	-305.8	-47.3	82.0	0.0000
100	-1.299	-24.7	-3.8	31.7	0.0000
101	-1.300	-23.7	-3.7	31.6	0.0000

102
103
104
105 *RUN COMPLETED*

106
EOT..

PROJECT	17th St Outfall Canal	PAGE	OF	COMPUTED BY	DATE
SUBJECT	STA 545+80 TO STA 552+70	CHECKED BY			6/87

STA 545+80 TO STA 552+70
ORLEANS SIDE



SCALE 1" = 20'

Q File : Q5460 F.S. = 1.5
ELEVATION IN FEET NGVD

12.9

LIST 05460

1 10001 17TH ST CANAL HLP STA. 545+80 TO STA 552+70
2 10002 I-WALL TOP EL. 14.0 BOTT. EL -6.6 F.S.=1.5
3 10003 3 0.13500000E+02 0.00000000E+01
4 10004 3 0.12500000E+02 0.62500000E+02
5 10005 3 0.11500000E+02 0.12500000E+03
6 10006 3 0.10500000E+02 0.18750000E+03
7 10007 3 0.95000000E+01 0.25000000E+03
8 10008 3 0.85000000E+01 0.31250000E+03
9 10009 3 0.85000000E+01 0.31250000E+03
10 10010 3 0.75000000E+01 0.18268148E+03
11 10011 3 0.65000000E+01 0.52862958E+02
12 10012 3 0.60927934E+01 0.00000000E+01
13 10013 3 0.60000000E+01 -0.12046303E+02
14 10014 3 0.60000000E+01 -0.12046303E+02
15 10015 3 0.55000000E+01 -0.65004993E+02
16 10016 3 0.55000000E+01 -0.57079683E+02
17 10017 3 0.45000000E+01 -0.15972973E+03
18 10018 3 0.35000000E+01 -0.26237979E+03
19 10019 3 0.25000000E+01 -0.36502984E+03
20 10020 3 0.15000000E+01 -0.45680299E+03
21 10021 3 0.50000000E+00 -0.50322142E+03
22 10022 3 0.00000000E+01 -0.52643064E+03
23 10023 3 0.00000000E+01 -0.52643064E+03

24 10024 3 -0.91593812E+00 -0.51533197E+03
25 10025 3 -0.27544540E+01 0.00000000E+01
26 10026 3 -0.61277475E+01 0.94552677E+03
27 10027 4 -0.61277475E+01 0.00000000E+01
28 10028 0 -0.61277475E+01 0.00000000E+01
29 10029 -0.61277475E+01 0.29802322E-07 0.31849671E+02

JT..

TYPE Q54600
File not found

A:\>B:

\>TYPE Q54600

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH ST CANAL HLP STA. 545+80 TO STA 552+70
WAL

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.56 INCHES AND OCCURS AT MEMBER COORDINATE
14.00 FT.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
-6.13	POINT LD	0.00 LBF
-6.13	COUPLE	-31.85 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
13.50	CONTN LD	0.00 LBF/SQ FT
12.50	CONTN LD	62.50 LBF/SQ FT
11.50	CONTN LD	125.00 LBF/SQ FT
10.50	CONTN LD	187.50 LBF/SQ FT
9.50	CONTN LD	250.00 LBF/SQ FT
8.50	CONTN LD	312.50 LBF/SQ FT
8.50	CONTN LD	312.50 LBF/SQ FT
7.50	CONTN LD	182.68 LBF/SQ FT
6.50	CONTN LD	52.86 LBF/SQ FT
6.09	CONTN LD	0.00 LBF/SQ FT
6.00	CONTN LD	-12.05 LBF/SQ FT
6.00	CONTN LD	-12.05 LBF/SQ FT
5.50	CONTN LD	-65.00 LBF/SQ FT
5.50	CONTN LD	-57.08 LBF/SQ FT
4.50	CONTN LD	-159.73 LBF/SQ FT
3.50	CONTN LD	-262.38 LBF/SQ FT
2.50	CONTN LD	-365.03 LBF/SQ FT
1.50	CONTN LD	-456.80 LBF/SQ FT

0.50	CONTN LD	-503.22	LBF/SQ FT
0.00	CONTN LD	-526.43	LBF/SQ FT
0.00	CONTN LD	-526.43	LBF/SQ FT
-0.92	CONTN LD	-515.33	LBF/SQ FT
-2.75	CONTN LD	0.00	LBF/SQ FT
-6.13	CONTN LD	945.53	LBF/SQ FT
-6.13	CONTN LD	0.00	LBF/SQ FT

Z-22

PROPERTIES ARE AS FOLLOWS.

MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
 CROSS SECTIONAL AREA= 6.47 SQ IN.
 ELASTIC MODULUS= 29000000. LBF/SQ IN.
 DEFLECTION REFERENCE IS AT -6.600

THE MAXIMUM BENDING MOMENT IS 7478.31 LBF-FT AND OCCURS AT 1.31
 WHICH HAS THE SHEAR FORCE OF 6.75 LBF.

DISTANCE (FEET)	SHEAR FOR (LBF)	SHEAR STR (LBF/SQIN)	BENDING MOM (LBF-FT)	DEFLECTION FROM TANG. THRU DEFLE REFERENCE (INCHES)
14.000	0.0	0.0	0.0	0.5649
13.999	0.0	0.0	0.0	0.5649
13.000	7.8	1.2	1.3	0.5181
12.000	70.3	10.9	35.2	0.4713
11.000	195.3	30.2	162.8	0.4245
10.000	382.8	59.2	446.6	0.3778
9.000	632.8	97.8	949.2	0.3315
8.000	921.3	142.4	1729.1	0.2859
7.000	1104.0	170.7	2752.5	0.2415
6.093	1157.4	178.9	3786.3	0.2029
6.000	1156.8	178.8	3893.7	0.1991
5.000	1096.2	169.4	5027.9	0.1594
4.000	936.5	144.8	6052.8	0.1233
3.000	674.1	104.2	6866.6	0.0914
2.000	310.4	48.0	7366.9	0.0643
1.312	6.8	1.0	7478.3	0.0487
1.000	-140.7	-21.8	7457.5	0.0424
0.000	-644.0	-99.5	7069.1	0.0257
-1.000	-1163.4	-179.8	6163.9	0.0140
-2.000	-1515.0	-234.2	4801.4	0.0065
-2.753	-1594.8	-246.5	3619.9	0.0032
-2.755	-1594.8	-246.5	3616.7	0.0032
-3.000	-1586.3	-245.2	3227.4	0.0024
-4.000	-1377.3	-212.9	1722.2	0.0006
-5.000	-888.1	-137.3	566.1	0.0001
-6.000	-118.5	-18.3	39.5	0.0000
-6.127	-0.9	-0.1	31.9	0.0000
-6.129	0.0	0.0	0.0	0.0000
-6.599	0.0	0.0	0.0	0.0000
-6.600	0.0	0.0	0.0	0.0000

RUN COMPLETED

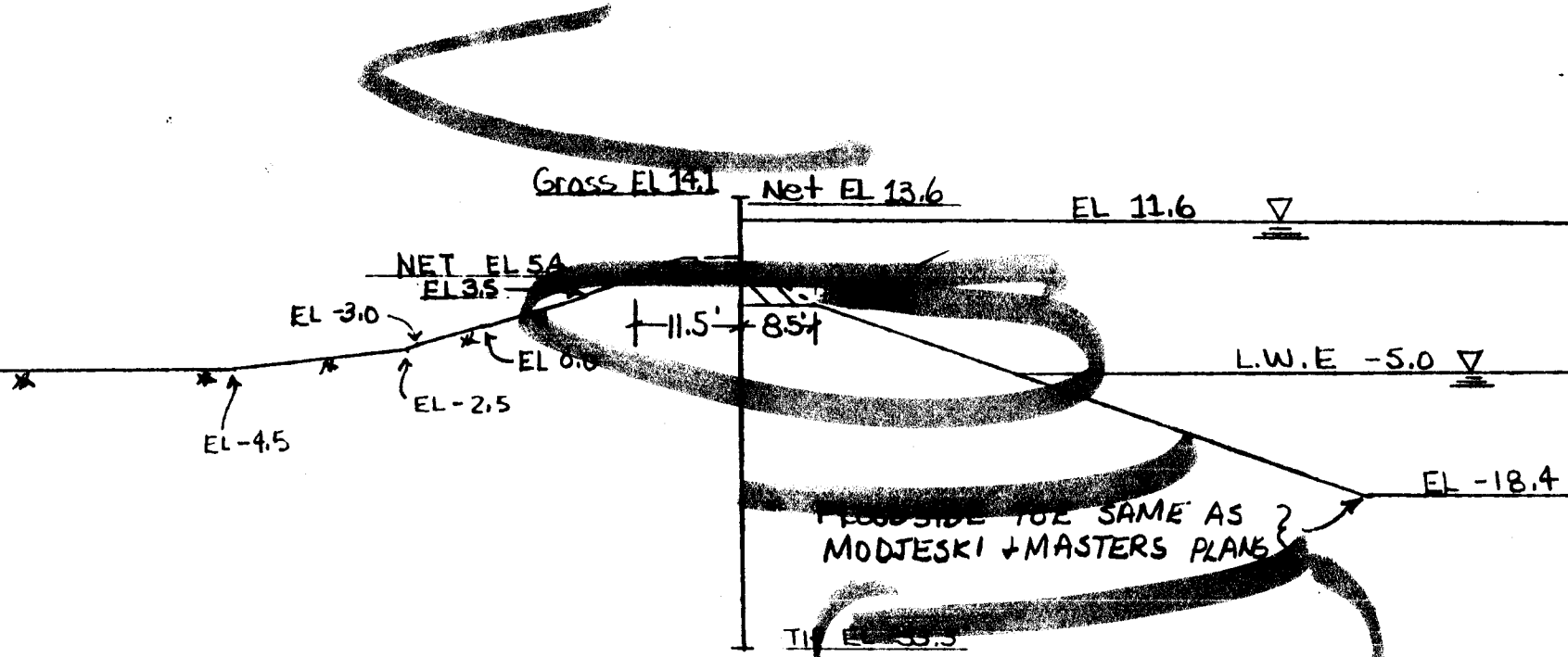
PROJECT 17th St Outfall Canal
 SUBJECT STA 553+70 TO STA 568+00

DATE 2/24/87
 DATE

COMPUTED BY
 CHECKED BY

PAGE OF

STA 553+70 TO STA 568+00
 ORLEANS SIDE



SCALE: 1" = 20'
 ELEVATION IN FEET N.G.V.D.
 Q-Files: Q 55410A FS, = 1.0

Encl 8

LIST Q55410

1	10001	17TH STREET CANAL HLP FROM STA 553+70 TO STA 568+00		
2	10002	I-WALL TOP EL. 13.6 BOTT. EL. -35.3 F.S.=1.5		
3	10003		3	0.136000000E+02 0.00000000E+01
4	10004		3	0.126000000E+02 0.62500000E+02
5	10005		3	0.116000000E+02 0.12500000E+03
6	10006		3	0.106000000E+02 0.18750000E+03
7	10007		3	0.960000000E+01 0.25000000E+03
8	10008		3	0.860000000E+01 0.31250000E+03
9	10009		3	0.760000000E+01 0.37500000E+03
10	10010		3	0.660000000E+01 0.43750000E+03
11	10011		3	0.560000000E+01 0.50000000E+03
12	10012		3	0.540000000E+01 0.51250000E+03
13	10013		3	0.540000000E+01 0.51250000E+03
14	10014		3	0.440000000E+01 0.36519798E+03
15	10015		3	0.350000000E+01 0.23262616E+03
16	10016		3	0.350000000E+01 0.23262616E+03
17	10017		3	0.250000000E+01 0.11504609E+03
18	10018		3	0.210000000E+01 0.68014058E+02
19	10019		3	0.210000000E+01 0.68014058E+02
20	10020		3	0.13796294E+01 0.00000000E+01
21	10021		3	0.110000000E+01 -0.26401316E+02
22	10022		3	0.100000000E+00 -0.12081669E+03
23	10023		3	0.000000000E+01 -0.13025823E+03
24	10024		3	0.000000000E+01 -0.13025823E+03
25	10025		3	-0.100000000E+01 -0.17790172E+03
26	10026		3	-0.200000000E+01 -0.22554520E+03
27	10027		3	-0.300000000E+01 -0.27318869E+03
28	10028		3	-0.400000000E+01 -0.26602237E+03
29	10029		3	-0.500000000E+01 -0.24948378E+03
30	10030		3	-0.600000000E+01 -0.23294518E+03
31	10031		3	-0.700000000E+01 -0.21640659E+03
32	10032		3	-0.800000000E+01 -0.19986799E+03
33	10033		3	-0.900000000E+01 -0.18332940E+03
34	10034		3	-0.100000000E+02 -0.17822789E+03
35	10035		3	-0.110000000E+02 -0.17793256E+03
36	10036		3	-0.120000000E+02 -0.17763724E+03
37	10037		3	-0.130000000E+02 -0.17734191E+03
38	10038		3	-0.140000000E+02 -0.17704659E+03
39	10039		3	-0.150000000E+02 -0.17787848E+03
40	10040		3	-0.160000000E+02 -0.17892531E+03
41	10041		3	-0.170000000E+02 -0.17997214E+03
42	10042		3	-0.180000000E+02 -0.18101897E+03
43	10043		3	-0.190000000E+02 -0.18206579E+03
44	10044		3	-0.200000000E+02 -0.18313404E+03
45	10045		3	-0.205000000E+02 -0.18372188E+03
46	10046		3	-0.205000000E+02 -0.18372188E+03
47	10047		3	-0.215000000E+02 -0.18372119E+03
48	10048		3	-0.225000000E+02 -0.21272843E+03
49	10049		3	-0.235000000E+02 -0.24339596E+03
50	10050		3	-0.245000000E+02 -0.27406349E+03
51	10051		3	-0.255000000E+02 -0.30473103E+03
52	10052		3	-0.265000000E+02 -0.33539856E+03
53	10053		3	-0.275000000E+02 -0.36606609E+03
54	10054		3	-0.285000000E+02 -0.39650244E+03
55	10055		3	-0.295000000E+02 -0.41946547E+03
56	10056		3	-0.30054500E+02 -0.43998029E+03
57	10057		3	-0.31004157E+02 0.00000000E+01

58 10058 .
59 10059
60 10060
EOT..

3 -0.35248374E+02 0.19663658E+04
4 -0.35248374E+02 0.00000000E+01
0 -0.35248374E+02 0.00000000E+01

BEAMS (SHEAR, MOMENT, DEFLECTION)

7TH STREET CANAL HLP FROM STA. 553+70 TO STA 568+00
WAL

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 10.11 INCHES AND OCCURS AT MEMBER COORDINATE
13.60 FT.

Z-27 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
-35.25	POINT LD	-0.51 LBF
-35.25	COUPLE	-13.32 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
13.60	CONTN LD	0.00 LBF/SQ FT
12.60	CONTN LD	62.50 LBF/SQ FT
11.60	CONTN LD	125.00 LBF/SQ FT
10.60	CONTN LD	187.50 LBF/SQ FT
9.60	CONTN LD	250.00 LBF/SQ FT
8.60	CONTN LD	312.50 LBF/SQ FT
7.60	CONTN LD	375.00 LBF/SQ FT
6.60	CONTN LD	437.50 LBF/SQ FT
5.60	CONTN LD	500.00 LBF/SQ FT
5.40	CONTN LD	512.50 LBF/SQ FT
5.40	CONTN LD	512.50 LBF/SQ FT
4.40	CONTN LD	365.20 LBF/SQ FT
3.50	CONTN LD	232.63 LBF/SQ FT
3.50	CONTN LD	232.63 LBF/SQ FT
2.50	CONTN LD	115.05 LBF/SQ FT
2.10	CONTN LD	68.01 LBF/SQ FT
2.10	CONTN LD	68.01 LBF/SQ FT
1.38	CONTN LD	0.00 LBF/SQ FT
1.10	CONTN LD	-26.40 LBF/SQ FT
0.10	CONTN LD	-120.82 LBF/SQ FT
0.00	CONTN LD	-130.26 LBF/SQ FT

3	0.00	CONTN LD	-130.26	LBF/SQ FT
7	-1.00	CONTN LD	-177.90	LBF/SQ FT
10	-2.00	CONTN LD	-225.55	LBF/SQ FT
11	-3.00	CONTN LD	-273.19	LBF/SQ FT
52	-4.00	CONTN LD	-266.02	LBF/SQ FT
53	-5.00	CONTN LD	-249.48	LBF/SQ FT
54	-6.00	CONTN LD	-232.95	LBF/SQ FT
55	-7.00	CONTN LD	-216.41	LBF/SQ FT
56	-8.00	CONTN LD	-199.87	LBF/SQ FT
57	-9.00	CONTN LD	-183.33	LBF/SQ FT
58	-10.00	CONTN LD	-178.23	LBF/SQ FT
59	-11.00	CONTN LD	-177.93	LBF/SQ FT
70	-12.00	CONTN LD	-177.64	LBF/SQ FT
71	-13.00	CONTN LD	-177.34	LBF/SQ FT
72	-14.00	CONTN LD	-177.05	LBF/SQ FT
73	-15.00	CONTN LD	-177.88	LBF/SQ FT
74	-16.00	CONTN LD	-178.93	LBF/SQ FT
75	-17.00	CONTN LD	-179.97	LBF/SQ FT
76	-18.00	CONTN LD	-181.02	LBF/SQ FT
77	-19.00	CONTN LD	-182.07	LBF/SQ FT
78	-20.00	CONTN LD	-183.13	LBF/SQ FT
79	-20.50	CONTN LD	-183.72	LBF/SQ FT
80	-20.50	CONTN LD	-183.72	LBF/SQ FT
81	-21.50	CONTN LD	-183.72	LBF/SQ FT
82	-22.50	CONTN LD	-212.73	LBF/SQ FT
83	-23.50	CONTN LD	-243.40	LBF/SQ FT
84	-24.50	CONTN LD	-274.06	LBF/SQ FT
85	-25.50	CONTN LD	-304.73	LBF/SQ FT
86	-26.50	CONTN LD	-335.40	LBF/SQ FT
87	-27.50	CONTN LD	-366.07	LBF/SQ FT
88	-28.50	CONTN LD	-396.50	LBF/SQ FT
89	-29.50	CONTN LD	-419.47	LBF/SQ FT
90	-30.05	CONTN LD	-439.98	LBF/SQ FT
91	-31.00	CONTN LD	0.00	LBF/SQ FT
92	-35.25	CONTN LD	1966.37	LBF/SQ FT
93	-35.25	CONTN LD	0.00	LBF/SQ FT
94				
95				

96 Z-27 PROPERTIES ARE AS FOLLOWS.

99 MOMENT OF INERTIA= 184.20 IN. TO THE 4TH PER FOOT OF WALL.
100 CROSS SECTIONAL AREA= 7.94 SQ IN.
101 ELASTIC MODULUS= 29000000. LBF/SQ IN.
102 DEFLECTION REFERENCE IS AT -35.300

105 THE MAXIMUM BENDING MOMENT IS 41192.44 LBF-FT AND OCCURS AT -14.36
106 WHICH HAS THE SHEAR FORCE OF 0.10 LBF.

111	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	DEFLECTION
112	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	FROM TANG.
					THRU DEFLE
					REFERENCE
					(INCHES)
113	13.600	0.0	0.0	0.0	10.1125
114	13.599	0.0	0.0	0.0	10.1125
115	13.000	11.2	1.4	2.2	9.8924

6	12.000	80.0	10.1	42.7	9.5250
7	11.000	211.2	26.6	183.1	9.1576
18	10.000	405.0	51.0	486.0	8.7902
19	9.000	661.2	83.3	1013.9	8.4231
20	8.000	980.0	123.4	1829.3	8.0562
121	7.000	1361.2	171.4	2994.7	7.6900
122	6.000	1805.0	227.3	4572.7	7.3248
123	5.000	2294.5	289.0	6623.3	6.9610
124	4.000	2674.4	336.8	9120.1	6.5995
125	3.000	2910.7	366.6	11923.7	6.2409
126	2.000	3025.9	381.1	14901.7	5.8861
127	1.380	3044.1	383.4	16786.4	5.6684
128	1.000	3037.3	382.5	17941.2	5.5362
129	0.000	2954.2	372.1	20944.8	5.1921
130	-1.000	2800.1	352.7	23825.9	4.8548
131	-2.000	2598.4	327.3	26529.1	4.5252
132	-3.000	2349.0	295.8	29006.8	4.2041
133	-4.000	2079.4	261.9	31220.5	3.8924
134	-5.000	1821.7	229.4	33169.7	3.5908
135	-6.000	1580.5	199.1	34869.4	3.2999
136	-7.000	1355.8	170.8	36336.1	3.0203
137	-8.000	1147.7	144.5	37586.5	2.7524
138	-9.000	956.1	120.4	38636.9	2.4966
139	-10.000	775.3	97.6	39502.2	2.2534
140	-11.000	597.2	75.2	40188.4	2.0229
141	-12.000	419.4	52.8	40696.7	1.8054
142	-13.000	241.9	30.5	41027.3	1.6011
143	-14.000	64.7	8.2	41180.6	1.4100
144	-14.365	0.1	0.0	41192.4	1.3436
145	-15.000	-112.7	-14.2	41156.7	1.2323
146	-16.000	-291.1	-36.7	40954.8	1.0678
147	-17.000	-470.6	-59.3	40574.1	0.9166
148	-18.000	-651.1	-82.0	40013.3	0.7785
149	-19.000	-832.6	-104.9	39271.6	0.6533
150	-20.000	-1015.2	-127.9	38347.7	0.5408
151	-21.000	-1198.8	-151.0	37240.8	0.4408
152	-22.000	-1386.1	-174.6	35949.5	0.3527
153	-23.000	-1599.1	-201.4	34459.4	0.2763
154	-24.000	-1842.5	-232.0	32741.1	0.2110
155	-25.000	-2116.5	-266.6	30764.2	0.1563
156	-26.000	-2421.3	-304.9	28497.8	0.1115
157	-27.000	-2756.7	-347.2	25911.4	0.0760
158	-28.000	-3122.7	-393.3	22974.3	0.0488
159	-29.000	-3518.3	-443.1	19656.0	0.0290
160	-30.000	-3939.5	-496.2	15929.6	0.0155
161	-31.000	-4172.3	-525.5	11835.5	0.0072
162	-31.003	-4172.3	-525.5	11822.3	0.0072
163	-31.005	-4172.3	-525.5	11814.0	0.0072
164	-32.000	-3942.6	-496.5	7739.4	0.0027
165	-33.000	-3249.6	-409.3	4104.7	0.0007
166	-34.000	-2093.2	-263.6	1394.7	0.0001
167	-35.000	-473.6	-59.6	72.7	0.0000
168	-35.247	-1.5	-0.2	13.3	0.0000
169	35.249	0.0	0.0	0.0	0.0000
170	35.299	0.0	0.0	0.0	0.0000
171	-35.300	0.0	0.0	0.0	0.0000
172					

3

'4

75 *RUN COMPLETED*

76

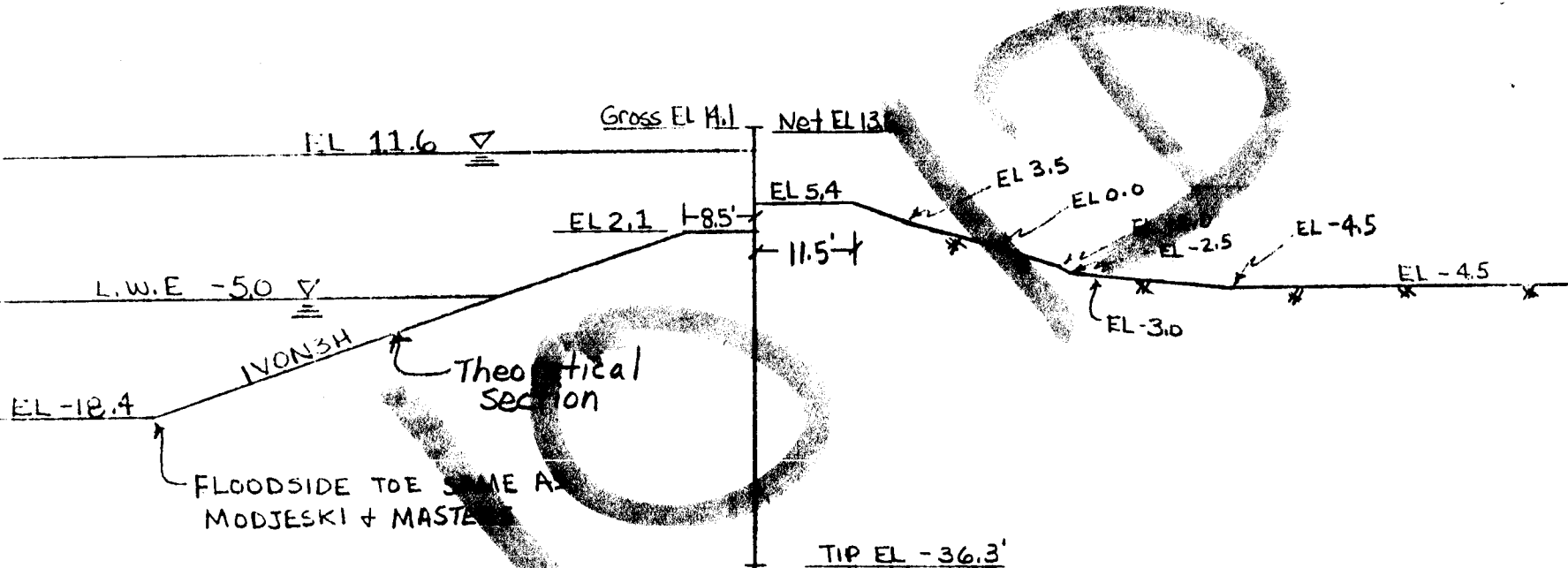
EOT..

Waiting...

Terminated.
DldfmxDLdkm

PROJECT	17th St Outfall Canal	PAGE	OF	COMPUTED BY	DATE
SUBJECT	STA 554+00 TO STA 568+00 ORLEANS			CHECKED BY	DATE

STA 554 TO STA 568+00
ORLEANS SIDE



SCALE : 1" = 20'

ELEVATION IN FEET N.G.V.D

TOP OF SAND EL -36.5

Q-FILES ~~Q 554 IOA~~ FS = 1.5

Q 554 IOA FS = 1.0

Benjamin
4/21

LIST Q55410

1	10001	17TH STREET CANAL HLP STA 554+00 TO STA 568+00		
2	10002	I-WALL TOP EL 14.1 BOTT EL -36.3 F.S.+1.5		
3	10003		3	0.136000000E+02 0.000000000E+01
4	10004		3	0.126000000E+02 0.625000000E+02
5	10005		3	0.116000000E+02 0.125000000E+03
6	10006		3	0.106000000E+02 0.187500000E+03
7	10007		3	0.960000000E+01 0.250000000E+03
8	10008		3	0.860000000E+01 0.312500000E+03
9	10009		3	0.760000000E+01 0.375000000E+03
10	10010		3	0.660000000E+01 0.437500000E+03
11	10011		3	0.560000000E+01 0.500000000E+03
12	10012		3	0.540000000E+01 0.512500000E+03
13	10013		3	0.540000000E+01 0.512500000E+03
14	10014		3	0.440000000E+01 0.36519798E+03
15	10015		3	0.350000000E+01 0.23262616E+03
16	10016		3	0.350000000E+01 0.23262616E+03
17	10017		3	0.250000000E+01 0.13564490E+03
18	10018		3	0.150000000E+01 0.38663639E+02
19	10019		3	0.11013288E+01 0.000000000E+01
20	10020		3	0.500000000E+00 -0.58317620E+02
21	10021		3	0.000000000E+01 -0.10680825E+03
22	10022		3	0.000000000E+01 -0.10680825E+03
23	10023		3	-0.100000000E+01 -0.15701762E+03
24	10024		3	-0.200000000E+01 -0.20722699E+03
25	10025		3	-0.300000000E+01 -0.25743636E+03
26	10026		3	-0.400000000E+01 -0.25283593E+03
27	10027		3	-0.500000000E+01 -0.23886322E+03
28	10028		3	-0.600000000E+01 -0.22489051E+03
29	10029		3	-0.700000000E+01 -0.21091780E+03
30	10030		3	-0.800000000E+01 -0.19694509E+03
31	10031		3	-0.900000000E+01 -0.18297238E+03
32	10032		3	-0.100000000E+02 -0.17875116E+03
33	10033		3	-0.110000000E+02 -0.17926945E+03
34	10034		3	-0.120000000E+02 -0.17978774E+03
35	10035		3	-0.130000000E+02 -0.18030603E+03
36	10036		3	-0.140000000E+02 -0.18082431E+03
37	10037		3	-0.150000000E+02 -0.18134260E+03
38	10038		3	-0.160000000E+02 -0.18186089E+03
39	10039		3	-0.170000000E+02 -0.18237918E+03
40	10040		3	-0.180000000E+02 -0.18289747E+03
41	10041		3	-0.190000000E+02 -0.18341575E+03
42	10042		3	-0.200000000E+02 -0.18395546E+03
43	10043		3	-0.205000000E+02 -0.18427904E+03
44	10044		3	-0.205000000E+02 -0.18427904E+03
45	10045		3	-0.215000000E+02 -0.18374980E+03
46	10046		3	-0.225000000E+02 -0.21272843E+03
47	10047		3	-0.235000000E+02 -0.24339596E+03
48	10048		3	-0.245000000E+02 -0.27406349E+03
49	10049		3	-0.255000000E+02 -0.30473103E+03
50	10050		3	-0.265000000E+02 -0.33539856E+03
51	10051		3	-0.275000000E+02 -0.36606609E+03
52	10052		3	-0.285000000E+02 -0.39650244E+03
53	10053		3	-0.295000000E+02 -0.41946547E+03
54	10054		3	-0.305000000E+02 -0.45646244E+03
55	10055		3	-0.30851837E+02 -0.47092340E+03
56	10056		3	-0.31887088E+02 0.000000000E+01
57	10057		3	-0.36291152E+02 0.20033575E+04

58 10058 4 -0.36291152E+02 0.00000000E+01
59 10059 0 -0.36291152E+02 0.00000000E+01
60 10060 -0.36291152E+02 0.29802322E-07 0.90545007E+01
EOT..

58	-2.00	CONTN LD	-207.23	LBF/SQ FT
59	-3.00	CONTN LD	-257.44	LBF/SQ FT
60	-4.00	CONTN LD	-252.84	LBF/SQ FT
61	-5.00	CONTN LD	-238.86	LBF/SQ FT
62	-6.00	CONTN LD	-224.89	LBF/SQ FT
63	-7.00	CONTN LD	-210.92	LBF/SQ FT
64	-8.00	CONTN LD	-196.95	LBF/SQ FT
65	-9.00	CONTN LD	-182.97	LBF/SQ FT
66	-10.00	CONTN LD	-178.75	LBF/SQ FT
67	-11.00	CONTN LD	-179.27	LBF/SQ FT
68	-12.00	CONTN LD	-179.79	LBF/SQ FT
69	-13.00	CONTN LD	-180.31	LBF/SQ FT
70	-14.00	CONTN LD	-180.82	LBF/SQ FT
71	-15.00	CONTN LD	-181.34	LBF/SQ FT
72	-16.00	CONTN LD	-181.86	LBF/SQ FT
73	-17.00	CONTN LD	-182.38	LBF/SQ FT
74	-18.00	CONTN LD	-182.90	LBF/SQ FT
75	-19.00	CONTN LD	-183.42	LBF/SQ FT
76	-20.00	CONTN LD	-183.96	LBF/SQ FT
77	-20.50	CONTN LD	-184.28	LBF/SQ FT
78	-20.50	CONTN LD	-184.28	LBF/SQ FT
79	-21.50	CONTN LD	-183.75	LBF/SQ FT
80	-22.50	CONTN LD	-212.73	LBF/SQ FT
81	-23.50	CONTN LD	-243.40	LBF/SQ FT
82	-24.50	CONTN LD	-274.06	LBF/SQ FT
83	-25.50	CONTN LD	-304.73	LBF/SQ FT
84	-26.50	CONTN LD	-335.40	LBF/SQ FT
85	-27.50	CONTN LD	-366.07	LBF/SQ FT
86	-28.50	CONTN LD	-396.50	LBF/SQ FT
87	-29.50	CONTN LD	-419.47	LBF/SQ FT
88	-30.50	CONTN LD	-456.46	LBF/SQ FT
89	-30.85	CONTN LD	-478.92	LBF/SQ FT
90	-31.89	CONTN LD	0.00	LBF/SQ FT
91	-36.29	CONTN LD	2003.36	LBF/SQ FT
92	-36.29	CONTN LD	0.00	LBF/SQ FT

93
94
95
96
97

Z-27 PROPERTIES ARE AS FOLLOWS.

98 MOMENT OF INERTIA= 184.28 IN. TO THE 4TH PER FOOT OF WALL
99 CROSS SECTIONAL AREA= 7.94 SQ IN.
100 ELASTIC MODULUS= 29000000. LBF/SQ IN.
101 DEFLECTION REFERENCE IS AT -36.300

102
103
104 THE MAXIMUM BENDING MOMENT IS 43660.55 LBF-FT AND OCCURS AT -15.31
105 WHICH HAS THE SHEAR FORCE OF 0.06 LBF.

107	DEFLECTION				
108	FROM TANG.				
109	THRU DEFLE				
110	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	DEFLECTION
111	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	(INCHES)
112	14.100	0.0	0.0	0.0	11.4214
113	14.099	0.0	0.0	0.0	11.4214
114	14.000	0.0	0.0	0.0	11.3820
115	13.000	11.2	1.4	2.2	10.9840

116	12.000	80.0	10.1	42.7	10.5860
117	11.000	211.2	26.6	183.1	10.1880
118	10.000	485.0	51.0	486.0	9.7901
119	9.000	661.2	83.3	1013.9	9.3924
120	8.000	988.0	123.4	1829.3	8.9950
121	7.000	1361.2	171.4	2994.7	8.5982
122	6.000	1885.0	227.3	4572.7	8.2024
123	5.000	2294.5	289.0	6623.3	7.8082
124	4.000	2674.4	336.8	9120.1	7.4160
125	3.000	2913.3	366.9	11924.1	7.0269
126	2.000	3049.0	384.0	14913.3	6.6416
127	1.101	3088.1	388.9	17676.8	6.2995
128	1.000	3087.6	388.9	17989.7	6.2612
129	0.000	3029.3	381.5	21056.2	5.8865
130	-1.000	2897.4	364.9	24023.7	5.5187
131	-2.000	2715.3	342.0	26834.2	5.1587
132	-3.000	2482.9	312.7	29437.5	4.8073
133	-4.000	2227.8	288.6	31792.5	4.4654
134	-5.000	1981.9	249.6	33896.2	4.1338
135	-6.000	1758.1	220.4	35761.1	3.8131
136	-7.000	1532.2	193.0	37401.0	3.5040
137	-8.000	1328.2	167.3	38830.1	3.2070
138	-9.000	1138.3	143.4	40062.2	2.9225
139	-10.000	957.4	120.6	41109.6	2.6510
140	-11.000	778.4	98.0	41977.6	2.3928
141	-12.000	598.9	75.4	42666.3	2.1481
142	-13.000	418.0	52.7	43175.2	1.9172
143	-14.000	238.3	30.0	43503.8	1.7003
144	-15.000	57.2	7.2	43651.5	1.4975
145	-15.315	0.1	0.0	43660.6	1.4365
146	-16.000	-124.4	-15.7	43618.0	1.3807
147	-17.000	-306.5	-38.6	43402.5	1.1341
148	-18.000	-489.2	-61.6	43004.7	0.9735
149	-19.000	-672.3	-84.7	42424.0	0.8268
150	-20.000	-856.0	-107.8	41659.9	0.6938
151	-21.000	-1040.2	-131.0	40711.8	0.5743
152	-22.000	-1227.6	-154.6	39579.1	0.4679
153	-23.000	-1440.5	-181.4	38247.5	0.3744
154	-24.000	-1683.9	-212.1	36687.8	0.2932
155	-25.000	-1958.0	-246.6	34869.4	0.2238
156	-26.000	-2262.7	-285.0	32761.6	0.1657
157	-27.000	-2598.1	-327.2	30333.8	0.1182
158	-28.000	-2964.2	-373.3	27555.2	0.0885
159	-29.000	-3359.7	-423.1	24395.5	0.0617
160	-30.000	-3780.9	-476.2	20827.6	0.0387
161	-31.000	-4232.5	-533.1	16821.7	0.0165
162	-31.886	-4411.5	-555.6	12965.7	0.0084
163	-31.888	-4411.5	-555.6	12956.9	0.0084
164	-32.000	-4488.6	-555.2	12463.3	0.0077
165	-33.000	-4129.8	-528.1	8156.2	0.0029
166	-34.000	-3396.1	-427.7	4355.4	0.0008
167	-35.000	-2207.5	-278.0	1515.7	0.0001
168	-36.000	-564.0	-71.0	92.1	0.0000
169	-36.290	-2.0	-0.3	9.1	0.0000
170	-36.292	0.0	0.0	0.0	0.0000
171	-36.299	0.0	0.0	0.0	0.0000
172	-36.300	0.0	0.0	0.0	0.0000

173

174

175

176 *RUN COMPLETED*

177

EOT..

PROJECT: 17th St Outfall Canal
 SUBJECT: STA 553+70 TO STA 568+00

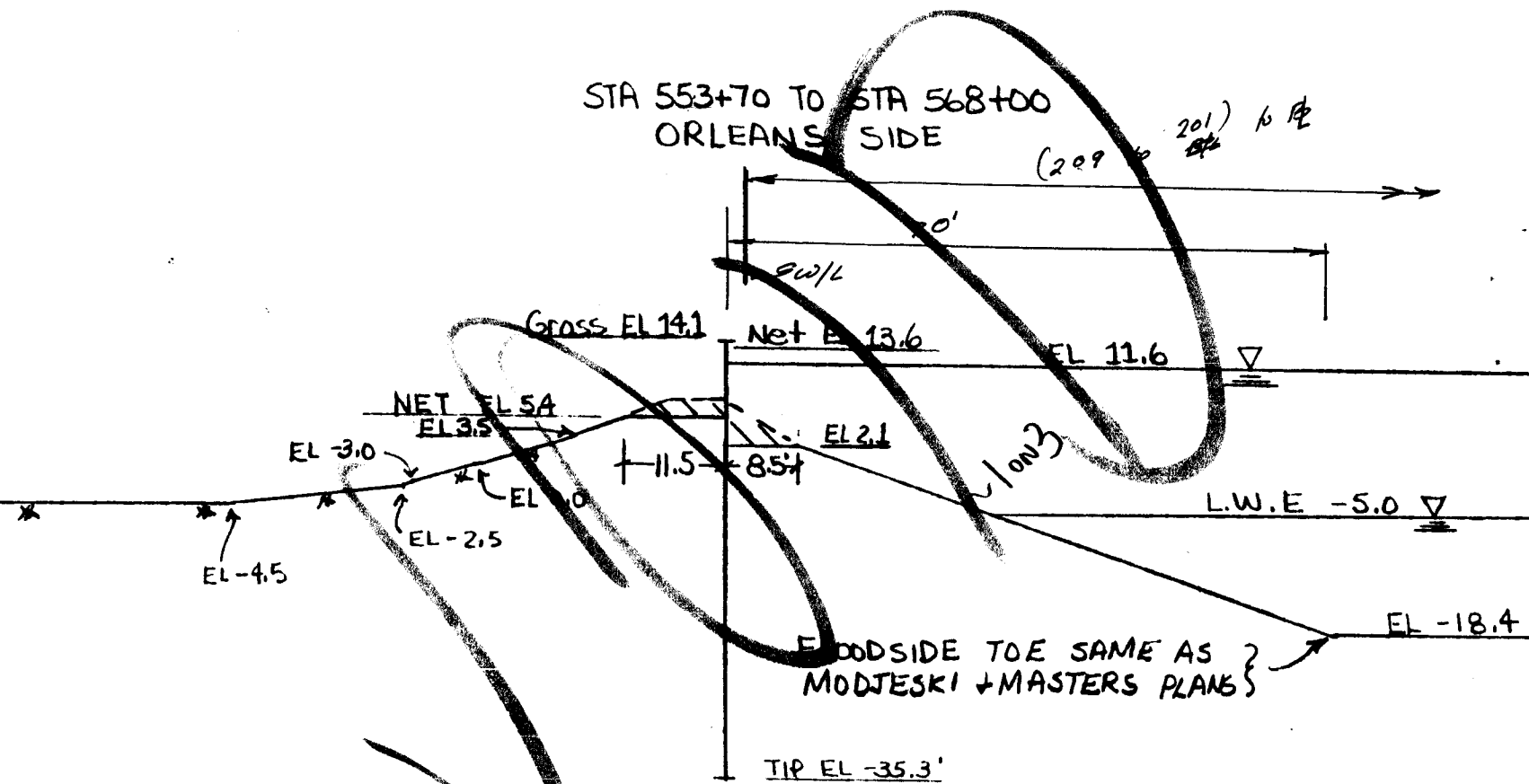
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DATE
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To be run

STA 553+70 TO STA 568+00
 ORLEANS SIDE



SCALE : 1" = 20'

ELEVATION IN FEET N.G.V.D.

Q-Files : Q 554 10 F.S. = 1.5
~~Q 554 10A F.S. = 1.0~~

1	10001	17TH STREET CANAL HLP, FROM STA 553+70 TO STA 568+00		
2	10002	I-WALL TOP EL 13.6 BOTT. EL -35.3 F.S.=1.0		
3	10003		3	0.13600000E+02 0.00000000E+01
4	10004		3	0.12600000E+02 0.62500000E+02
5	10005		3	0.11600000E+02 0.12500000E+03
6	10006		3	0.10600000E+02 0.18750000E+03
7	10007		3	0.96000000E+01 0.25000000E+03
8	10008		3	0.86000000E+01 0.31250000E+03
9	10009		3	0.76000000E+01 0.37500000E+03
10	10010		3	0.66000000E+01 0.43750000E+03
11	10011		3	0.56000000E+01 0.50000000E+03
12	10012		3	0.54000000E+01 0.51250000E+03
13	10013		3	0.54000000E+01 0.51250000E+03
14	10014		3	0.44000000E+01 0.30108524E+03
15	10015		3	0.35000000E+01 0.11081196E+03
16	10016		3	0.35000000E+01 0.11081196E+03
17	10017		3	0.28580218E+01 0.00000000E+01
18	10018		3	0.25000000E+01 -0.61798203E+02
19	10019		3	0.21000000E+01 -0.13084227E+03
20	10020		3	0.21000000E+01 -0.13084227E+03
21	10021		3	0.11000000E+01 -0.28570969E+03
22	10022		3	0.10000000E+00 -0.44057711E+03
23	10023		3	0.00000000E+01 -0.45606385E+03
24	10024		3	0.00000000E+01 -0.45606385E+03
25	10025		3	-0.10000000E+01 -0.53076734E+03
26	10026		3	-0.20000000E+01 -0.60547082E+03
27	10027		3	-0.30000000E+01 -0.59570664E+03
28	10028		3	-0.40000000E+01 -0.57866657E+03
29	10029		3	-0.50000000E+01 -0.56162650E+03
30	10030		3	-0.60000000E+01 -0.54458643E+03
31	10031		3	-0.70000000E+01 -0.52754636E+03
32	10032		3	-0.80000000E+01 -0.51461430E+03
33	10033		3	-0.90000000E+01 -0.51414241E+03
34	10034		3	-0.93444668E+01 -0.51397986E+03
35	10035		3	-0.10735381E+02 0.00000000E+01
36	10036		3	-0.15034919E+02 0.15887934E+04
37	10037		4	-0.15034919E+02 0.00000000E+01
38	10038		0	-0.15034919E+02 0.00000000E+01
39	10039	-0.15034919E+02	0	0.45954168E-01 0.25101522E+02

EOT..

LIST Q55410A0

1
2 BEAMS (SHEAR, MOMENT, DEFLECTION)

3
4
5 17TH STREET CANAL HLP, FROM STA 553+70 TO STA 568+00
6 WAL

7
8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
10 AS COUNTERCLOCKWISE.

11
12
13 THE MAXIMUM DEFLECTION IS 1.68 INCHES AND OCCURS AT MEMBER COORDINATE
14 13.60 FT.

15
16
17
18 Z-27 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19
20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21
22
23 CALCULATED EXTERNAL LOADS

24

25 DISTANCE FROM	26 TYPE OF	27 MAGNITUDE OF
REFERENCE (FT)	LOAD	LOAD
28 -15.03	POINT LD	-0.05 LBF
29 -15.03	COUPLE	-25.11 LBF-FT

30
31 INPUTTED LOADS

34 DISTANCE FROM	35 TYPE OF	36 MAGNITUDE OF
REFERENCE (FT)	LOAD	LOAD
37 13.60	CONTN LD	0.00 LBF/SQ FT
38 12.60	CONTN LD	62.50 LBF/SQ FT
39 11.60	CONTN LD	125.00 LBF/SQ FT
40 10.60	CONTN LD	187.50 LBF/SQ FT
41 9.60	CONTN LD	250.00 LBF/SQ FT
42 8.60	CONTN LD	312.50 LBF/SQ FT
43 7.60	CONTN LD	375.00 LBF/SQ FT
44 6.60	CONTN LD	437.50 LBF/SQ FT
45 5.60	CONTN LD	500.00 LBF/SQ FT
46 5.40	CONTN LD	512.50 LBF/SQ FT
47 5.40	CONTN LD	512.50 LBF/SQ FT
48 4.40	CONTN LD	301.09 LBF/SQ FT
49 3.50	CONTN LD	110.81 LBF/SQ FT
50 3.50	CONTN LD	110.81 LBF/SQ FT
51 2.86	CONTN LD	0.00 LBF/SQ FT
52 2.50	CONTN LD	-61.80 LBF/SQ FT
53 2.10	CONTN LD	-130.84 LBF/SQ FT
54 2.10	CONTN LD	-130.84 LBF/SQ FT
55 1.10	CONTN LD	-285.71 LBF/SQ FT
56 0.10	CONTN LD	-440.58 LBF/SQ FT
57 0.00	CONTN LD	-456.06 LBF/SQ FT

58	0.00	CONTN LD	-456.06	LBF/SQ FT
59	-1.00	CONTN LD	-530.77	LBF/SQ FT
60	-2.00	CONTN LD	-605.47	LBF/SQ FT
61	-3.00	CONTN LD	-595.71	LBF/SQ FT
62	-4.00	CONTN LD	-578.67	LBF/SQ FT
63	-5.00	CONTN LD	-561.63	LBF/SQ FT
64	-6.00	CONTN LD	-544.59	LBF/SQ FT
65	-7.00	CONTN LD	-527.55	LBF/SQ FT
66	-8.00	CONTN LD	-514.61	LBF/SQ FT
67	-9.00	CONTN LD	-514.14	LBF/SQ FT
68	-9.34	CONTN LD	-513.98	LBF/SQ FT
69	-10.74	CONTN LD	0.00	LBF/SQ FT
70	-15.03	CONTN LD	1588.79	LBF/SQ FT
71	-15.03	CONTN LD	0.00	LBF/SQ FT

Z-27 PROPERTIES ARE AS FOLLOWS.

MOMENT OF INERTIA= 184.20 IN. TO THE 4TH PER FOOT OF WALL
 CROSS SECTIONAL AREA= 7.94 SQ IN.
 ELASTIC MODULUS= 29000000. LBF/SQ IN.
 DEFLECTION REFERENCE IS AT -35.300

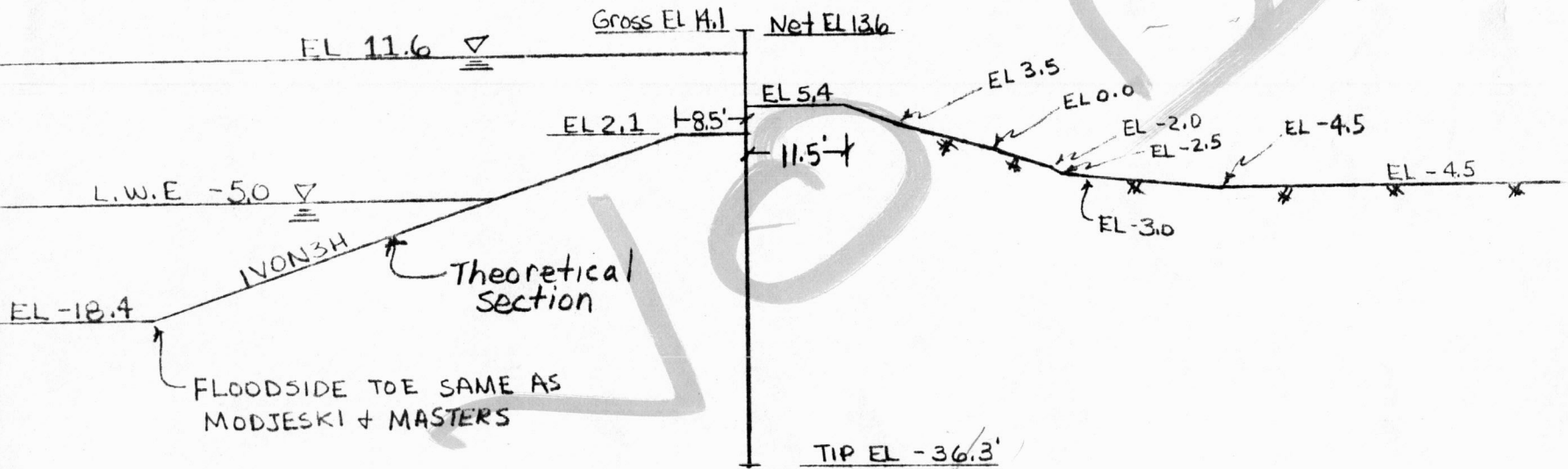
THE MAXIMUM BENDING MOMENT IS 23269.56 LBF-FT AND OCCURS AT -3.68
 WHICH HAS THE SHEAR FORCE OF -1.84 LBF.

89	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	DEFLECTION
90	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	FROM TANG.
91					THRU DEFLE
92					REFERENCE
					(INCHES)
91	13.600	0.0	0.0	0.0	1.6793
92	13.599	0.0	0.0	0.0	1.6793
93	13.000	11.2	1.4	2.2	1.6197
94	12.000	80.0	10.1	42.7	1.5200
95	11.000	211.2	26.6	183.1	1.4204
96	10.000	405.0	51.0	486.0	1.3209
97	9.000	661.2	83.3	1013.9	1.2215
98	8.000	980.0	123.4	1829.3	1.1224
99	7.000	1361.2	171.4	2994.7	1.0240
100	6.000	1805.0	227.3	4572.7	0.9266
101	5.000	2289.3	288.3	6622.7	0.8307
102	4.000	2611.6	328.9	9090.7	0.7369
103	3.000	2727.2	343.5	11776.1	0.6461
104	2.858	2729.0	343.7	12163.5	0.6335
105	2.000	2665.5	335.7	14486.8	0.5591
106	1.000	2441.8	307.5	17053.4	0.4768
107	0.000	2063.1	259.8	19318.7	0.3999
108	-1.000	1569.7	197.7	21141.4	0.3293
109	-2.000	1001.6	126.1	22433.2	0.2656
110	-3.000	401.0	50.5	23133.7	0.2090
111	-3.683	-1.8	-0.2	23269.6	0.1746
112	-4.000	-186.2	-23.4	23239.7	0.1599
113	-5.000	-756.3	-95.3	22767.0	0.1183
114	-6.000	-1309.4	-164.9	21732.7	0.0840
115	-7.000	-1845.5	-232.4	20153.8	0.0567

116	-8.000	-2366.6	-298.1	18046.7	0.0359
117	-9.000	-2881.0	-362.8	15422.9	0.0210
118	-10.000	-3315.6	-417.6	12302.3	0.0109
119	-10.734	-3415.5	-430.2	9818.5	0.0062
120	-10.736	-3415.5	-430.2	9811.7	0.0062
121	-11.000	-3402.6	-428.5	8912.4	0.0049
122	-12.000	-3120.0	-392.9	5620.3	0.0017
123	-13.000	-2467.9	-310.8	2795.6	0.0004
124	-14.000	-1446.3	-182.2	807.6	0.0000
125	-15.000	-55.2	-7.0	26.1	0.0000
126	-15.034	-1.5	-0.2	25.1	0.0000
127	-15.036	0.0	0.0	0.0	0.0000
128	-16.000	0.0	0.0	0.0	0.0000
129	-17.000	0.0	0.0	0.0	0.0000
130	-18.000	0.0	0.0	0.0	0.0000
131	-19.000	0.0	0.0	0.0	0.0000
132	-20.000	0.0	0.0	0.0	0.0000
133	-21.000	0.0	0.0	0.0	0.0000
134	-22.000	0.0	0.0	0.0	0.0000
135	-23.000	0.0	0.0	0.0	0.0000
136	-24.000	0.0	0.0	0.0	0.0000
137	-25.000	0.0	0.0	0.0	0.0000
138	-26.000	0.0	0.0	0.0	0.0000
139	-27.000	0.0	0.0	0.0	0.0000
140	-28.000	0.0	0.0	0.0	0.0000
141	-29.000	0.0	0.0	0.0	0.0000
142	-30.000	0.0	0.0	0.0	0.0000
143	-31.000	0.0	0.0	0.0	0.0000
144	-32.000	0.0	0.0	0.0	0.0000
145	-33.000	0.0	0.0	0.0	0.0000
146	-34.000	0.0	0.0	0.0	0.0000
147	-35.000	0.0	0.0	0.0	0.0000
148	-35.299	0.0	0.0	0.0	0.0000
149	-35.300	0.0	0.0	0.0	0.0000
150					
151					
152					
153	*RUN COMPLETED*				
154					
EOT..					

PROJECT	17th St Outfall Canal	PAGE	OF	COMPUTED BY	DATE
SUBJECT	STA 554+00 TO STA 568+00 ORLEANS			FJV	
				CHECKED BY	DATE

STA 554 TO STA 568+00
ORLEANS SIDE



SCALE : 1" = 20'

ELEVATION IN FEET N.G.M.D

TOP OF SAND EL -36.5

Q-FILES : Q554-10 F.S. = 1.5

Q55410A F.S. = 1.0

Revised
4/21

LIST Q55410A

1	10001	17TH STREET CANAL HLP STA 554+00 TO SAT 568+00		
2	10002	I-WALL TOP EL. 14.1 BOTT EL -36.3 F.S.=1.00		
3	10003		3	0.136000000E+02 0.000000000E+01
4	10004		3	0.126000000E+02 0.625000000E+02
	10005		3	0.116000000E+02 0.125000000E+03
o	10006		3	0.106000000E+02 0.187500000E+03
7	10007		3	0.960000000E+01 0.250000000E+03
8	10008		3	0.860000000E+01 0.312500000E+03
9	10009		3	0.760000000E+01 0.375000000E+03
10	10010		3	0.660000000E+01 0.437500000E+03
11	10011		3	0.560000000E+01 0.500000000E+03
12	10012		3	0.540000000E+01 0.512500000E+03
13	10013		3	0.540000000E+01 0.512500000E+03
14	10014		3	0.440000000E+01 0.30108524E+03
15	10015		3	0.350000000E+01 0.11081196E+03
16	10016		3	0.350000000E+01 0.11081196E+03
17	10017		3	0.27924406E+01 0.000000000E+01
18	10018		3	0.250000000E+01 -0.45799571E+02
19	10019		3	0.150000000E+01 -0.20241111E+03
20	10020		3	0.500000000E+00 -0.35902264E+03
21	10021		3	0.000000000E+01 -0.43732841E+03
22	10022		3	0.000000000E+01 -0.43732841E+03
23	10023		3	-0.100000000E+01 -0.51377600E+03
24	10024		3	-0.200000000E+01 -0.59022360E+03
25	10025		3	-0.300000000E+01 -0.58220354E+03
26	10026		3	-0.400000000E+01 -0.56690758E+03
27	10027		3	-0.500000000E+01 -0.55161163E+03
28	10028		3	-0.600000000E+01 -0.53631567E+03
29	10029		3	-0.700000000E+01 -0.52101972E+03
30	10030		3	-0.800000000E+01 -0.51010934E+03
31	10031		3	-0.900000000E+01 -0.51250105E+03
32	10032		3	-0.97228925E+01 -0.51423000E+03
33	10033		3	-0.11095682E+02 0.000000000E+01
34	10034		3	-0.15387120E+02 0.16075191E+04
35	10035		4	-0.15387120E+02 0.000000000E+01
36	10036		0	-0.15387120E+02 0.000000000E+01
37	10037	-0.15387120E+02	0.12144107E+00	0.40808202E+02

EOT..

LIST Q55410AO

1
2
3
4

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH STREET CANAL HLP STA 554+00 TO SAT 560+00
WAL

5
6
7

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

11

12

THE MAXIMUM DEFLECTION IS 1.81 INCHES AND OCCURS AT MEMBER COORDINATE
14.10 FT.

15

16

17

Z-27 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21

22

23

CALCULATED EXTERNAL LOADS

24

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
-15.39	POINT LD	-0.12 LBF
-15.39	COUPLE	-40.01 LBF-FT

27

28

29

30

31

INPUTTED LOADS

32

33

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
13.60	CONTN LD	0.00 LBF/SQ FT
12.60	CONTN LD	62.50 LBF/SQ FT
11.60	CONTN LD	125.00 LBF/SQ FT
10.60	CONTN LD	187.50 LBF/SQ FT
9.60	CONTN LD	250.00 LBF/SQ FT
8.60	CONTN LD	312.50 LBF/SQ FT
7.60	CONTN LD	375.00 LBF/SQ FT
6.60	CONTN LD	437.50 LBF/SQ FT
5.60	CONTN LD	500.00 LBF/SQ FT
5.40	CONTN LD	512.50 LBF/SQ FT
5.40	CONTN LD	512.50 LBF/SQ FT
4.40	CONTN LD	301.09 LBF/SQ FT
3.50	CONTN LD	110.01 LBF/SQ FT
3.50	CONTN LD	110.01 LBF/SQ FT
2.79	CONTN LD	0.00 LBF/SQ FT
2.50	CONTN LD	-45.00 LBF/SQ FT
1.50	CONTN LD	-202.41 LBF/SQ FT
0.50	CONTN LD	-359.02 LBF/SQ FT
0.00	CONTN LD	-437.33 LBF/SQ FT
0.00	CONTN LD	-437.33 LBF/SQ FT
-1.00	CONTN LD	-513.70 LBF/SQ FT

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58	-2.00	CONTN LD	-590.22	LBF/SQ FT
59	-3.00	CONTN LD	-582.20	LBF/SQ FT
60	-4.00	CONTN LD	-566.91	LBF/SQ FT
61	-5.00	CONTN LD	-551.61	LBF/SQ FT
62	-6.00	CONTN LD	-536.32	LBF/SQ FT
63	-7.00	CONTN LD	-521.02	LBF/SQ FT
64	-8.00	CONTN LD	-510.11	LBF/SQ FT
65	-9.00	CONTN LD	-512.50	LBF/SQ FT
66	-9.72	CONTN LD	-514.23	LBF/SQ FT
67	-11.10	CONTN LD	0.00	LBF/SQ FT
68	-15.39	CONTN LD	1607.52	LBF/SQ FT
69	-15.39	CONTN LD	0.00	LBF/SQ FT

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72 Z-27 PROPERTIES ARE AS FOLLOWS.

73
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75 MOMENT OF INERTIA= 184.20 IN. TO THE 4TH PER FOOT OF WALL
76 CROSS SECTIONAL AREA= 7.94 SQ IN.
77 ELASTIC MODULUS= 29000000. LBF/SQ IN.
78 DEFLECTION REFERENCE IS AT -36.300

79
80
81 THE MAXIMUM BENDING MOMENT IS 23697.28 LBF-FT AND OCCURS AT -3.88
82 WHICH HAS THE SHEAR FORCE OF -0.78 LBF.

87	DISTANCE (FEET)	SHEAR FOR (LBF)	SHEAR STR (LBF/SQIN)	BENDING MOM (LBF-FT)	DEFLECTION FROM TANG. THRU DEFLE REFERENCE (INCHES)
89	14.100	0.0	0.0	0.0	1.0124
90	14.099	0.0	0.0	0.0	1.0124
91	14.000	0.0	0.0	0.0	1.0022
92	13.000	11.2	1.4	2.2	1.6989
93	12.000	80.0	10.1	42.7	1.5957
94	11.000	211.2	26.6	183.1	1.4925
95	10.000	405.0	51.0	486.0	1.3893
96	9.000	661.2	83.3	1013.9	1.2863
97	8.000	0.0	0.0	0.0000	0.0000
128	-17.000	0.0	0.0	0.0	0.0000
02	0.00	.00.			
11	.0 .0				
100	.0				
1320	0.				
13	0				
1060	0.000 .00 0				
11					
101	4.000	2611.6	328.9	9090.7	0.7837
102	3.000	2729.2	343.7	11776.5	0.6893
103	2.792	2732.6	344.2	12343.4	0.6701
104	2.000	2683.4	338.0	14495.8	0.5987
105	1.000	2481.0	312.5	17091.1	0.5127
106	0.000	2122.0	267.3	19405.7	0.4323
107	-1.000	1646.4	207.4	21296.2	0.3582
108	-2.000	1094.4	137.8	22673.1	0.2909
109	-3.000	508.2	64.0	23473.7	0.2309

110	-3.885	-0.8	-0.1	23697.3	0.1841
111	-4.000	-66.3	-8.4	23693.4	0.1784
112	-5.000	-625.6	-78.8	23346.2	0.1336
113	-6.000	-1169.6	-147.3	22447.3	0.0964
114	-7.000	-1698.2	-213.9	21012.2	0.0663
115	-8.000	-2213.8	-278.8	19855.3	0.0431

116	-9.000	-2725.1	-343.2	16586.8	0.0259
117	-10.000	-3224.3	-406.1	13605.6	0.0141
118	-11.000	-3447.4	-434.2	10238.5	0.0067
119	-11.095	-3449.2	-434.4	9912.8	0.0062
120	-11.097	-3449.2	-434.4	9905.1	0.0062
121	-12.000	-3296.8	-415.1	6835.6	0.0026
122	-13.000	-2770.8	-348.9	3771.4	0.0007
123	-14.000	-1869.3	-235.4	1420.5	0.0001
124	-15.000	-594.1	-74.8	157.6	0.0000
125	-15.386	-1.5	-0.2	40.8	0.0000
126	-15.388	0.0	0.0	0.0	0.0000
127	-16.000	0.0	0.0	0.0	0.0000
128	-17.000	0.0	0.0	0.0	0.0000

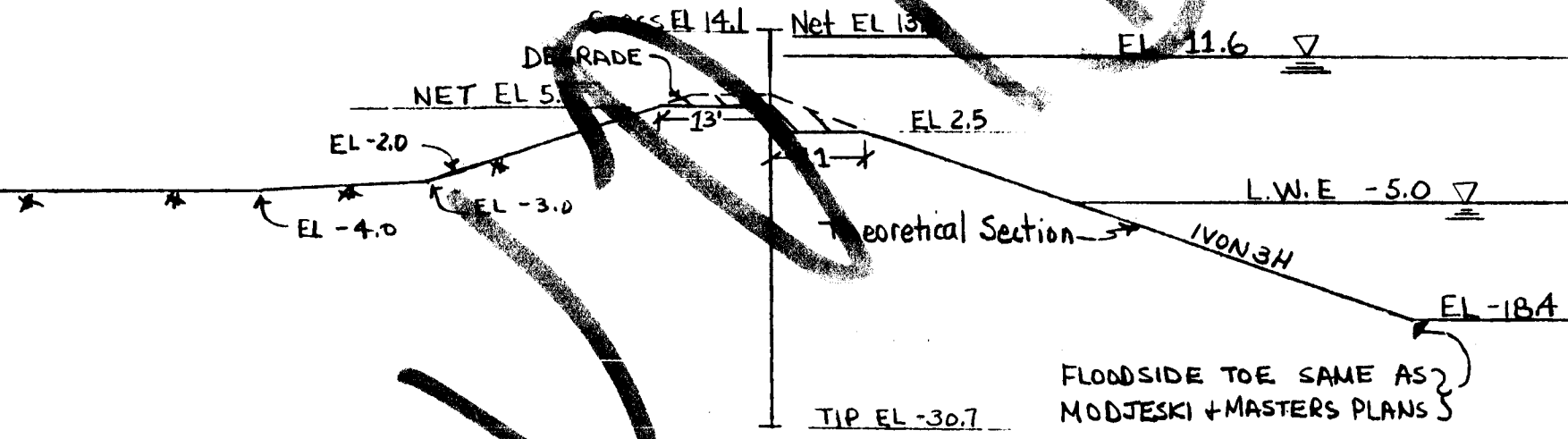
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11.. 0 . 0
1 0 0 . 0
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1320 0.
13 0
1060 r20000 . .00 0

139	-28.000	0.0	0.0	0.0	0.0000
140	-29.000	0.0	0.0	0.0	0.0000
141	-30.000	0.0	0.0	0.0	0.0000
142	-31.000	0.0	0.0	0.0	0.0000
143	-32.000	0.0	0.0	0.0	0.0000
144	-33.000	0.0	0.0	0.0	0.0000
145	-34.000	0.0	0.0	0.0	0.0000
146	-35.000	0.0	0.0	0.0	0.0000
147	-36.000	0.0	0.0	0.0	0.0000
148	-36.299	0.0	0.0	0.0	0.0000
149	-36.300	0.0	0.0	0.0	0.0000

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153 *RUN COMPLETED*
154
EOT..

PROJECT 17th St Outfall Canal	PAGE OF	COMPUTED BY FSV	DATE 4/24/87
SUBJECT STA 568+00 TO STA 589+00	CHECKED BY	DATE	DATE

STA 568+00 TO STA 589+00
ORLEANS SIDE



SCALE 1" = 20'
 ELEVATION IN FEET NGVD
 Q Files 8 Q 589 IO FS = 1.5
 Q 589 IOA FS = 1.0

(Enc 19

LIST 058910

1 10001 17TH STREET CANAL HLP STA 568+00 TO STA 589+00

2 10002 I-WALL TOP EL. 13.6 BOTT. EL. -30.7 F.S.=1.5

3	10003	3	0.13600000E+02	0.00000000E+01
4	10004	3	0.12600000E+02	0.62500000E+02
	10005	3	0.11600000E+02	0.12500000E+03
6	10006	3	0.10600000E+02	0.18750000E+03
7	10007	3	0.96000000E+01	0.25000000E+03
8	10008	3	0.86000000E+01	0.31250000E+03
9	10009	3	0.76000000E+01	0.37500000E+03
10	10010	3	0.66000000E+01	0.43750000E+03
11	10011	3	0.56000000E+01	0.50000000E+03
12	10012	3	0.55000000E+01	0.50625000E+03
13	10013	3	0.55000000E+01	0.50625000E+03
14	10014	3	0.45000000E+01	0.35894798E+03
15	10015	3	0.35000000E+01	0.21164595E+03
16	10016	3	0.35000000E+01	0.21164595E+03
17	10017	3	0.25000000E+01	0.94065884E+02
18	10018	3	0.25000000E+01	0.94065884E+02
19	10019	3	0.15037016E+01	0.00000000E+01
20	10020	3	0.15000000E+01	-0.34949055E+00
21	10021	3	0.50000000E+00	-0.94764865E+02
22	10022	3	0.00000000E+01	-0.14197255E+03
23	10023	3	0.00000000E+01	-0.14197255E+03
24	10024	3	-0.10000000E+01	-0.18961604E+03
25	10025	3	-0.20000000E+01	-0.23725953E+03
26	10026	3	-0.30000000E+01	-0.28490301E+03
27	10027	3	-0.40000000E+01	-0.33254650E+03
28	10028	3	-0.50000000E+01	-0.33466467E+03
29	10029	3	-0.60000000E+01	-0.31418586E+03
30	10030	3	-0.70000000E+01	-0.29370706E+03
31	10031	3	-0.80000000E+01	-0.27322825E+03
32	10032	3	-0.90000000E+01	-0.25274945E+03
33	10033	3	-0.10000000E+02	-0.23227065E+03
34	10034	3	-0.11000000E+02	-0.21842180E+03
35	10035	3	-0.12000000E+02	-0.21097566E+03
36	10036	3	-0.13000000E+02	-0.20798467E+03
37	10037	3	-0.14000000E+02	-0.20578687E+03
38	10038	3	-0.15000000E+02	-0.20544107E+03
39	10039	3	-0.16000000E+02	-0.20509526E+03
40	10040	3	-0.17000000E+02	-0.20474946E+03
41	10041	3	-0.18000000E+02	-0.20440365E+03
42	10042	3	-0.19000000E+02	-0.20405785E+03
43	10043	3	-0.20000000E+02	-0.20371204E+03
44	10044	3	-0.20500000E+02	-0.20353914E+03
45	10045	3	-0.20500000E+02	-0.20353914E+03
46	10046	3	-0.21500000E+02	-0.20201696E+03
47	10047	3	-0.22500000E+02	-0.20637010E+03
48	10048	3	-0.23500000E+02	-0.23583970E+03
49	10049	3	-0.24500000E+02	-0.26530853E+03
50	10050	3	-0.25500000E+02	-0.29475797E+03
51	10051	3	-0.26229139E+02	-0.31623070E+03
52	10052	3	-0.26880619E+02	0.00000000E+01
53	10053	3	-0.30683518E+02	0.18459400E+04
54	10054	4	-0.30683518E+02	0.00000000E+01
55	10055	0	-0.30683518E+02	0.00000000E+01
56	10056		-0.30683518E+02	0.14901161E-06
				0.83978972E+01

EOT..

LIST Q589100

1
2 BEAMS (SHEAR, MOMENT, DEFLECTION)
3

4
5 17TH STREET CANAL HLP STA 568+00 TO STA 589+00
6 WAL

7
8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
10 AS COUNTERCLOCKWISE.

11
12
13 THE MAXIMUM DEFLECTION IS 6.99 INCHES AND OCCURS AT MEMBER COORDINATE
14 13.60 FT.

15
16
17
18 Z-27 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19
20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21
22
23 CALCULATED EXTERNAL LOADS

24
25 DISTANCE FROM TYPE OF MAGNITUDE OF
26 REFERENCE (FT) LOAD LOAD
27
28 -30.68 POINT LD 0.00 LBF
29 -30.68 COUPLE -8.40 LBF-FT

30
31
32 INPUTTED LOADS

33
34 DISTANCE FROM TYPE OF MAGNITUDE OF
35 REFERENCE (FT) LOAD LOAD
36
37 13.60 CONTN LD 0.00 LBF/SQ FT
38 12.60 CONTN LD 62.50 LBF/SQ FT
39 11.60 CONTN LD 125.00 LBF/SQ FT
40 10.60 CONTN LD 187.50 LBF/SQ FT
41 9.60 CONTN LD 250.00 LBF/SQ FT
42 8.60 CONTN LD 312.50 LBF/SQ FT
43 7.60 CONTN LD 375.00 LBF/SQ FT
44 6.60 CONTN LD 437.50 LBF/SQ FT
45 5.60 CONTN LD 500.00 LBF/SQ FT
46 5.50 CONTN LD 506.25 LBF/SQ FT

47 5.50 CONTN LD 506.25 LBF/SQ FT
48 4.50 CONTN LD 358.95 LBF/SQ FT
49 3.50 CONTN LD 211.65 LBF/SQ FT
50 3.50 CONTN LD 211.65 LBF/SQ FT
51 2.50 CONTN LD 94.07 LBF/SQ FT
52 2.50 CONTN LD 94.07 LBF/SQ FT
53 1.50 CONTN LD 0.00 LBF/SQ FT
54 1.50 CONTN LD -0.35 LBF/SQ FT
55 0.50 CONTN LD -94.76 LBF/SQ FT
56 0.00 CONTN LD -141.97 LBF/SQ FT
57 0.00 CONTN LD -141.97 LBF/SQ FT

58	-1.00	CONTN LD	-189.62	LBF/SQ FT
59	-2.00	CONTN LD	-237.26	LBF/SQ FT
60	-3.00	CONTN LD	-284.90	LBF/SQ FT
61	-4.00	CONTN LD	-332.55	LBF/SQ FT
62	-5.00	CONTN LD	-334.66	LBF/SQ FT
	-6.00	CONTN LD	-314.19	LBF/SQ FT
64	-7.00	CONTN LD	-293.71	LBF/SQ FT
65	-8.00	CONTN LD	-273.23	LBF/SQ FT
66	-9.00	CONTN LD	-252.75	LBF/SQ FT
67	-10.00	CONTN LD	-232.27	LBF/SQ FT
68	-11.00	CONTN LD	-218.42	LBF/SQ FT
69	-12.00	CONTN LD	-210.98	LBF/SQ FT
70	-13.00	CONTN LD	-207.98	LBF/SQ FT
71	-14.00	CONTN LD	-205.79	LBF/SQ FT
72	-15.00	CONTN LD	-205.44	LBF/SQ FT
73	-16.00	CONTN LD	-205.10	LBF/SQ FT
74	-17.00	CONTN LD	-204.75	LBF/SQ FT
75	-18.00	CONTN LD	-204.40	LBF/SQ FT
76	-19.00	CONTN LD	-204.06	LBF/SQ FT
77	-20.00	CONTN LD	-203.71	LBF/SQ FT
78	-20.50	CONTN LD	-203.54	LBF/SQ FT
79	-20.50	CONTN LD	-203.54	LBF/SQ FT
80	-21.50	CONTN LD	-202.02	LBF/SQ FT
81	-22.50	CONTN LD	-206.37	LBF/SQ FT
82	-23.50	CONTN LD	-235.84	LBF/SQ FT
83	-24.50	CONTN LD	-265.31	LBF/SQ FT
84	-25.50	CONTN LD	-294.76	LBF/SQ FT
85	-26.23	CONTN LD	-316.23	LBF/SQ FT
86	-26.88	CONTN LD	0.00	LBF/SQ FT
87	-30.68	CONTN LD	1845.94	LBF/SQ FT
8	-30.68	CONTN LD	0.00	LBF/SQ FT

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Z-27 PROPERTIES ARE AS FOLLOWS.

93
94 MOMENT OF INERTIA= 184.20 IN. TO THE 4TH PER FOOT OF WALL
95 CROSS SECTIONAL AREA= 7.94 SQ IN.
96 ELASTIC MODULUS= 29000000. LBF/SQ IN.
97 DEFLECTION REFERENCE IS AT -30.700

98
99
100 THE MAXIMUM BENDING MOMENT IS 36396.76 LBF-FT AND OCCURS AT -10.72
101 WHICH HAS THE SHEAR FORCE OF -1.41 LBF.

					DEFLECTION FROM TANG. THRU DEFLE REFERENCE
106	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	
107	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	(INCHES)
108	13.600	0.0	0.0	0.0	6.9871
109	13.599	0.0	0.0	0.0	6.9871
110	13.000	11.2	1.4	2.2	6.8184
111	12.000	80.0	10.1	42.7	6.5369
112	11.000	211.2	26.6	183.1	6.2554
113	10.000	405.0	51.0	486.0	5.9740
114	9.000	661.2	83.3	1013.9	5.6927
115	8.000	980.0	123.4	1829.3	5.4118

116	7.000	1361.2	171.4	2994.7	5.1315
117	6.000	1805.0	227.3	4572.7	4.8522
118	5.000	2285.0	287.8	6621.2	4.5744
119	4.000	2644.0	333.0	9098.0	4.2987
120	3.000	2859.3	360.1	11860.7	4.0260
121	2.000	2956.3	372.3	14777.3	3.7572
122	1.504	2967.9	373.8	16248.4	3.6255
123	1.000	2955.9	372.3	17741.3	3.4931
124	0.000	2861.2	360.4	20657.7	3.2348
125	-1.000	2695.4	339.5	23440.0	2.9831
126	-2.000	2481.9	312.6	26032.6	2.7391
127	-3.000	2220.9	279.7	28388.0	2.5034
128	-4.000	1912.1	240.8	30458.5	2.2769
129	-5.000	1578.5	198.8	32204.0	2.0602
130	-6.000	1254.1	157.9	33618.6	1.8540
131	-7.000	950.2	119.7	34719.1	1.6585
132	-8.000	666.7	84.0	35525.8	1.4743
133	-9.000	403.7	50.8	36059.3	1.3016
134	-10.000	161.2	20.3	36340.0	1.1406
135	-10.715	-1.4	-0.2	36396.8	1.0326
136	-11.000	-64.1	-8.1	36387.4	0.9912
137	-12.000	-278.8	-35.1	36215.3	0.8537
138	-13.000	-488.3	-61.5	35831.4	0.7278
139	-14.000	-695.2	-87.6	35239.5	0.6136
140	-15.000	-900.8	-113.5	34441.4	0.5107
141	-16.000	-1106.1	-139.3	33438.0	0.4189
142	-17.000	-1311.0	-165.1	32229.4	0.3380
143	-18.000	-1515.6	-190.9	30816.0	0.2674
144	-19.000	-1719.8	-216.6	29198.3	0.2068
145	-20.000	-1923.7	-242.3	27376.5	0.1557
146	-21.000	-2127.1	-267.9	25351.0	0.1134
147	-22.000	-2329.9	-293.4	23122.7	0.0793
148	-23.000	-2539.4	-319.8	20689.5	0.0526
149	-24.000	-2775.2	-349.5	18034.6	0.0327
150	-25.000	-3040.5	-382.9	15129.2	0.0185
151	-26.000	-3335.3	-420.1	11943.8	0.0093
152	-26.880	-3510.0	-442.1	8910.6	0.0044
153	-26.882	-3510.0	-442.1	8903.6	0.0044
154	-27.000	-3506.5	-441.6	8488.2	0.0039
155	-28.000	-3205.9	-403.8	5091.6	0.0012
156	-29.000	-2419.8	-304.8	2238.3	0.0002
157	-30.000	-1148.3	-144.6	413.8	0.0000
158	-30.683	-1.8	-0.2	8.4	0.0000
159	-30.685	0.0	0.0	0.0	0.0000
160	-30.699	0.0	0.0	0.0	0.0000
161	-30.700	0.0	0.0	0.0	0.0000

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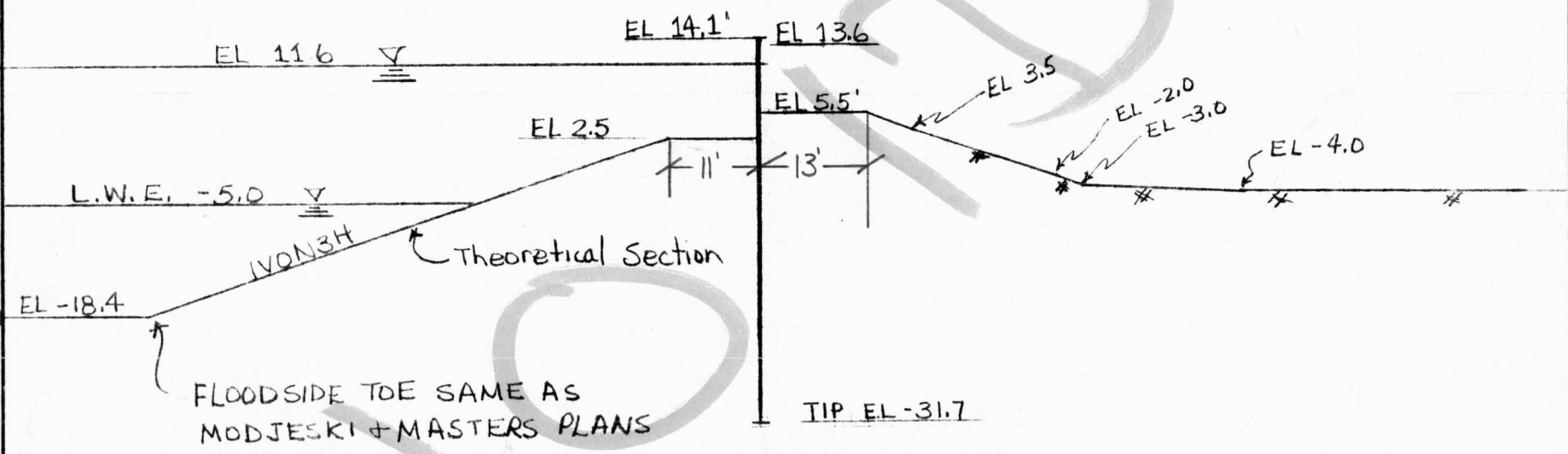
165 *RUN COMPLETED*

166

EOT..

PROJECT	17th St Outfall Canal	PAGE	OF	COMPUTED BY	DATE
SUBJECT	STA 568+00 TO STA 589+00			CHECKED BY	DATE

STA 568+00 TO STA 589+00
ORLEANS SIDE



SCALE: 1" = 20'
 ELEVATION IN FEET N.G.V.D
 TOP OF SAND EL -34.0
 Q Files 8 Q589I0 F.S. = 1.5
 Q589I0A F.S. = 1.0

Review
4/21

LIST Q58910

Station	Description	Value 1	Value 2
1	10001 17TH STREE CANAL HLP STA 568+00 TO STA 589+00		
2	10002 I-WALL TOP EL. 14.1 BOTT EL. -31.7 S.S.=1.5		
3	10003	3 0.136000000E+02	0.000000000E+01
4	10004	3 0.126000000E+02	0.625000000E+02
5	10005	3 0.116000000E+02	0.125000000E+03
6	10006	3 0.106000000E+02	0.187500000E+03
7	10007	3 0.960000000E+01	0.250000000E+03
8	10008	3 0.860000000E+01	0.312500000E+03
9	10009	3 0.760000000E+01	0.375000000E+03
10	10010	3 0.660000000E+01	0.437500000E+03
11	10011	3 0.560000000E+01	0.500000000E+03
12	10012	3 0.550000000E+01	0.506250000E+03
13	10013	3 0.550000000E+01	0.506250000E+03
14	10014	3 0.450000000E+01	0.35894798E+03
15	10015	3 0.350000000E+01	0.21164595E+03
16	10016	3 0.350000000E+01	0.21164595E+03
17	10017	3 0.250000000E+01	0.11574039E+03
18	10018	3 0.150000000E+01	0.19834821E+02
19	10019	3 0.12931838E+01	0.000000000E+01
20	10020	3 0.500000000E+00	-0.76070746E+02
21	10021	3 0.000000000E+01	-0.12402353E+03
22	10022	3 0.000000000E+01	-0.12402353E+03
23	10023	3 -0.100000000E+01	-0.17315721E+03
24	10024	3 -0.200000000E+01	-0.22229089E+03
25	10025	3 -0.300000000E+01	-0.27142457E+03
26	10026	3 -0.400000000E+01	-0.32055825E+03
27	10027	3 -0.500000000E+01	-0.32416660E+03
28	10028	3 -0.600000000E+01	-0.30517799E+03
29	10029	3 -0.700000000E+01	-0.28618938E+03
30	10030	3 -0.800000000E+01	-0.26720077E+03
31	10031	3 -0.900000000E+01	-0.24821216E+03
32	10032	3 -0.100000000E+02	-0.22922355E+03
33	10033	3 -0.110000000E+02	-0.21776152E+03
34	10034	3 -0.120000000E+02	-0.21356812E+03
35	10035	3 -0.130000000E+02	-0.21240639E+03
36	10036	3 -0.140000000E+02	-0.21138675E+03
37	10037	3 -0.150000000E+02	-0.21036710E+03
38	10038	3 -0.160000000E+02	-0.20934746E+03
39	10039	3 -0.170000000E+02	-0.20832782E+03
40	10040	3 -0.180000000E+02	-0.20730818E+03
41	10041	3 -0.190000000E+02	-0.20628854E+03
42	10042	3 -0.200000000E+02	-0.20526889E+03
43	10043	3 -0.205000000E+02	-0.20475907E+03
44	10044	3 -0.205000000E+02	-0.20475907E+03
45	10045	3 -0.215000000E+02	-0.20256305E+03
46	10046	3 -0.225000000E+02	-0.20637010E+03
47	10047	3 -0.235000000E+02	-0.23583970E+03
48	10048	3 -0.245000000E+02	-0.26530853E+03
49	10049	3 -0.255000000E+02	-0.29475797E+03
50	10050	3 -0.265000000E+02	-0.32420742E+03
51	10051	3 -0.27051466E+02	-0.34044778E+03
52	10052	3 -0.27757218E+02	0.000000000E+01
53	10053	3 -0.31654092E+02	0.18798117E+04
54	10054	4 -0.31654092E+02	0.000000000E+01
55	10055	0 -0.31654092E+02	0.000000000E+01
56	10056 -0.31654092E+02	0.29802322E-07	0.12715392E+02

EOT..

LIST Q589100

1
2 BEAMS (SHEAR, MOMENT, DEFLECTION)
3
4

17TH STREE CANAL HLP STA 568+00 TO STA 589+00
WAL

8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
10 AS COUNTERCLOCKWISE.

13 THE MAXIMUM DEFLECTION IS 7.81 INCHES AND OCCURS AT MEMBER COORDINATE
14 14.18 FT.

18 Z-27 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

23 CALCULATED EXTERNAL LOADS

24	25	26	27
DISTANCE FROM	TYPE OF	MAGNITUDE OF	
REFERENCE (FT)	LOAD	LOAD	
28 -31.65	POINT LD	8.88 LBF	
29 -31.65	COUPLE	-12.72 LBF-FT	

32 INPUTTED LOADS

34	35	36	37
DISTANCE FROM	TYPE OF	MAGNITUDE OF	
REFERENCE (FT)	LOAD	LOAD	
37 13.60	CONTN LD	8.88 LBF/SQ FT	
38 12.60	CONTN LD	62.50 LBF/SQ FT	
39 11.60	CONTN LD	125.00 LBF/SQ FT	
40 10.60	CONTN LD	187.50 LBF/SQ FT	
41 9.60	CONTN LD	250.00 LBF/SQ FT	
42 8.60	CONTN LD	312.50 LBF/SQ FT	
43 7.60	CONTN LD	375.00 LBF/SQ FT	
44 6.60	CONTN LD	437.50 LBF/SQ FT	
45 5.60	CONTN LD	500.00 LBF/SQ FT	
46 5.50	CONTN LD	506.25 LBF/SQ FT	
47 5.50	CONTN LD	506.25 LBF/SQ FT	
48 4.50	CONTN LD	350.95 LBF/SQ FT	
49 3.50	CONTN LD	211.65 LBF/SQ FT	
50 3.50	CONTN LD	211.65 LBF/SQ FT	
51 2.50	CONTN LD	115.74 LBF/SQ FT	
52 1.50	CONTN LD	19.83 LBF/SQ FT	
53 1.29	CONTN LD	8.88 LBF/SQ FT	
54 0.50	CONTN LD	-76.87 LBF/SQ FT	
55 0.80	CONTN LD	-124.82 LBF/SQ FT	
56 0.80	CONTN LD	-124.82 LBF/SQ FT	
57 -1.80	CONTN LD	-173.16 LBF/SQ FT	

58	-2.00	CONTN LD	-222.29	LBF/SQ FT
59	-3.00	CONTN LD	-271.42	LBF/SQ FT
60	-4.00	CONTN LD	-320.56	LBF/SQ FT
61	-5.00	CONTN LD	-324.17	LBF/SQ FT
62	-6.00	CONTN LD	-305.10	LBF/SQ FT
63	-7.00	CONTN LD	-286.19	LBF/SQ FT
64	-8.00	CONTN LD	-267.20	LBF/SQ FT
65	-9.00	CONTN LD	-248.21	LBF/SQ FT
66	-10.00	CONTN LD	-229.22	LBF/SQ FT
67	-11.00	CONTN LD	-217.76	LBF/SQ FT
68	-12.00	CONTN LD	-213.57	LBF/SQ FT
69	-13.00	CONTN LD	-212.41	LBF/SQ FT
70	-14.00	CONTN LD	-211.39	LBF/SQ FT
71	-15.00	CONTN LD	-210.37	LBF/SQ FT
72	-16.00	CONTN LD	-209.35	LBF/SQ FT
73	-17.00	CONTN LD	-208.33	LBF/SQ FT
74	-18.00	CONTN LD	-207.31	LBF/SQ FT
75	-19.00	CONTN LD	-206.29	LBF/SQ FT
76	-20.00	CONTN LD	-205.27	LBF/SQ FT
77	-20.50	CONTN LD	-204.76	LBF/SQ FT
78	-20.50	CONTN LD	-204.76	LBF/SQ FT
79	-21.50	CONTN LD	-202.56	LBF/SQ FT
80	-22.50	CONTN LD	-200.37	LBF/SQ FT
81	-23.50	CONTN LD	-235.04	LBF/SQ FT
82	-24.50	CONTN LD	-265.31	LBF/SQ FT
83	-25.50	CONTN LD	-294.76	LBF/SQ FT
84	-26.50	CONTN LD	-324.21	LBF/SQ FT
85	-27.00	CONTN LD	-340.45	LBF/SQ FT
86	-27.76	CONTN LD	0.00	LBF/SQ FT
87	-31.65	CONTN LD	1079.81	LBF/SQ FT
88	-31.65	CONTN LD	0.00	LBF/SQ FT

91 Z-27

PROPERTIES ARE AS FOLLOWS.

94 MOMENT OF INERTIA= 104.20 IN. TO THE 4TH PER FOOT OF WALL
 95 CROSS SECTIONAL AREA= 7.94 SQ IN.
 96 ELASTIC MODULUS= 29000000. LBF/SQ IN.
 97 DEFLECTION REFERENCE IS AT -31.700

100 THE MAXIMUM BENDING MOMENT IS 37925.64 LBF-FT AND OCCURS AT -11.40
 101 WHICH HAS THE SHEAR FORCE OF -0.52 LBF.

DISTANCE (FEET)	SHEAR FOR (LBF)	SHEAR STR (LBF/SQIN)	BENDING MOM (LBF-FT)	DEFLECTION FROM TANG. THRU DEFLE REFERENCE (INCHES)
108 14.100	0.0	0.0	0.0	7.0001
109 14.099	0.0	0.0	0.0	7.0001
110 14.000	0.0	0.0	0.0	7.7782
111 13.000	11.2	1.4	2.2	7.4767
112 12.000	80.0	10.1	42.7	7.1752
113 11.000	211.2	26.6	183.1	6.8737
114 10.000	405.0	51.0	406.0	6.5723
115 9.000	661.2	83.3	1013.9	6.2710

116	8.000	980.0	123.4	1829.3	5.9701
117	7.000	1361.2	171.4	2994.7	5.6698
118	6.000	1805.0	227.3	4572.7	5.3705
119	5.000	2285.0	287.8	6621.2	5.0727
120	4.000	2644.0	333.0	9098.0	4.7770
121	3.000	2862.0	368.5	11861.1	4.4844
122	2.000	2977.8	375.0	14789.0	4.1955
123	1.293	3001.7	378.1	16905.1	3.9942
124	1.000	2997.6	377.5	17704.7	3.9115
125	0.000	2921.5	368.0	20752.3	3.6332
126	-1.000	2773.0	349.2	23603.6	3.3616
127	-2.000	2575.2	324.3	26281.8	3.0976
128	-3.000	2328.4	293.2	28737.7	2.8421
129	-4.000	2032.4	256.0	30922.2	2.5959
130	-5.000	1710.0	215.4	32793.7	2.3597
131	-6.000	1395.3	175.7	34344.8	2.1341
132	-7.000	1099.7	138.5	35598.7	1.9196
133	-8.000	823.0	103.6	36550.5	1.7166
134	-9.000	565.3	71.2	37243.0	1.5253
135	-10.000	326.5	41.1	37687.3	1.3462
136	-11.000	103.1	13.0	37901.2	1.1792
137	-11.478	-0.5	-0.1	37925.6	1.1037
138	-12.000	-112.6	-14.2	37896.1	1.0244
139	-13.000	-325.6	-41.0	37676.9	0.8819
140	-14.000	-537.5	-67.7	37245.2	0.7516
141	-15.000	-748.4	-94.3	36602.2	0.6333
142	-16.000	-958.2	-120.7	35748.8	0.5268
143	-17.000	-1167.1	-147.0	34686.1	0.4319
144	-18.000	-1374.9	-173.2	33415.0	0.3482
145	-19.000	-1581.7	-199.2	31936.7	0.2753
146	-20.000	-1787.5	-225.1	30252.0	0.2127
147	-21.000	-1992.1	-250.9	28362.1	0.1599
148	-22.000	-2195.4	-276.5	26268.4	0.1163
149	-23.000	-2405.0	-302.9	23969.6	0.0811
150	-24.000	-2640.8	-332.6	21449.2	0.0537
151	-25.000	-2906.1	-366.0	18678.2	0.0332
152	-26.000	-3200.9	-403.1	15627.2	0.0188
153	-27.000	-3525.1	-444.0	12266.7	0.0093
154	-27.756	-3662.7	-461.3	9531.0	0.0049
155	-27.758	-3662.7	-461.3	9524.4	0.0049
156	-28.000	-3648.5	-459.5	8640.0	0.0039
157	-29.000	-3298.2	-414.4	5130.5	0.0012
158	-30.000	-2449.5	-308.5	2220.5	0.0002
159	-31.000	-1126.4	-141.9	392.3	0.0000
160	-31.653	-1.9	-0.2	12.7	0.0000
161	-31.655	0.0	0.0	0.0	0.0000
162	-31.699	0.0	0.0	0.0	0.0000
163	-31.700	0.0	0.0	0.0	0.0000
164					
165					
166					
167	*RUN COMPLETED*				
168					

EDT..

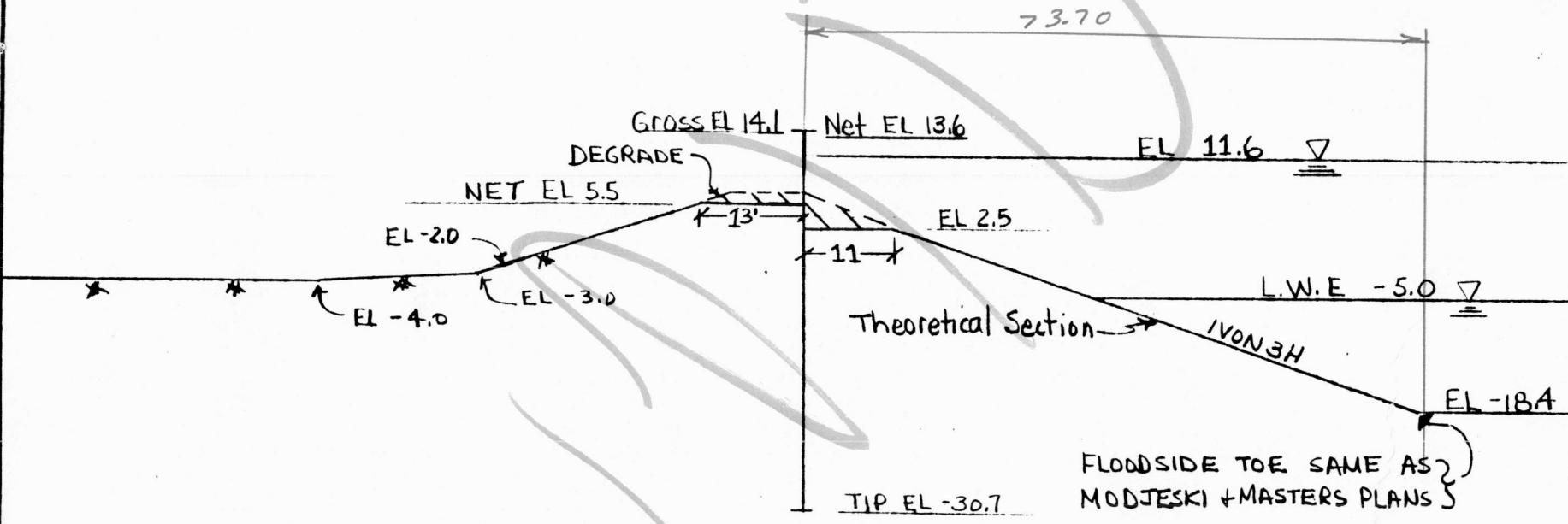
PROJECT 17th St Outfall Canal
 SUBJECT STA 568+00 TO STA 589+00

COMPUTED BY FSJ
 CHECKED BY

DATE 4/24/87
 DATE

PAGE OF

STA 568+00 TO STA 589+00
 ORLEANS SIDE



SCALE 1" = 20'
 ELEVATION IN FEET NGVD
 Q Files : Q 589 I O F.S.=1.5
 Q 589 I O A F.S.=1.0

Enc 19

LIST 058910A

1 10001 17TH STREET CANAL HLP FROM STA 568+00 TO STA 589+00

2 10002 I-WALL TOP EL. 13.6 BOTT EL. -30.7 F.S. = 1.00

3 10003 3 0.13600000E+02 0.00000000E+01

A 10004 3 0.12600000E+02 0.62500000E+02

10005 3 0.11600000E+02 0.12500000E+03

6 10006 3 0.10600000E+02 0.18750000E+03

7 10007 3 0.96000000E+01 0.25000000E+03

8 10008 3 0.86000000E+01 0.31250000E+03

9 10009 3 0.76000000E+01 0.37500000E+03

10 10010 3 0.66000000E+01 0.43750000E+03

11 10011 3 0.56000000E+01 0.50000000E+03

12 10012 3 0.55000000E+01 0.50625000E+03

13 10013 3 0.55000000E+01 0.50625000E+03

14 10014 3 0.45000000E+01 0.29483524E+03

15 10015 3 0.35000000E+01 0.83420487E+02

16 10016 3 0.35000000E+01 0.83420487E+02

17 10017 3 0.30167116E+01 0.00000000E+01

18 10018 3 0.25000000E+01 -0.89189679E+02

19 10019 3 0.25000000E+01 -0.89189679E+02

20 10020 3 0.15000000E+01 -0.24405710E+03

21 10021 3 0.50000000E+00 -0.39892452E+03

22 10022 3 0.00000000E+01 -0.47635823E+03

23 10023 3 0.00000000E+01 -0.47635823E+03

24 10024 3 -0.10000000E+01 -0.55106171E+03

25 10025 3 -0.20000000E+01 -0.62576520E+03

26 10026 3 -0.30000000E+01 -0.70046868E+03

27 10027 3 -0.40000000E+01 -0.68819657E+03

28 10028 3 -0.50000000E+01 -0.66577355E+03

29 10029 3 -0.60000000E+01 -0.64335052E+03

30 10030 3 -0.70000000E+01 -0.62092749E+03

31 10031 3 -0.77201276E+01 -0.60478005E+03

32 10032 3 -0.94119342E+01 0.00000000E+01

33 10033 3 -0.13803914E+02 0.15700268E+04

34 10034 4 -0.13803914E+02 0.00000000E+01

35 10035 0 -0.13803914E+02 0.00000000E+01

36 10036 -0.13803914E+02 0.17488388E+01 -0.10979499E+03

EOT..

LIST Q589IOAO

1
2
3
4

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH STREET CANAL HLP FROM STA 568+00 TO STA 589+00
WAL

6
7

8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
10 AS COUNTERCLOCKWISE.

11

12

13 THE MAXIMUM DEFLECTION IS 1.44 INCHES AND OCCURS AT MEMBER COORDINATE
14 13.60 FT.

15

16

17

18 Z-27 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19

20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21

22

23

CALCULATED EXTERNAL LOADS

24

25 DISTANCE FROM TYPE OF MAGNITUDE OF
26 REFERENCE(FT) LOAD LOAD

27

28 -13.80 POINT LD -1.75 LBF
29 -13.80 COUPLE 109.79 LBF-FT

31

32

INPUTTED LOADS

33

34 DISTANCE FROM TYPE OF MAGNITUDE OF
35 REFERENCE(FT) LOAD LOAD

36

37 13.60 CONTN LD 0.00 LBF/SQ FT

38 12.60 CONTN LD 62.50 LBF/SQ FT

39 11.60 CONTN LD 125.00 LBF/SQ FT

40 10.60 CONTN LD 187.50 LBF/SQ FT

41 9.60 CONTN LD 250.00 LBF/SQ FT

42 8.60 CONTN LD 312.50 LBF/SQ FT

43 7.60 CONTN LD 375.00 LBF/SQ FT

44 6.60 CONTN LD 437.50 LBF/SQ FT

45 5.60 CONTN LD 500.00 LBF/SQ FT

46 5.50 CONTN LD 506.25 LBF/SQ FT

47 5.50 CONTN LD 506.25 LBF/SQ FT

48 4.50 CONTN LD 294.84 LBF/SQ FT

49 3.50 CONTN LD 83.42 LBF/SQ FT

50 3.50 CONTN LD 83.42 LBF/SQ FT

51 3.02 CONTN LD 0.00 LBF/SQ FT

52 2.50 CONTN LD -89.19 LBF/SQ FT

53 2.50 CONTN LD -89.19 LBF/SQ FT

54 1.50 CONTN LD -244.06 LBF/SQ FT

55 0.50 CONTN LD -398.92 LBF/SQ FT

56 0.00 CONTN LD -476.36 LBF/SQ FT

57 0.00 CONTN LD -476.36 LBF/SQ FT

58	-1.00	CONTN LD	-551.06	LBF/SQ FT
59	-2.00	CONTN LD	-625.77	LBF/SQ FT
60	-3.00	CONTN LD	-700.47	LBF/SQ FT
61	-4.00	CONTN LD	-688.20	LBF/SQ FT
62	-5.00	CONTN LD	-665.77	LBF/SQ FT
63	-6.00	CONTN LD	-643.35	LBF/SQ FT
64	-7.00	CONTN LD	-620.93	LBF/SQ FT
65	-7.72	CONTN LD	-604.78	LBF/SQ FT
66	-9.41	CONTN LD	0.00	LBF/SQ FT
67	-13.80	CONTN LD	1570.03	LBF/SQ FT
68	-13.80	CONTN LD	0.00	LBF/SQ FT
69				

70
71 Z-27 PROPERTIES ARE AS FOLLOWS.

72
73
74 MOMENT OF INERTIA= 184.20 IN. TO THE 4TH PER FOOT OF WALL
75 CROSS SECTIONAL AREA= 7.94 SQ IN.
76 ELASTIC MODULUS= 29000000. LBF/SQ IN.
77 DEFLECTION REFERENCE IS AT -30.700

78
79
80 THE MAXIMUM BENDING MOMENT IS 22260.89 LBF-FT AND OCCURS AT -3.24
81 WHICH HAS THE SHEAR FORCE OF -1.11 LBF.

82					DEFLECTION
83					FROM TANG.
84					THRU DEFLE
85					REFERENCE
86	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	(INCHES
	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT))
87	13.600	0.0	0.0	0.0	1.4400
89	13.599	0.0	0.0	0.0	1.4400
90	13.000	11.2	1.4	2.2	1.3867
91	12.000	80.0	10.1	42.7	1.2979
92	11.000	211.2	26.6	183.1	1.2091
93	10.000	405.0	51.0	486.0	1.1203
94	9.000	661.2	83.3	1013.9	1.0317
95	8.000	980.0	123.4	1829.3	0.9435
96	7.000	1361.2	171.4	2994.7	0.8558
97	6.000	1805.0	227.3	4572.7	0.7692
98	5.000	2277.0	286.8	6619.9	0.6840
99	4.000	2571.8	323.9	9061.9	0.6011
100	3.017	2660.1	335.0	11649.5	0.5223
101	3.000	2660.1	335.0	11693.9	0.5210
102	2.000	2573.1	324.1	14324.2	0.4448
103	1.000	2329.1	293.3	16788.2	0.3731
104	0.000	1930.2	243.1	18930.7	0.3069
105	-1.000	1416.5	178.4	20610.3	0.2468
106	-2.000	828.0	104.3	21738.7	0.1933
107	-3.000	164.9	20.8	22241.4	0.1468
108	-3.238	-1.1	-0.1	22260.9	0.1368
109	-4.000	-529.4	-66.7	22058.2	0.1075
110	-5.000	-1206.4	-151.9	21188.4	0.0753
111	-6.000	-1861.0	-234.4	19652.9	0.0499
112	-7.000	-2493.1	-314.0	17474.0	0.0308
113	-8.000	-3089.7	-389.1	14675.4	0.0174
114	-9.000	-3415.7	-430.2	11392.9	0.0086
115	-9.411	-3446.0	-434.0	9981.0	0.0062

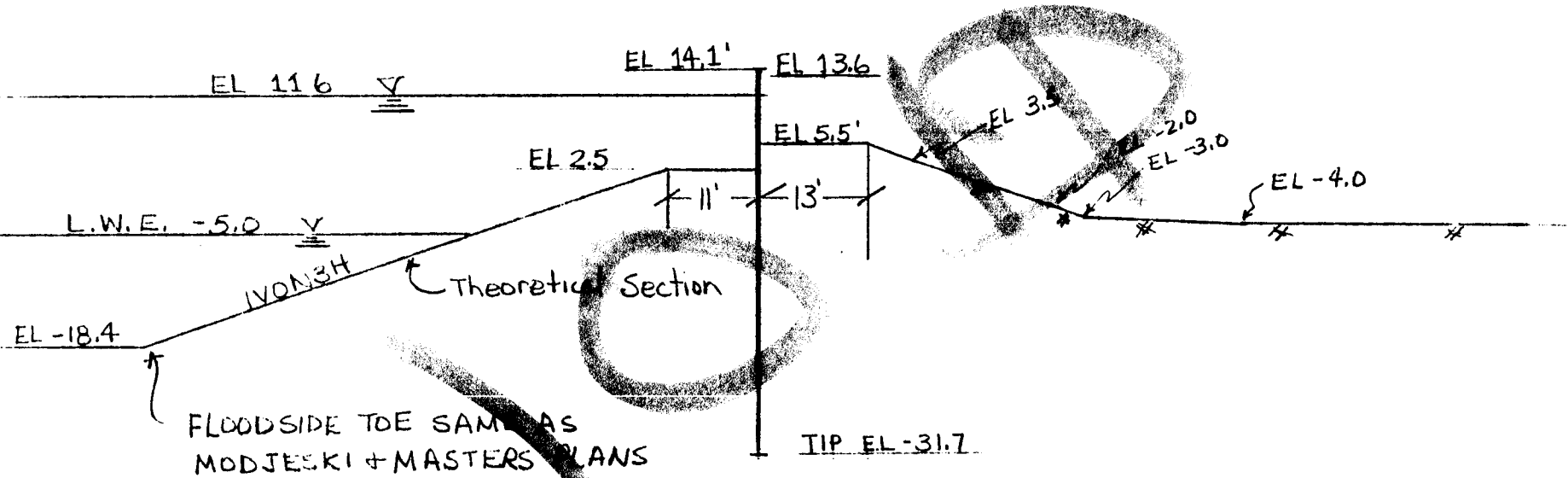
116	-9.413	-3446.0	-434.0	9974.1	0.0062
117	-10.000	-3384.2	-426.2	7963.2	0.0036
118	-11.000	-2995.2	-377.2	4743.7	0.0011
119	-12.000	-2248.8	-283.2	2091.8	0.0002
120	-13.000	-1144.9	-144.2	365.2	0.0000
121	-13.803	0.2	0.0	-109.8	0.0000
122	-13.805	0.0	0.0	0.0	0.0000
123	-14.000	0.0	0.0	0.0	0.0000
124	-15.000	0.0	0.0	0.0	0.0000
125	-16.000	0.0	0.0	0.0	0.0000
126	-17.000	0.0	0.0	0.0	0.0000
127	-18.000	0.0	0.0	0.0	0.0000
128	-19.000	0.0	0.0	0.0	0.0000
129	-20.000	0.0	0.0	0.0	0.0000
130	-21.000	0.0	0.0	0.0	0.0000
131	-22.000	0.0	0.0	0.0	0.0000
132	-23.000	0.0	0.0	0.0	0.0000
133	-24.000	0.0	0.0	0.0	0.0000
134	-25.000	0.0	0.0	0.0	0.0000
135	-26.000	0.0	0.0	0.0	0.0000
136	-27.000	0.0	0.0	0.0	0.0000
137	-28.000	0.0	0.0	0.0	0.0000
138	-29.000	0.0	0.0	0.0	0.0000
139	-30.000	0.0	0.0	0.0	0.0000
140	-30.699	0.0	0.0	0.0	0.0000
141	-30.700	0.0	0.0	0.0	0.0000

142
143
144
145
146
EOT..

RUN COMPLETED

PROJECT	THA S - Cuthall Canal	COMPUTED BY	DATE
SUBJECT	STA 568+00 TO STA 589+00	CHECKED BY	DATE
	PAGE	OF	

STA 568+00 TO STA 589+00
ORLEANS SIDE



SCALE 1" = 20'
ELEVATION IN FEET N.G.V.D
TOP OF SAND EL -34.0
Q Files 8 Q58910 F.S. = 1.5

20
4/91

LIST

1	10001	17TH STREET CANAL HLP STA. 568+00 TO STA 569+00		
2	10002	I-WALL TOP EL.14.1 BOTT EL -31.7		
3	10003		3	0.13600000E+02 0.00000000E+01
4	10004		3	0.12600000E+02 0.62000000E+02
5	10005		3	0.11600000E+02 0.12500000E+03
6	10006		3	0.10600000E+02 0.10700000E+03
7	10007		3	0.96000000E+01 0.20000000E+03
8	10008		3	0.86000000E+01 0.31200000E+03
9	10009		3	0.76000000E+01 0.37500000E+03
10	10010		3	0.66000000E+01 0.43750000E+03
11	10011		3	0.56000000E+01 0.50000000E+03
12	10012		3	0.55000000E+01 0.50625000E+03
13	10013		3	0.55000000E+01 0.50625000E+03
14	10014		3	0.45000000E+01 0.29403024E+03
15	10015		3	0.35000000E+01 0.83420407E+02
16	10016		3	0.35000000E+01 0.83420407E+02
17	10017		3	0.29400225E+01 0.00000000E+01
18	10018		3	0.25000000E+01 -0.72453946E+02
19	10019		3	0.15000000E+01 -0.22832030E+03
20	10020		3	0.50000000E+00 -0.30420201E+03
21	10021		3	0.00000000E+01 -0.46214003E+03
22	10022		3	0.00000000E+01 -0.46214003E+03
23	10023		3	-0.10000000E+01 -0.53705052E+03
24	10024		3	-0.20000000E+01 -0.61356102E+03
25	10025		3	-0.30000000E+01 -0.60927152E+03
26	10026		3	-0.40000000E+01 -0.67000642E+03
27	10027		3	-0.50000000E+01 -0.65659041E+03
28	10028		3	-0.60000000E+01 -0.63517440E+03
29	10029		3	-0.70000000E+01 -0.61375030E+03
30	10030		3	-0.79093070E+01 -0.59257137E+03
31	10031		3	-0.96327560E+01 0.00000000E+01
32	10032		3	-0.14012017E+02 0.15792990E+04
33	10033		4	-0.14012017E+02 0.00000000E+01
34	10034		0	-0.14012017E+02 0.00000000E+01
35	10035	-0.14012017E+02	0.30092462E+01	0.31200940E+02

EOT..

LIST Q589IOAD

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BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH STREET CANAL MLP STA. 560+00 TO STA 589+00

WAL

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 1.54 INCHES AND OCCURS AT MEMBER COORDINATE
14.10 FT.

Z-27 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE(FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
-14.01	POINT LD	-3.89 LBF
-14.01	COUPLE	-31.21 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE(FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
13.60	CONTN LD	0.00 LBF/80 FT
12.60	CONTN LD	62.50 LBF/80 FT
11.60	CONTN LD	125.00 LBF/80 FT
10.60	CONTN LD	187.50 LBF/80 FT
9.60	CONTN LD	250.00 LBF/80 FT
8.60	CONTN LD	312.50 LBF/80 FT
7.60	CONTN LD	375.00 LBF/80 FT
6.60	CONTN LD	437.50 LBF/80 FT
5.60	CONTN LD	500.00 LBF/80 FT
5.50	CONTN LD	506.25 LBF/80 FT
5.50	CONTN LD	506.25 LBF/80 FT
4.50	CONTN LD	294.04 LBF/80 FT
3.50	CONTN LD	83.42 LBF/80 FT
3.50	CONTN LD	83.42 LBF/80 FT
2.96	CONTN LD	0.00 LBF/80 FT
2.50	CONTN LD	-72.45 LBF/80 FT
1.50	CONTN LD	-228.33 LBF/80 FT
0.50	CONTN LD	-384.20 LBF/80 FT
0.00	CONTN LD	-462.14 LBF/80 FT
0.00	CONTN LD	-462.14 LBF/80 FT
-1.00	CONTN LD	-537.05 LBF/80 FT

58	-2.00	CONTN LD	-613.56	LBF/SQ FT
59	-3.00	CONTN LD	-689.27	LBF/SQ FT
60	-4.00	CONTN LD	-678.01	LBF/SQ FT
61	-5.00	CONTN LD	-656.59	LBF/SQ FT
62	-6.00	CONTN LD	-635.17	LBF/SQ FT
63	-7.00	CONTN LD	-613.76	LBF/SQ FT
64	-7.99	CONTN LD	-592.57	LBF/SQ FT
65	-9.63	CONTN LD	0.00	LBF/SQ FT
66	-14.01	CONTN LD	1579.30	LBF/SQ FT
67	-14.01	CONTN LD	0.00	LBF/SQ FT
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69				

70 Z-27 PROPERTIES ARE AS FOLLOWS.

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73 MOMENT OF INERTIA= 184.20 IN. TO THE 4TH PER FOOT OF WALL
74 CROSS SECTIONAL AREA= 7.94 SQ IN.
75 ELASTIC MODULUS= 29000000. LBF/SQ IN.
76 DEFLECTION REFERENCE IS AT -31.700

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79 THE MAXIMUM BENDING MOMENT IS 22568.93 LBF-FT AND OCCURS AT -3.37
80 WHICH HAS THE SHEAR FORCE OF -1.31 LBF.

85	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	DEFLECTION
86	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	FROM TANG. THRU DEFLE REFERENCE (INCHES)
87	14.100	0.0	0.0	0.0	1.5374
88	14.099	0.0	0.0	0.0	1.5374
89	14.000	0.0	0.0	0.0	1.5283
90	13.000	11.2	1.4	2.2	1.4371
91	12.000	88.0	10.1	42.7	1.3459
92	11.000	211.2	26.6	183.1	1.2547
93	10.000	485.0	51.0	486.0	1.1635
94	9.000	661.2	83.3	1013.9	1.0725
95	8.000	988.0	123.4	1829.3	0.9819
96	7.000	1361.2	171.4	2994.7	0.8919
97	6.000	1885.0	227.3	4572.7	0.8029
98	5.000	2277.0	286.0	6619.9	0.7153
99	4.000	2571.8	323.9	9061.9	0.6308
100	3.000	2662.2	335.3	11694.3	0.5476
101	2.965	2662.3	335.3	11787.9	0.5447
102	2.000	2589.8	326.2	14333.2	0.4689
103	1.000	2361.4	297.4	16821.0	0.3949
104	0.000	1977.2	249.0	19884.1	0.3263
105	-1.000	1477.2	186.0	20737.7	0.2639
106	-2.000	981.5	113.5	21933.3	0.2081
107	-3.000	258.1	31.5	22515.5	0.1594
108	-3.366	-1.3	-0.2	22568.9	0.1434
109	-4.000	-433.5	-54.6	22422.8	0.1179
110	-5.000	-1188.8	-138.6	21653.8	0.0837
111	-6.000	-1746.7	-228.0	20228.3	0.0564
112	-7.000	-2371.2	-298.6	18167.6	0.0356
113	-8.000	-2974.2	-374.6	15493.1	0.0287
114	-9.000	-3382.6	-426.0	12284.6	0.0188
115	-9.632	-3454.8	-435.1	10117.2	0.0066

116	-9.634	-3454.8	-435.1	10110.3	0.0066
117	-10.000	-3430.5	-432.1	8848.0	0.0048
118	-11.000	-3117.8	-392.7	5543.8	0.0017
119	-12.000	-2444.5	-307.9	2732.5	0.0004
120	-13.000	-1410.7	-177.7	774.9	0.0000
121	-14.000	-16.3	-2.1	31.3	0.0000
122	-14.012	2.3	0.3	31.2	0.0000
123	-14.014	0.0	0.0	0.0	0.0000
124	-15.000	0.0	0.0	0.0	0.0000
125	-16.000	0.0	0.0	0.0	0.0000
126	-17.000	0.0	0.0	0.0	0.0000
127	-18.000	0.0	0.0	0.0	0.0000
128	-19.000	0.0	0.0	0.0	0.0000
129	-20.000	0.0	0.0	0.0	0.0000
130	-21.000	0.0	0.0	0.0	0.0000
131	-22.000	0.0	0.0	0.0	0.0000
132	-23.000	0.0	0.0	0.0	0.0000
133	-24.000	0.0	0.0	0.0	0.0000
134	-25.000	0.0	0.0	0.0	0.0000
135	-26.000	0.0	0.0	0.0	0.0000
136	-27.000	0.0	0.0	0.0	0.0000
137	-28.000	0.0	0.0	0.0	0.0000
138	-29.000	0.0	0.0	0.0	0.0000
139	-30.000	0.0	0.0	0.0	0.0000
140	-31.000	0.0	0.0	0.0	0.0000
141	-31.699	0.0	0.0	0.0	0.0000
142	-31.700	0.0	0.0	0.0	0.0000

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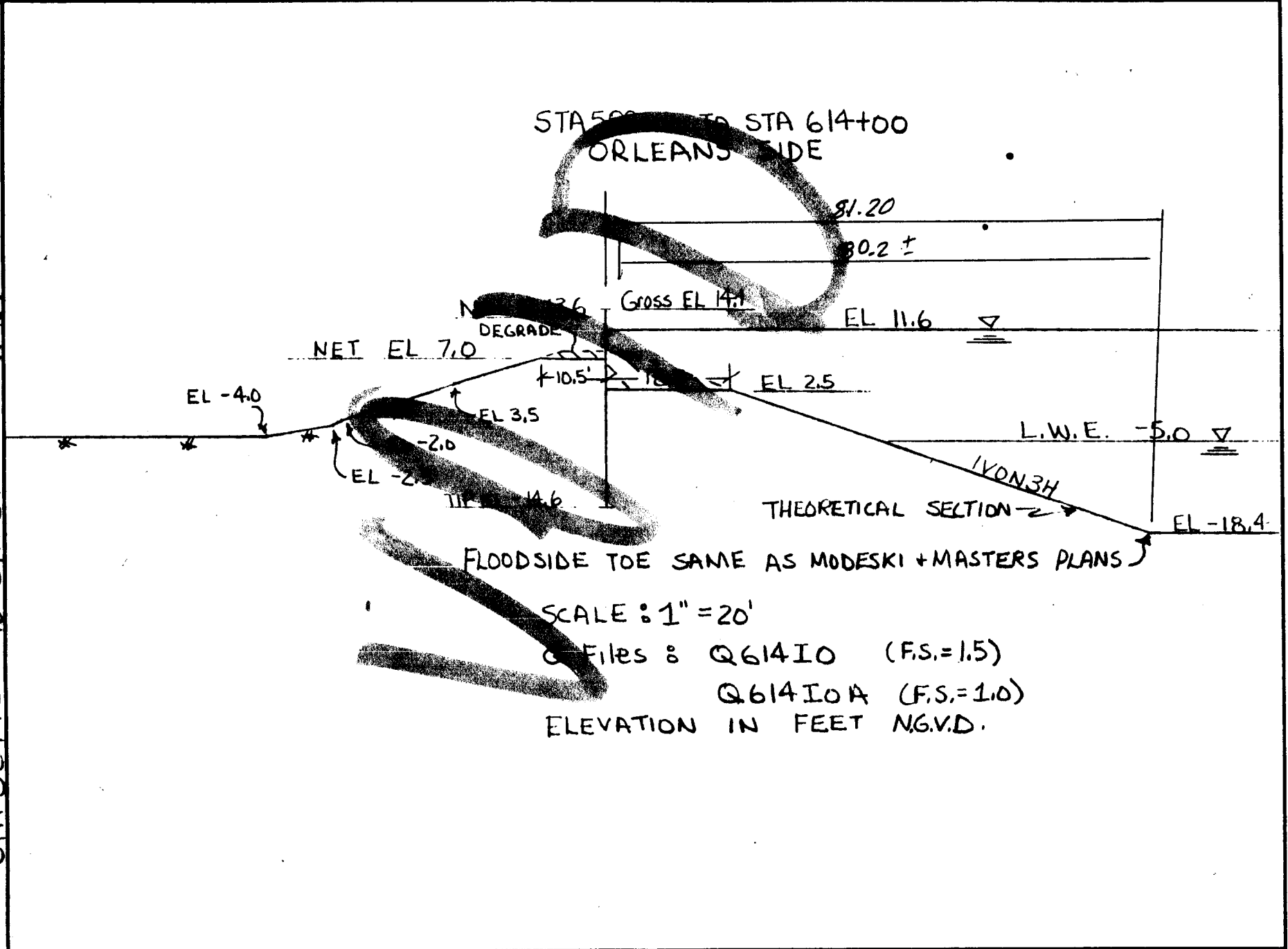
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146 *RUN COMPLETED*

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EOT..

Waiting...

PROJECT 17th St Outfall Canal
 SUBJECT STA 589+00 To STA 614+00 Orleans
 DATE 2/24/87
 COMPUTED BY FJK
 CHECKED BY
 DATE



LMV FORM 107e MAR 82
 PREVIOUS EDITIONS MAY BE USED
 (FOR USE WITH 10 x 10 GRID)
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Enc 10

LIST 061410

1 10001 17TH STREET CANAL FROM STA 589+00 TO STA 614+00
2 10002 I-WALL TOP EL. 13.6 BOTT. EL. -14.6 F.S.=1.5
3 10003 3 0.70000000E+01 0.00000000E+01
4 10004 3 0.60000000E+01 0.68636101E+02
5 10005 3 0.50000000E+01 0.13727220E+03
6 10006 3 0.40000000E+01 0.20590830E+03
7 10007 3 0.35000000E+01 0.24022635E+03
8 10008 3 0.35000000E+01 0.24022635E+03
9 10009 3 0.25000000E+01 0.29913901E+03
10 10010 3 0.25000000E+01 0.29913901E+03
11 10011 3 0.15000000E+01 0.17797150E+03
12 10012 3 0.50000000E+00 0.56804000E+02
13 10013 3 0.31194440E-01 0.00000000E+01
14 10014 3 0.00000000E+01 -0.37797524E+01
15 10015 3 0.00000000E+01 -0.37797524E+01
16 10016 3 -0.10000000E+01 -0.51423285E+02
17 10017 3 -0.20000000E+01 -0.99066819E+02
18 10018 3 -0.30000000E+01 -0.14671035E+03
19 10019 3 -0.40000000E+01 -0.19435388E+03
20 10020 3 -0.50000000E+01 -0.24199742E+03
21 10021 3 -0.60000000E+01 -0.28964095E+03
22 10022 3 -0.70000000E+01 -0.33895592E+03
23 10023 3 -0.80000000E+01 -0.40096489E+03

24 10024 3 -0.90000000E+01 -0.46297386E+03
25 10025 3 -0.97271898E+01 -0.50806615E+03
26 10026 3 -0.11337941E+02 0.00000000E+01
27 10027 3 -0.14620593E+02 0.10354201E+04
28 10028 4 -0.14620593E+02 0.00000000E+01
29 10029 0 -0.14620593E+02 0.00000000E+01
J 10030 -0.14620593E+02 -0.14901161E-07 -0.92250505E+02

EOT..

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH STREET CANAL FROM STA 589+00 TO STA 614+00

WAL

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 1.13 INCHES AND OCCURS AT MEMBER COORDINATE
13.60 FT.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
-14.62	POINT LD	21.26 LBF
-14.62	COUPLE	92.47 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
7.00	CONTN LD	0.00 LBF/SQ FT
6.00	CONTN LD	68.64 LBF/SQ FT
5.00	CONTN LD	137.27 LBF/SQ FT
4.00	CONTN LD	205.91 LBF/SQ FT
3.50	CONTN LD	240.23 LBF/SQ FT
3.50	CONTN LD	240.23 LBF/SQ FT
2.50	CONTN LD	299.14 LBF/SQ FT
2.50	CONTN LD	299.14 LBF/SQ FT
1.50	CONTN LD	177.97 LBF/SQ FT
0.50	CONTN LD	56.80 LBF/SQ FT

0.03	CONTN LD	0.00 LBF/SQ FT
0.00	CONTN LD	-3.78 LBF/SQ FT
0.00	CONTN LD	-3.78 LBF/SQ FT
-1.00	CONTN LD	-51.42 LBF/SQ FT
-2.00	CONTN LD	-99.07 LBF/SQ FT
-3.00	CONTN LD	-146.71 LBF/SQ FT
-4.00	CONTN LD	-194.35 LBF/SQ FT
-5.00	CONTN LD	-242.00 LBF/SQ FT
-6.00	CONTN LD	-289.64 LBF/SQ FT
-7.00	CONTN LD	-338.96 LBF/SQ FT
-8.00	CONTN LD	-400.96 LBF/SQ FT
-9.00	CONTN LD	-462.97 LBF/SQ FT
-9.73	CONTN LD	-508.07 LBF/SQ FT
-11.34	CONTN LD	0.00 LBF/SQ FT
-14.62	CONTN LD	1035.42 LBF/SQ FT

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PROPERTIES ARE AS FOLLOWS.

MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
CROSS SECTIONAL AREA= 6.47 SQ IN.

ELASTIC MODULUS= 29000000. LBF/SQ IN.
DEFLECTION REFERENCE IS AT -14.600

THE MAXIMUM BENDING MOMENT IS 8011.18 LBF-FT AND OCCURS AT -6.57
WHICH HAS THE SHEAR FORCE OF 6.05 LBF.

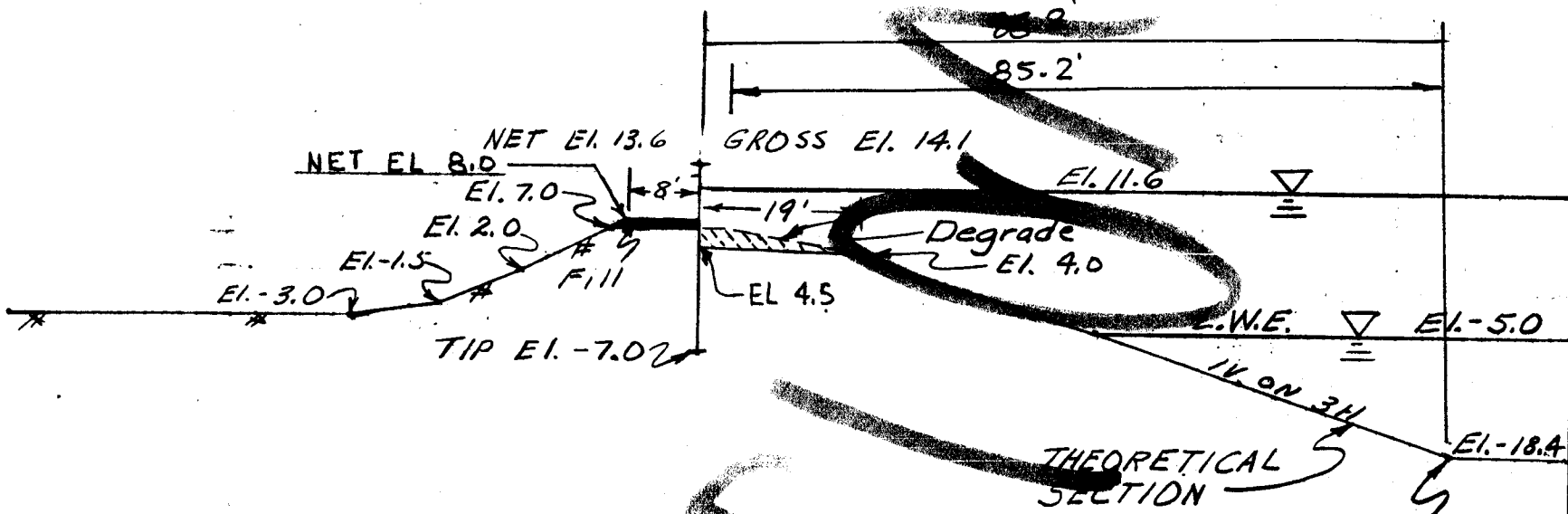
DISTANCE (FEET)	SHEAR FOR (LBF)	SHEAR STR (LBF/SQIN)	BENDING MOM (LBF-FT)	DEFLECTION FROM TANG. THRU DEFLE REFERENCE (INCHES)
13.600	0.0	0.0	0.0	1.1308
13.599	0.0	0.0	0.0	1.1308
13.000	0.0	0.0	0.0	1.0956
12.000	0.0	0.0	0.0	1.0367
11.000	0.0	0.0	0.0	0.9778
10.000	0.0	0.0	0.0	0.9190
9.000	0.0	0.0	0.0	0.8601
8.000	0.0	0.0	0.0	0.8012
7.000	0.0	0.0	0.0	0.7423
6.000	34.3	5.3	11.4	0.6835
5.000	137.3	21.2	91.5	0.6246
4.000	308.9	47.7	308.9	0.5658
3.000	547.9	84.7	731.9	0.5073
2.000	824.5	127.5	1420.7	0.4493
1.000	1002.5	155.0	2344.3	0.3924
0.031	1059.3	163.8	3352.2	0.3388
0.000	1059.3	163.7	3385.3	0.3371
-1.000	1031.7	159.5	4434.7	0.2842
-2.000	956.4	147.8	5432.7	0.2344
-3.000	833.5	128.8	6331.7	0.1885
-4.000	663.0	102.5	7083.9	0.1470
-5.000	444.8	68.8	7641.8	0.1105
-6.000	179.0	27.7	7957.7	0.0793
-6.570	6.0	0.9	8011.2	0.0641
-7.000	-135.3	-20.9	7983.7	0.0538
-8.000	-505.2	-78.1	7668.6	0.0338
-9.000	-937.2	-144.9	6952.5	0.0192
-10.000	-1417.1	-219.1	5774.8	0.0095
-11.000	-1681.5	-259.9	4199.2	0.0038
-11.337	-1699.5	-262.7	3628.6	0.0026
-11.339	-1699.5	-262.7	3625.2	0.0026
-12.000	-1630.3	-252.0	2517.0	0.0011
-13.000	-1263.8	-195.4	1043.7	0.0001
-14.000	-581.8	-89.9	94.6	0.0000
-14.599	-22.3	-3.4	-92.0	0.0000
-14.600	-21.3	-3.3	-92.0	0.0000

RUN COMPLETED

EOT..
BYE

PROJECT	17TH ST. Outfall Canal	PAGE	OF	COMPUTED BY	ELV	DATE	27 April 83
SUBJECT	STA. 614+00 TO STA. 625+00 Orleans Par.	CHECKED BY					

STA. 614+00 TO STA. 625+00
ORLEANS SIDE



FLOODSIDE TOE SAME AS MODESKI & MASTERS PLAN
SCALE 1" = 20'
Q FILE # 062510 F.S. 1.5
ELEVATION IN FEET N.G.V.D.

LIST, Q62510

1	10001	17TH STREET CANAL HLP FROM STA. 614+00 TO STA. 625+00		
2	10002	I-WALL TOP EL. 13.6 BOTT. EL. -7.0 F. S. =1.5		
3	10003		3	0.136000000E+02 0.000000000E+01
4	10004		3	0.126000000E+02 0.625000000E+02
	10005		3	0.116000000E+02 0.125000000E+03
6	10006		3	0.106000000E+02 0.187500000E+03
7	10007		3	0.960000000E+01 0.250000000E+03
8	10008		3	0.860000000E+01 0.312500000E+03
9	10009		3	0.800000000E+01 0.350000000E+03
10	10010		3	0.800000000E+01 0.350000000E+03
11	10011		3	0.700000000E+01 0.20269798E+03
12	10012		3	0.600000000E+01 0.55395954E+02
13	10013		3	0.56239294E+01 0.000000000E+01
14	10014		3	0.500000000E+01 -0.91906069E+02
15	10015		3	0.450000000E+01 -0.16555708E+03
16	10016		3	0.450000000E+01 -0.16555708E+03
17	10017		3	0.350000000E+01 -0.28061273E+03
18	10018		3	0.350000000E+01 -0.28061273E+03
19	10019		3	0.250000000E+01 -0.37566988E+03
20	10020		3	0.150000000E+01 -0.43472350E+03
21	10021		3	0.500000000E+00 -0.44972223E+03
22	10022		3	0.000000000E+01 -0.45722160E+03
23	10023		3	0.000000000E+01 -0.45722160E+03
24	10024		3	-0.100000000E+01 -0.42544845E+03
25	10025		3	-0.200000000E+01 -0.39367530E+03
26	10026		3	-0.300000000E+01 -0.36190215E+03
27	10027		3	-0.31804408E+01 -0.35616898E+03
28	10028		3	-0.42595855E+01 0.000000000E+01
29	10029		3	-0.75698046E+01 0.10925294E+04
30	10030		4	-0.75698046E+01 0.000000000E+01
31	10031		0	-0.75698046E+01 0.000000000E+01
32	10032	-0.75698046E+01	0	0.14901161E-07 0.99096339E+01

EOT..

LIST, Q625100

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BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH STREET CANAL HLP FROM STA. 614+00 TO STA. 625+00
WAL

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.78 INCHES AND OCCURS AT MEMBER COORDINATE
13.60 FT.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
-7.57	POINT LD	568.95 LBF
-7.57	COUPLE	147.09 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
13.60	CONTN LD	0.00 LBF/SQ FT
12.60	CONTN LD	62.50 LBF/SQ FT
11.60	CONTN LD	125.00 LBF/SQ FT
10.60	CONTN LD	187.50 LBF/SQ FT
9.60	CONTN LD	250.00 LBF/SQ FT
8.60	CONTN LD	312.50 LBF/SQ FT
8.00	CONTN LD	350.00 LBF/SQ FT
8.00	CONTN LD	350.00 LBF/SQ FT
7.00	CONTN LD	202.70 LBF/SQ FT
6.00	CONTN LD	55.40 LBF/SQ FT
5.62	CONTN LD	0.00 LBF/SQ FT
5.00	CONTN LD	-91.91 LBF/SQ FT
4.50	CONTN LD	-165.56 LBF/SQ FT
4.50	CONTN LD	-165.56 LBF/SQ FT
3.50	CONTN LD	-280.61 LBF/SQ FT
3.50	CONTN LD	-280.61 LBF/SQ FT
2.50	CONTN LD	-375.67 LBF/SQ FT
1.50	CONTN LD	-434.72 LBF/SQ FT
0.50	CONTN LD	-449.72 LBF/SQ FT
0.00	CONTN LD	-457.22 LBF/SQ FT
0.00	CONTN LD	-457.22 LBF/SQ FT

58 -1.00 CONTN LD -425.45 LBF/SQ FT
 59 -2.00 CONTN LD -393.68 LBF/SQ FT
 60 -3.00 CONTN LD -361.90 LBF/SQ FT
 61 -3.18 CONTN LD -356.17 LBF/SQ FT
 62 -4.26 CONTN LD 0.00 LBF/SQ FT
 63 -7.57 CONTN LD 1092.53 LBF/SQ FT
 64 -7.57 CONTN LD 0.00 LBF/SQ FT

Z-22 PROPERTIES ARE AS FOLLOWS.

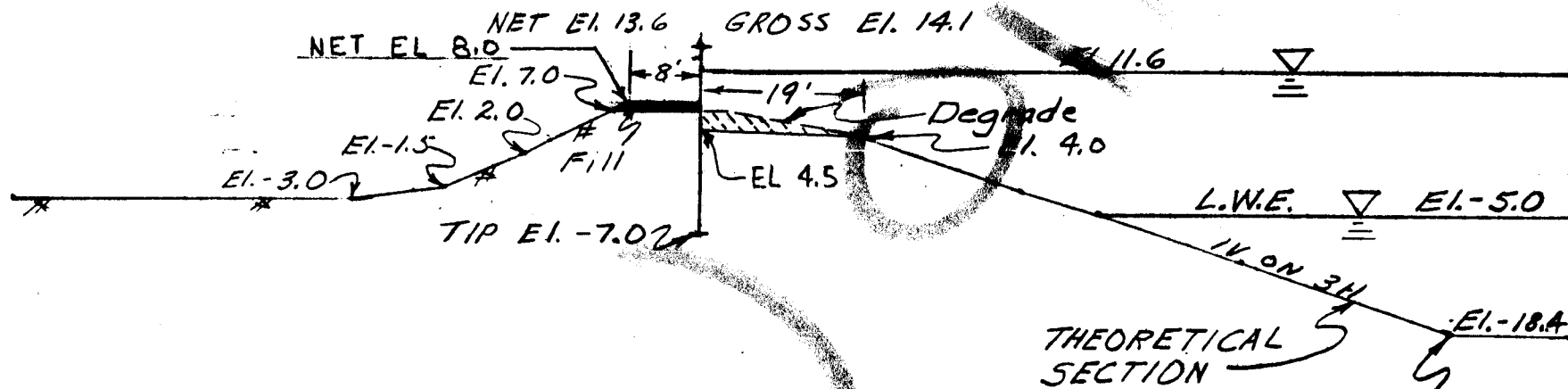
70 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
 71 CROSS SECTIONAL AREA= 6.47 SQ IN.
 72 ELASTIC MODULUS= 29000000. LBF/SQ IN.
 73 DEFLECTION REFERENCE IS AT -7.000

76 THE MAXIMUM BENDING MOMENT IS 9165.45 LBF-FT AND OCCURS AT 0.72
 77 WHICH HAS THE SHEAR FORCE OF 1.52 LBF.

					DEFLECTION FROM TANG. THRU DEFLE REFERENCE
82	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	(INCHES)
83	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	
84	13.600	0.0	0.0	0.0	0.7824
85	13.599	0.0	0.0	0.0	0.7824
86	13.000	11.2	1.7	2.2	0.7447
87	12.000	80.0	12.4	42.7	0.6817
88	11.000	211.2	32.7	183.1	0.6188
89	10.000	405.0	62.6	486.0	0.5561
90	9.000	661.2	102.2	1013.9	0.4937
91	8.000	980.0	151.5	1829.3	0.4321
92	7.000	1256.3	194.2	2959.8	0.3718
93	6.000	1385.4	214.2	4292.9	0.3136
94	5.624	1395.8	215.8	4816.5	0.2925
95	5.000	1367.1	211.3	5681.5	0.2585
96	4.000	1205.6	186.4	6978.8	0.2073
97	3.000	927.5	143.4	8054.1	0.1611
98	2.000	556.3	86.0	8802.4	0.1205
99	1.000	127.1	19.6	9147.2	0.0861
100	0.717	1.5	0.2	9165.5	0.0775
101	0.000	-322.6	-49.9	9050.7	0.0581
102	-1.000	-763.9	-118.1	8504.8	0.0364
103	-2.000	-1173.5	-181.4	7533.5	0.0207
104	-3.000	-1551.3	-239.8	6168.4	0.0103
105	-4.000	-1797.1	-277.8	4468.8	0.0042
106	-4.259	-1808.3	-279.5	4002.2	0.0032
107	-4.261	-1808.3	-279.5	3998.6	0.0032
108	-5.000	-1717.8	-265.5	2683.9	0.0012
109	-6.000	-1308.4	-202.3	1143.3	0.0002
110	-6.999	-569.9	-88.1	177.7	0.0000
111	-7.000	-568.9	-87.9	177.1	0.0000

RUN COMPLETED

STA. 614+00 TO STA. 625+00
ORLEANS SIDE



FLOODSIDE TOE SAME AS MODESKI & MASTERS PLAN

SCALE: 1" = 20'

Q FILE: Q62510 F.S. 1.5

ELEVATION IN FEET NG.V.D.

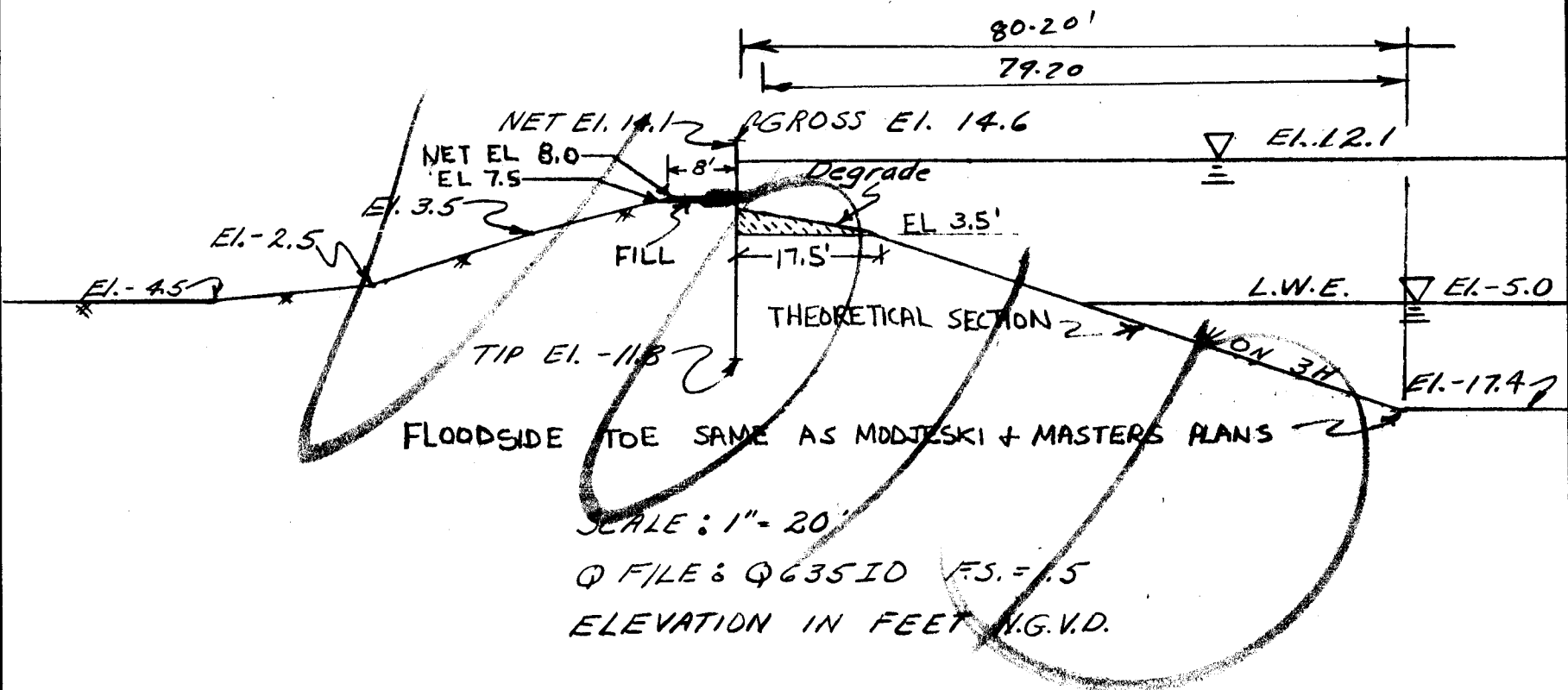
ADVANCE COPY
SUBJECT TO CORRECTIONS

Received
Apr. 30-87

PROJECT	17TH ST. Outfall Canal	PAGE	OF	COMPUTED BY	DATE
SUBJECT	STA. 614+00 TO STA. 625+00	Orleans Br.		F.I.V.	27 APR 1987
		CHECKED BY			

PROJECT	17TH ST Outfall Canal	PAGE	OF	COMPUTED BY	F.J.V.	DATE	28 April 87
SUBJECT	STA. 625+00 TO STA 635+00 ORLEANS SIDE	CHECKED BY					

STA. 625+00 TO STA 635+00
ORLEANS SIDE



58 -8.87 CONTN LD 0.00 LBF/SQ FT
 59 -11.77 CONTN LD 1166.06 LBF/SQ FT
 60 -11.77 CONTN LD 0.00 LBF/SQ FT

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62

63 Z-22 PROPERTIES ARE AS FOLLOWS.

64

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66 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
 67 CROSS SECTIONAL AREA= 6.47 SQ IN.
 68 ELASTIC MODULUS= 29000000. LBF/SQ IN.
 69 DEFLECTION REFERENCE IS AT -11.800

70

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72 THE MAXIMUM BENDING MOMENT IS 7245.90 LBF-FT AND OCCURS AT -4.51
 73 WHICH HAS THE SHEAR FORCE OF 7.42 LBF.

74

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78	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	DEFLECTION
79	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	FROM TANG. THRU DEFLE REFERENCE (INCHES)
80	14.100	0.0	0.0	0.0	0.8576
81	14.099	0.0	0.0	0.0	0.8576
82	14.000	0.0	0.0	0.0	0.8528
83	13.000	0.0	0.0	0.0	0.8046
84	12.000	0.0	0.0	0.0	0.7564
85	11.000	0.0	0.0	0.0	0.7082
86	10.000	0.0	0.0	0.0	0.6600
87	9.000	0.0	0.0	0.0	0.6118
88	8.000	0.0	0.0	0.0	0.5636
89	7.000	34.3	5.3	11.4	0.5154
90	6.000	137.3	21.2	91.5	0.4672
91	5.000	308.9	47.7	308.9	0.4191
92	4.000	549.1	84.9	732.1	0.3713
93	3.000	834.2	129.0	1426.0	0.3239
94	2.000	1021.9	158.0	2364.1	0.2777
95	1.000	1088.4	168.3	3429.4	0.2331
96	0.951	1088.6	168.3	3482.8	0.2309
97	0.000	1033.8	159.8	4500.6	0.1909
98	-1.000	894.8	138.3	5468.9	0.1519
99	-2.000	708.1	109.5	6274.3	0.1167
100	-3.000	471.3	72.8	6868.3	0.0860
101	-4.000	178.5	27.6	7198.2	0.0600
102	-4.507	7.4	1.1	7245.9	0.0488
103	-5.000	-173.7	-26.9	7205.5	0.0391
104	-6.000	-585.3	-90.5	6830.9	0.0233
105	-7.000	-1056.2	-163.3	6015.2	0.0122
106	-8.000	-1538.7	-237.9	4706.0	0.0054
107	-8.870	-1691.1	-261.4	3279.4	0.0021
108	-8.872	-1691.1	-261.4	3276.1	0.0021
109	-9.000	-1687.8	-260.9	3059.2	0.0018
110	-10.000	-1434.8	-221.8	1464.5	0.0004
111	-11.000	-779.8	-120.5	323.7	0.0000
2	-11.770	-1.2	-0.2	7.6	0.0000
113	-11.772	0.0	0.0	0.0	0.0000
114	-11.799	0.0	0.0	0.0	0.0000
115	-11.800	0.0	0.0	0.0	0.0000

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BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH STREET CANAL HLP FROM STA. 625+00 TO STA. 635+00
WAL

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8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
10 AS COUNTERCLOCKWISE.

11
12

13 THE MAXIMUM DEFLECTION IS 0.86 INCHES AND OCCURS AT MEMBER COORDINATE
14 14.10 FT.

15
16

18 Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19
20

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21
22

23 CALCULATED EXTERNAL LOADS

24
25

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
------------------------------	--------------	-------------------

26
27

-11.77	POINT LD	0.00 LBF
-11.77	COUPLE	-7.60 LBF-FT

28
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30
31

32 INPUTTED LOADS

33
34

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
------------------------------	--------------	-------------------

35
36

8.00	CONTN LD	0.00 LBF/SQ FT
7.00	CONTN LD	68.64 LBF/SQ FT
6.00	CONTN LD	137.27 LBF/SQ FT
5.00	CONTN LD	205.91 LBF/SQ FT
4.00	CONTN LD	274.54 LBF/SQ FT
3.50	CONTN LD	308.86 LBF/SQ FT
3.50	CONTN LD	308.86 LBF/SQ FT
2.50	CONTN LD	187.69 LBF/SQ FT
1.50	CONTN LD	66.53 LBF/SQ FT
0.95	CONTN LD	0.00 LBF/SQ FT

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0.50	CONTN LD	-54.64 LBF/SQ FT
0.00	CONTN LD	-115.22 LBF/SQ FT
0.00	CONTN LD	-115.22 LBF/SQ FT
-1.00	CONTN LD	-162.87 LBF/SQ FT
-2.00	CONTN LD	-210.51 LBF/SQ FT
-3.00	CONTN LD	-263.11 LBF/SQ FT
-4.00	CONTN LD	-322.49 LBF/SQ FT
-5.00	CONTN LD	-381.87 LBF/SQ FT
-6.00	CONTN LD	-441.25 LBF/SQ FT
-7.00	CONTN LD	-500.63 LBF/SQ FT
-7.54	CONTN LD	-532.98 LBF/SQ FT

55
56

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LIST Q63510

1	10001	17TH STREET CANAL HLP FROM STA. 625+00 TO STA. 635+00		
2	10002	I-WALL TOP EL. 14.1 BOT. EL. -11.8 F.S.=1.5		
3	10003		3	0.80000000E+01 0.00000000E+01
4	10004		3	0.70000000E+01 0.68636101E+02
5	10005		3	0.60000000E+01 0.13727220E+03
6	10006		3	0.50000000E+01 0.20590830E+03
7	10007		3	0.40000000E+01 0.27454440E+03
8	10008		3	0.35000000E+01 0.30886246E+03
9	10009		3	0.35000000E+01 0.30886246E+03
10	10010		3	0.25000000E+01 0.18769495E+03
11	10011		3	0.15000000E+01 0.66527447E+02
12	10012		3	0.95094646E+00 0.00000000E+01
13	10013		3	0.50000000E+00 -0.54640057E+02
14	10014		3	0.00000000E+01 -0.11522381E+03
15	10015		3	0.00000000E+01 -0.11522381E+03
16	10016		3	-0.10000000E+01 -0.16286734E+03
17	10017		3	-0.20000000E+01 -0.21051087E+03
18	10018		3	-0.30000000E+01 -0.26310686E+03
19	10019		3	-0.40000000E+01 -0.32248655E+03
20	10020		3	-0.50000000E+01 -0.38186624E+03
21	10021		3	-0.60000000E+01 -0.44124594E+03
22	10022		3	-0.70000000E+01 -0.50062563E+03
23	10023		3	-0.75449058E+01 -0.53298197E+03
24	10024		3	-0.88707033E+01 0.00000000E+01
25	10025		3	-0.11771284E+02 0.11660582E+04
26	10026		4	-0.11771284E+02 0.00000000E+01
27	10027		0	-0.11771284E+02 0.00000000E+01
28	10028	-0.11771284E+02	0	0.59604645E-07 0.75981326E+01

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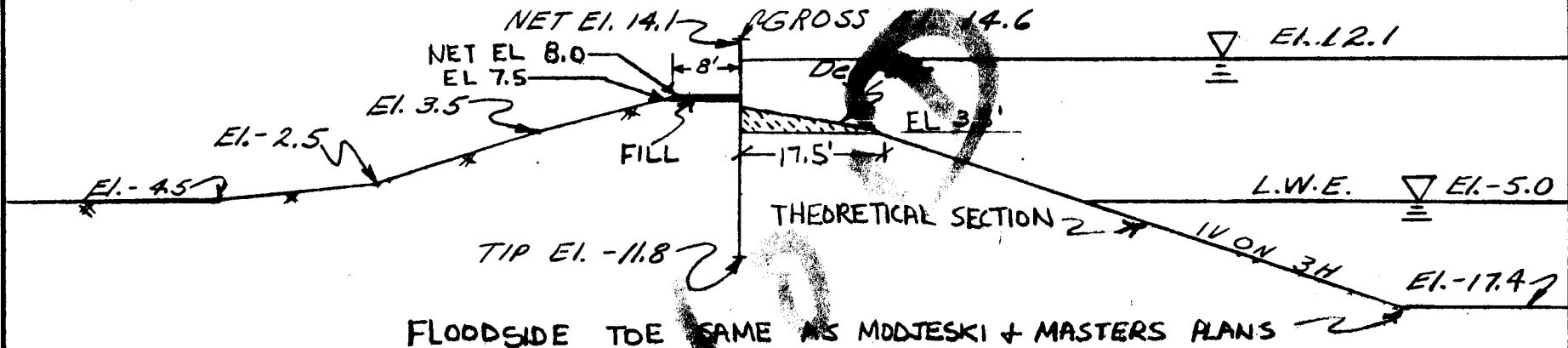
119 *RUN COMPLETED*

120

EOT..

PROJECT	17TH ST Outfall Canal	PAGE	OF	COMPUTED BY	F.V.	DATE	28 April 87
SUBJECT	STA. 625+00 TO STA 635+00 ORLEANS SIDE	CHECKED BY					

STA. 625+00 TO STA 635+00
ORLEANS SIDE



FLOODSIDE TOE SAME AS MODTESKI + MASTERS PLANS

SCALE: 1" = 20'

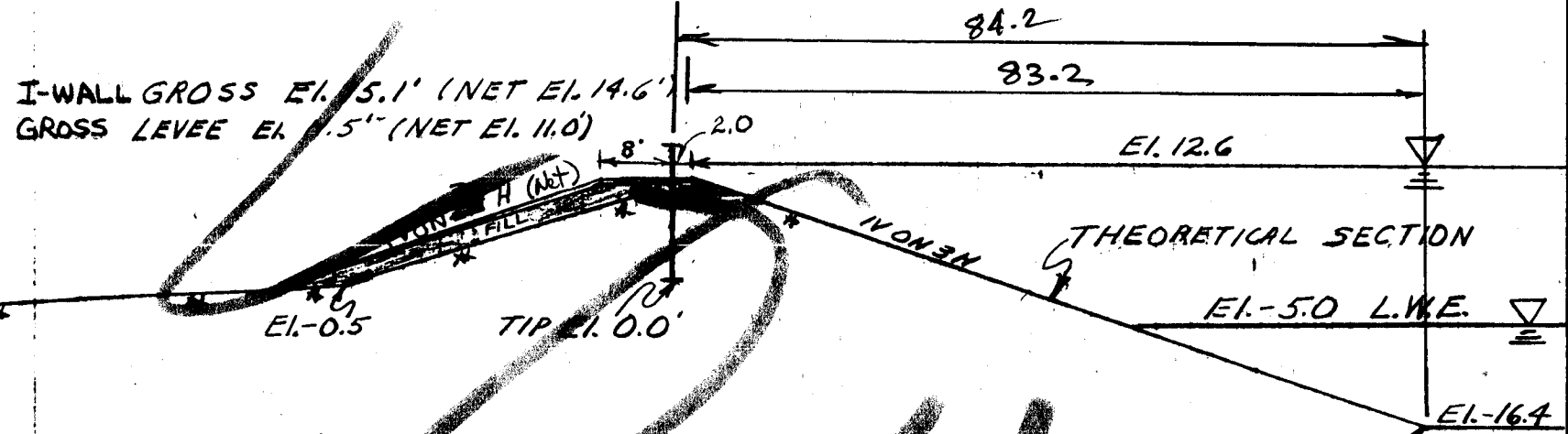
Q FILE: Q63510 F.S. = 1.5
ELEVATION IN FEET N.G.V.D.

ADVANCE COPY
SUBJECT TO CORRECTION

Received
April-30-87

PROJECT	17TH ST. Outfall Canal	PAGE	OF	COMPUTED BY	EJV	DATE	5 May 82
SUBJECT	Sta. 635+00 to Sta. 647+00 (Orleans side)	CHECKED BY					

STA. 635+00 TO STA. 647+00
ORLEANS SIDE



FLOOD SIDE TOE SAME AS MADJESKI & MASTERS

SCALE 1" = 20'
ELEVATION IN FEET N.G.M.D.

Q FILE: Q64711 (Previously furnished for Jefferson side)
Sta 635+00 to 647+00

ty TYPE 04 647IJ

10001 17TH ST CANAL HLP STA 635+00 TO 647+00

10002 I-WALL TOP EL. 15.1 BOT. EL. 0.0 F.S.=1.5

10003	3	0.14600000E+02	0.00000000E+01
10004	3	0.13600000E+02	0.62500000E+02
10005	3	0.12600000E+02	0.12500000E+03
10006	3	0.11600000E+02	0.18750000E+03
10007	3	0.11000000E+02	0.22500000E+03
10008	3	0.11000000E+02	0.22500000E+03
10009	3	0.10000000E+02	0.12285702E+03
10010	3	0.90000000E+01	0.20714046E+02
10011	3	0.88016456E+01	0.00000000E+01
10012	3	0.80000000E+01	-0.83715426E+02
10013	3	0.70000000E+01	-0.19229024E+03
10014	3	0.60000000E+01	-0.30086506E+03
10015	3	0.55000000E+01	-0.35515247E+03
10016	3	0.55000000E+01	-0.35515247E+03
10017	3	0.45000000E+01	-0.43376576E+03
10018	3	0.36081352E+01	-0.47447892E+03
10019	3	0.23944853E+01	0.00000000E+01
10020	3	0.11380161E+00	0.89163793E+03
10021	4	0.11380161E+00	0.00000000E+01
10022	0	0.11380161E+00	0.00000000E+01
10023		0.11380161E+00	0.74505806E-08
			0.68945313E+01

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BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH ST CANAL HLF STA 635+00 TO SAT 647+00
WAL

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.14 INCHES AND OCCURS AT MEMBER COORDINATE
15.10 FT.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
0.15	POINT LD	0.00 LBF
0.15	COUPLE	-9.47 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
14.60	CONTN LD	0.00 LBF/SQ FT
13.60	CONTN LD	62.50 LBF/SQ FT
12.60	CONTN LD	125.00 LBF/SQ FT
11.60	CONTN LD	187.50 LBF/SQ FT
11.00	CONTN LD	225.00 LBF/SQ FT
11.00	CONTN LD	225.00 LBF/SQ FT
10.00	CONTN LD	122.86 LBF/SQ FT
9.00	CONTN LD	20.71 LBF/SQ FT
8.80	CONTN LD	0.00 LBF/SQ FT
8.00	CONTN LD	-83.26 LBF/SQ FT
7.00	CONTN LD	-190.56 LBF/SQ FT
6.00	CONTN LD	-297.85 LBF/SQ FT
5.50	CONTN LD	-351.50 LBF/SQ FT
5.50	CONTN LD	-351.50 LBF/SQ FT
4.50	CONTN LD	-428.84 LBF/SQ FT
3.50	CONTN LD	-473.21 LBF/SQ FT
3.48	CONTN LD	-473.95 LBF/SQ FT
2.37	CONTN LD	0.00 LBF/SQ FT
0.15	CONTN LD	938.37 LBF/SQ FT
0.15	CONTN LD	0.00 LBF/SQ FT

Z-22

PROPERTIES ARE AS FOLLOWS.

MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
 CROSS SECTIONAL AREA= 6.47 SQ IN.
 ELASTIC MODULUS= 29000000. LBF/SQ IN.
 DEFLECTION REFERENCE IS AT 0.000

THE MAXIMUM BENDING MOMENT IS 3270.51 LBF-FT AND OCCURS AT 5.31
 WHICH HAS THE SHEAR FORCE OF 9.52 LBF.

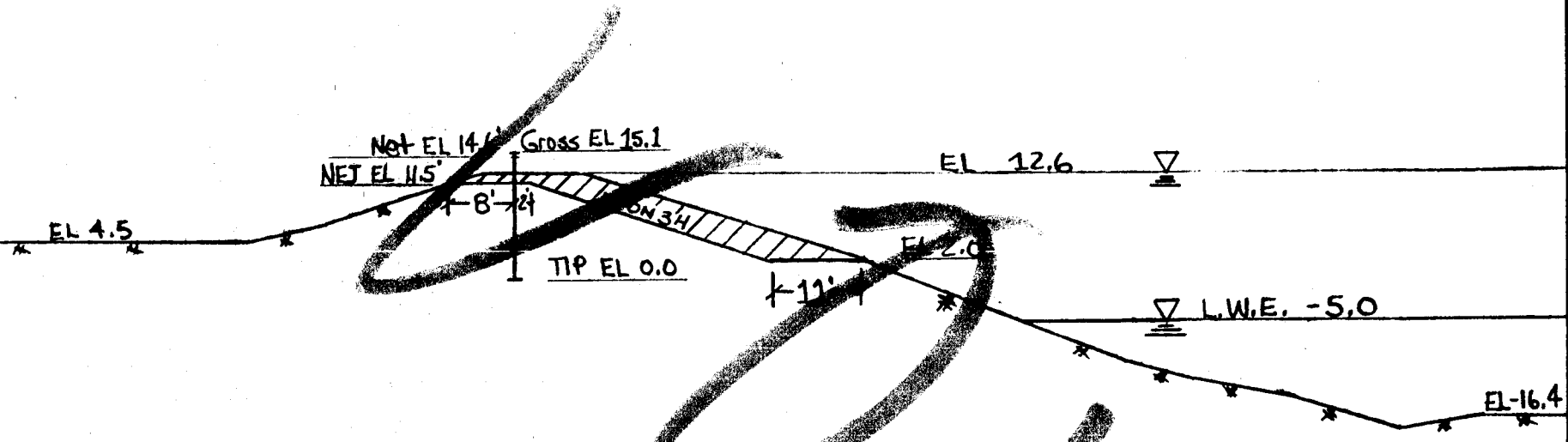
DISTANCE (FEET)	SHEAR FOR (LBF)	SHEAR STR (LBF/SQIN)	BENDING MOM (LBF-FT)	DEFLECTION FROM TANG. THRU DEFLE REFERENCE (INCHES)
15.100	0.0	0.0	0.0	0.1388
15.099	0.0	0.0	0.0	0.1388
15.000	0.0	0.0	0.0	0.1373
14.000	11.2	1.7	2.2	0.1222
13.000	80.0	12.4	42.7	0.1070
12.000	211.2	32.7	183.1	0.0920
11.000	405.0	62.6	486.0	0.0771
10.000	578.9	89.5	986.5	0.0625
9.000	650.7	100.6	1609.8	0.0487
8.801	652.8	100.9	1739.7	0.0461
8.000	619.4	95.8	2253.6	0.0360
7.000	482.5	74.6	2813.5	0.0249
6.000	238.3	36.8	3182.8	0.0157
5.315	9.5	1.5	3270.5	0.0107
5.000	-109.4	-16.9	3255.0	0.0088
4.000	-534.1	-82.6	2938.3	0.0041
3.000	-952.6	-147.3	2180.0	0.0015
2.367	-1037.9	-160.4	1541.1	0.0006
2.365	-1037.9	-160.4	1539.0	0.0006
2.000	-1009.5	-156.0	1163.6	0.0003
1.000	-642.1	-99.3	302.5	0.0000
0.155	-0.9	-0.1	9.5	0.0000
0.153	0.0	0.0	0.0	0.0000
0.001	0.0	0.0	0.0	0.0000
0.000	0.0	0.0	0.0	0.0000

RUN COMPLETED

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PROJECT	17th St Outfall Canal	PAGE	OF	COMPUTED BY	DATE
SUBJECT	STA 663+00 TO STA 670+00 Orleans Parish Side			ESV	1/18/87
		CHECKED BY			

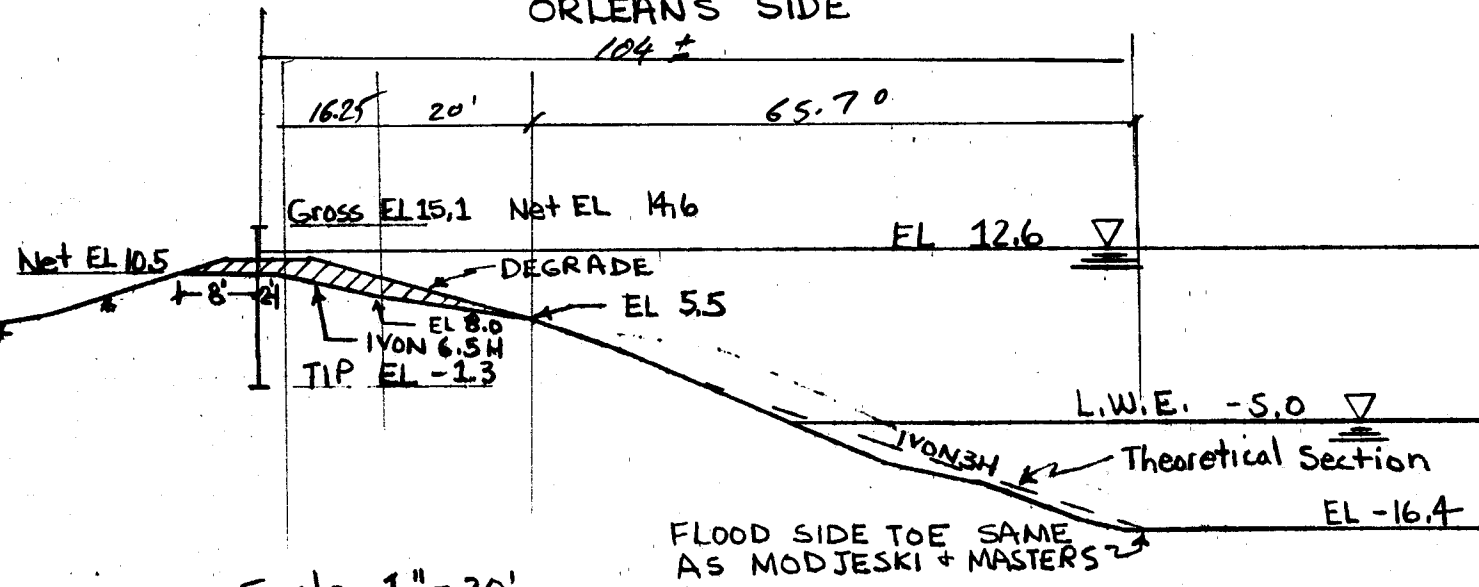
STA 663+00 TO STA 670+00
ORLEANS SIDE



SCALE 1" = 20'
ELEVATION IN FEET NEG'D

PROJECT	17th St Offfall Canal	PAGE	OF	COMPUTED BY	DATE
SUBJECT	STA 663+00 TO 670+00 (Orleans Side)			FJV	June 87
		CHECKED BY			

STA 663+00 TO STA 670+00
ORLEANS SIDE



Scale 1" = 20'

ELEVATION IN FEET N.G.V.D.

Q File: Q 670 IJ (Previously furnished for Jefferson side)
Sta 663+00 to 670+00

TYPE Q670IJ

10003	3	0.14600000E+02	0.00000000E+01
10004	3	0.13600000E+02	0.62500000E+02
10005	3	0.12600000E+02	0.12500000E+03
10006	3	0.11600000E+02	0.18750000E+03
10007	3	0.10600000E+02	0.25000000E+03
10008	3	0.10500000E+02	0.25625000E+03
10009	3	0.10500000E+02	0.25625000E+03
10010	3	0.95000000E+01	0.14536527E+03
10011	3	0.85000000E+01	0.34480544E+02
10012	3	0.81951751E+01	0.00000000E+01
10013	3	0.75000000E+01	-0.78635341E+02
10014	3	0.65000000E+01	-0.19579625E+03
10015	3	0.55000000E+01	-0.31295715E+03
10016	3	0.55000000E+01	-0.31295715E+03
10017	3	0.45000000E+01	-0.41835424E+03
10018	3	0.35000000E+01	-0.46432045E+03
10019	3	0.25000000E+01	-0.50610877E+03
10020	3	0.24098109E+01	-0.50973138E+03
10021	3	0.11583886E+01	0.00000000E+01
10022	3	-0.13235832E+01	0.10109607E+04
10023	4	-0.13235832E+01	0.00000000E+01
10024	0	-0.13235832E+01	0.00000000E+01
10025		-0.13235832E+01	0.14901161E-07
			0.31351822E+02

A:\>

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH STREET HLP STA. 663+00 TO STA 670+00
WAL

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.23 INCHES AND OCCURS AT MEMBER COORDINATE
15.10 FT.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
-1.32	POINT LD	23.73 LBF
-1.32	COUPLE	-31.07 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
14.60	CONTN LD	0.00 LBF/SQ FT
13.60	CONTN LD	62.50 LBF/SQ FT
12.60	CONTN LD	125.00 LBF/SQ FT
11.60	CONTN LD	187.50 LBF/SQ FT
10.60	CONTN LD	250.00 LBF/SQ FT
10.50	CONTN LD	256.25 LBF/SQ FT
10.50	CONTN LD	256.25 LBF/SQ FT
9.50	CONTN LD	145.37 LBF/SQ FT
8.50	CONTN LD	34.48 LBF/SQ FT
8.20	CONTN LD	0.00 LBF/SQ FT
7.50	CONTN LD	-78.64 LBF/SQ FT
6.50	CONTN LD	-195.80 LBF/SQ FT
5.50	CONTN LD	-312.96 LBF/SQ FT
5.50	CONTN LD	-312.96 LBF/SQ FT
4.50	CONTN LD	-418.35 LBF/SQ FT
3.50	CONTN LD	-464.32 LBF/SQ FT
2.50	CONTN LD	-506.11 LBF/SQ FT
2.41	CONTN LD	-509.73 LBF/SQ FT
1.16	CONTN LD	0.00 LBF/SQ FT
-1.32	CONTN LD	1010.96 LBF/SQ FT
-1.32	CONTN LD	0.00 LBF/SQ FT

MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL

CROSS SECTIONAL AREA= 6.47 SQ IN.

ELASTIC MODULUS= 29000000. LBF/SQ IN.

DEFLECTION REFERENCE IS AT -1.300

THE MAXIMUM BENDING MOMENT IS 4451.92 LBF-FT AND OCCURS AT 4.43
WHICH HAS THE SHEAR FORCE OF 8.87 LBF.

DISTANCE (FEET)	SHEAR FOR (LBF)	SHEAR STR (LBF/SQIN)	BENDING MOM (LBF-FT)	DEFLECTION FROM TANG. THRU DEFLE REFERENCE (INCHES)
15.100	0.0	0.0	0.0	0.2273
15.099	0.0	0.0	0.0	0.2273
15.000	0.0	0.0	0.0	0.2251
14.000	11.2	1.7	2.2	0.2025
13.000	80.0	12.4	42.7	0.1799
12.000	211.2	32.7	183.1	0.1573
11.000	405.0	62.6	486.0	0.1349
10.000	639.6	98.9	1010.3	0.1129
9.000	784.9	121.3	1731.8	0.0916
8.195	821.3	127.0	2383.0	0.0754
8.000	819.1	126.6	2543.2	0.0716
7.000	740.0	114.4	3332.3	0.0533
6.000	544.2	84.1	3984.2	0.0374
5.000	232.7	36.0	4382.0	0.0242
4.434	8.9	1.4	4451.9	0.0181
4.000	-178.2	-27.5	4415.5	0.0141
3.000	-642.0	-99.2	4009.1	0.0071
2.000	-1110.3	-171.6	3122.7	0.0029
1.159	-1254.6	-193.9	2108.5	0.0010
1.157	-1254.6	-193.9	2106.0	0.0010
1.000	-1249.5	-193.1	1908.8	0.0008
0.000	-981.3	-151.7	759.5	0.0001
-1.000	-305.8	-47.3	82.0	0.0000
-1.299	-24.7	-3.8	31.7	0.0000
-1.300	-23.7	-3.7	31.6	0.0000

RUN COMPLETED

A:\>

FRANK VOJKOVICH
4/16/90
EXT 1034

<u>Per EL</u>	<u>Q-FILES</u>	<u>Q-CASE</u>	
-5.0	MET17A	FS. = 1.5	PLATE 100
-5.0	MET17B	FS. = 1.0	
-5.0	MET18A	FS. = 1.5	PLATE 121
-5.0	MET18B	FS. = 1.0	

QUALIFIER 213 AB ϕ 4