MEMORANDUM THRU

Area Engineer, NOAO Contr Admin Br

FOR C/Engr Div

SUBJECT: Narrative Completion Report for Contract No. DACW29-94-C-0003, Lake Pontchartrain, Louisiana & Vicinity, High Level Plan, London Avenue Outfall Canal, Parallel Protection, Pumping Station No. 3 to Mirabeau Avenue Floodwall, Orleans Parish, Louisiana

- 1. The subject contract dated 15 Oct 93, was awarded to Boh Bros Construction Company, Incorporated, Post Office Drawer 53266, New Orleans, Louisiana 70153. The Notice to Proceed was issued on 18 Nov 93, with construction to start no later than 28 Nov 93. The original completion date was set for 12 May 95, with the estimated amount of the contract being \$6,684,517.00.
- 2. Required work under this contract included Clearing and Grubbing, Selective Demolition of 13,100 ft. of existing floodwall along with a pedestrian bridge at Mirabeua Avenue and an abandoned vehicular bridge at Benefit Street, construction of 13,100 feet of sheet piling and steel reinforced concrete I-type floodwall, structural excavation and backfill necessary to construct floodwall, embankment along the floodwall, asphalt paving repair, construction of two swing gate structures, complete with steel floodgates at railroad crossing, construction of temporary falsework necessary to build the swing gate structures, modifications of waterlines at several locations, driving of steel "H" piles necessary for swing gate construction, temporary and permanent relocation of electric feeder cables, temporary flood protection & coffer dams, and fertilizing & seeding.
- 3. The Preconstruction Conference was held at the New Orleans Area Office on 18 Nov 93. Detailed minutes of this meeting are located in the contract file. The Notice to Proceed was signed by the contractor on 18 Nov 93 and the contractor began mobilizing equipment and performing preliminary survey work on 26 Nov 93.
- 4. The contractor mobilized project trailers on 5 Dec 93 and began installing safety fence and erosion control on 30 Dec 93. The entire jobsite was enclosed during construction with the 48" safety

- fence. The contractor installed 10,241 linear feet of 36" silt fence manufactured by Mirafi, Incorporated.
- 5. This contract provided for 17 major construction phases:
 (1) Clearing and Grubbing, (2) Selective Demolition, (3)
 Embankment, (4) Structural Excavation and Backfill, (5) Fertilizing and seeding, (6) Temporary Falsework for R/R Gates, (7) Driving Steel Sheet Piling, Type PZ22 and PSA23, (8) Driving Steel H-Piles, (9) Reinforced Concrete Floodwall, (10) Utility Modifications, (11) Incidental Paving, (12) Temporary and Permanent Relocation of Electric Pump Station Feeder Lines, (13) Structural Steel Gates and Metalwork, (14) Painting, (15) Temporary Flood Protection, (16) Erosion Control, and (17) Relocation of Trees and Shrubs.
- 6. The contractor started the major phases of work on 13 Jan 94 with the removal of the pedestrian foot bridge located near Mirabeau Avenue. The contractor used hand labor and a small hydraulic boom truck mounted crane to remove the walkway and handrails. The timber pilings were pulled on 9 Dec 94, with the use of Lorain 60-ton motor crane mounted on flexifloats, and the holes left were filled with a cement slurry.
- 7. In order to provide access to and from the jobsite, the grass on the landside of the existing floodwall was cleared and the existing levee was degraded approximately 2 feet. This material was rolled down the slope and moved around the jobsite to make a 15-20 foot shelf for the contractor's crawler cranes. The contractor used a caterpillar D-5 dozer to clear and degrade the existing levee, and a caterpillar 235 backhoe with a dump truck to move degraded material to stockpile areas. Also, the concrete steps at Mirabeau were removed to provide a clear access from the north. Additional access to the jobsite was also obtained by the contractor through Dillard University on the east side of the canal and the Parkway Commission's property on the east and west sides of the canal. This phase of work began on 25 Jan 95 and was continually maintained during floodwall construction between Mirabeau Avenue and Gentilly Boulevard.
- 8. The contractor relocated all of the trees and shrubs located along the levee near the residence at Dillard University on 1 Feb 95. The trees were replanted near the Physical Plant in Dillard University and maintained during the life of the project. On 12 Jul 95 all trees and shrubs were replanted at the original location along the levee and all clean-up work was completed at the residence.
- 9. Demolition of the Benefit Street bridge began on 9 Feb 94 with the contractor mobilizing an American 5299 crawler type crane with

an 80-foot boom, and a 410C caterpillar backhoe with a hydro-hammer, to the jobsite. The bridge deck was broken into sections with the backhoe and lifted out with the crane utilizing a fabricated harness. A small barge placed in the canal was used to catch any broken concrete from the demolition process. The demolition and removal of the bridge deck and beams was completed on 1 Mar 94. The remaining timber pilings were pulled at a later date on 17 Oct 94 by the contractor when the permanent sheetpile for the floodwall were driven in this area and all holes left were filled with a bentonite cement slurry. The contractor utilized an American 5299A crawler crane with an 80' boom to pull these timber pilings.

- Modification to the existing waterlines began on 3 Mar 94. existing waterline was replaced with a new 6-inch waterline between E B/L Station 20+60 to 14+31 and E B/L Station 6+36 to 1+23 and a new 4 in waterline encased in a 12" steel pipe between E B/L Station 1+23 to 0+82. All new water lines were Class 150 PVC pipe with a minimum of 3 feet of cover, except under the Railroad, which recieved 5.5 feet of cover. Also installed were five new water meters, three water valves, and one fire hydrant between E B/L Station 0+82 to 6+36. The existing waterline was removed between E B/L Station 0+82 to 1+23, and 6+36 to 13+25 and cut, plugged, and abandoned between E B/L Station 1+23 to 6+36 and 14+31 to 20+60. All newly installed waterline was pressure tested and chlorinated in the presence of a S&WB representative and a government representative. Service through the existing waterlines was not interrupted until all necessary preparations were made to perform the tie-ins at the designated locations. The waterline work was completed on 20 Jun 95.
- The temporary relocation of the electric feederlines began on 15 Mar 94. A subcontractor, Hazard Construction Company, Inc. performed all relocation work for the contractor. The relocation of Electric Feederline No. 340 from its original position located on existing slope pavement to the vertical face of the concrete channel lining started at E B/L Station 21+00 and continued thru E B/L Station 1+25. Upon completion of this temporary relocation, feederline No. 400 was replaced with new cable in 5" conduit and temporarily installed on the vertical face of the channel lining from W/B/L Station 21+00 to 14+10. The remaining feederline cable (FL-340 from E B/L Station 21+00 to 68+56 and FL-400 from W B/L Station 21+00 to 68+88) was temporarily relocated to a trench located on the floodside of the existing floodwall to protect it during floodwall demolition. The cables were deenergized during relocation work, and tested at the pumping station to assure no damage was done to the cables during the relocation. The cables were all temporarily relocated by 16 Feb 95.

12. On 29 Apr 94 the driving of 283,866.40 sq. ft. of steel sheet piling commenced. The contractor used Syro steel sheet pile type SPZ-22, SPZ-26, and special fabricated zee and tee type sheetpile for this contract. The contractor utilized two separate driving crews on both the east and west sides of the canal. Sheetpile driving started at Mirabeau and proceeded south to pumping station No. 1.

The contractor used the following equipment to drive the sheetpile on the west side of the canall-50 ton American 5299A crane with 80' boom, 1-416 ICE vibratory hammer, 1- clothes pin hammer, and 1-40 feet steel I-beam to maintain alignment. The contractor utilized a 60-ton 670 Lorain motor crane with 80' boom mounted on (6) six-10'x40'x7' and (2) two-10'x20'x7' flexi-float barges equipped with an ICE416 vibratory hammer and a 40' steel I-beam for alignment to drive the sheetpile on the east side of the canal between Mirabeau Avenue and Gentilly Blvd and also between W B/L 17+95 to 14+21.

Prior to driving the sheetpile under interstate 610 and the uncapped sheetpile at the floodgates, the contractor procured the services of Specialty Coatings to apply the approved coal tar epoxy to these sheetpiles. These sheets were sandblasted to white metal finish and the coal tar epoxy was applied (sprayed) in two coats to provide a 16 mil thickness. The sheets under I-610 were field cut into five sections and driven in spliced (welded from interlock to interlock) sections with the use of a Gradall 880 backhoe and an ICE 216 vibratory hammer.

Within the railroad right-of-way, the contractor utilized the American 5299A crane with 100' boom, a Vulcan 08 single acting air hammer, a clothes pin hammer, and a 40' steel I-beam for alignment to drive the cofferdam sheetpile (PMA22) and the new sheetpile under the gate slab. All sheetpile driven on subject contract were driven to grade and work was completed on 25 May 95.

A private testing laboratory was retained by the Orleans Levee District, which monitored vibrations during sheet pile operations.

13. The contractor commenced construction of the reinforced concrete floodwalls on 4 May 94. There were four hundred thirty-eight (438) permanent concrete I-wall monoliths, (6) six temporary concrete I-wall monoliths, and two (2) concrete gate monoliths placed on subject contract. Prior to placing the 4-inch stabilization slab, cathodic protection was installed on the sheetpile at all monolith joints. The I-walls were placed from Elev 3.5, 4.0, and 4.5 to Elev 14.4 on both the east and west sides of

the canal. All rebar was Grade 60 and placed according to contract drawings. An approved three bulb (Tamms/Horn Products) waterstop was placed between each monolith joint along with a fibre expansion joint filler (Sealtight). The contractor used EFCO (Economy Forms Corporation) steel forms along with plyform material backed with steel double channel wales. The forms were connected with 1 1/4" x 1" taper ties spaced according to the contractor's formwork design. A rubber fractured fin form material was attached to the land side form to provide the required finish, except between Station E B/L 21+40.96 to 33+10.96 and W B/L 48+65.18 to 58+55.18 which received a repeated pattern of three BAS- relief architectural panels followed by three fractured fin panels. All formwork was set using a 5299A American Crane. The I-wall monoliths were made in one placement, and the gate monoliths were constructed in two placements.

Concrete was delivered to the jobsite by concrete mixer trucks and placed into forms with a concrete bucket and rubber trunks of various lengths. Concrete pump trucks were utilized for I-wall placements between approximate E B/L station 56+00 to 48+00, E B/L Station 20+70 to 14+50, under the I-610 on both the east and west sides, and for the two gate monoliths. The concrete pump trucks were used due to limited access along the existing levee. Quality control was maintained daily through use of the 1246 checkout list with air content tests, slump tests, temperature checks, and test cylinders done by the contractor. The contractor placed concrete on both sides of the canal simultaneously by two different concrete These crews started at Mirabeau Avenue and worked work crews. behind the sheetpile driving crews proceeding south to the pumping Concrete monoliths were moist cured for the entire seven day curing period. The contractor averaged three I-wall placements per week per side, between Mirabeau Avenue and Gentilly Boulevard, and two per week per side south of Gentilly Boulevard. The last concrete I-wall was placed on 8 Jun 95. The contractor utilized the following equipment per side for this item of work; 1-5299A American Crawler-type crane, 1-2CY concrete bucket, 2-concrete vibrators, form oil, several rubber chutes of various lengths, curing blankets, miscellaneous hand tools used for concrete wall construction, and occasionally a Schwing BPL 1200 concrete pump truck equipped with feeder pipe.

14. On 20 Jun 94, the contractor commenced demolition of the existing floodwall at E BL Station 12+26.95. The contractor used a caterpillar 325 backhoe equipped with a hydraulic ram (hammer) to break the concrete cap off in pieces between the pump station and approx station 1+00 on both east and west sides. The existing sheetpile were then pulled and hauled off site between E B/L Station 1+46-5+87, 12+26.95-12+85, and 14+50-20+70, and W B/L Station 10+55-12+55, and 14+24-17+95. Within the other reaches the

sheetpile was cut off at elevations from EL 5.0 to 7.0. Between W and E B/L Station 21+00 to Mirabeau Avenue, the existing concrete cap was scored in 30' sections and lifted out with the 325 Backhoe and hauled off site to a recycling yard (Pontchartrain Materials) for disposal. The existing concrete channel lining and the new perm sheetpile served as temporary flood protection in areas where the alignment of the new floodwall coincided with the old floodwall. At no time during hurricane season was there an opening in flood protection more than 300 linear feet, and non-hurricane season of 750 linear feet opened. All existing floodwall was demolished and removed from the jobsite by 25 May 95.

The contractor began the permanent installation of electric feeder cable (FL-400) on 26 Aug 94. Hazard construction was the subcontractor that performed all electrical work on the feederlines. Prior to any wall construction, new cable was installed 2' deep, except within LDOTD ROW which was 4' deep, in a 12" wide trench, encapsulated with 3000 psi concrete between W B/L Station 2+60 to This new section of cable was spliced to the existing cable at Station 2+60 and new cable at 12+70. Between W B/L Station 12+70 to 14+10 new cable was installed in 5" conduit and secured to the channel lining under the Gentilly Boulevard bridge. The contractor then replaced the existing cable between W/B/L Station 14+10 to 21+00 with new cable encased in 5" conduit and made two splices at This cable was installed on the new floodwall after placement of the 4" incidental paving between the new floodwall and the channel lining. Finally, the contractor relocated the existing cable to the new floodwall between W B/L Station 21+00 to 68+88. This work started at Mirabeau Avenue and proceeded south to allow the contractor to obtain slack in the line. The subcontractor utilized a small truck mounted crane and sling along with hand labor to install the cable on the new wall. The new cable was brought out on large reels and stretched out with use of a 215 caterpillar rubber tire backhoe. The backhoe was also used to excavate and backfill the trench.

The permanent installation of the electric feeder cable FL-340 began on 14 Nov 94. This work consisted of relocating the feederline to the floodside of the new floodwall using bolts and clamps to secure the line to the wall. The line was installed after the new wall was painted and all structural backfill was in place from E B/L station 1+25 to 12+85 and 14+70 to 68+56. Between E B/L Station 12+85 to 14+70, new 3/C #500 15KV cable was installed under the Gentilly Bridge and apliced to the existing FL-340 at both ends. This new cable installation at the above location, was the last electrical work done on the project, and was completed on 29 Jun 95. Also on the east side, FL-432 was passed thru the new permanent

sheetpile at approximate E B/L Station 21+60 installed in a 16" diasplit steel sleeve packed with plastic sealant and sealed with neoprene rubber sleeves.

- 16. The contractor commenced application of the approved Tammoseal cementitous items paint (1st coat) beginning at Mirabeau Avenue on the floodside to both the east and west floodwalls on 13 Sep 94. The cementitous paint was applied at a rate of 2 pounds/square yard of concrete surface. Upon completion of the floodside, the landside of the floodwalls were painted with the Tammoseal. Two coats of acrylic emulsion paint (Tammosheen) was then placed over the cementitous paint on the floodside and then the landside. The contractor initially used a sprayer to apply the cementitous paint, but did not fill all voids and pits in the concrete surface. A roller was then used which provided the required finish and filled all voids in the surfaces. The final two coats of acrylic emulsion paint was sprayed on with an air sprayer. The painting operation was completed on 24 Jul 95.
- Structural backfill was begun on 26 Aug 94 on the floodside of both the east and west floodwalls. The floodside was backfilled first to allow the contractor to relocate the feeder cable to the new wall. The material was cast over the new wall with a 880 Gradall Backhoe and compacted with hand tampers and plate compactors (Mikasa Products). The backfill was placed within 2 feet on both sides of the wall in 8" thick lifts and compacted to 90% of the max dry density. Soil samples were taken to develop standard proctor curves and compaction tests were performed by Alpha Testing Laboratory. The contractor utilized a vibratory roller (RT560-Wacker Corp) and a Tandem vibratory roller (BW90AD-Bomag) to compact the fill on the landside of the floodwall. Make note that at the R/R monoliths, the subballast supplied by Norfolk Southern was compacted as dense as the material in the same area, with hand tampers to the satisfaction of railroad personnel. All structural backfill was completed on 15 Jun 95 when the last lift was placed at E B/L Station 2+00.
- 18. The work within the railroad rights-of-way commenced on 7 Dec 94. This phase of work consisted of driving cofferdam and permanent sheetpile, pipe piles and steel H-piles, constructing and installing four falsework structures to maintain traffic on the railroad tracks, placing two concrete gate monoliths, and installing two steel swing gates. Prior to any work within the P/R ROW, meetings were held to establish a working relationship with the Railroad Company (Norfolk Southern). At these meetings the railroad POC established a work window for the contractor from 0730 to 1430 within the R/R ROW. During the life of the contract, the contractor was continually denied this allowable schedule and encountered delay time.

The contractor worked on the west side first by driving two rows of steel sheetpile (PMA-22), 20 feet long, perpendicular with the tracks, and one row down the centerline of both sets of tracks. Continuous pipe piles were then driven to EL-60.0 to support the falsework bridges. Structural excavation for the gate base monolith The contractor removed the falsework bridge, excavated the material for the base slab, and reset the falsework bridge for one track in an 8 hour workday. After all structural excavation, the steel H-piles and the permanent sheetpile were driven. Quality assurance was maintained during the driving of the H-piles by using a level, marking the pile footage, and recording blow counts for the 2,016 LF of HP 14x73 steel H-piles. All piles were driven to grade. The contractor then constructed the reinforced gate base slabs and columns using 4000 psi concrete and epoxy coated rebar in the gate The contractor completed the concrete work and removed all falsework (5' below grade) on the west side on 9 Mar 95. contractor commenced operations on the east side on 13 Mar 95 and completed work on 15 May 95. The following equipment was utilized; 1-5299 American Crane with 100' Boom, 80' pile leads, 1-Vulcan 08 air hammer, 1-clothes pin hammer, 1-880 Gradall backhoe, 1-Schwing 1200 concrete pump truck, crane mats, 2 CY concrete bucket, concrete vibrators, and other miscellaneous hand tools for concrete work.

- 19. The two railroad swing gates were fabricated and painted by Manufab, Incorporated of Pearlington, Mississippi. Quality assurance was maintained by visits to Manufab's yard. The gates were installed by Manufab on 6 Apr 95 (west) and 22 Jun 95 (east) and two trial operations were conducted. Some adjustments were made on both gates and they were accepted on 24 Jul 95. The contractor used a Caterpillar 325 backhoe to set the gates
- 20. After all floodwall was constructed and painted, the contractor started rebuilding and reshaping the landside levee. The embankment was built to the lines and grades shown on the plans from the existing levee material using D-4 and D-5 caterpillar dozers, 1-235 caterpillar backhoe, 1-215 caterpillar backhoe, and several dump trucks. Work commenced on 2 Feb 95 and was completed on 5 Jul 95.
- 21. Fertilizing, seeding, and mulching of the completed levee on the east and west side began on 19 Jun 95 between Mirabeau Avenue and Gentilly Blvd. Subcontractor, Economy Grassing performed this phase of work. After harrowing the dressed levee, an approved fertilizer and unhulled Bermuda seed was broadcast and cultipacked and wood cellulose fiber mulch was spraye, over the entire bare surface of levee. The subcontractor completed this first section 21 Jun 95. The area between the pumping station no. 1 and Gentilly

Boulevard was completed on 13 Jul 95. The following equipment was used; 1-2130 John Deere tractor, 1-seed broadcaster, 1-spike tooth harrow and 1-Hydro mulch sprayer.

- 22. The last phase of work was to repave London Avenue between E B/L Station 2+35 to 5+75 and W B/L Station 14+00+0 17+75, and the Benefit Street Bridge ramps on both sides of the canal. The areas damaged along London Avenue during construction operations received a 3.5" binder and 1.5" wearing course. The areas previously occupied by the Benefit Street bridge ramps received a 8" thick crushed concrete base course compacted to 95% max dry density, along with the overlay. All asphalt work was started and completed on 26 Jul 95. The following equipment was used; 1- vibratory roller, 1-D3 Caterpillar dozer, 1-Paving machine and other incidental equipment.
- 23. The contractor had difficulty in coordinating with utility owners for relocation of their lines located with contract R.O.W.. There were Cox Cable lines crossing the canal and running parallel with the canal located within the ROW not shown on the contract drawings. No substantial construction delays were encountered by the contractor, however, the coordination efforts of the contractor were increased due to the utility owners poor relocation work.
- 24. There were twenty-two modifications on this contract and a summary of each follows:
- a. P00001 (FM-001) dated 23 Mar 94. This modification provided additional funds for payment in accordance with SPECIAL CLAUSE H-26, an increase of \$1,972,000.00.
- b. P00002 (CIN-01) dated 31 Mar 94. This modification corrected stationing and offsets, and dimension discrepancies found after award, and increased the amount of coated sheetpile due to an error in the original stations on the east side. The modification increased the contract by \$3,879.22 with no time extension.
- c. P00003 (TE-001) dated 11 Apr 94. This modification extended the required completion date of the contract (7) seven calendar days due to unusually severe weather encountered between 18 Nov 93 to 31 Mar 94. The contract price remains the same.
- d. P00004 (UCO-01) dated 25 Apr 94. This modification was to provide approximately 200 feet of additional 15 KV feederline cable to be installed under the east side of Gentilly Boulevard bridge. The existing cable was damaged prior to any electrical work commencing. The UCO was written in order for the contractor to obtain the additional cable within a reasonable time from the supplier due to manufacturing time restraints. This modification

was settled at a later date.

- e. P00005 (FM-002) dated 25 Apr 94. This modification provided additional funds for payment in accordance with SPECIAL CLAUSE H-26, an increase of \$3,884,517.00.
- f. P00006 (CIN-03) dated 2 May 94. This modification substituted acrylic emulsion paint for cementitous paint on the landside BAS-relief architectural monoliths, and to provide a sack rubbed finish to these surfaces. The originally specified cementitous paint would cover the detail of the artwork panels. The modification increased the contract by \$5,799.30 with no time extension.
- g. P00007 (CIN-02) dated 29 Jun 94. This modification changed the type and number of sill plates, stiffners, and railroad plate deflectors, added cofferdam sheeting, modified the falsework pipe piles and changed all rebar to be epoxy coated within the gate sills on both the east and west sides. The Norfolk Southern Railroad Company requested these changes after contract award. The modification increased the contract by \$23,005.20 with no time extension.
- h. P00008 (CIN-05) dated 29 Jun 94. This modification changed the size of the new waterline installed between E B/L Station 2+08 to 6+36 from 4" to 6" diameter and realigned the waterline to eliminate two bends. The S&WB of New Orleans requested a larger size waterline that was supplying a fire hydrant due to fire prevention codes. The modification increased the contract by \$1,500.00 with no time extension.
- i. P00009 (TE-002) dated 11 Jul 94. This modification extended the required completion date of the contract eleven (11) calendar days due to unusually severe weather encountered between 1 Apr 94 to 30 Jun 94. The contract price remains the same.
- j. P00010 (CIN-06) dated 5 Aug 94. This modification was to replace the 200 linear feet of additional electric feederline under the east side of Gentilly Boulevard. The existing cable was damaged prior to any work done on this cable. This modification definitizes UCO-01. The modification increased the contract by \$24,993.59 with an increase in contract time of ten (10) calendar days.
- k. P00011 (CIN-07) dated 12 Sep 94. This modification added a feederline cable splice to the existing FL400 at W B/L Station 14+10. Due to the closing of Gentilly Boulevard Bridge, this additional splice was needed to allow the contractor to stay clear from working under the bridge for safety purposes and not be

impacted or delayed in his progress of work. The modification increased the contract by \$5,000.00 with no time extension.

- 1. P00012 (UCO-02) dated 16 Sep 94. This modification directed the contractor to close any gaps in hurricane protection due to expected high tides from an approaching hurricane. This work took place on 15 and 16 Sep 94 in which the contractor pulled 60' of sheet pile up to EL 11.5 between E B/L Station 20+80 to 21+40. This modification was settled at a later date.
- m. P00013 (TE-003) dated 18 October 94. This modification extended the required completion date of the contract seven (7) calendar days due to unusually severe weather encountered between 1 Jul 94 to 30 Sep 94. The contract price remains the same.
- n. P00014 (CIN-08) dated 17 Nov 94. This modification was for the contractor to close the gaps in hurricane protection due to an approaching hurricane. This modification definitizes UCO-02. The modification increased the contract amount by \$12,620.00 with an increase in contract time of six (6) calendar days.
- o. P00015 (CIN-10) dated 12 Apr 95. This modification was for formwork modifications made by the contractor due to varying elevations of the concrete channel liner. The channel liner elevation differed in places as much as 6 inches from the plan elevation. This modification increased the contract by \$10,200.00 with no time extension.
- p. P00016 (TE-004) dated 14 Apr 95. This modification extended the required completion date of the contract thirteen (13) calendar days due to unusually severe weather encountered between 1 Oct 94 to 31 Mar 95. The contract price remains the same.
- q. P00017 (CIN-11) dated 23 May 95. This modification added slope pavement and reinforced retaining walls at the two railroad gates. The original contract drawings provided no details at these two ends of the job. The modification increased the contract price by \$7,500.00 with no time extension.
- r. P00018 (CIN-09) dated 9 Jun 95. This modification was to remove approximately 1300 linear feet of sheetpiling above elevation 7.5 to allow placement of slope pavement and for additional demolition costs due to the sheetpile extending further in the concrete cap than shown. The contract drawings contained discrepancies as to the actual elevation of the existing sheetpile embedded in the existing concrete cap. The modification increased the contract price by \$43,000.00 with an increase in contract time of fourteen (14) calendar days.

- s. P00019 (FM-003) dated 19 Jun 95. This modification provided additional funds for payment in accordance with SPECIAL CLAUSE H-26 and Contract Clause I-18, an increase of \$105,742.10.
- t. P00020 (TE-005) dated 12 Jul 95. This modification extended the required completion date of the contract fifteen (15) calendar days due to unusually severe weather encountered between 1 Apr 95 to 30 Jun 95. The contract price remains the same.
- u. Pending time modification (TE-006) for an additional twenty-two (22) calendar days due to unusually severe weather encountered between 1 Jul 95 to 17 Aug 95. The contract price will remain the same.
- 25. The contractor submitted a claim on August 11, 1995, in the amount of \$144,917.74 for construction delays encountered while working within Norfolk Southern Railroad Rights-of-Way. The claim is being analyzed for merit.
- 26. The following is a list of major suppliers on the subject contract:
 - a. Steel Sheet Piles Syro, Inc., Girard, OH
 - b. Reinforcing Steel Lulich Steel Corp., Slidell, LA
 - c. Concrete Carlo Ditta, Inc., New Orleans, LA
- d. Waterstop/Silt Fence/Expansion Joint Filler/Form Oil/Cementitous and Acrylic Emulsion Paint-Building Specialties, Incorporated, New Orleans, LA
 - e. Waterline Supplies Louisiana Utilities, Jefferson, LA
 - f. 15 KV Feederline Cable-Okonite Co., Ramsey, NJ
 - g. Feederline Splice Kits Mac Products, Inc., Kearney, NJ
- h. Misc. Electric Feederline Supplies Nulite Electrical, New Orleans, LA
 - i. Asphalt T. L. James & Co., Kenner, LA
- j. Crushed Concrete Pontchartrain Materials Corp., New Orleans, LA
 - k. Steel H-Piles LB Foster Co., Pittsburgh, PA

- 1. Floodgates & Metal Items Manufab, Pearlington, MS
- m. Coal Tar Epoxy Application Specialty Ctgs, Kenner, LA
- n. Formwork EFCC, Des Moines, IW
- o. Seed Alexandria Seed, Alexandria, LA
- p. Hi-potential feederline tests Point Eight, Belle Chasse, LA
 - q. Density Tests Alpha Testing, Kenner, LA
 - r. Artwork Molds H. H. Horil & Assn., Mobile, AL
- 27. Subcontractors performing work on this project along with the contract responsibilities are as follows:
- a. Hazard Construction & Drayage Company, 701 S. Alexandar, New Orleans, LA 70119. All electrical relocation work.
- b. Economy Grassing, Innc., 7054 W. T. Hall Roadd, Ethel, LA 70730. Seeding, Fertilizing, and Mulching.
- 28. The contractor submitted and enforced an adequate Accident Prevention Program. The contractor was very cooperative in the performance of the work and performed daily safety inspections in addition to holding weekly safety meetings. There were no lost time accidents throughout the duration of the project.
- 29. The contractor was efficient and professional in the performance of the work and any extra work required to complete this project. The equipment used was kept in good working condition. Quality Control was maintained throughout the life of the contract.
- 30. Following is a comparison of contract quantities versus actual quantities:

Item	Description	Qty & Unit	Unit Price	Est. Amt	Final Qty	Earnings to Date
0001	Mob & Demob	LS	LS	\$100,000	100%	\$100,000
0002	Clearing & Grub	LS	LS	\$ 20,000	100%	\$ 20,000
0003	Selective Demo (P00018)	LS	LS	\$282,000	100%	\$282,000

0004	Ped. Bridge Demo	LS	LS	\$ 5,000	100%	\$ 5,000
0005	Embankment Semicompact. fill	LS	LS	\$40,000	100%	\$40,000
0006	Structural Excav	LS	LS	\$400,000	100%	\$400,000
0007	Fertilizing & Seeding	LS	LS	\$10,000	100%	\$10,000
	Temp. Falsework for gates (P00007)	LS	LS	\$113,505.20	100%	\$113,505.20
0009	Piling Steel Sht Type PZ-22 (P00002 273,5		\$8.58	\$2,347,256.34		6.40SF \$2,435,573.71
0010	Piling, Steel Sht Type PSA-23	10 SI	F \$18	\$19,980	850.29S	F \$15,305.22
0011	Furnish & Deliver H piles 2,	020LI	F \$16	\$32,320	2,016L	F \$32,256
0012	Driving Piles (Steel H piles) 2,	020	\$5	\$10,100	2,016L	F \$10,080
0013	Reinforced Concrete Floodwall (P00002),(P00006) (P00015),(P00017)	LS	LS	\$2,113,569.57	100%	\$2,113.569.57
	Utility Mods.	LS	LS	\$ 168,500	100%	\$168,500
	Temp. Relocation of Feeder lines	LS	LS	\$400,000	100%	\$400,000
	Permanent Relocation of Feeder lines (P00010) (P00011) Structural Steel	on LS	LS	\$429,993.59	100%	\$429,993.59
	Gates & Misc. Metals	LS	LS	\$ 70,000	100%	\$70,000
0018	Painting (P00002)	LS	LS	\$134,469.61	100%	\$134,469.61

0019	Rail	road Ins.	LS	LS	\$	7,000	100%	\$	7,000
0020		. Flood Protc ffer Dams	LS	LS	\$	5,000	100%	\$	5,000
0021	State Perf	e Req ormance Bond	LS	LS	\$7	0,000	100%	\$7	0,000
0022	Eros	ion Control							
	AA.	First 13,900	13,	900LF	\$2	\$27,800	10,241	LF	\$20,482
	вв.	All over 13,900	1,	450LF	\$2	\$ 2,900			
0023		Flood ection 014)	LS	LS		\$12,620.	100%		\$12,620

- 31 A copy of as-built drawings are attached.
- 32. The contract was completed in accordance with contract plans and specifications with final acceptance on 17 $\,\mathrm{Aug}$ 95.

Jules Boudreaux Project Engineer New Orleans Area Ofc

Jules Bordrany

CF:
Proj Engr (Boudreaux)
Proj Insp (Bollent)
Ofc Engr w/as-built
CELMN-CD-Q
CELMN-PA
CELMN-CT
CELMN-CD-C
CELMN-CD-CS
CELMN-CD-B
CELMN-PP
CELMN-OD-ON

Safety is a Part of Your Contract

PLANS FOR LAKE PONTCHARTRAIN, LOUISIANA AND VICINITY HURRICANE PROTECTION HIGH LEVEL PLAN ORLEANS PARISH, LA.

LONDON AVE. OUTFALL CANAL, PARALLEL PROTECTION PUMP STATION NO. 3 TO SUMIRABEAU AVE. FLOODWALL.





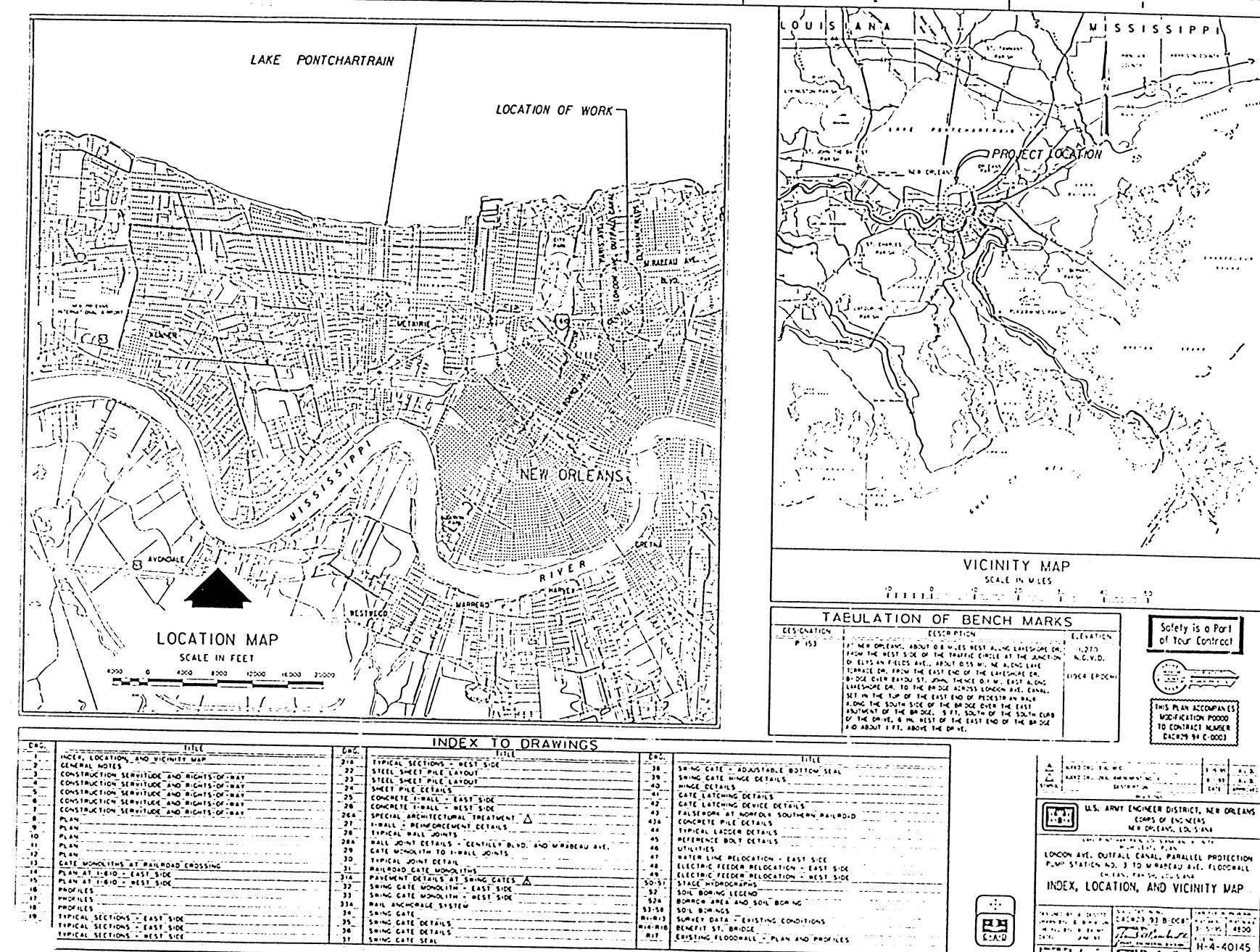


US Army Corps of Engineers New Orleans District 1993

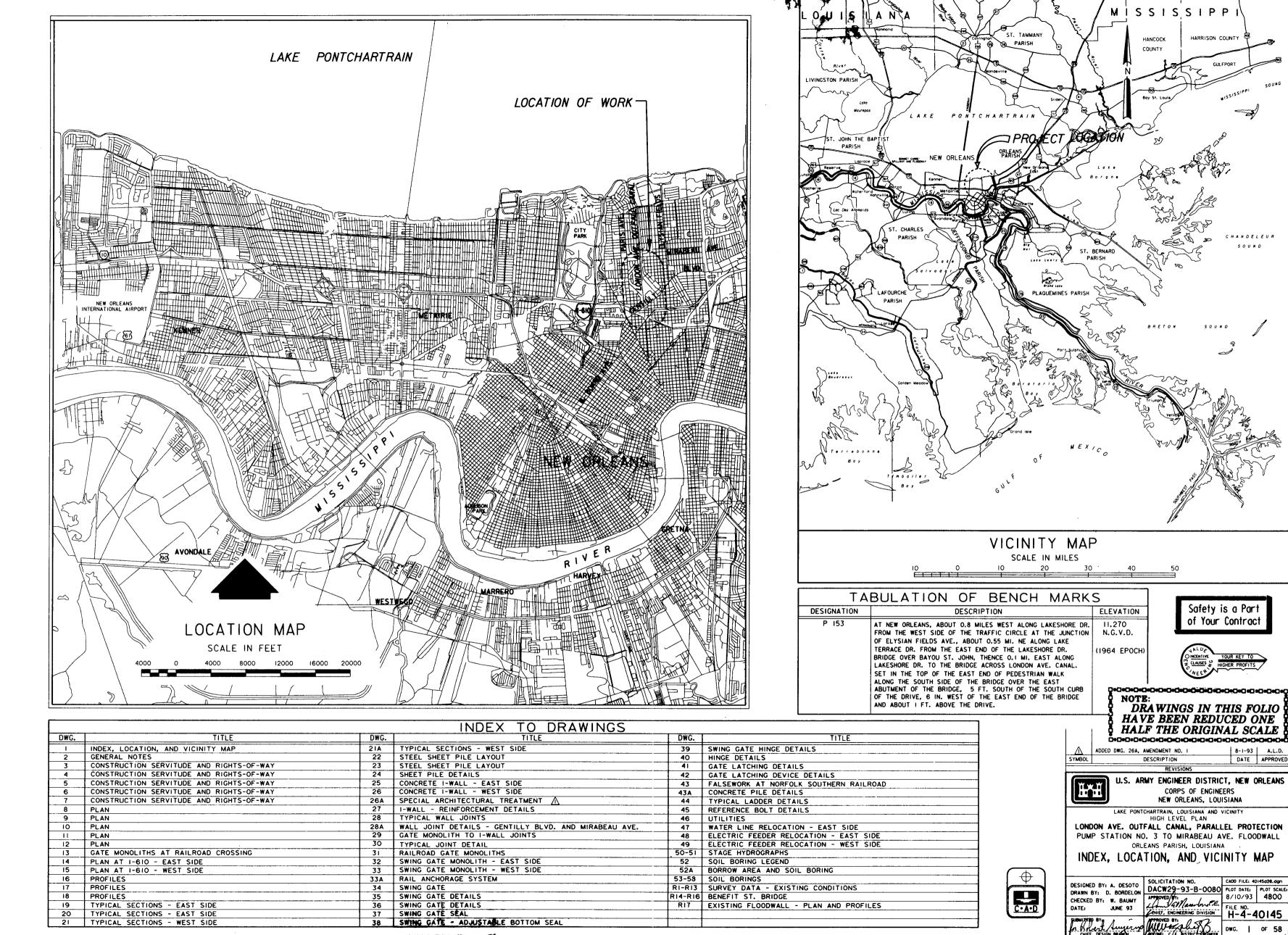
DRAWINGS IN THIS FOLIO HAVE BEEN REDUCED ONE HALF THE ORIGINAL SCALE



H-4-40145



130/2-6-5e H-4-40145



3

5

2

2 '

ı

GENERAL NOTES:

- I. AZIMUTHS SHOWN ARE MEASURED CLOCKWISE FROM THE NORTH.
- 2. ELEVATIONS ARE IN FEET AND REFER TO NATIONAL GEODETIC VERTICAL DATUM (N.G.V.D.).
- 3. DIMENSIONS AND/OR ELEVATIONS MARKED THUS (±) ARE APPROXIMATE. CONTRACTOR SHALL VERIFY ACTUAL DIMENSIONS
- 4. DIMENSIONS AND/OR ELEVATIONS MARKED THUS (N.T.S.) ARE NOT SHOWN TO SCALE.
- 5. DRAWINGS ARE GENERALLY TO SCALE, BUT SHOULD NOT BE SCALED. N.T.S. IS SHOWN ONLY WHERE DRAWING IS OBVIOUSLY
- 6. BENCH MARKS AND BASE LINES HAVE BEEN ESTABLISHED AT THE SITE BY THE GOVERNMENT; SEE DWG. I FOR BENCH MARK DESCRIPTION.
- 7. THE BASELINE STATIONING REFERS TO THE CORPS OF ENGINEERS TRAVERSE NO. 92-098.
- 8. FOR BORING LOGS, SEE DWGS. 52A-58.
- 9. UNCONTROLLED MOSAICS PREPARED FROM AERIAL PHOTOS FLOWN MARCH 1992.
- 10. FOR EXISTING CONDITIONS, SEE DWGS. RI-RI7.

STEEL NOTES:

- ALL STRUCTURAL STEEL SHALL BE ASTM A36, UNLESS OTHERWISE NOTED.
- 2. TO PREVENT CORROSION BY MOISTURE BETWEEN STEEL SURFACES IN CONTACT, ALL SUCH CONTACTS SHALL BE SEALED WATERTIGHT BY RUNNING A CONTINUOUS 1/8" FILLET WELD ALONG ALL EDGES OF THE CONTACT, UNLESS OTHERWISE NOTED.
- 3. ALL WELDING SHALL BE ELECTRIC WELDING. WORKMANSHIP AND TECHNIQUE, WHERE APPLICABLE, SHALL CONFORM TO THE AMERICAN WELDING SPECIFICATIONS (SEE SPECS.) STRUCTURAL WELDING CODE.
- 4. WELDING SYMBOLS SHOWN ARE THOSE ADOPTED BY THE AMERICAN WELDING SOCIETY AND INDICATE ONLY SIZE AND TYPE OF WELDS REQUIRED. DETAILED INFORMATION SHALL BE SHOWN ON THE SHOP DRAWINGS AND SUBMITTED BY THE CONTRACTOR FOR APPROVAL.
- 5. DIMENSIONS SHOWN OR CALLED FOR ARE THE FINAL DIMENSIONS; ALLOWANCES MUST BE MADE FOR MACHINING.
- ITEMS MARKED C.R.S. SHALL BE CORROSION RESISTANT STEEL (STAINLESS STEEL), SEE SPECIFICATIONS.

CONCRETE NOTES:

- 1. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (F_C') OF 3000 PSI AT 28 DAYS, 90 DAYS IF POZZOLAN IS USED, UNLESS OTHER WISE NOTED.
- 2. STABILIZATION SLAB CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (F'_C) OF 2500 PSI AT 28 DAYS, 90 DAYS IF POZZOLAN IS USED.
- 3. REINFORCING STEEL SHALL HAVE A MINIMUM YIELD STRENGTH (Fy) OF 60,000 PSI.
- 4. REINFORCING SHALL BE SPACED TO MISS RECESSES FOR GATE LATCHES.
- 5. CONSTRUCTION JOINTS SHALL BE PROVIDED WHERE SHOWN. WHERE NOT SHOWN, CONSTRUCTION JOINTS SHALL BE PLACED AT LOCATIONS LEAST LIKELY TO IMPAIR THE INTERGRITY OF THE CONCRETE STRUCTURE. CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE CONTRACTING OFFICER.
- 6. UNLESS OTHERWISE NOTED, PROVIDE ¾ " CHAMFER AT ALL EXPOSED JOINTS, EDGES, EXTERNAL CORNERS, AND VERTICAL EXPANSION JOINTS.
- 7. ALL PRIMARY REINFORCEMENT SHALL HAVE A MINIMUM COVER OF 3" UNLESS OTHERWISE NOTED. THE COVER FOR SECONDARY REINFORCEMENT MAY BE REDUCED FROM THE ABOVE BY THE DIAMETER OF THE BAR.
- 8. ALL BENDS OF REINFORCEMENT AND ALL BAR SPACERS AND SUPPORTS SHALL BE IN ACCORDANCE WITH SP-66, AMERICAN CONCRETE INSTITUTE DETAILING MANUAL 1980.
- 9. REINFORCING BAR DESIGNATION NUMBERS CONFORM TO THE NUMBERING SYSTEM OF THE CONCRETE REINFORCING STEEL INSTITUTE.
- 10. REINFORCING BARS SHALL BE CONTINUOUS AT ALL CORNERS UNLESS OTHERWISE NOTED.
- 11. REINFORCEMENT, WHERE NECESSARY TO AVOID OPENINGS, PIPES, EMBEDDED ITEMS AND OTHER OBSTRUCTIONS, SHALL BE BENT OR SHIFTED AS DIRECTED BY THE CONTRACTING OFFICER.
- 12. THE EMBEDMENT AND SPLICE TABLE SHALL BE USED IN DETERMINING LAP SPLICES AND EMBEDMENT LENGTHS WHERE LENGTHS ARE NOT OTHERWISE INDICATED. SPLICE LENGTHS SHALL BE BASED ON THE SMALLER BAR BEING LAPPED. THE CONTRACTOR WILL BE ALLOWED TO MAKE SPLICES IN ADDITION TO THOSE INDICATED IN THE DRAWINGS, WHERE ESSENTIAL TO CONSTRUCTIBILITY, SUBJECT TO APPROVAL BY THE CONTRACTING OFFICER. SPLICES OTHER THAN THOSE SHOWN ON THE DRAWINGS AND OTHER THAN ANY ADDITIONAL SPLICES REQUIRED BY THE CONTRACTING OFFICER, WILL BE AT THE CONTRACTOR'S
- 13. ALL EXTERIOR FORMED SURFACES NOT COVERED BY BACKFILL SHALL BE CLASS "A" FINISH AND SURFACES COVERED BY BACKFILL SHALL BE CLASS "D" FINISH, UNLESS OTHERWISE
- 14. FOR "SWING GATE MONOLITH" CONCRETE PLACEMENT. THE CONTRACTOR SHALL EITHER PLACE A CONSTRUCTION JOINT AT APPROXIMATELY MID-WALL HEIGHT OR SHALL EMPLOY TEMPORARY "WINDOWS" IN THE FORMS TO FACILITATE CONCRETE PLACEMENT AND CONSOLIDATION.
- 15. THE MINIMUM LAP LENGTH FOR WELDED WIRE FABRIC SHALL BE 8 INCHES.

	REI	NFORCEME	ENT EMB	EDMENT	AND SPL	ICE TABL	_ES		
		BASIC	TABLE		ALTERNATE TABLE				
BAR SIZE	MINIMUM EMBEDMENT MINII LENGTH, INCHES			NIMUM LAP LENGTH INCHES		MINIMUM EMBEDMENT LENGTH, INCHES		MINIMUM LAP LENGTH	
	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER	
3	16	' 12	21	16	16	12	21	16	
4	21	16	28	21	21	16	28	21	
5	27	21	35	27	27	21	35	27	
6	32	25	42	32	32	25	42	32	
7	37	29	49	37	37	29	49	37	
8	45	35	59	45	43	33	56	43	
9	57	44	74	57	48	37	63	48	
10	72	56	94	72	58	45	75	58	
11	89	68	116	89	71	55	92	71	

NOTES:

- I. USE THE BASIC TABLE IF ALL OF THE FOLLOWING CONDITIONS ARE MET:
 - A) CENTER TO CENTER BAR SPACING LATERALLY IS AT LEAST 4 BAR DIAMETERS
 - B) CONCRETE COVER IS AT LEAST 2 BAR DIAMETERS, AND
 - C) EDGE DISTANCE TO THE FIRST BAR IN A LAYER IS AT LEAST 2 BAR DIAMETERS.
- 2. THE ALTERNATE TABLE MAY BE USED IF ALL OF THE FOLLOWING CONDITIONS ARE MET: A) CENTER TO CENTER BAR SPACING LATERALLY IS AT LEAST 6 BAR DIAMETERS
 - B) CONCRETE COVER IS AT LEAST 2 BAR DIAMETERS, AND
 - C) EDGE DISTANCE TO THE FIRST BAR IN A LAYER IS AT LEAST 2.5 BAR DIAMETERS.
- 3. IF CONCRETE COVER OR EDGE DISTANCE IS LESS THAN 2 BAR DIAMETERS OR THE CENTER TO CENTER BAR SPACING LATERALLY IS LESS THAN 4 DIAMETERS, SEE ACI 318 FOR APPROPRIATE GUIDANCE.
- 4. TOP BARS ARE HORIZONTAL BARS AND BARS INCLINED LESS THAN 45 DEGREES WITH RESPECT TO A HORIZONTAL PLANE WHICH ARE PLACED SUCH THAT MORE THAN 12 INCHES OF CONCRETE IS CAST IN THE MEMBER BELOW THE BAR.
- 5. THE TABLES SHOWN ABOVE ARE FOR NORMAL WEIGHT CONCRETE AND UNCOATED REINFORCING BARS. IF EPOXY COATED BARS ARE USED, SEE ACI 318 FOR ADDITIONAL CONSIDERATIONS.

ABBREVIATIONS

BF	= BOTTOM FACE
BL	= BOTTOM LAYER
С	= CENTER
C.1.	= CAST IRON
CJ	= CONSTRUCTION JOINT
CL	= CLEAR COVER
C/L OR &	= CENTER LINE
C.R.S.	= CORROSION RESISTANT STEEL
Ø	= DIAMETER
D	= DRAIN
D.1.	= DROP INLET
D.P.	= DRAIN PIPE
D/S	= DOWN STREAM
D.V.	= DRAIN VALVE MANHOLE
EB/L	= EAST BASELINE
EF	= EACH FACE
EL.	= ELEVATION
ES	= EQUALLY SPACED
F.H.	= FIRE HYDRANT
FF	. = FAR FACE

D

= LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT MARKER LDH = MANHOLE

= ELECTRIC FEEDER LINE

= NEAR FACE = NOT TO SCALE

= GAS

0.C. = ON CENTER = OPTIONAL

FL

P.C. = POINT OF CURVATURE

= POINT OF TANGENCY

= SEWER

S.C.O. = SEWER CLEANOUT ST = STANDARD HOOK

STA. = STATION = TELEPHONE

TF = TOP FACE TEL.M.H. = TELEPHONE MANHOLE

= UP STREAM

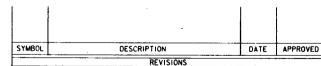
TL = TOP LAYER = TEST PILE

= WATER WB/L = WEST BASELINE

U/S

W/L = WALL LINE W.M. = WATER METER

= WATER VALVE W.V.





U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA

LAKE PONTCHARTRAIN, LA. AND VICINITY HIGH LEVEL PLAN LONDON AVE. OUTFALL CANAL, PARALLEL PROTECTION PUMP STATION NO.3 TO MIRABEAU AVE. FLOODWALL

ORLEANS PARISH, LOUISIANA

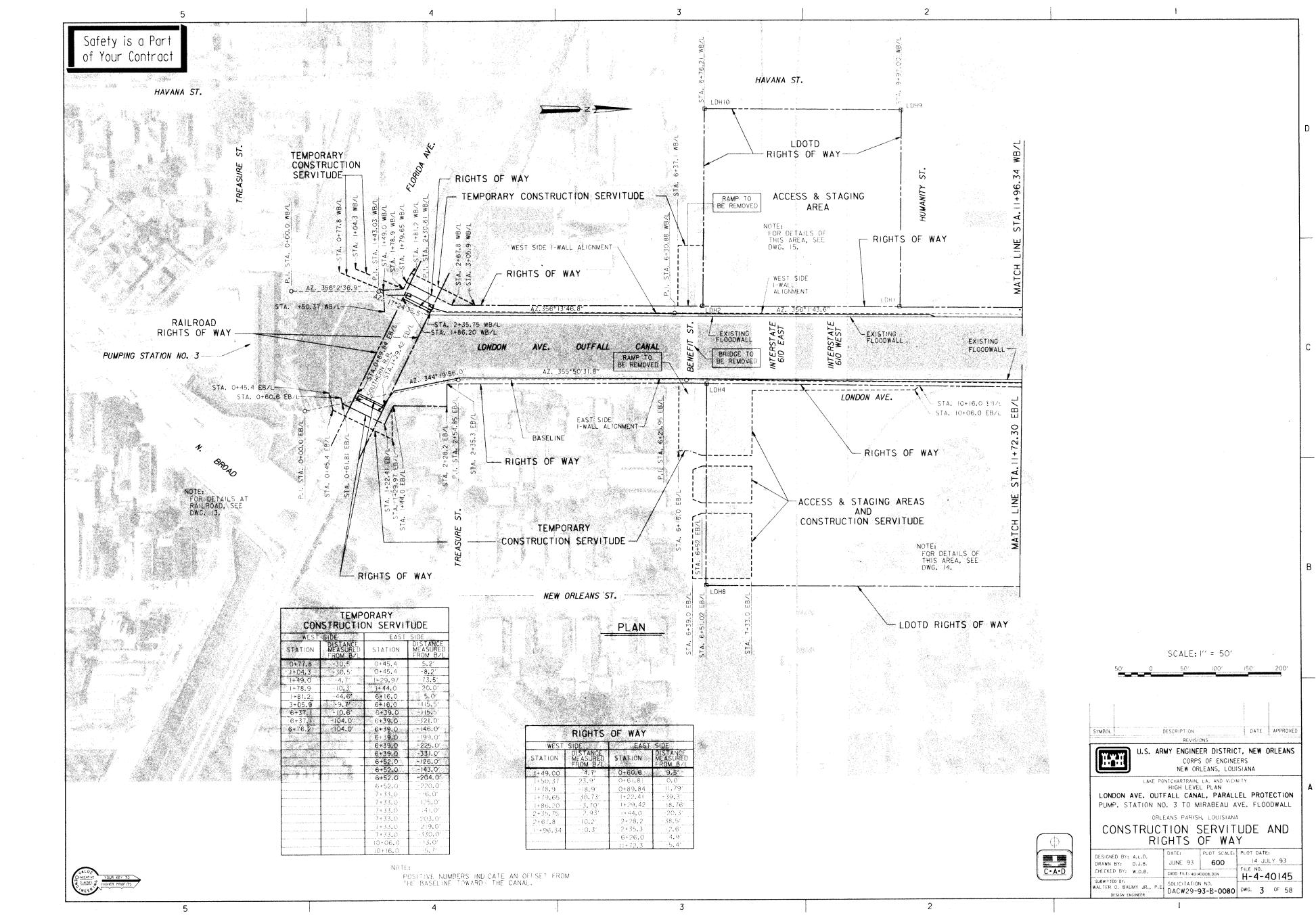
GENERAL NOTES

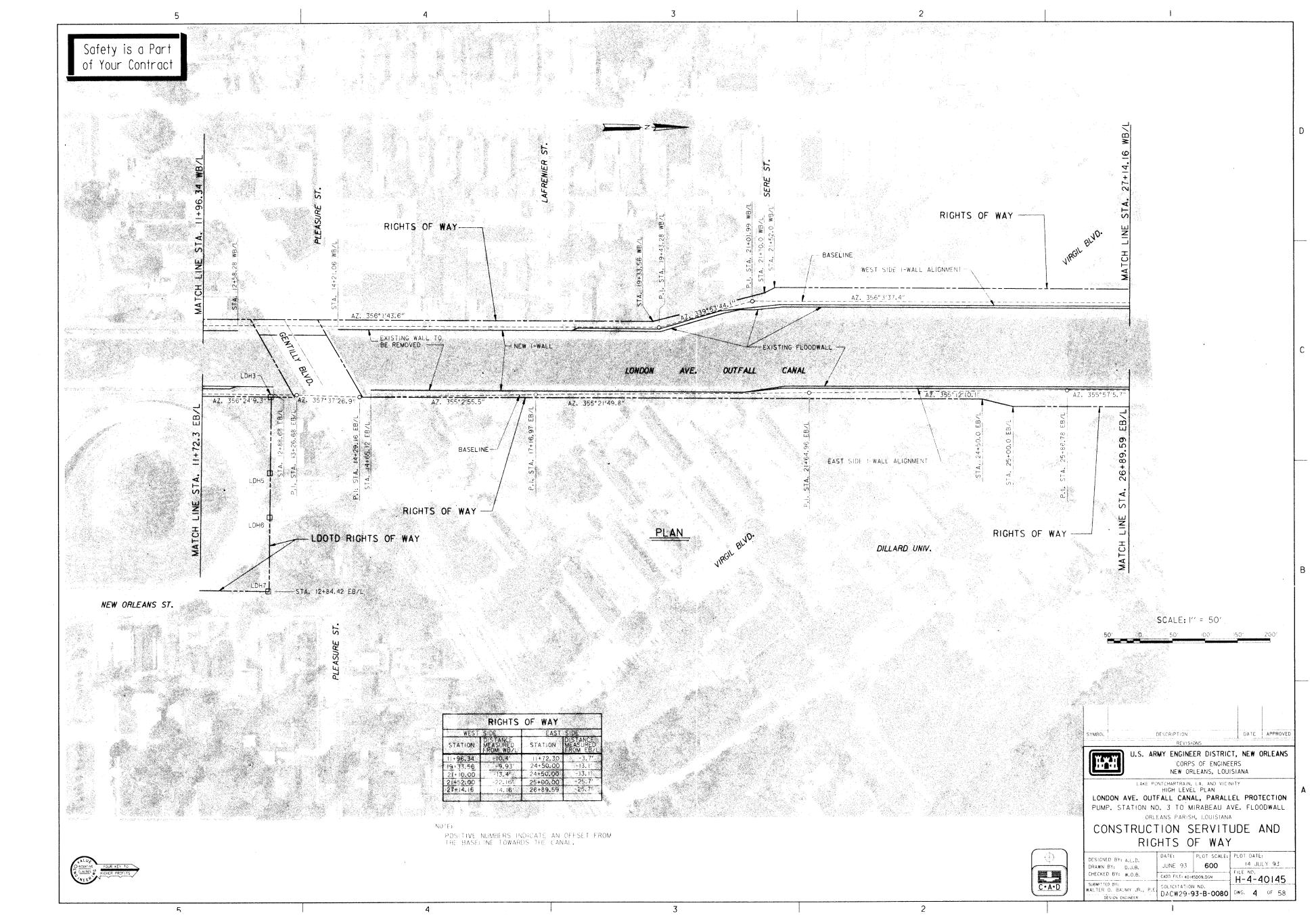


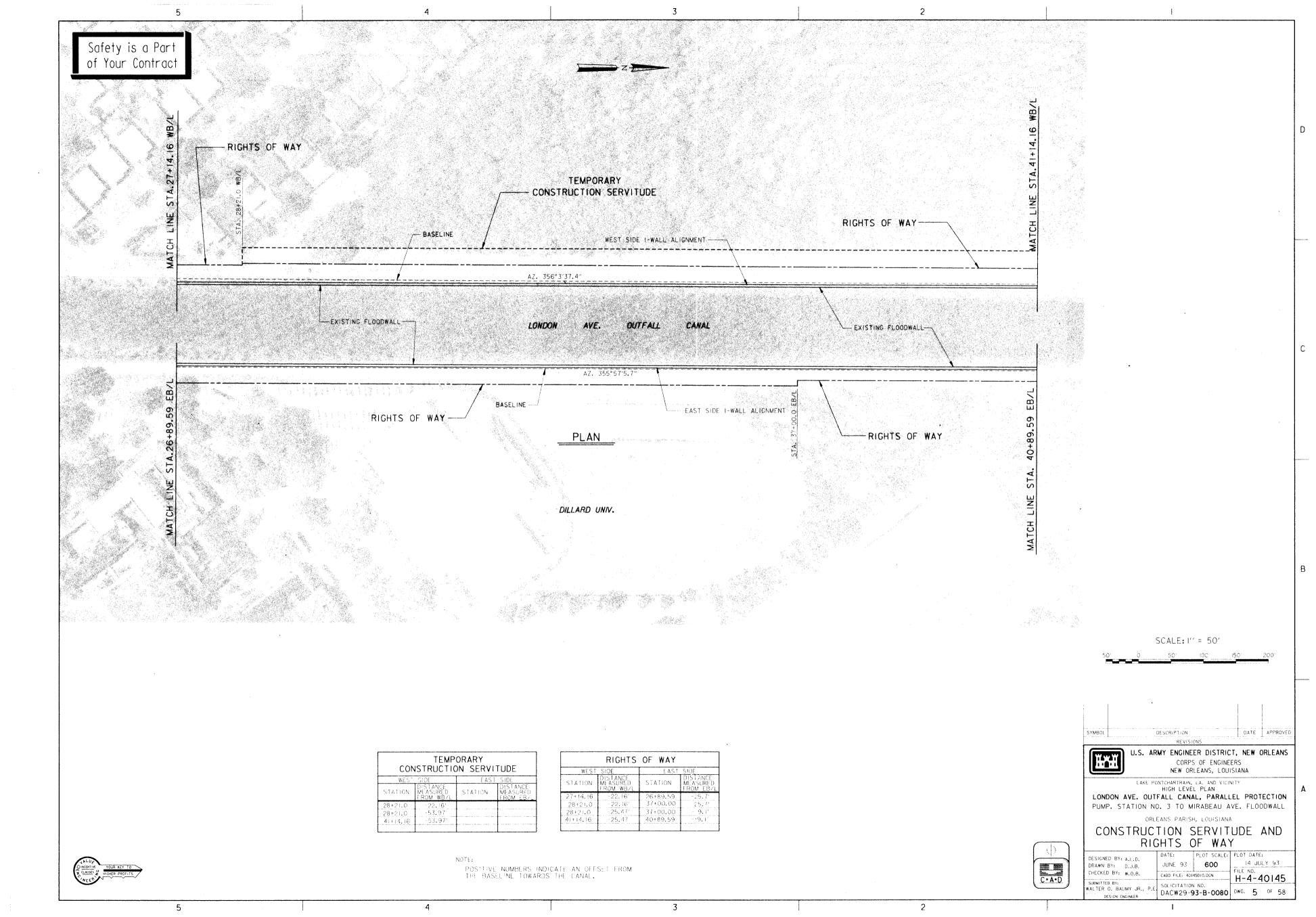
PLOT SCALE: DESIGNED BY: A.L.D. 20 JULY 93 JUNE 93 - 1 CHECKED BY: W.O.B. FILE NO. H-4-40145 SUBMITTED BY: WALTER O. BAUMY JR., P. SOLICITATION NO DACW29-93-B-0080 DWG. 2 OF 58

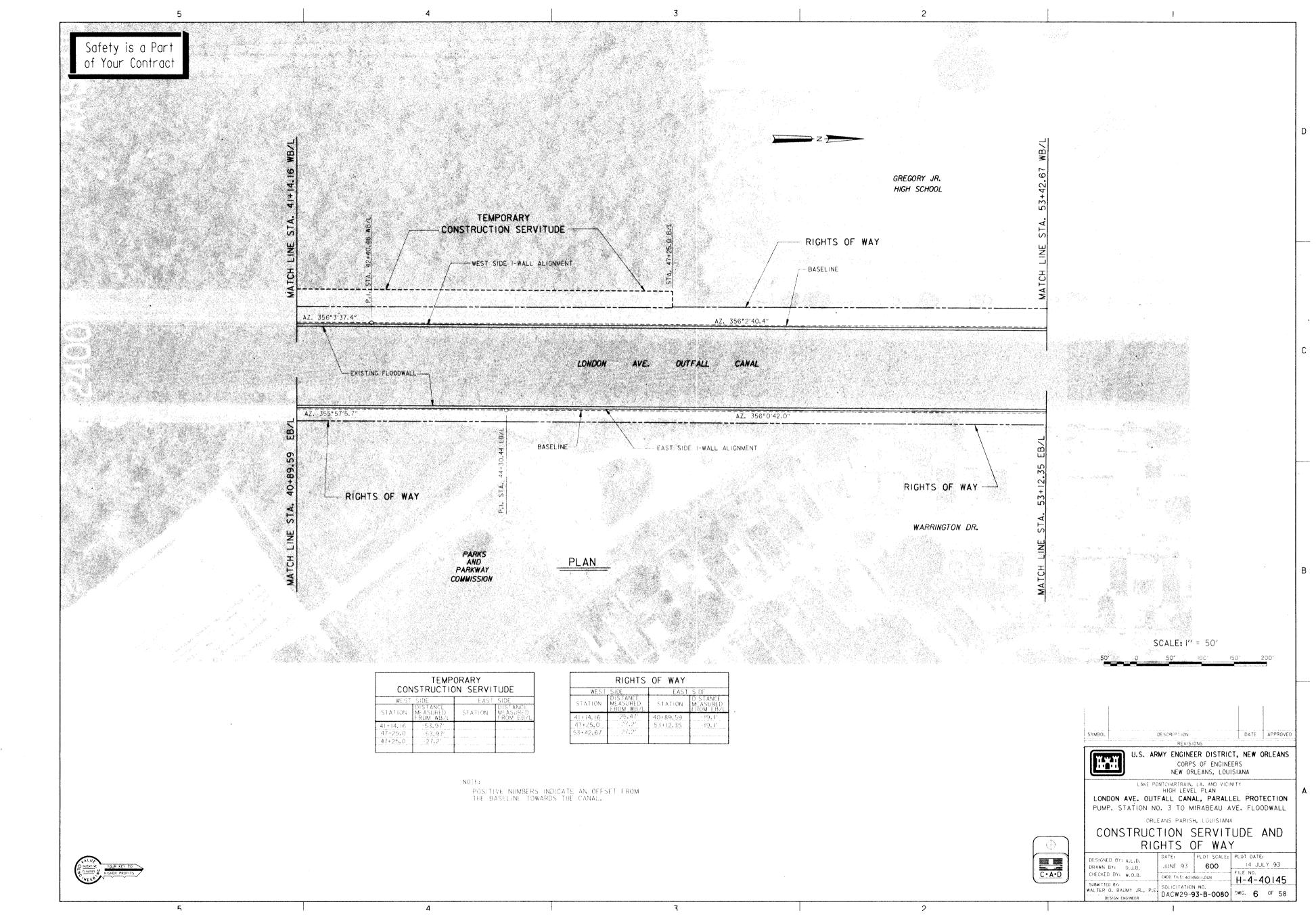
3

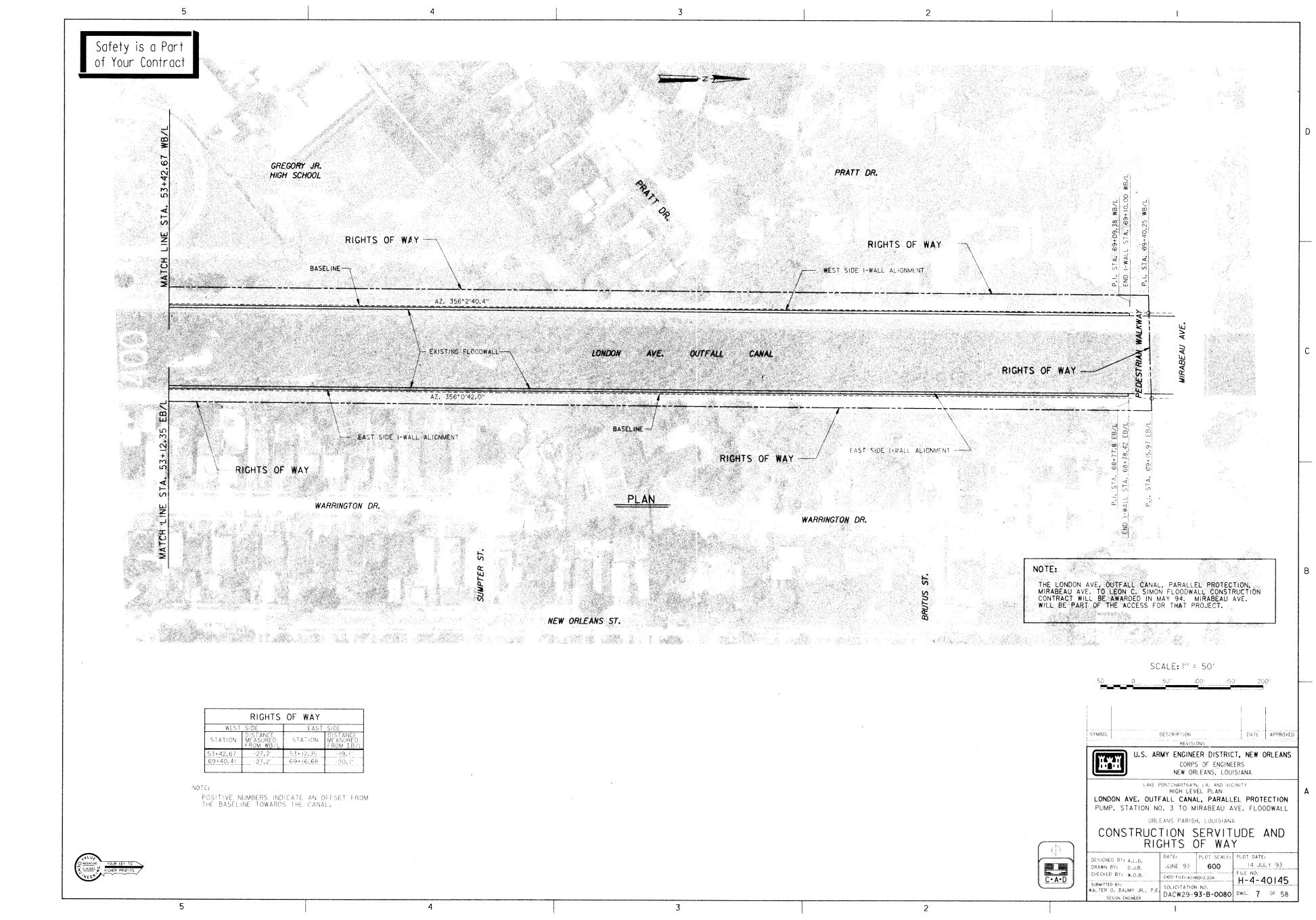
2

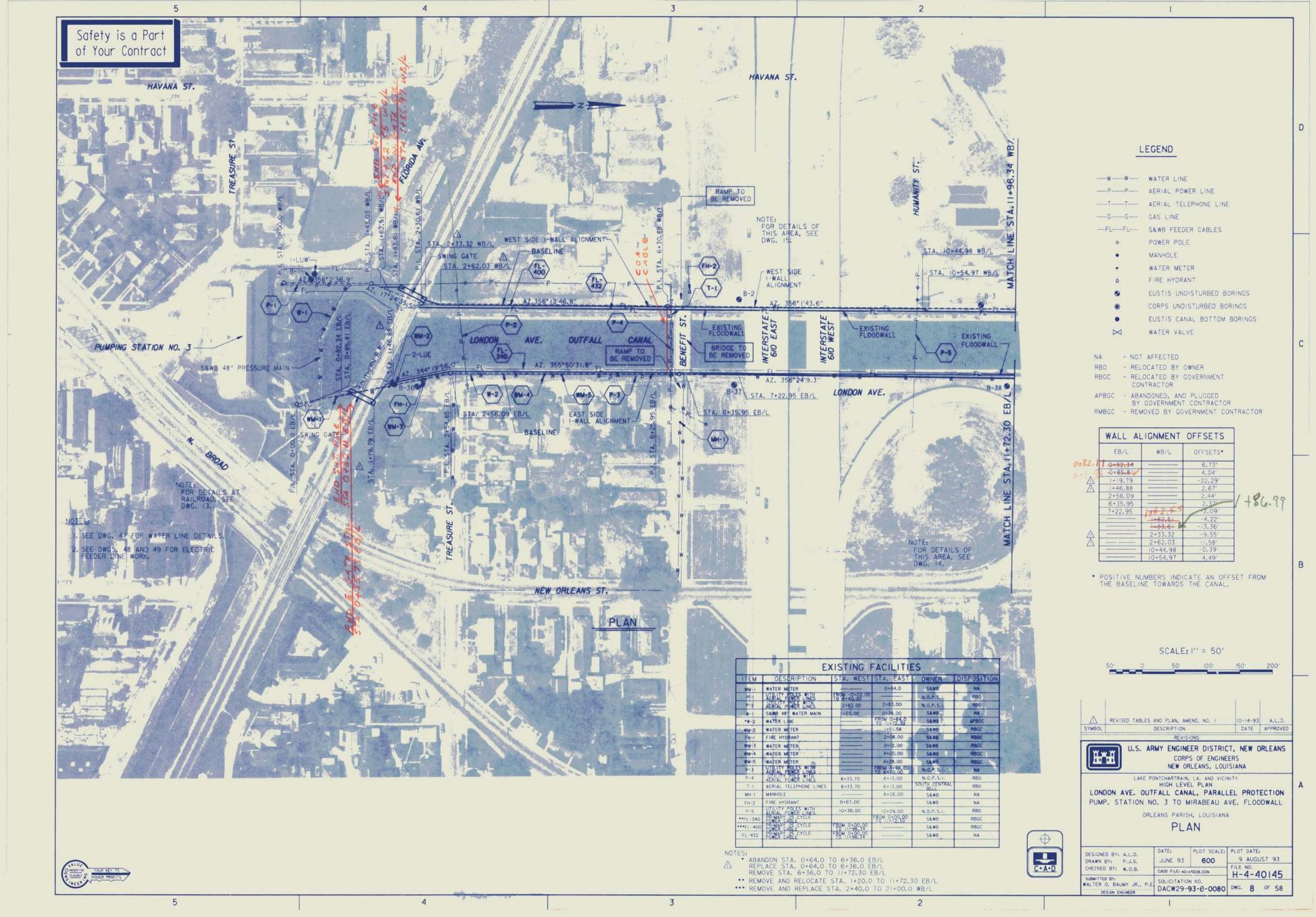


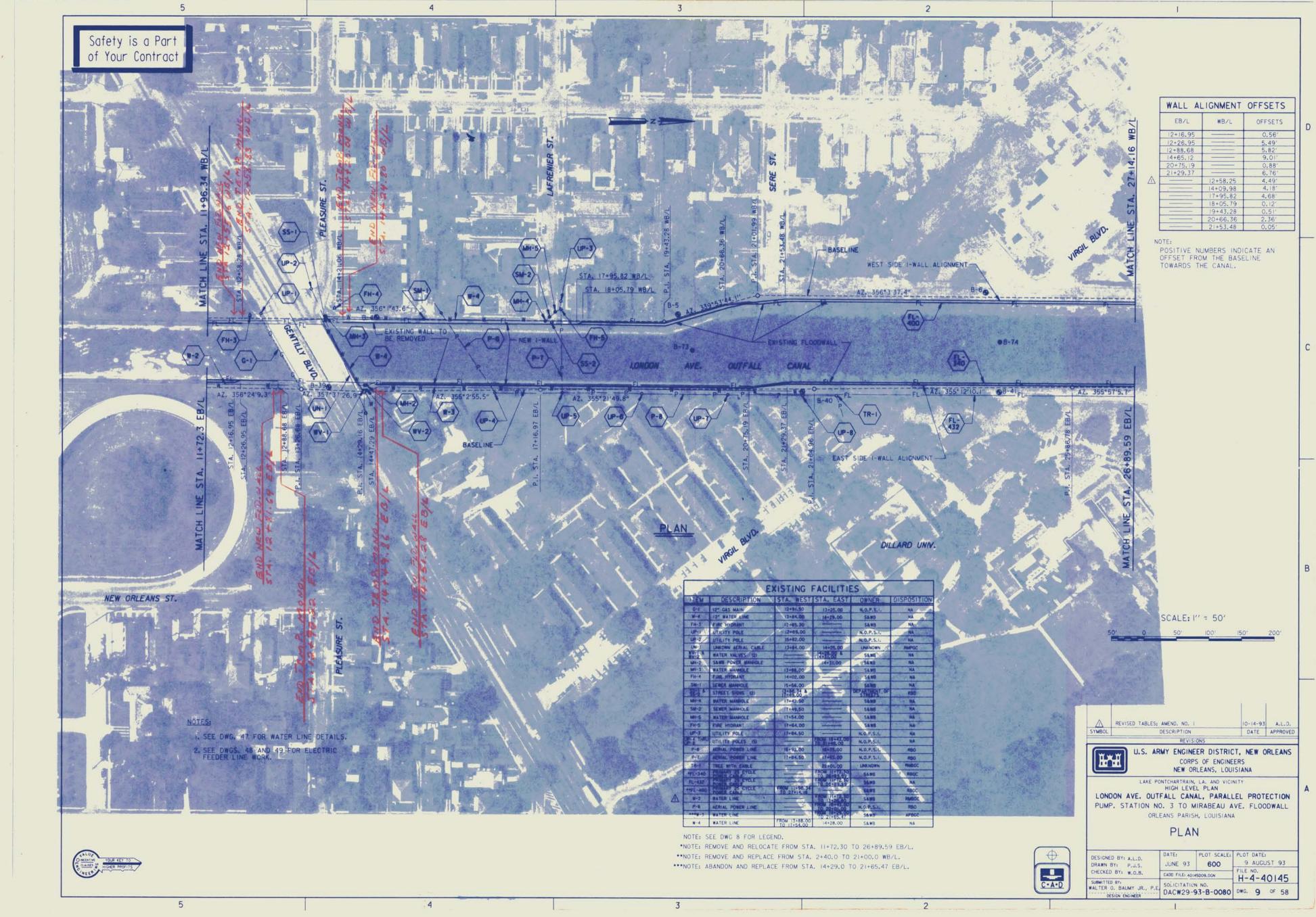


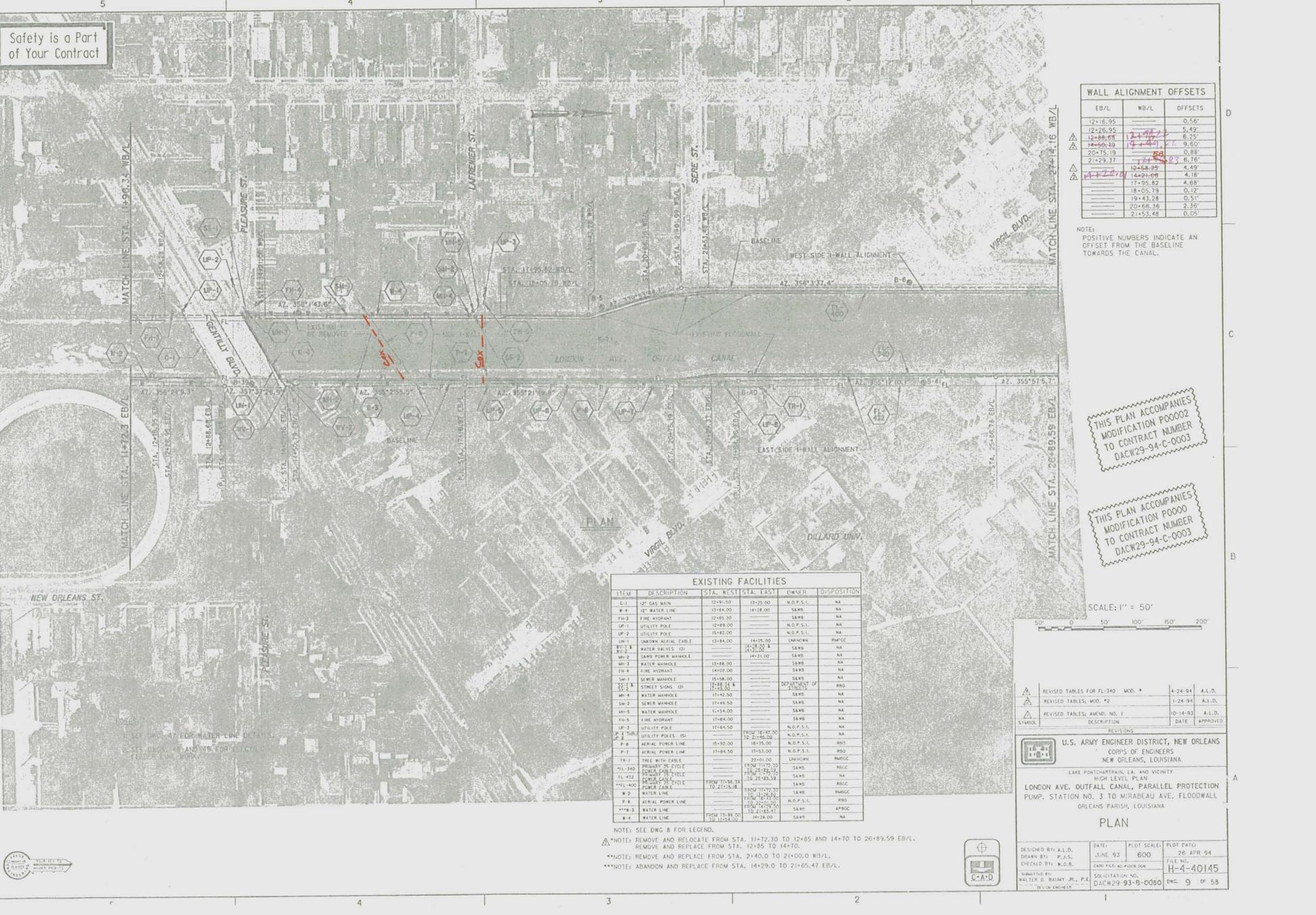




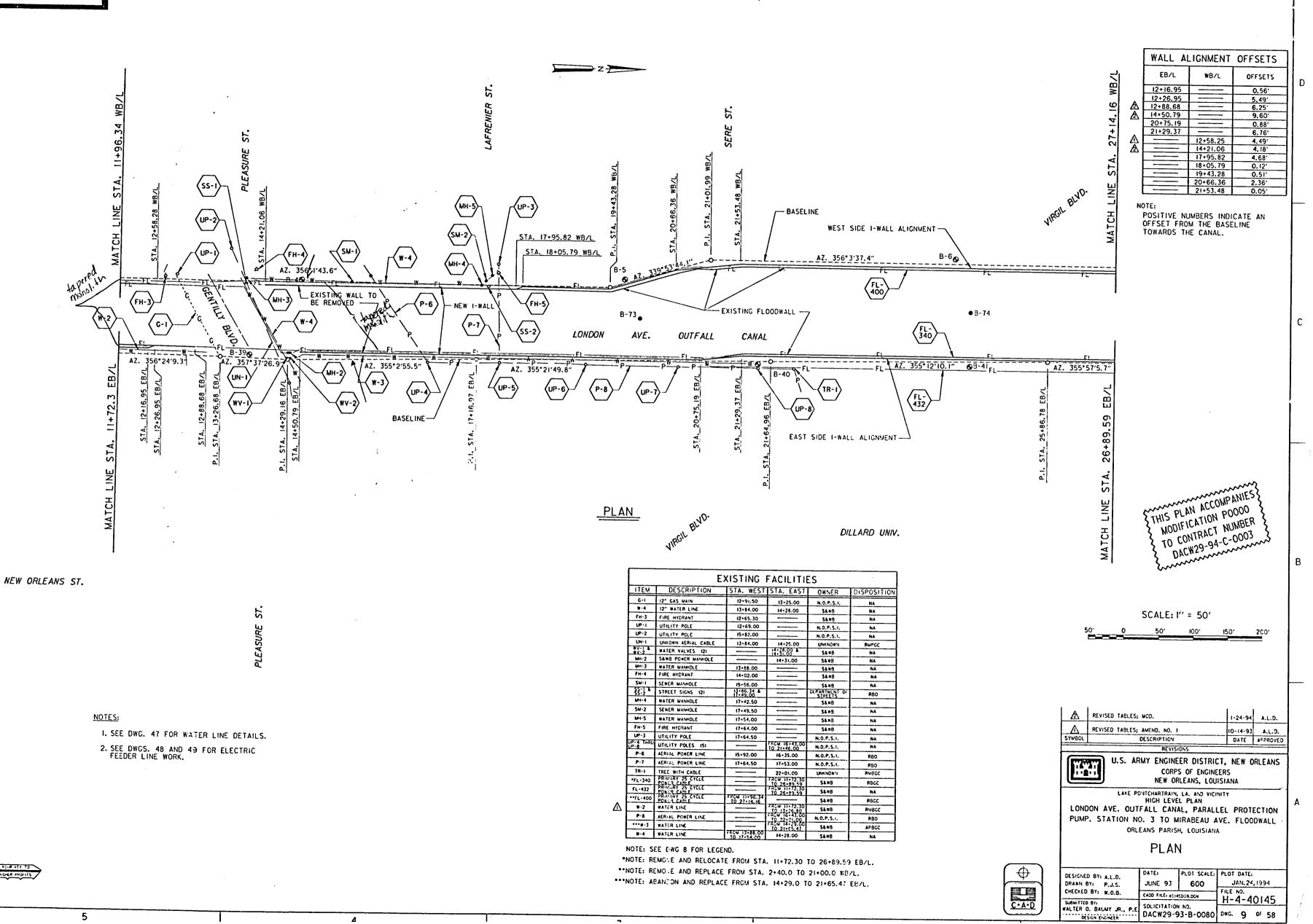


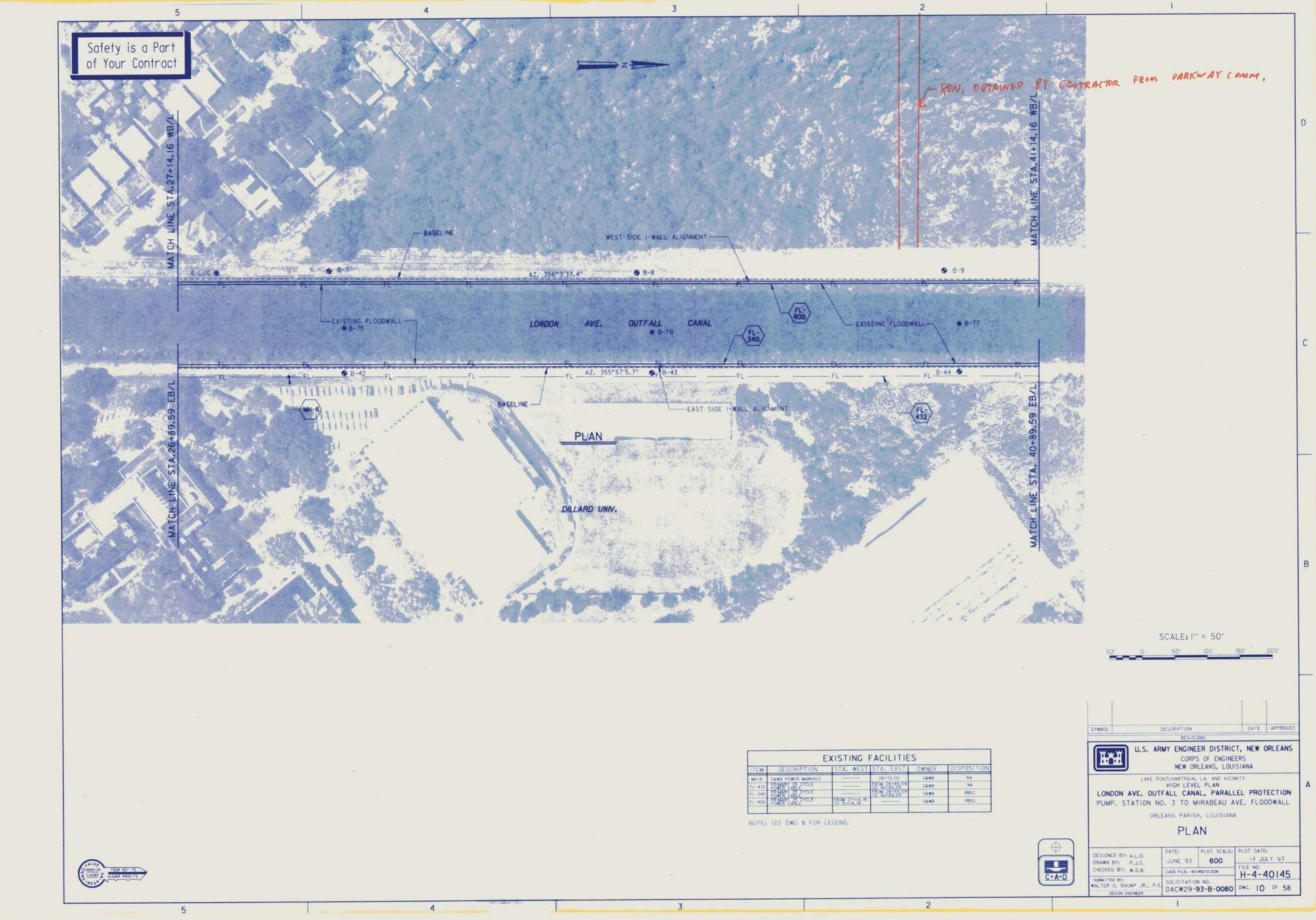


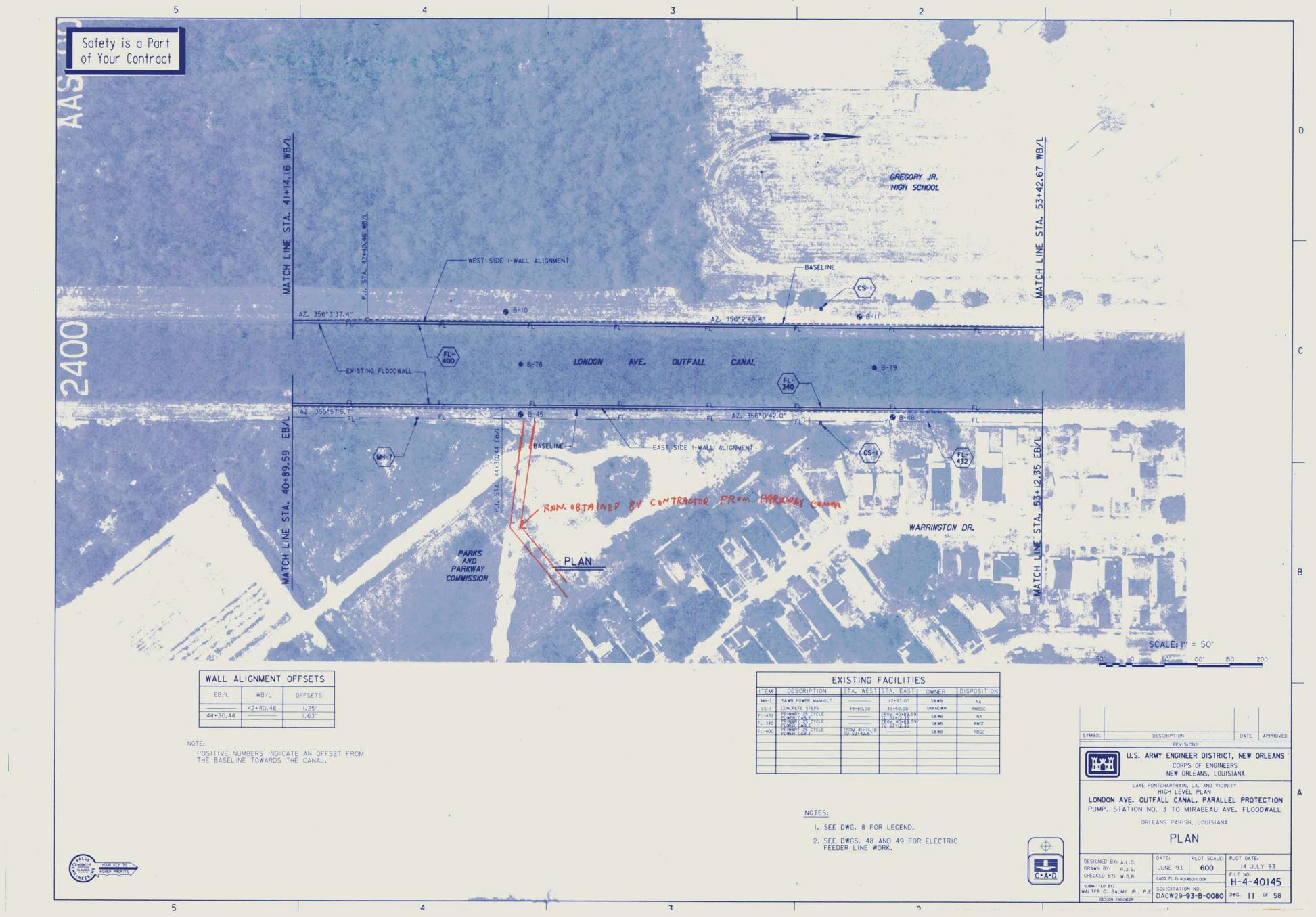


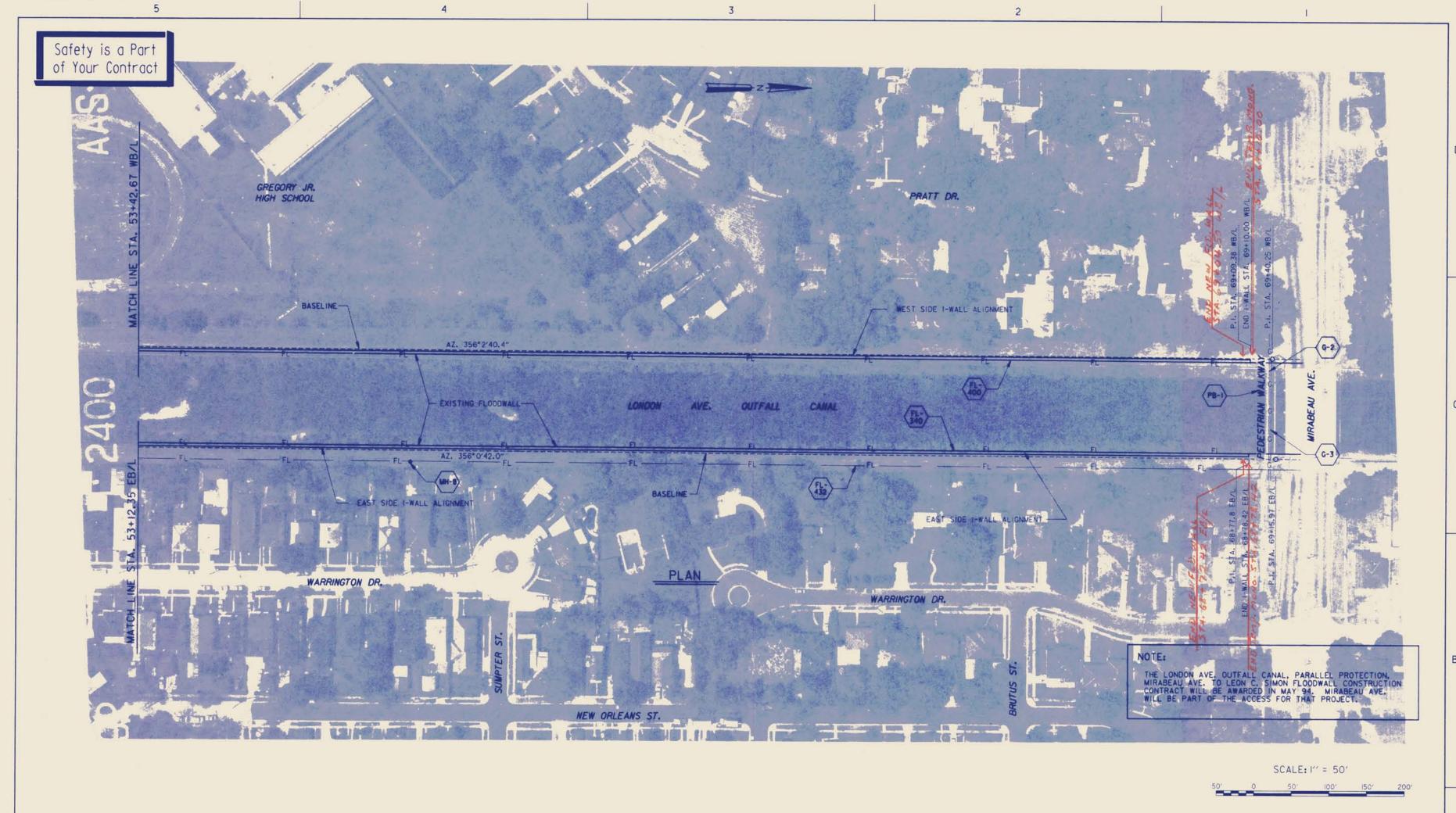


Safety is a Part of Your Contract









WALL ALIGNMENT OFFSETS OFFSETS 68+78.42

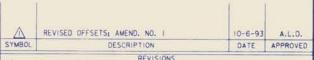
EXISTING FACILITIES N.O.P.S.I. N.O.P.S.I. 69+38.00 S&WB S&WB S&WB

NOTE:

POSITIVE NUMBERS INDICATE AN OFFSET FROM THE BASELINE TOWARDS THE CANAL.

NOTES:

- I. SEE DWG. 8 FOR LEGEND.
- 2. CONCRETE STEPS LEADING TO PB-I TO BE REMOVED.
- 3. SEE DWGS. 48 AND 49 FOR ELECTRIC FEEDER LINE WORK.





C-A-D

U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA

LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN
LONDON AVE. OUTFALL CANAL, PARALLEL PROTECTION

PUMP. STATION NO. 3 TO MIRABEAU AVE. FLOODWALL

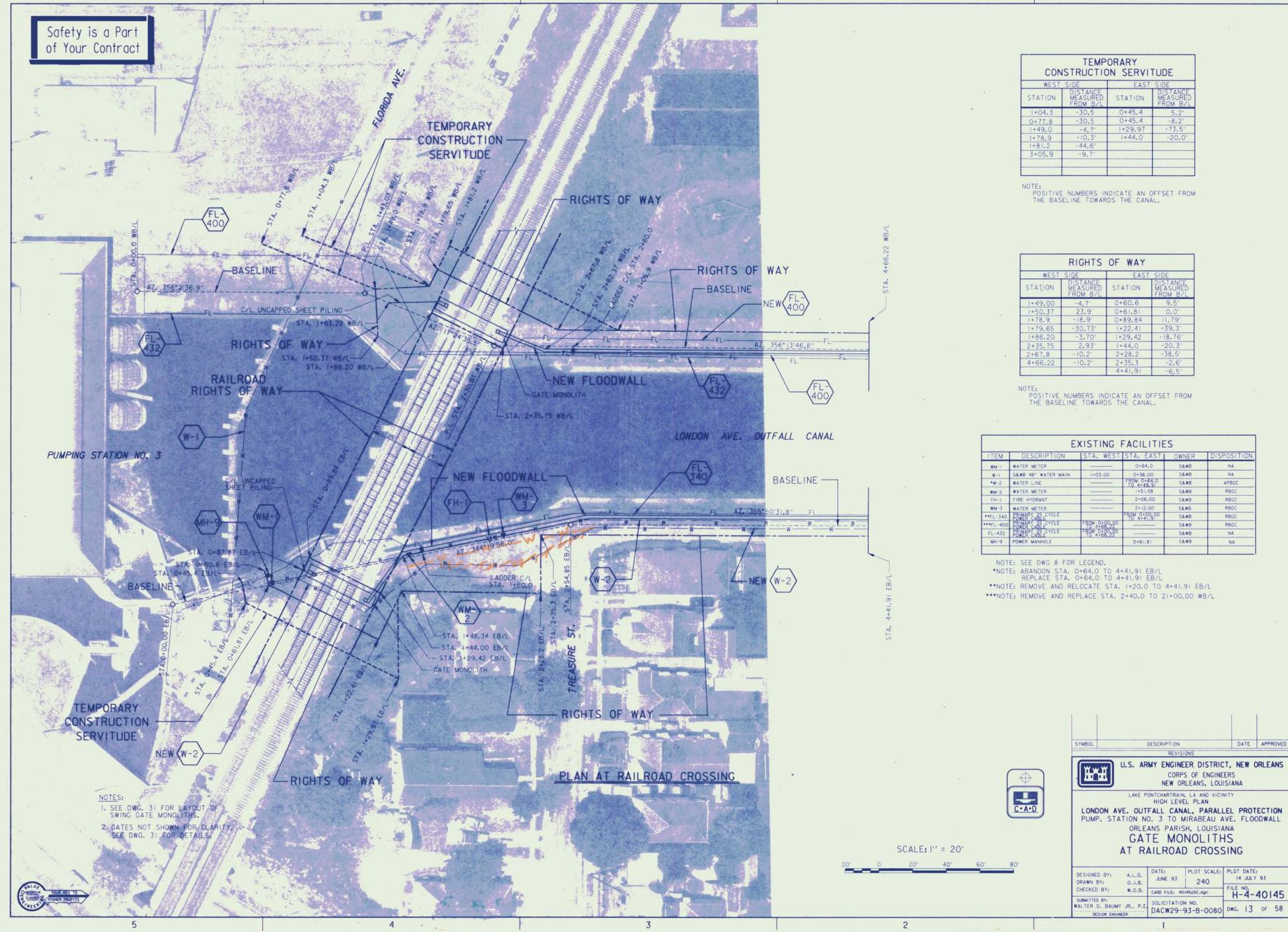
ORLEANS PARISH, LOUISIANA

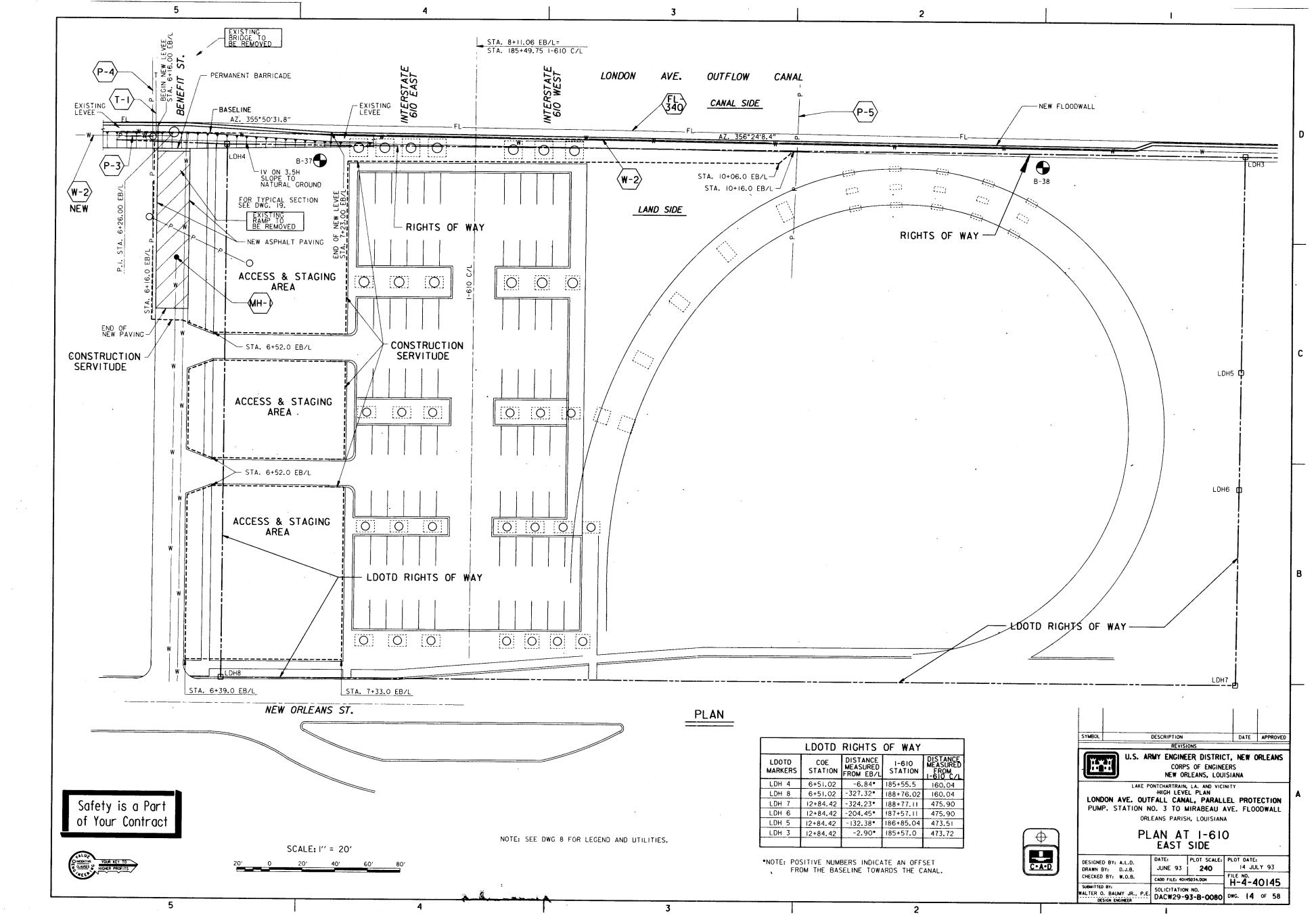
PLAN

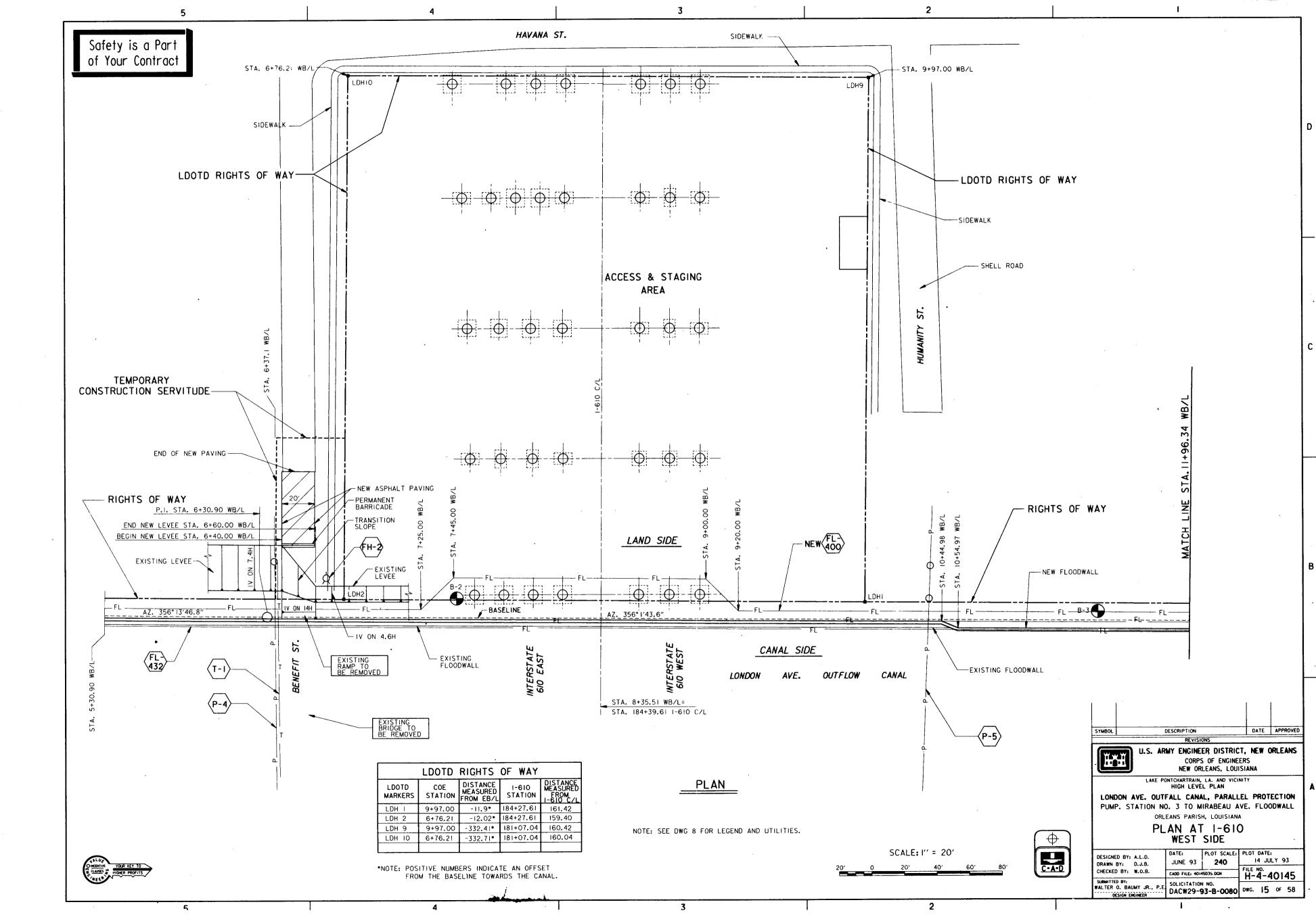
DESIGNED BY: A.L.D. DRAWN BY: P.J.S.	DATE: PLOT SCALE: JUNE 93 600		PLOT DATE: 9 AUGUST 93		
CHECKED BY: W.O.B.	CADD FILE: 401	45D12.06N	FILE NO.		
SUBMITTED BY:			H-4-40145		
WALTER O. BAUMY JR., P.E. DESIGN ENGINEER	DACW29-	93-B-0080	DWG. 12 OF 58		

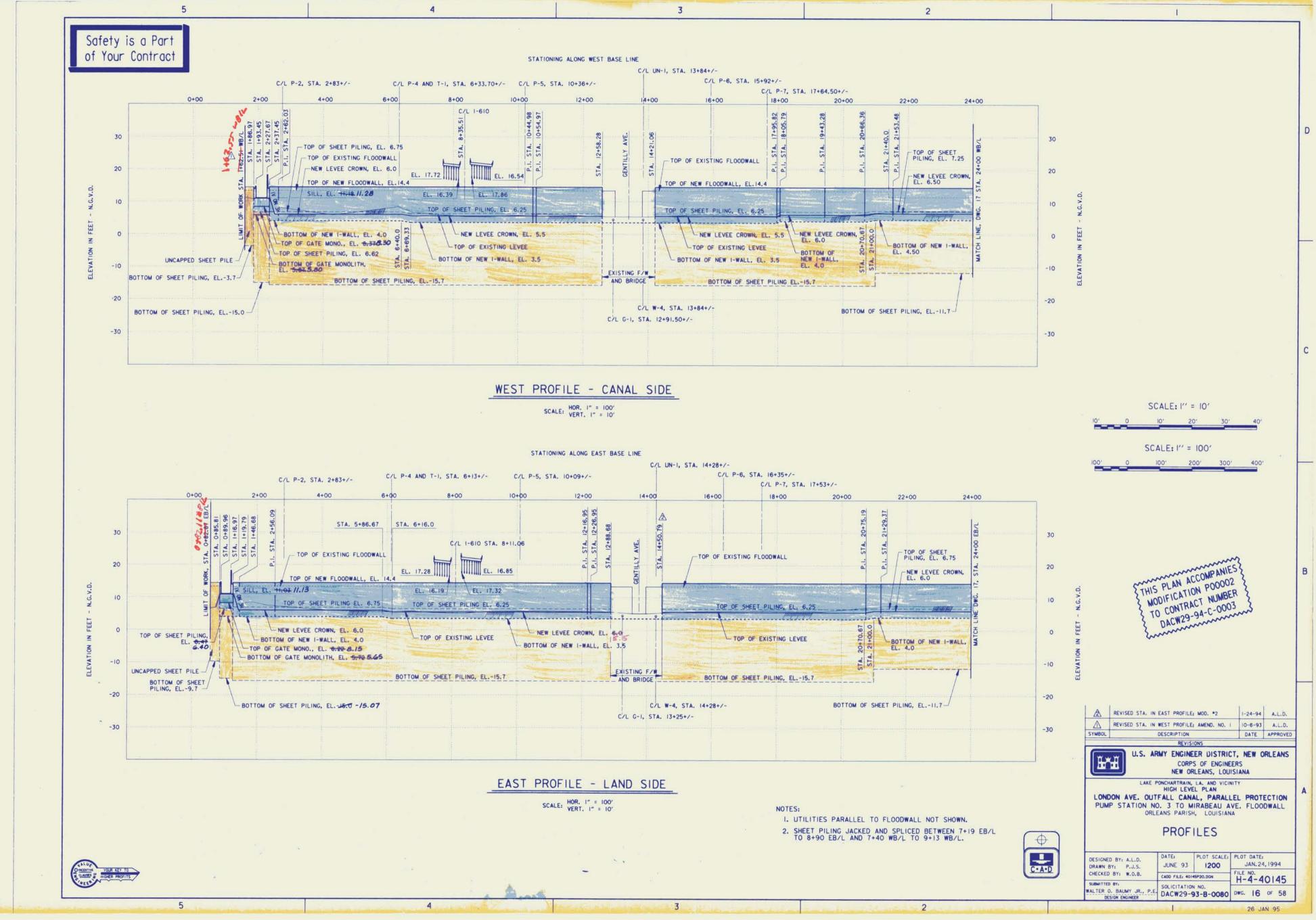


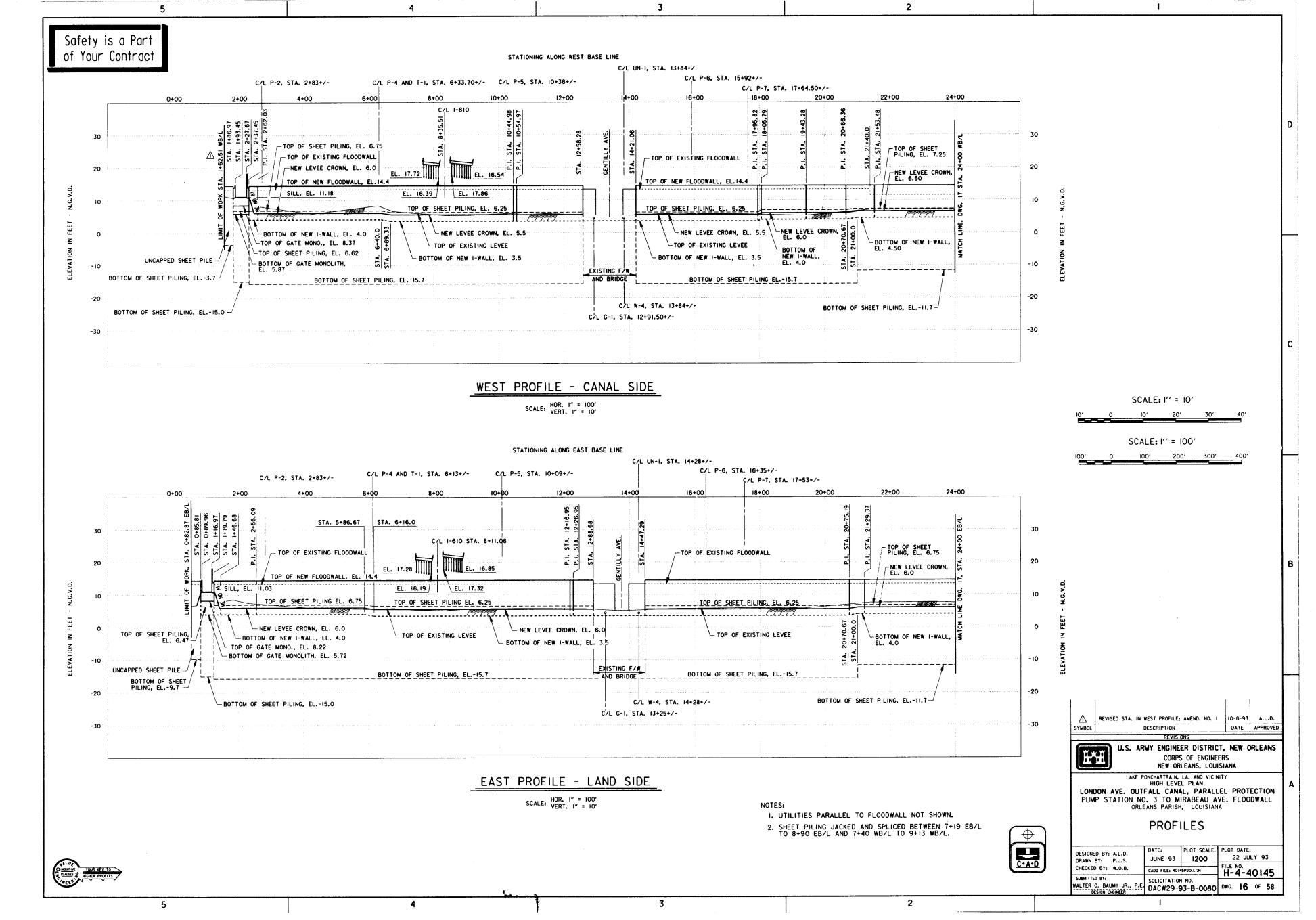
2

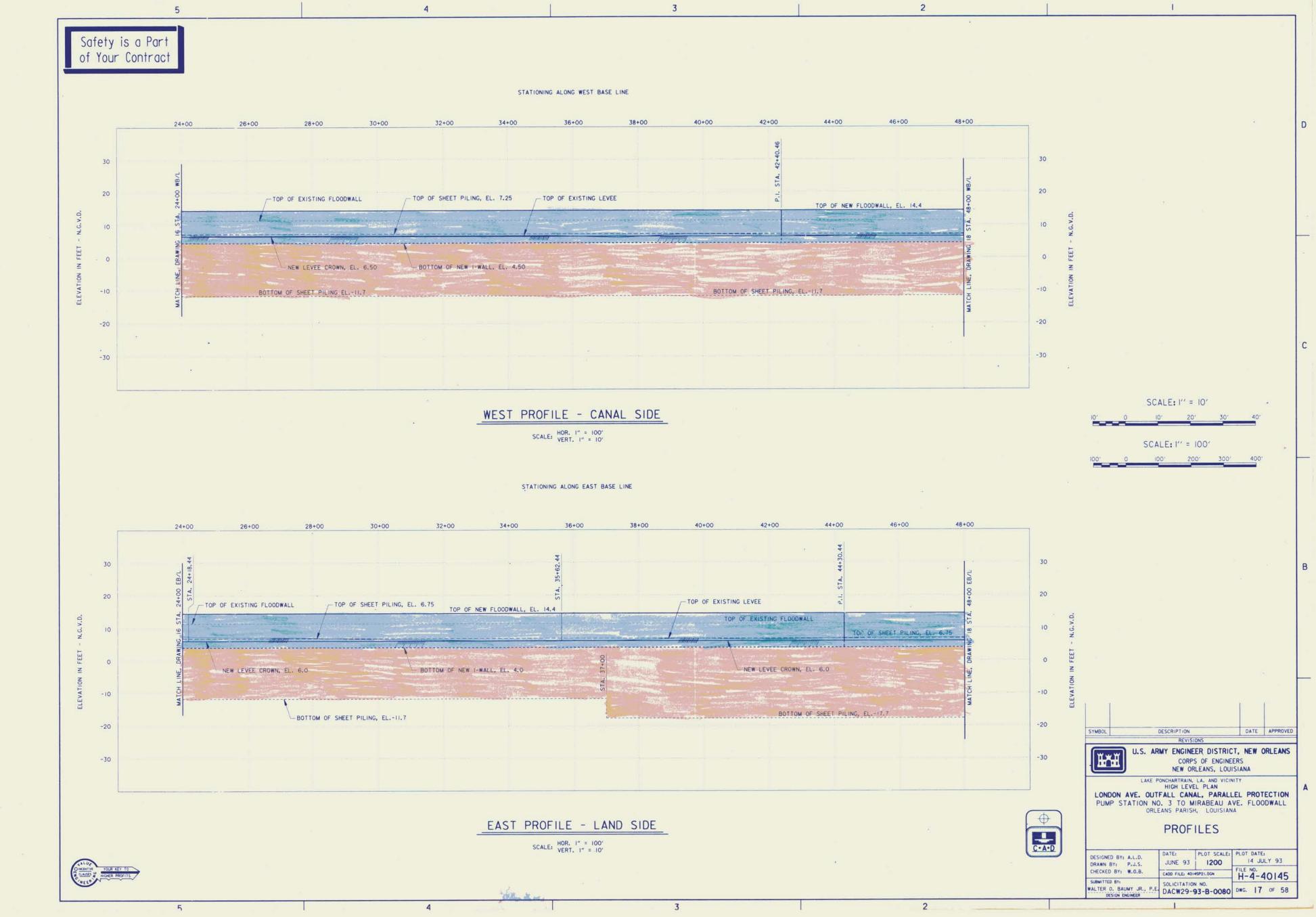


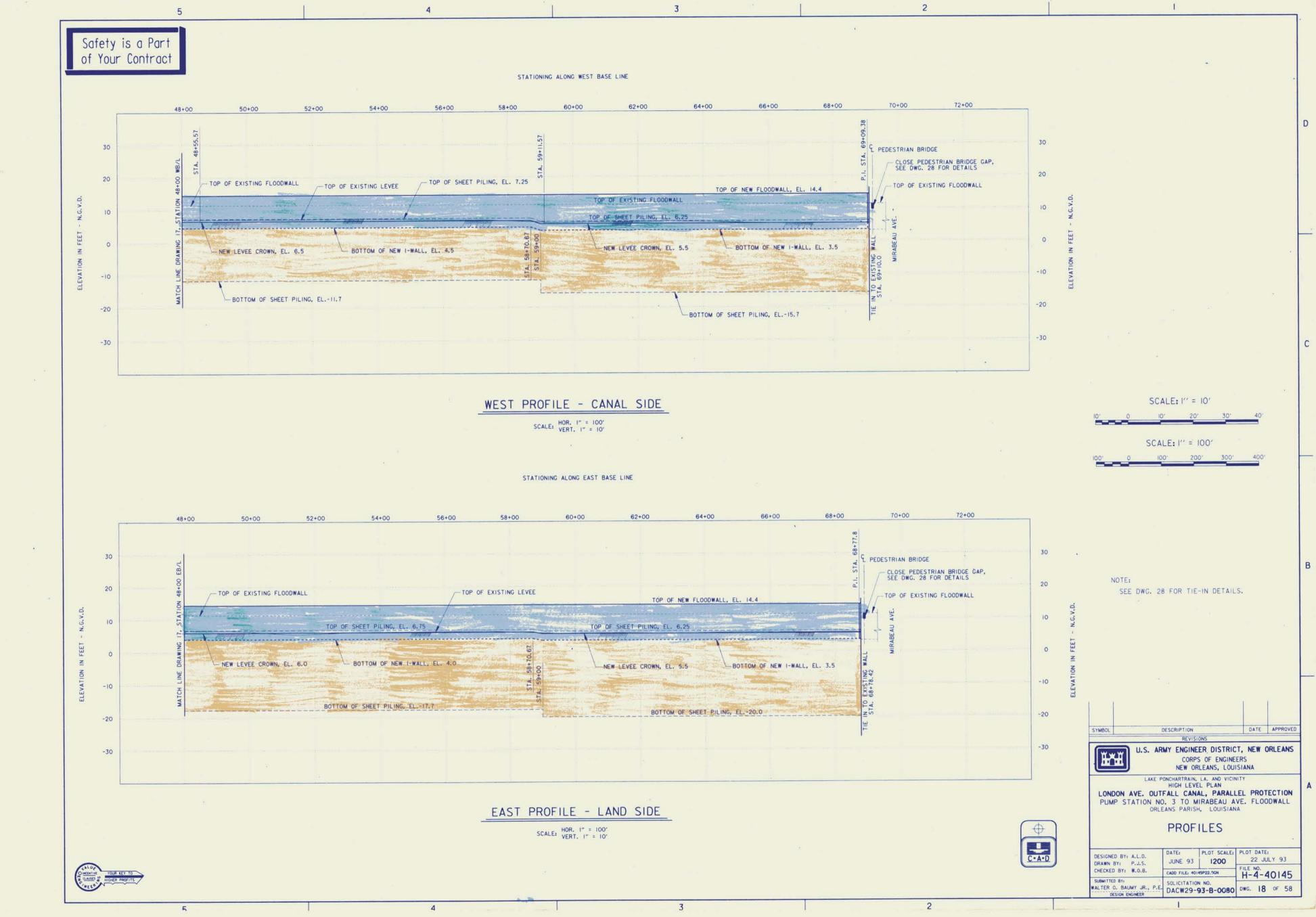


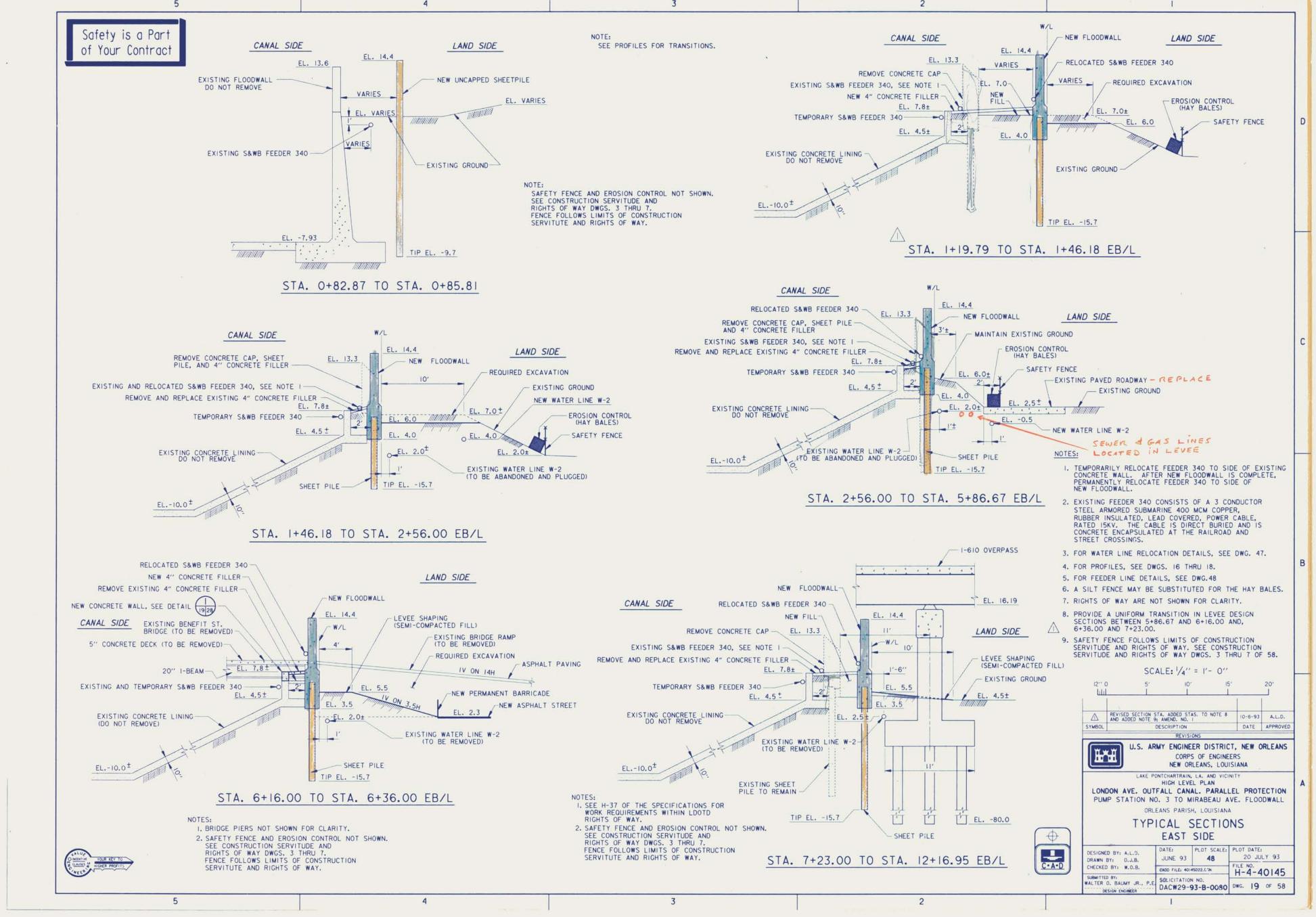












2 SEE PROFILES FOR TRANSITIONS. Safety is a Part of Your Contract CANAL SIDE LAND SIDE NEW FLOODWALL-LAND SIDE CANAL SIDE EL. 14.4 NEW FLOODWALL RELOCATED S&WB FEEDER 340 COMPACTED FILL-EL. 14.4 MAINTAIN EXISTING GROUND REMOVE CONCRETE CAP AND SHEET PILE EXISTING FLOODWALL TO BE REMOVED EROSION CONTROL (HAY BALES) NEW LEVEE SHAPING (SEMICOMPACTED FILL) EXISTING S&WB FEEDER 340, SEE NOTE 1, DWG 19-RELOCATED S&WB FEEDER 340 REMOVE AND REPLACE 4" CONCRETE FILLER - SAFETY FENCE EL. 8.0 EL. 7.8± EXISTING S&WB FEEDER 340, SEE NOTE TEMPORARY S&WB FEEDER 340 -I' (TYP) EXISTING FENCE EXISTING S&WB FEEDER 432 (FROM STA. 14+20-TO APPROX. STA. 21+00 ONLY) EL. 5.5 TEMPORARY S&WB FEEDER 340 EL. 4.5 ± EL. 4.0 REMOVE PILING AND CONCRETE CAP ABOVE EL. 5.5 O- EL. 2.5 + NEW WATER LINE W-3
(FROM STA 14+29 TO 21+00 ONLY) EXISTING CONCRETE LINING EL. 1.0± NEW WATER LINE W-3 (FROM STA 21+00 TO 21+65,47)

EL. 2.5 + EXISTING WATER LINE W-3 (FROM STA 14+29 TO 21+00 ONLY)

EXISTING WATER LINE W-2 (FROM STA 10+00 TO 13+26 ONLY)

(TO BE REMOVED)

(TO BE ABANDONED AND PLUGGED)

STA. 12+26.95 TO STA. 12+85.18 EB/L STA. 14+50.79 TO STA. 20+70.67 EB/L SCALE: 1/4" = 1'-0"

TIP EL. -15.7

SHEET PILE-

EL.-10.0±

STA. 21+00.0 TO STA. 36+70.67 EB/L SCALE: 1/4" = 1'-0"

EXISTING WATER LINE (FROM STA. 21+00 TO 21+65.47) (TO BE REMOVED)

TIP EL. -11.7

CANAL SIDE LAND SIDE CANAL SIDE LAND SIDE EL. 14.4 NEW FLOODWALL NEW LEVEE SHAPING (SEMICOMPACTED FILL) NEW FLOODWALL EL. 14.4 NEW LEVEE SHAPING (SEMICOMPACTED FILL) EXISTING FLOODWALL TO BE REMOVED -RELOCATED S&WB FEEDER 340 -EROSION CONTROL (HAY BALES) EXISTING FLOODWALL TO BE REMOVED -EXISTING S&WB FEEDER 340, SEE NOTE I RELOCATED S&WB FEEDER 340 L. 6.0 5.5 EROSION CONTROL (HAY BALES) EXISTING S&WB FEEDER 340, SEE NOTE !-SAFETY FENCE REMOVE PILING AND CAP ABOVE EL. 5.5 REMOVE PILING AND CAP ABOVE EL. 5.5 -SAFETY FENCE EL. 3.5 TEMPORARY S&WB FEEDER 340 EL. 4.0 - EXISTING FENCE - EXISTING FENCE EL. 0.0± SLOPE TO DRAIN TEMPORARY S&WB FEEDER 340 EXISTING GROUND EL.-1.5± EXISTING GROUND EXISTING GROUND EL. -2.7± I' (TYP) I' (TYP) EXISTING S&WB FEEDER 432 TIP EL. -20.0 - EXISTING S&WB FEEDER 432 SHEET PILE TO REMAIN-EL. -9.5 ± EL. -9.5 + TIP EL. -17.7 SHEET PILE TO REMAIN -EXISTING GROUND

EXISTING GROUND

EL.-9.5 ±

STA. 37+00 TO STA. 58+70.67 EB/L

SCALE: 1/4" = 1'-0"

4

I. TEMPORARILY RELOCATE FEEDER 340 BY DIGGING A TRENCH, COVERING WITH STEEL PLATE AND LEVEE MATERIAL. AFTER THE CONCRETE CAP IS REMOVED FROM THE SHEET PILE, FEEDER 340 SHALL BE REMOVED FROM THE TEMPORARY TRENCH AND ATTACHED TO THE NEW FLOODWALL.

1 2. SEE NOTES 2 THRU 7 AND 9, DWG. 19.

3. PROVIDE A UNIFORM TRANSITION IN LEVEE DESIGN SECTIONS BETWEEN STAS. 12+16.95 AND 12+26.95; STAS. 20+70.67 AND 21+00; STAS. 36+70.67 AND 37+00 (CANAL SIDE); STAS. 58+70.67 AND 59+00 EB/L.

STA. 59+00.0 TO STA. 68+78.42 EB/L

SCALE: 1/4" = 1'-0"

SHEET PILE TO REMAIN

SCALE: 1/4" = 1'- 0"

		REVISIONS		
U.S.	ARMY	CORPS OF ENGINE NEW ORLEANS, LOU	EERS	ORLEANS

REVISED NOTE 2; AMEND. NO. 1

LAKE PONTCHARTRAIN, LA. AND VICINITY HIGH LEVEL PLAN
LONDON AVE. OUTFALL CANAL. PARALLEL PROTECTION
PUMP STATION NO. 3 TO MIRABEAU AVE. FLOODWALL

-EROSION CONTROL (HAY BALES)

EXISTING FENCE

-SAFETY FENCE

I' (TYP)

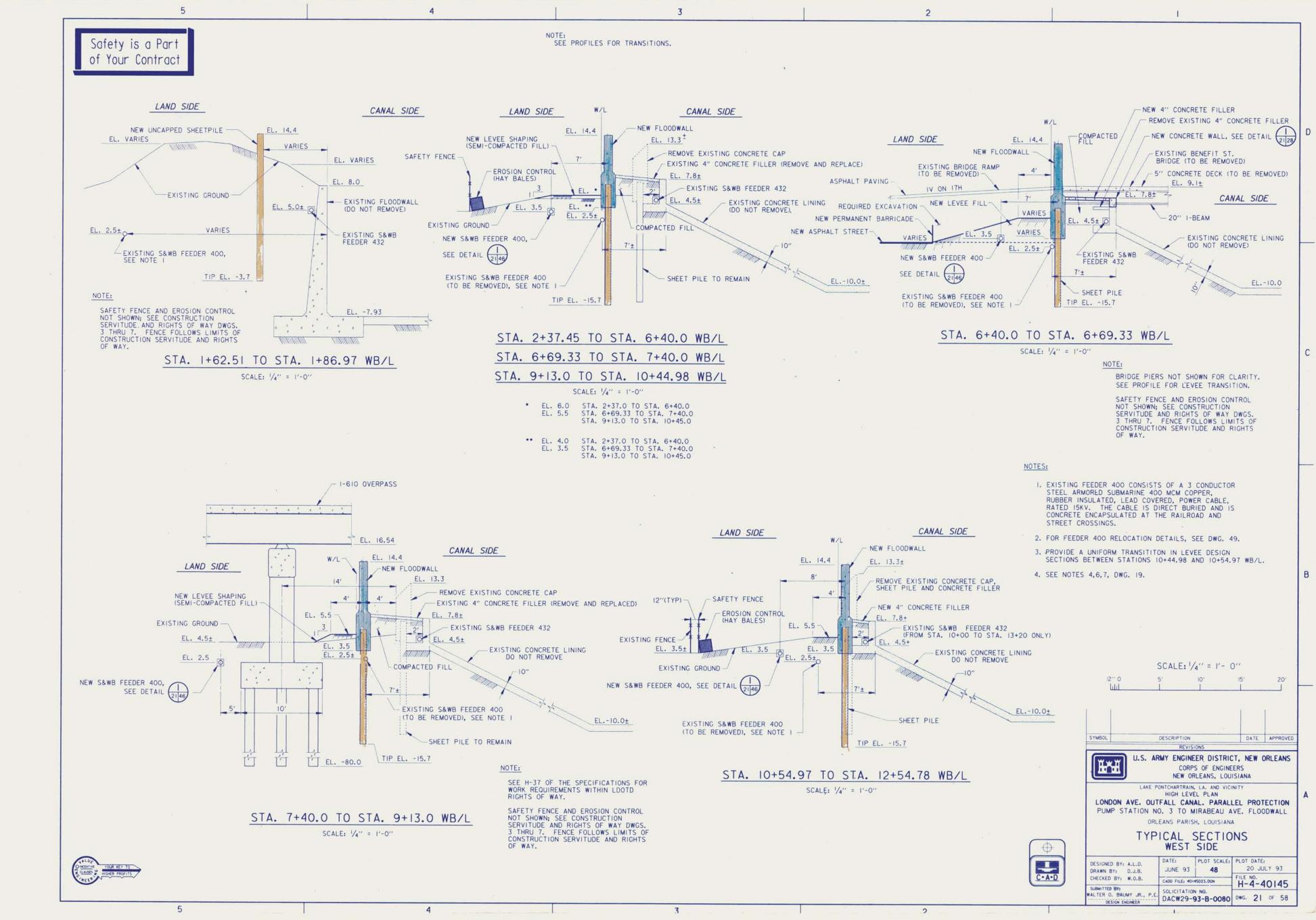
- EXISTING S&WB FEEDER 432

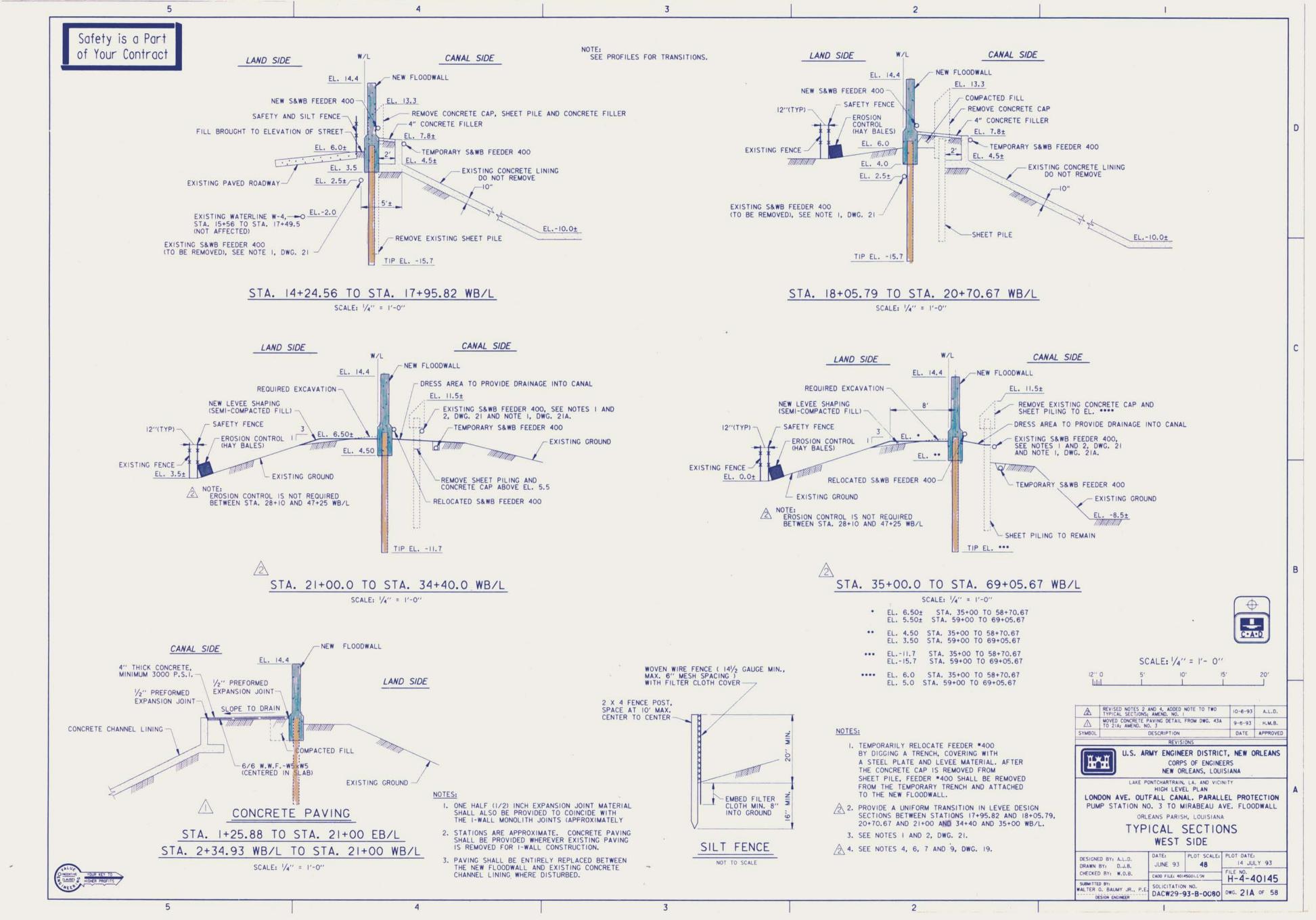
ORLEANS PARISH, LOUISIANA TYPICAL SECTIONS EAST SIDE

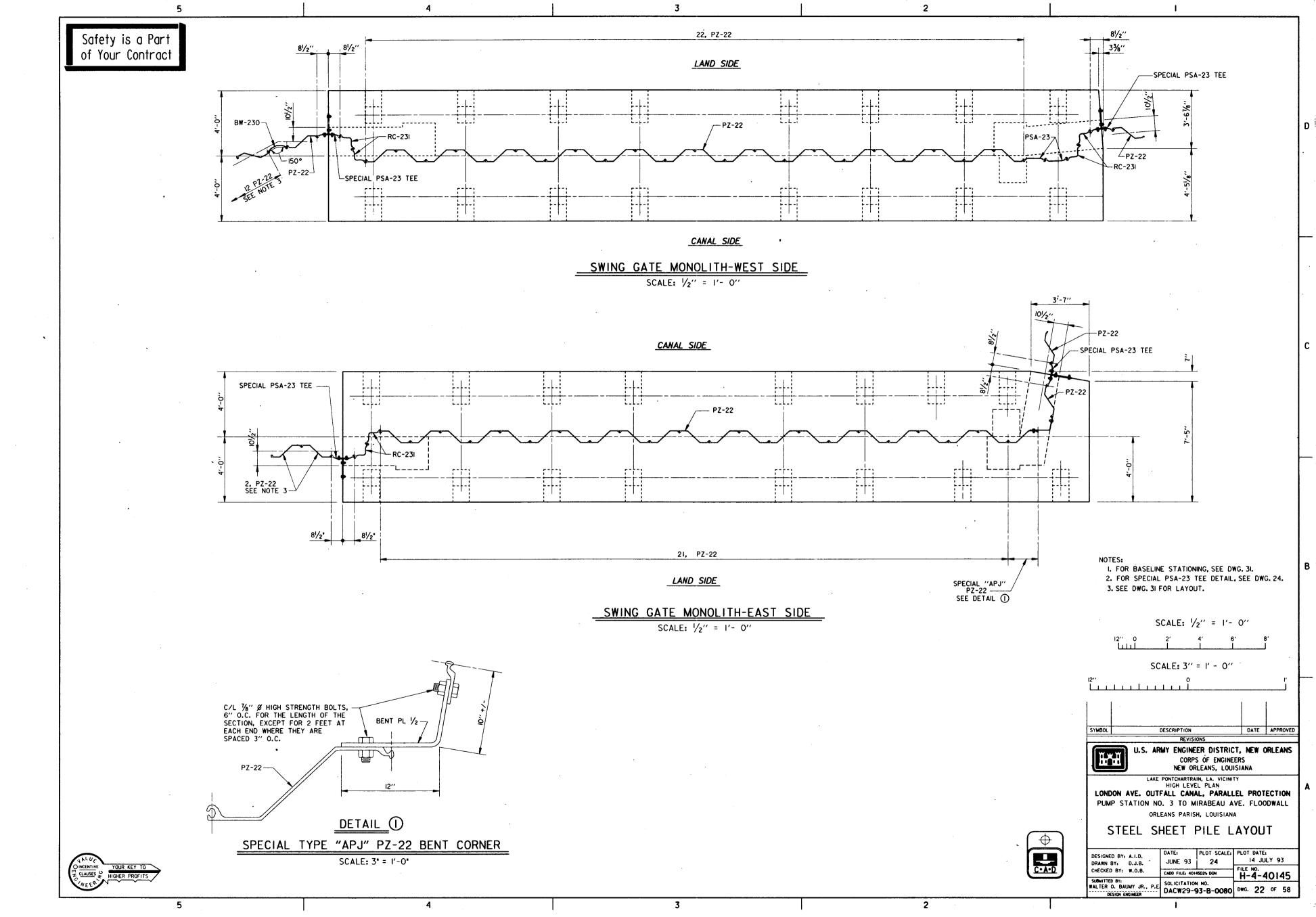
DESIGNED BY: A.L.D. DRAWN BY: P.J.S. CHECKED BY: W.O.B. JUNE 93 48 CADD FILE: 40145503.DON H-4-40145 SUBMITTED BY: WALTER O. BAUMY JR., P.E SOLICITATION NO. DACW29-93-B-0080 DWG. 20 OF 58

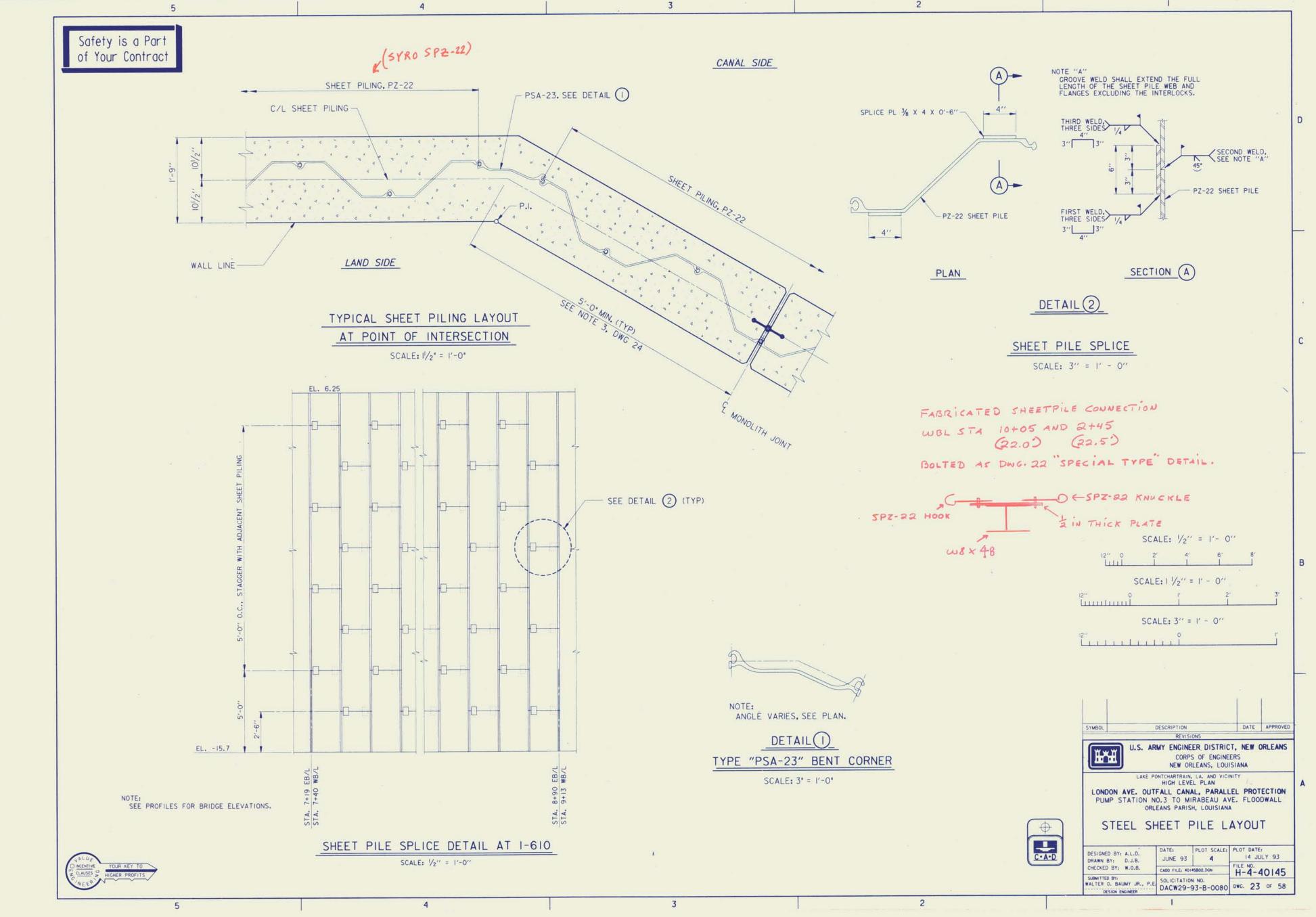
2

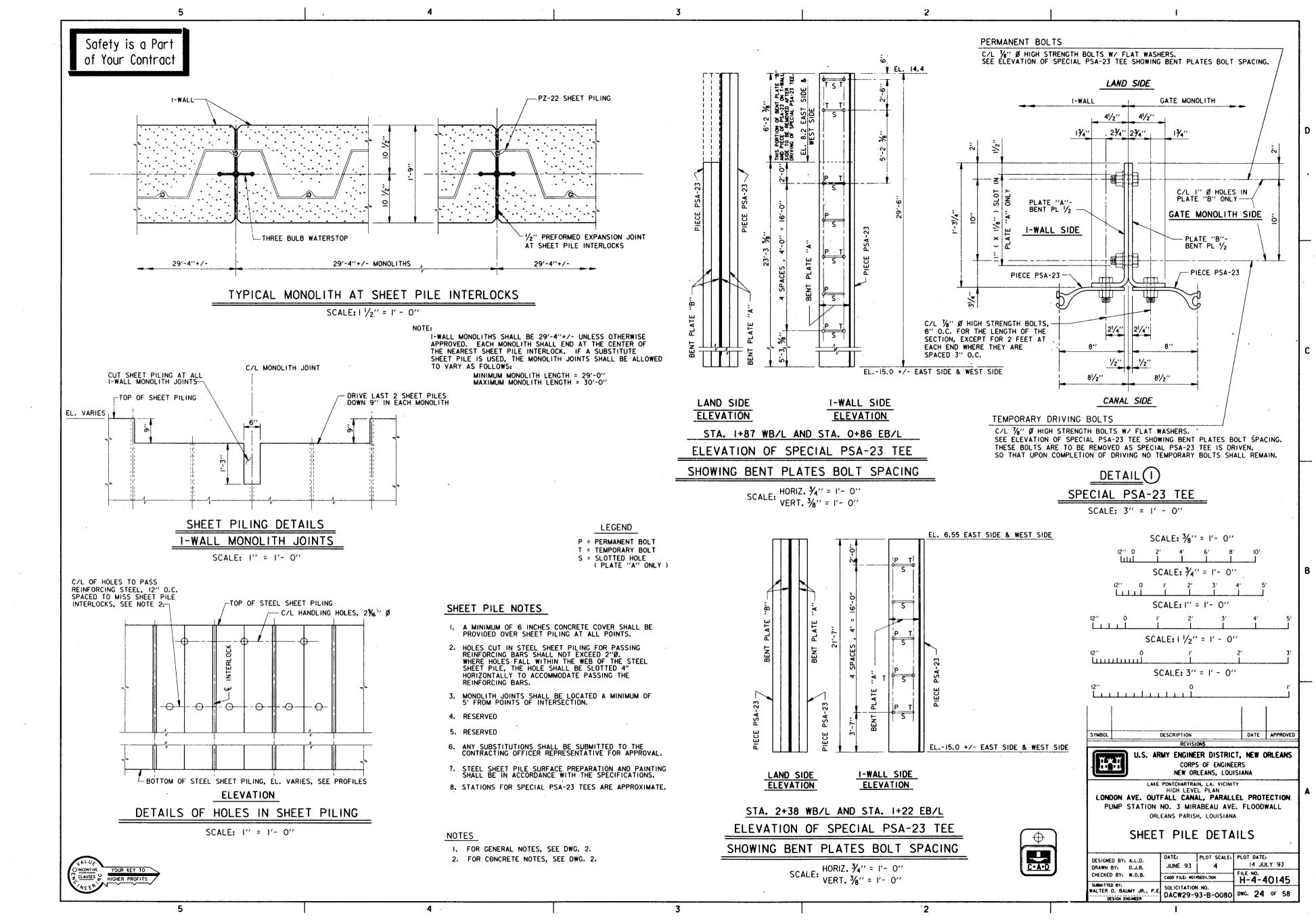
C-A-D

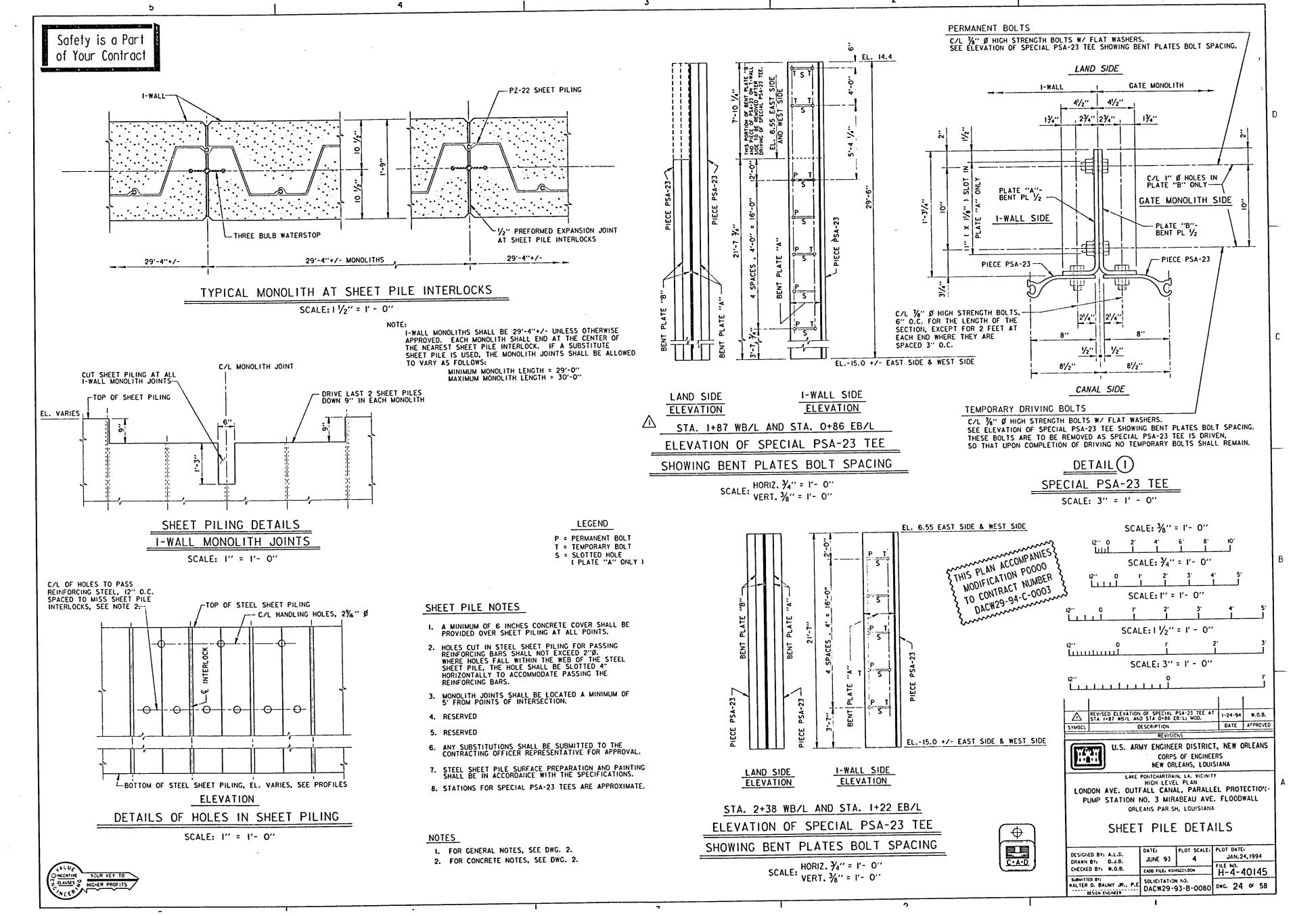


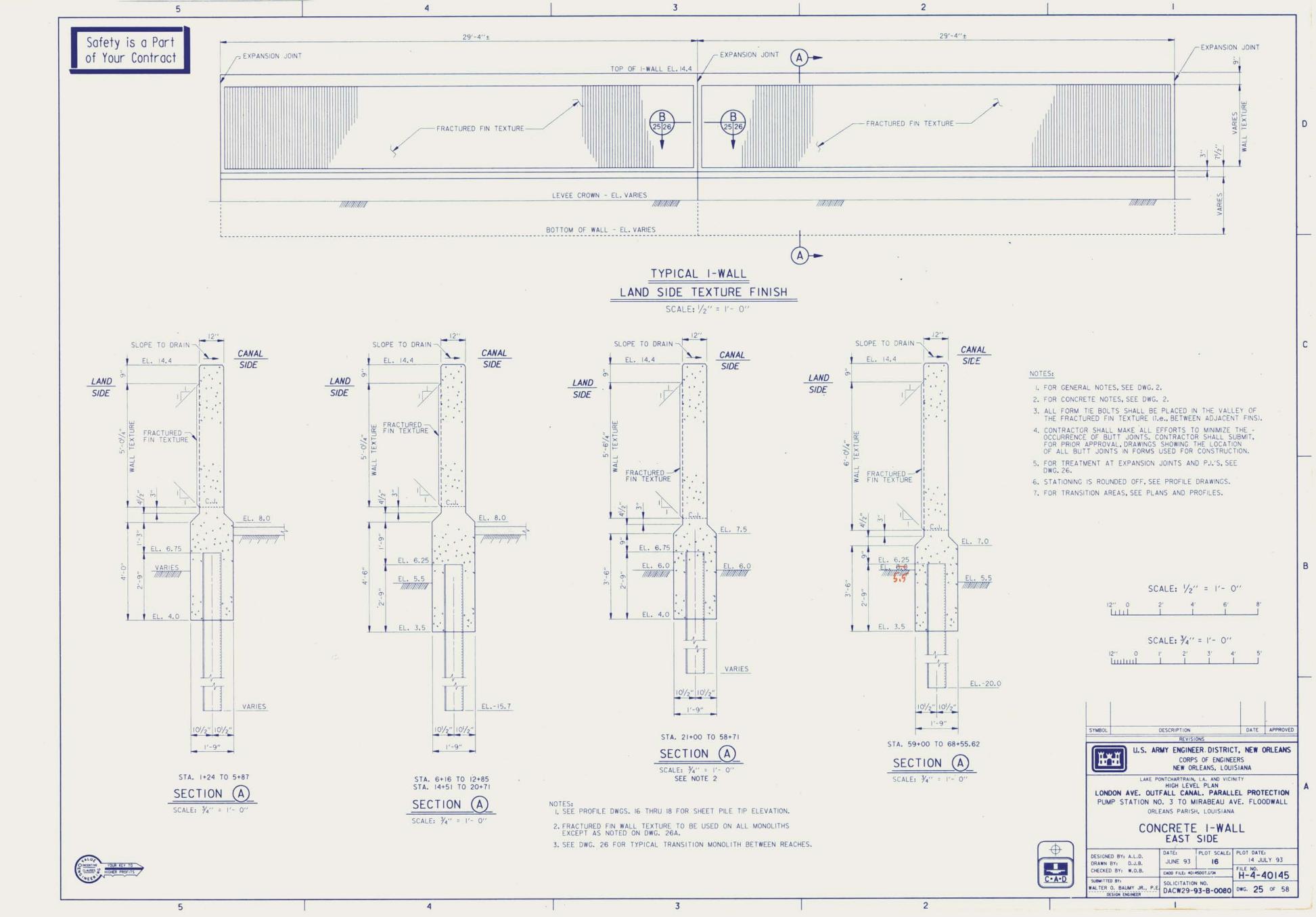


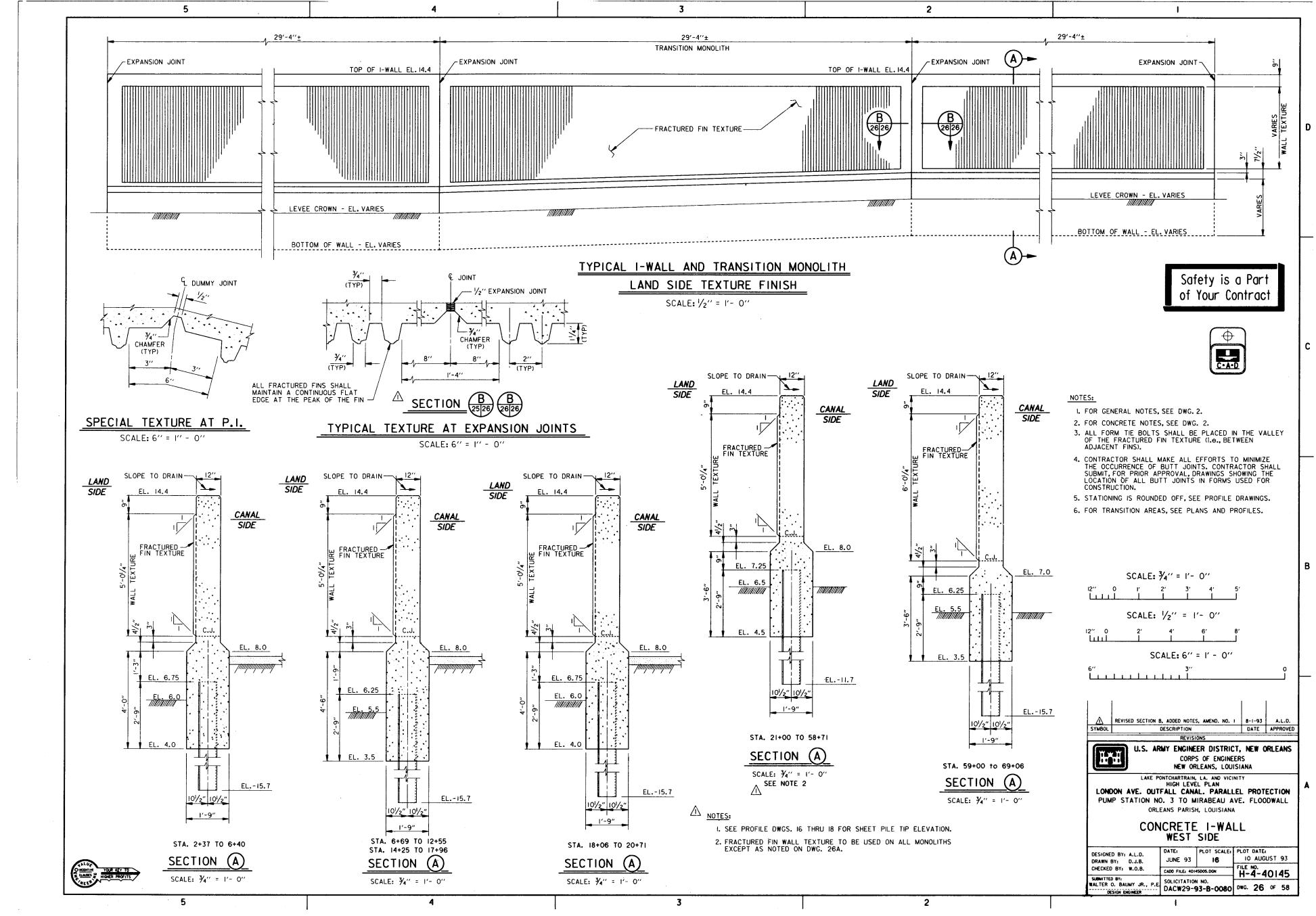


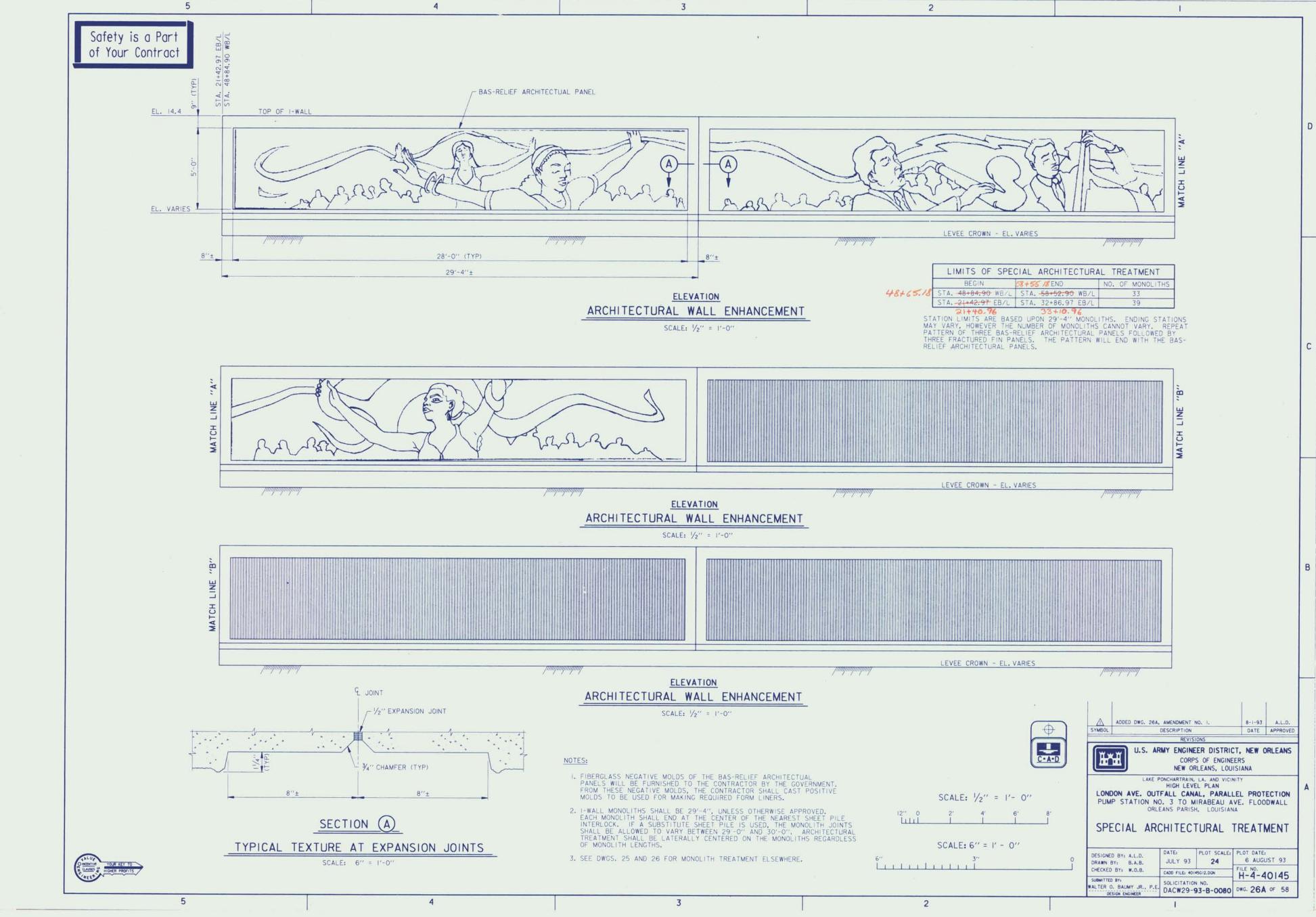


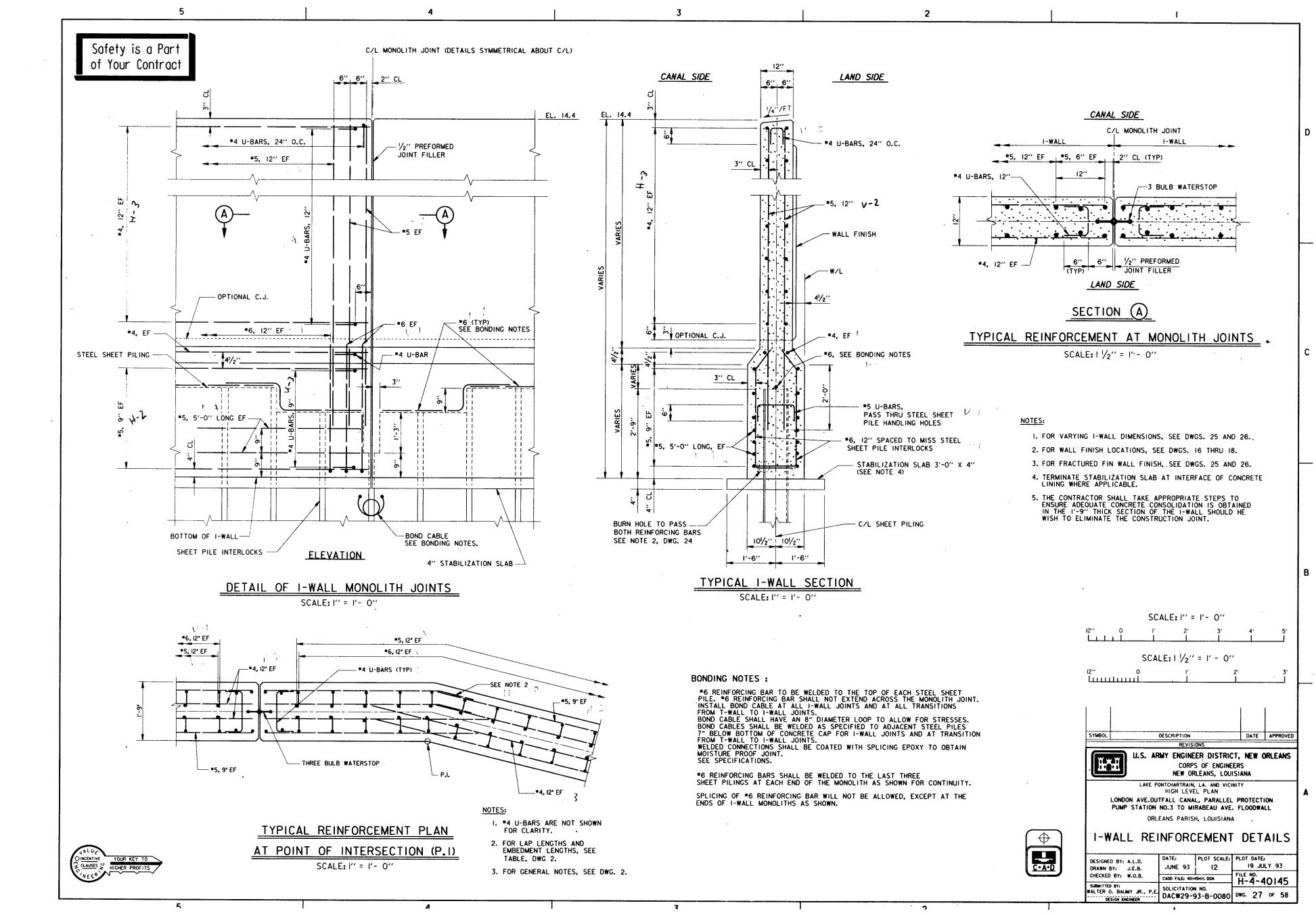


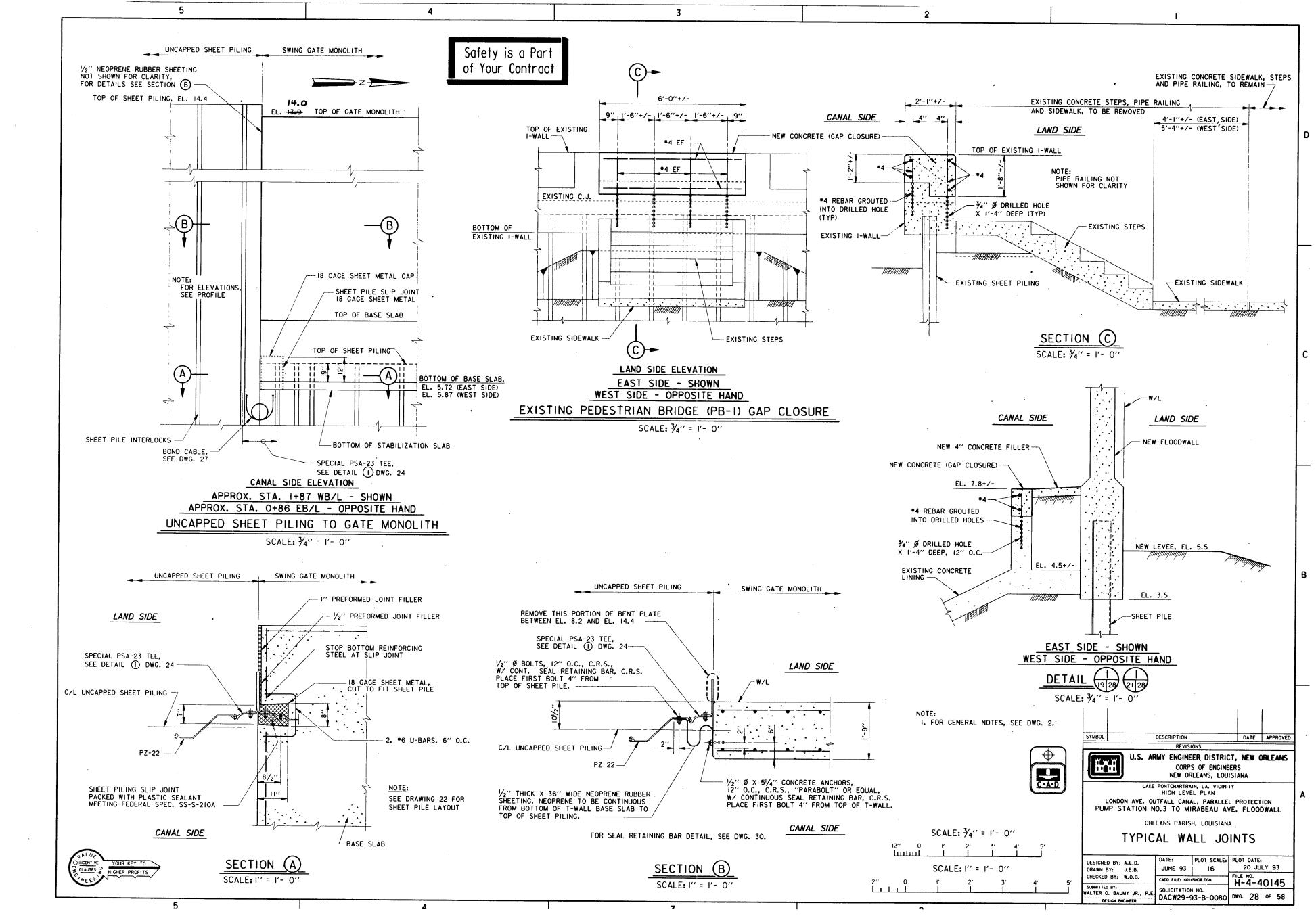


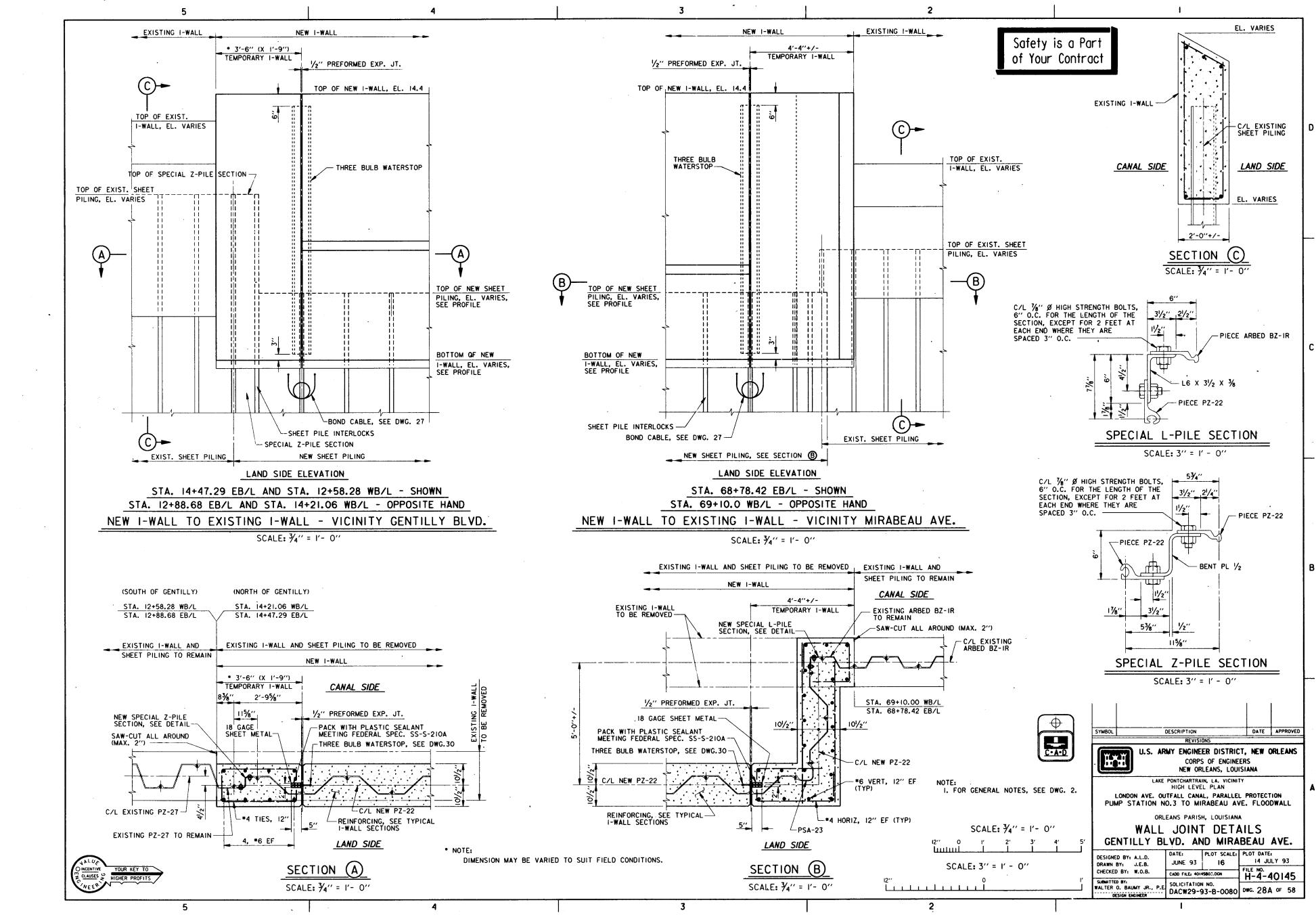


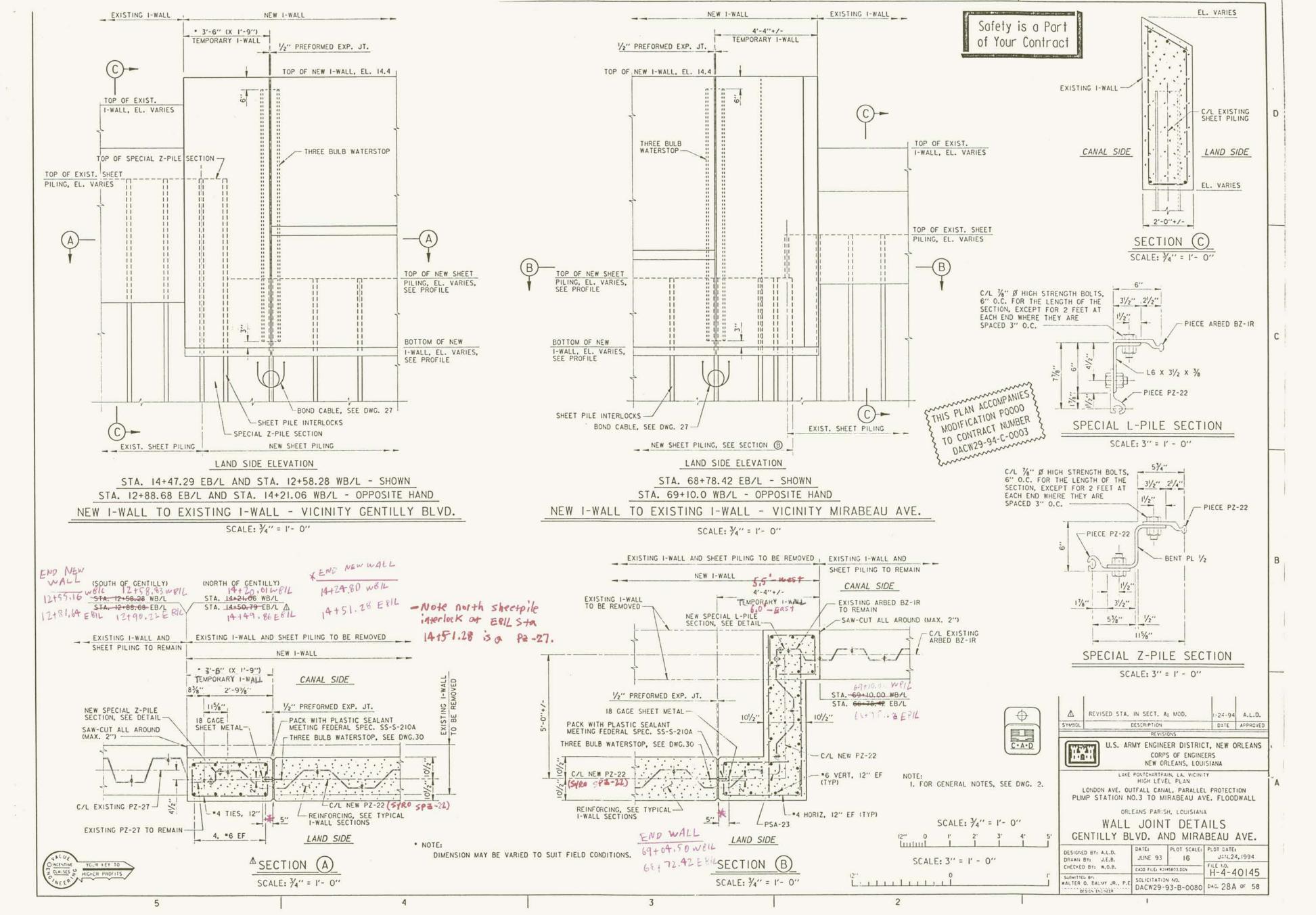


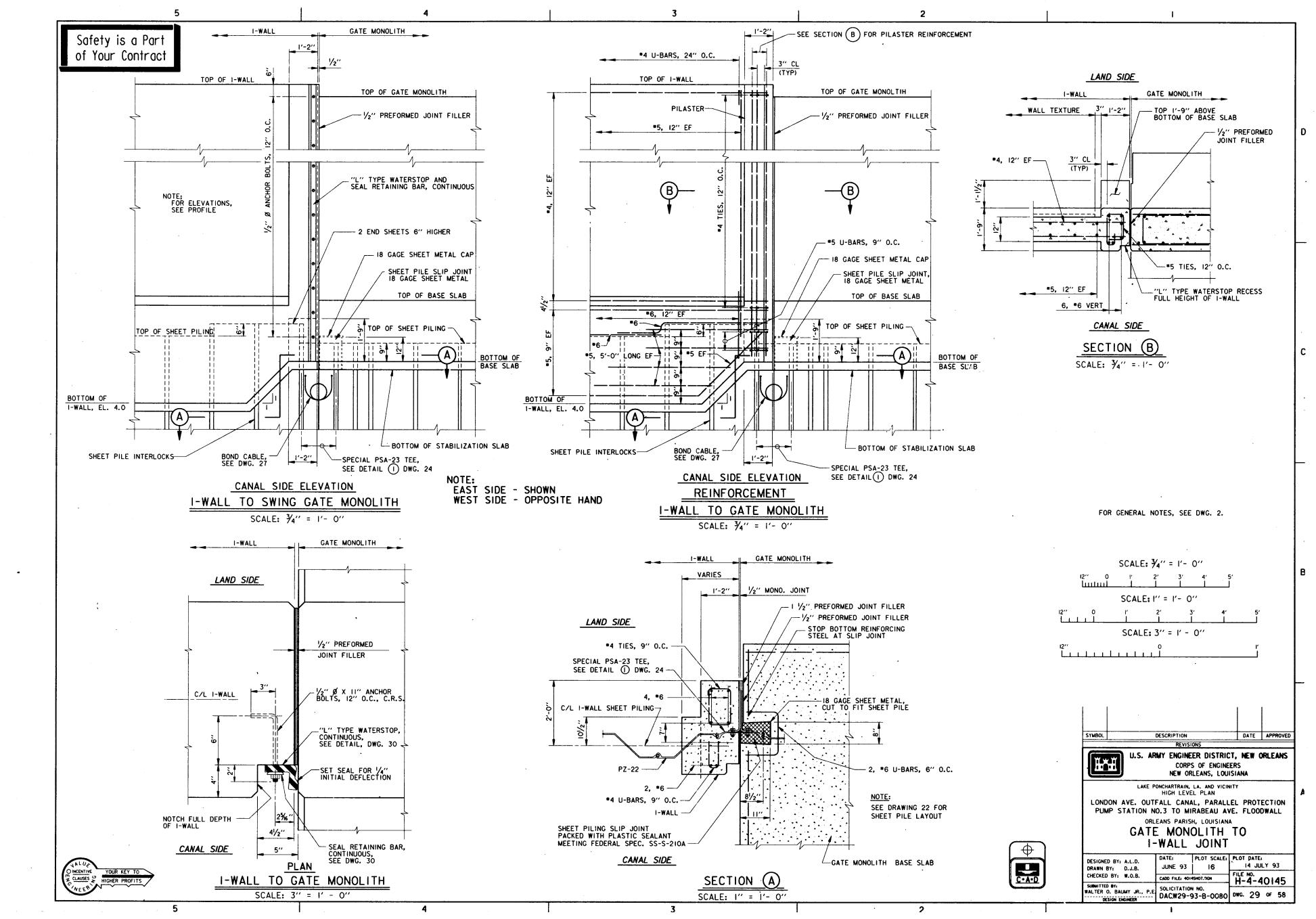




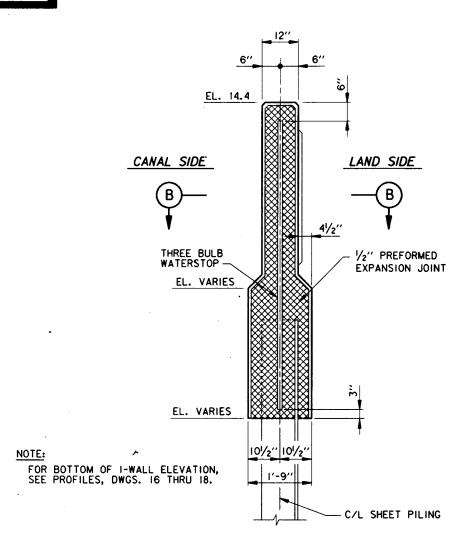




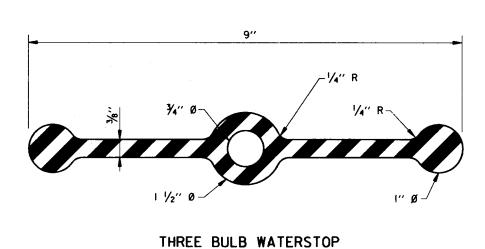




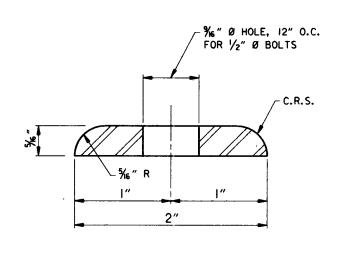
Safety is a Part of Your Contract



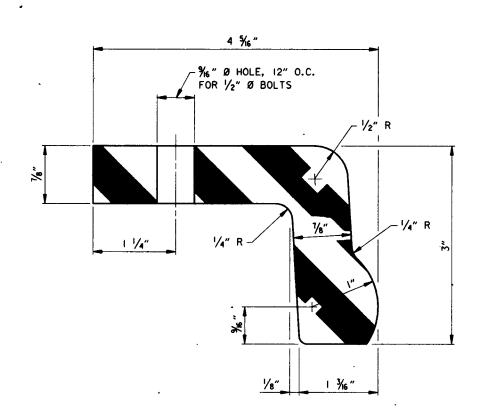
TYPICAL I-WALL JOINT SCALE: 3/4" = 1'- 0"



SCALE: 12" = 1' - 0"

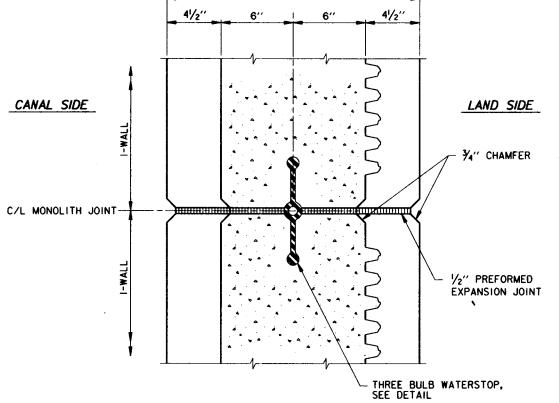


SEAL RETAINING BAR SCALE: 6" = 1'- 0"



"L" TYPE WATERSTOP

FOR GENERAL NOTES, SEE DWG. 2.



SECTION (B) SCALE: 3" = 1' - 0"

SCALE: 3/4" = 1'- 0" SCALE: 3" = 1' - 0" SCALE: 6" = 1' - 0" SCALE: 12" = 1' - 0" DESCRIPTION REVISIONS

U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS CORPS OF ENGINEERS
NEW ORLEANS, LOUISIANA

LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN
LONDON AVE. OUTFALL CANAL, PARALLEL PROTECTION PUMP STATION NO.3 TO MIRABEAU AVE.FLOODWALL ORLEANS PARISH, LOUISIANA

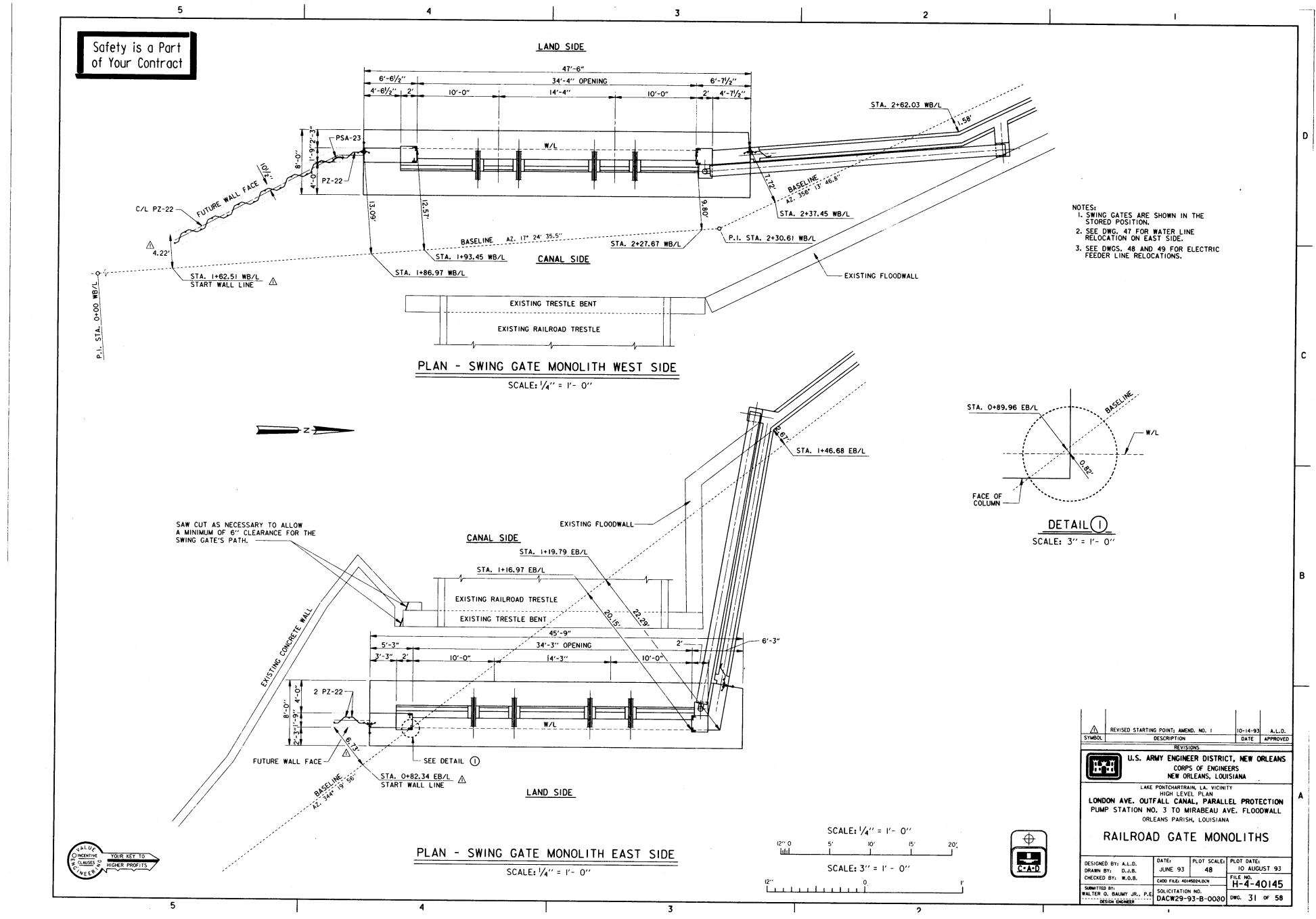
TYPICAL JOINT DETAILS

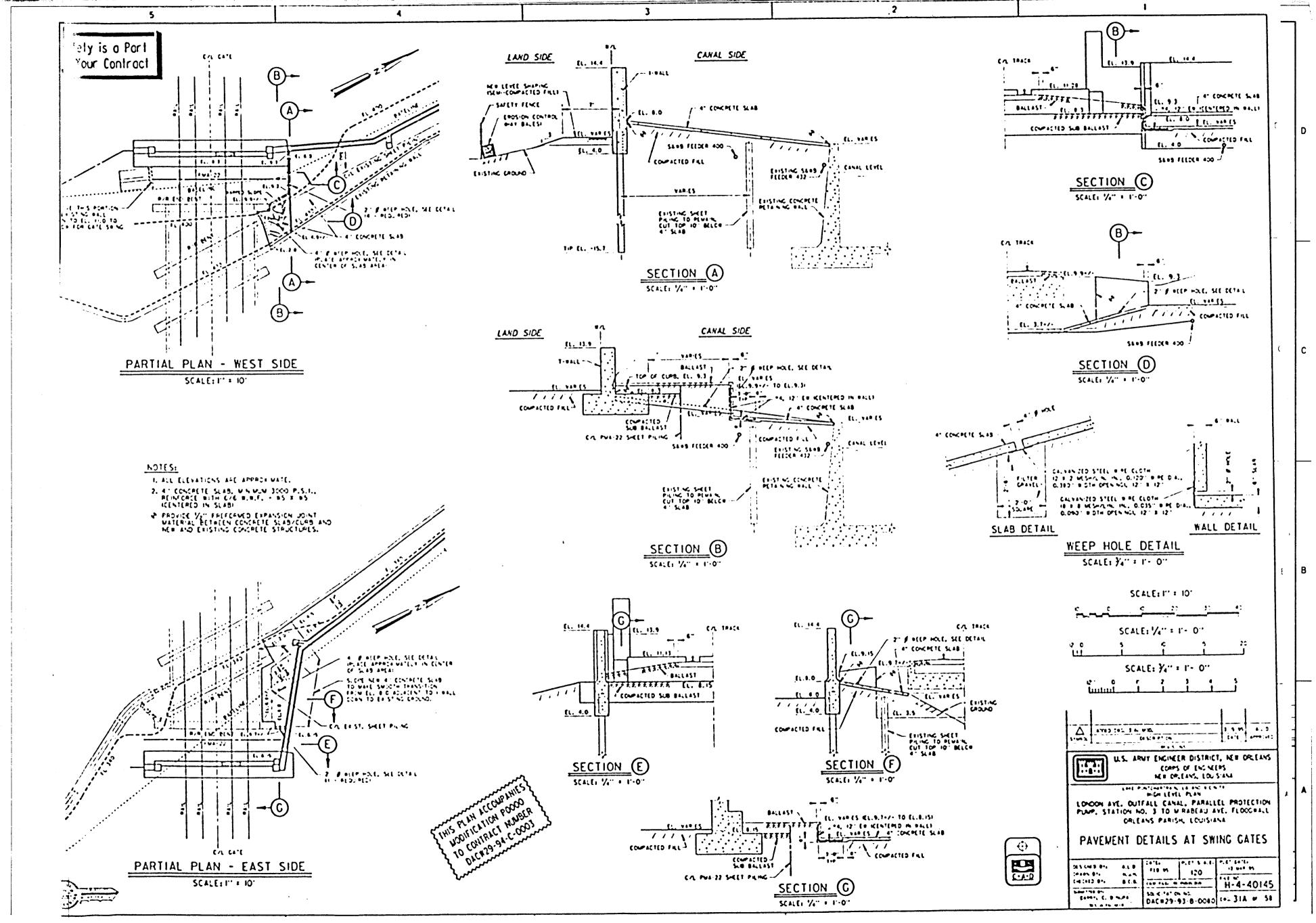


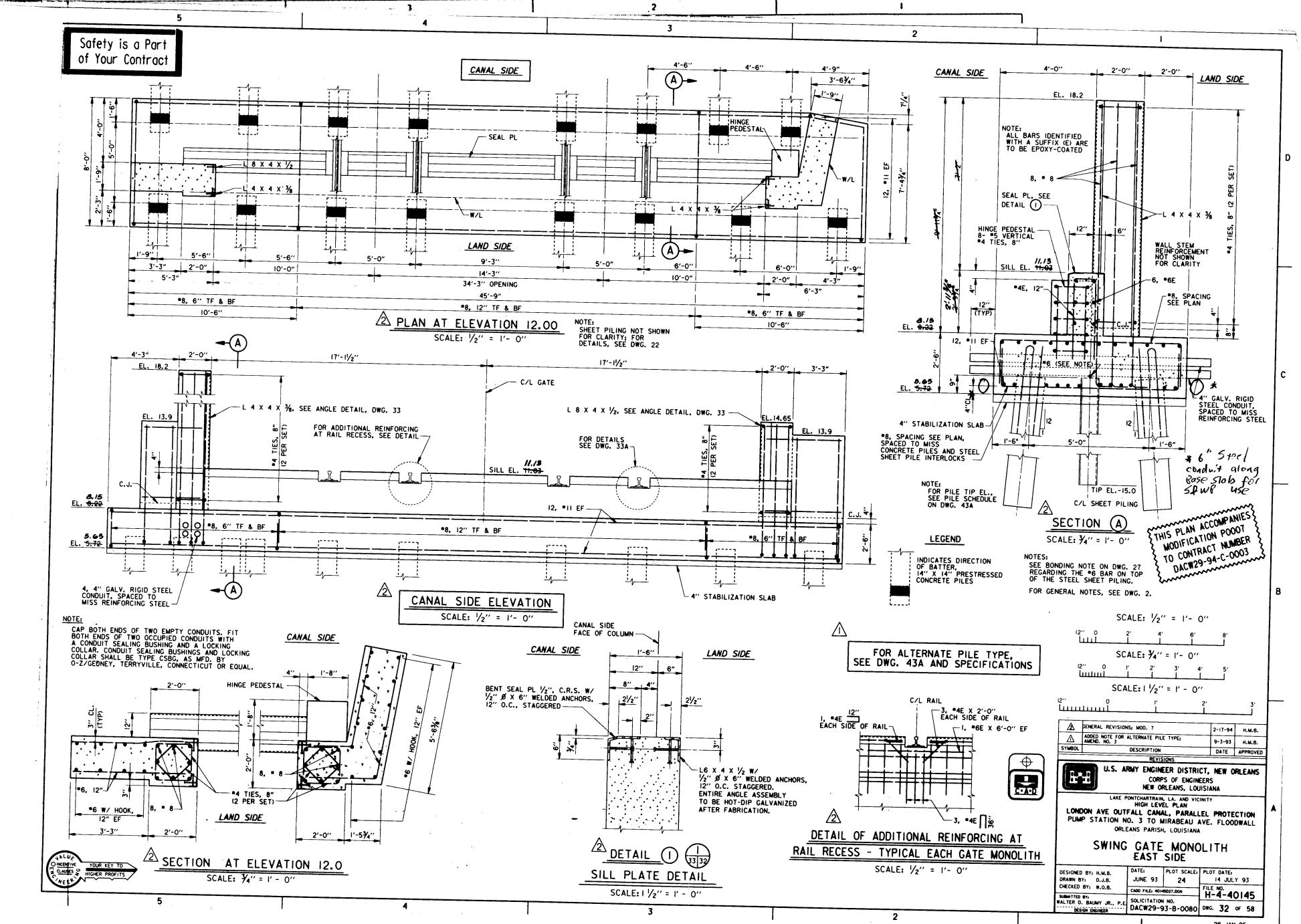
DESIGNED BY: A.L.O. DRAWN BY: J.E.B. CHECKED BY: W.O.B. JUNE 93

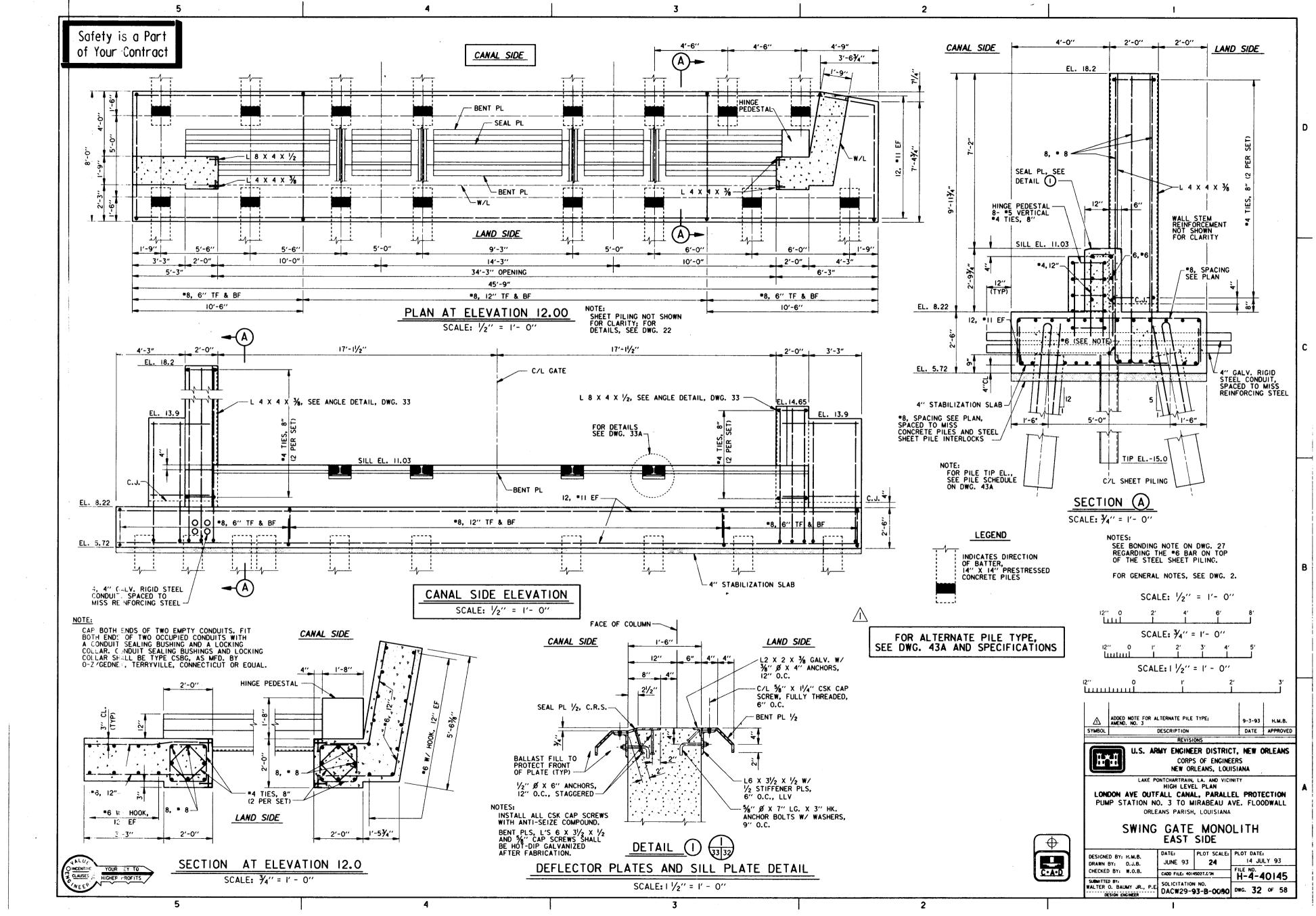
FILE NO. H-4-40145 SUBMITTED BY:
WALTER O. BAUMY JR., P.E DACW29-93-B-0080 DWG. 30 OF 58

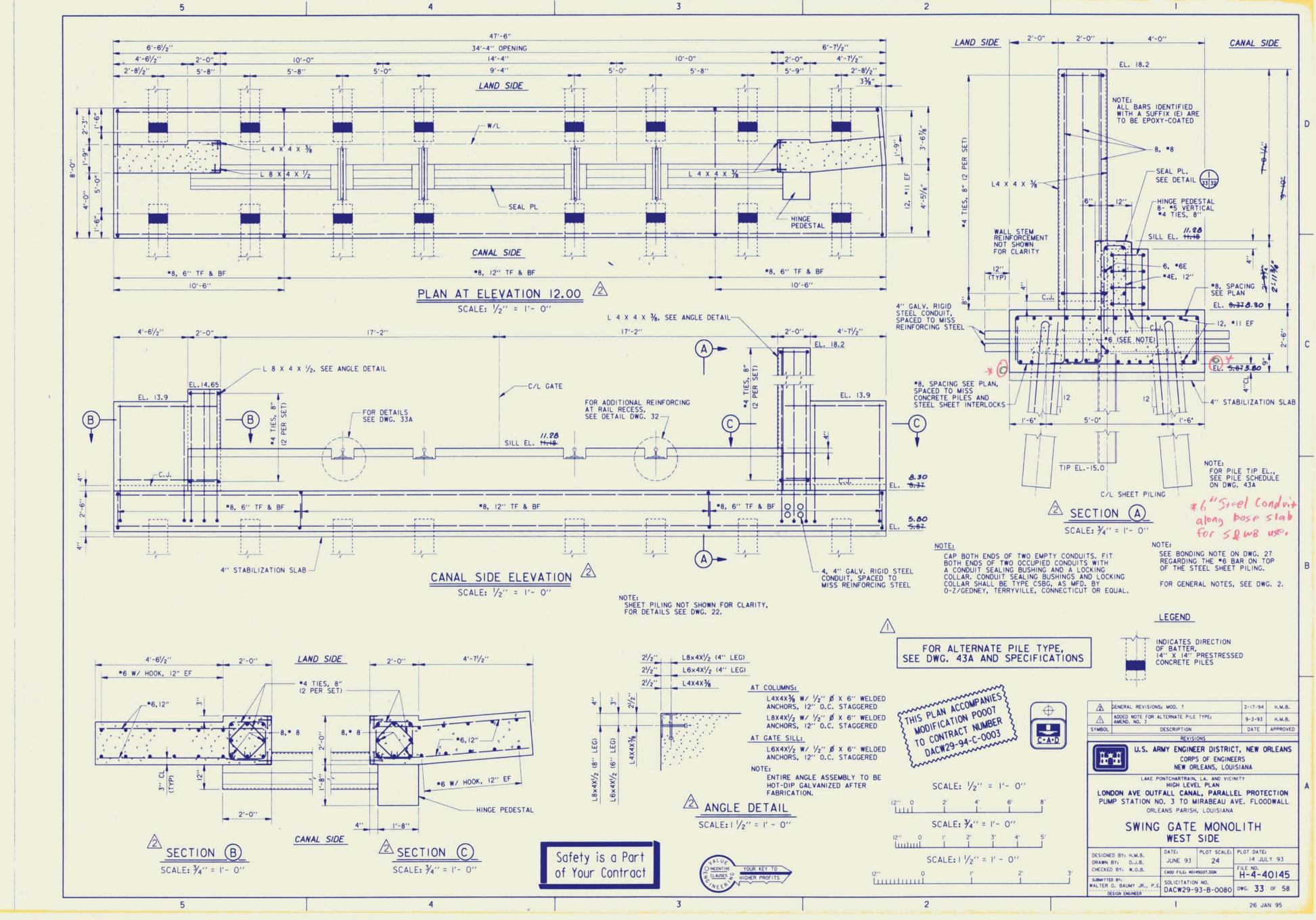
2

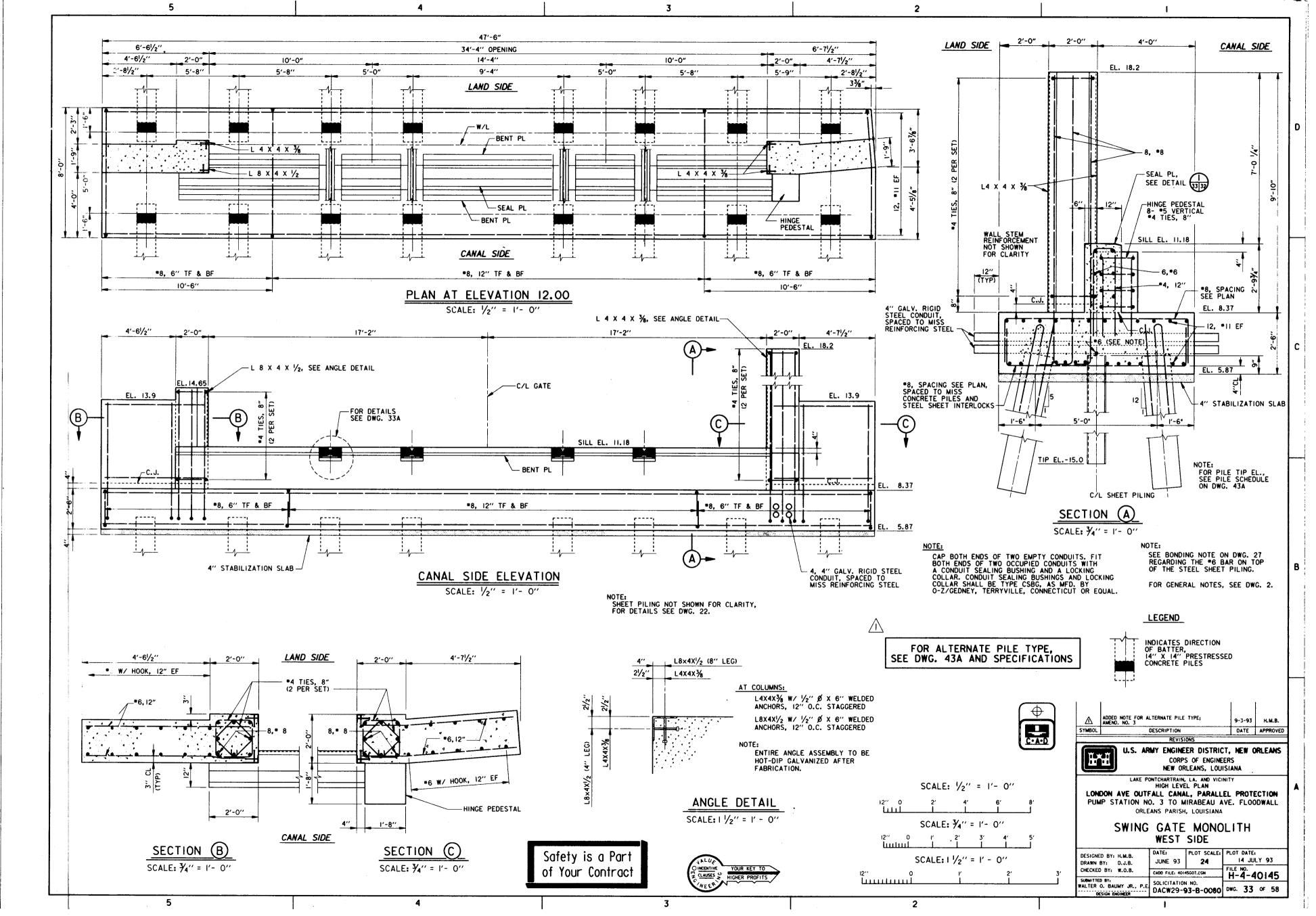


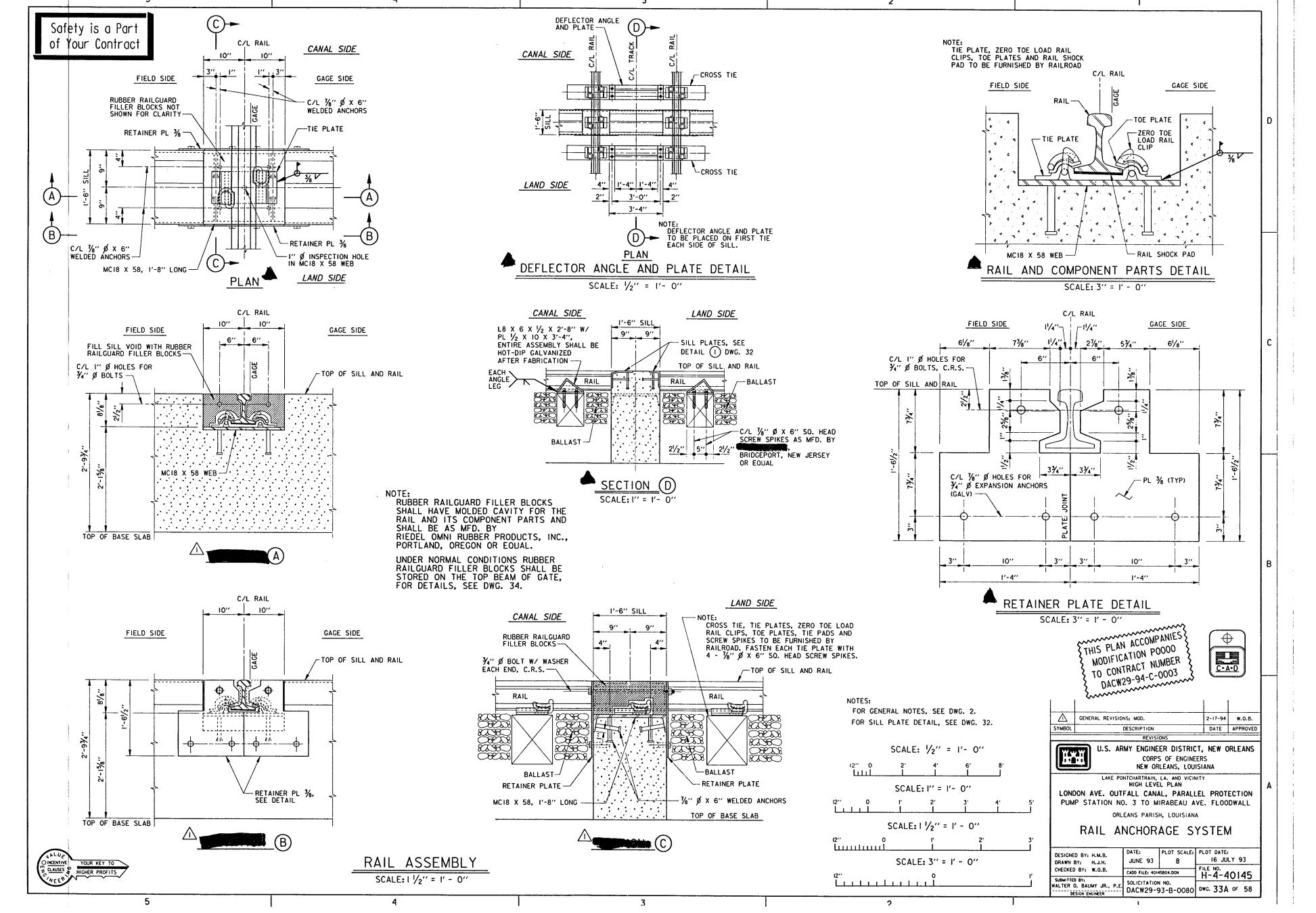


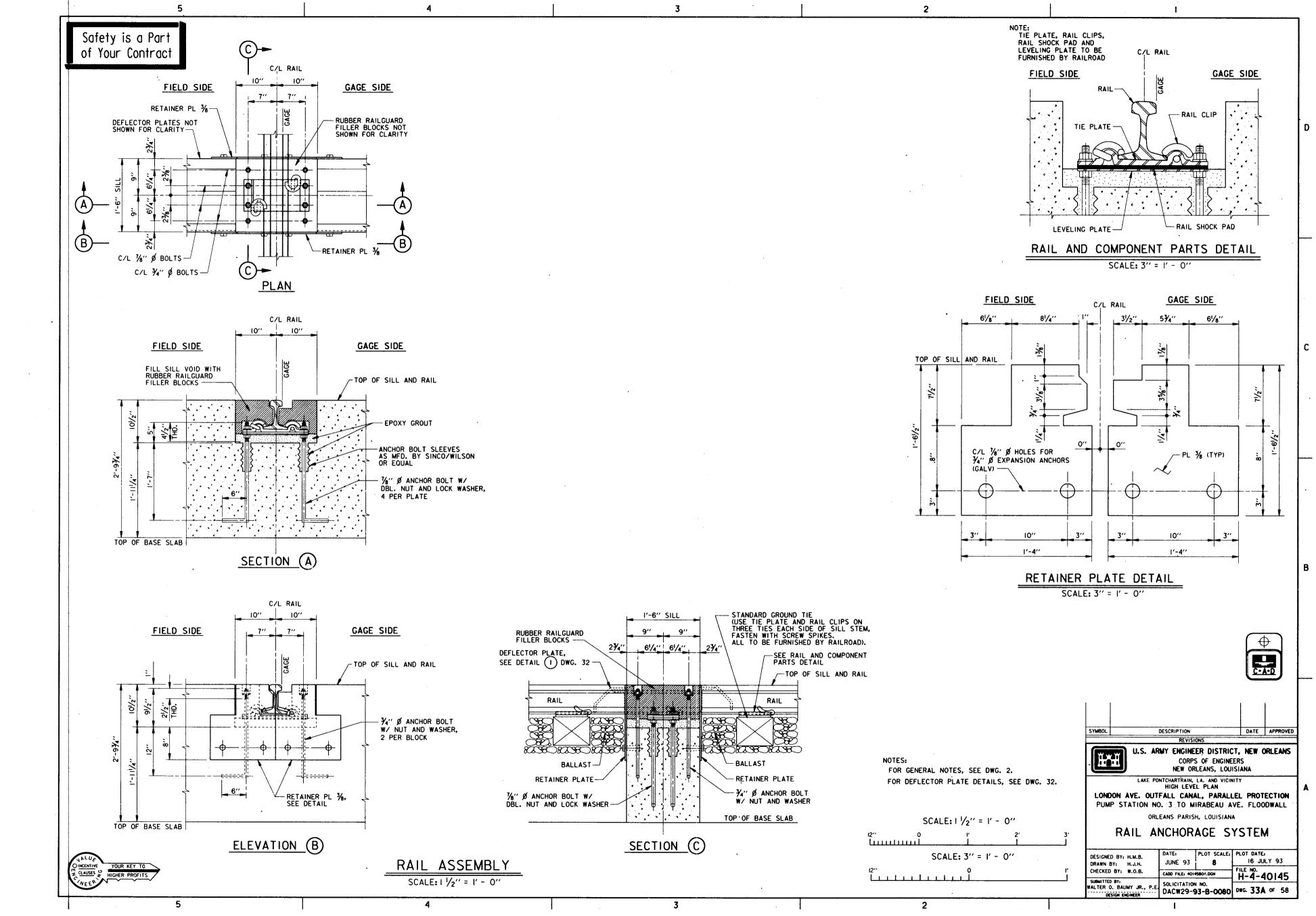


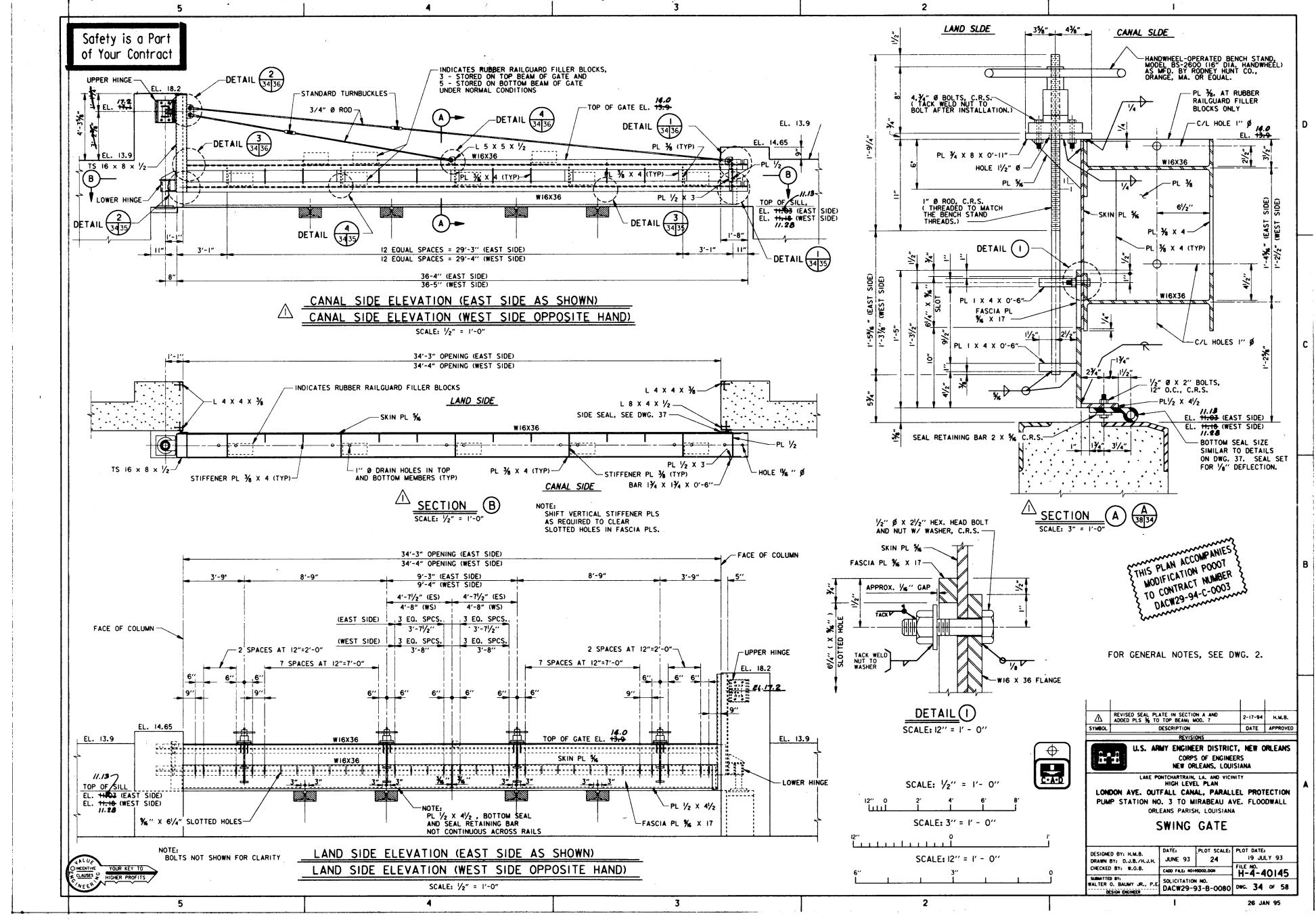


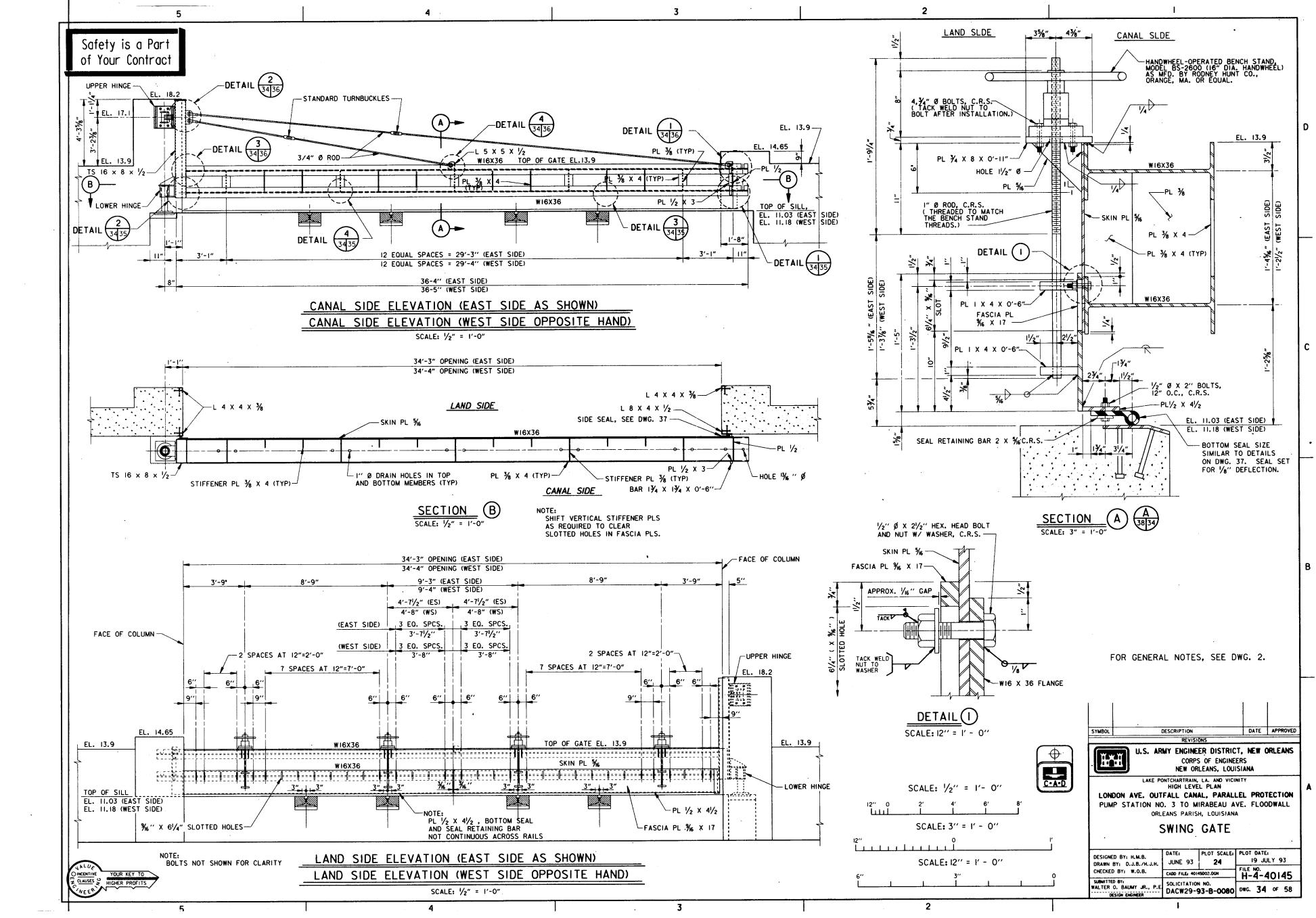


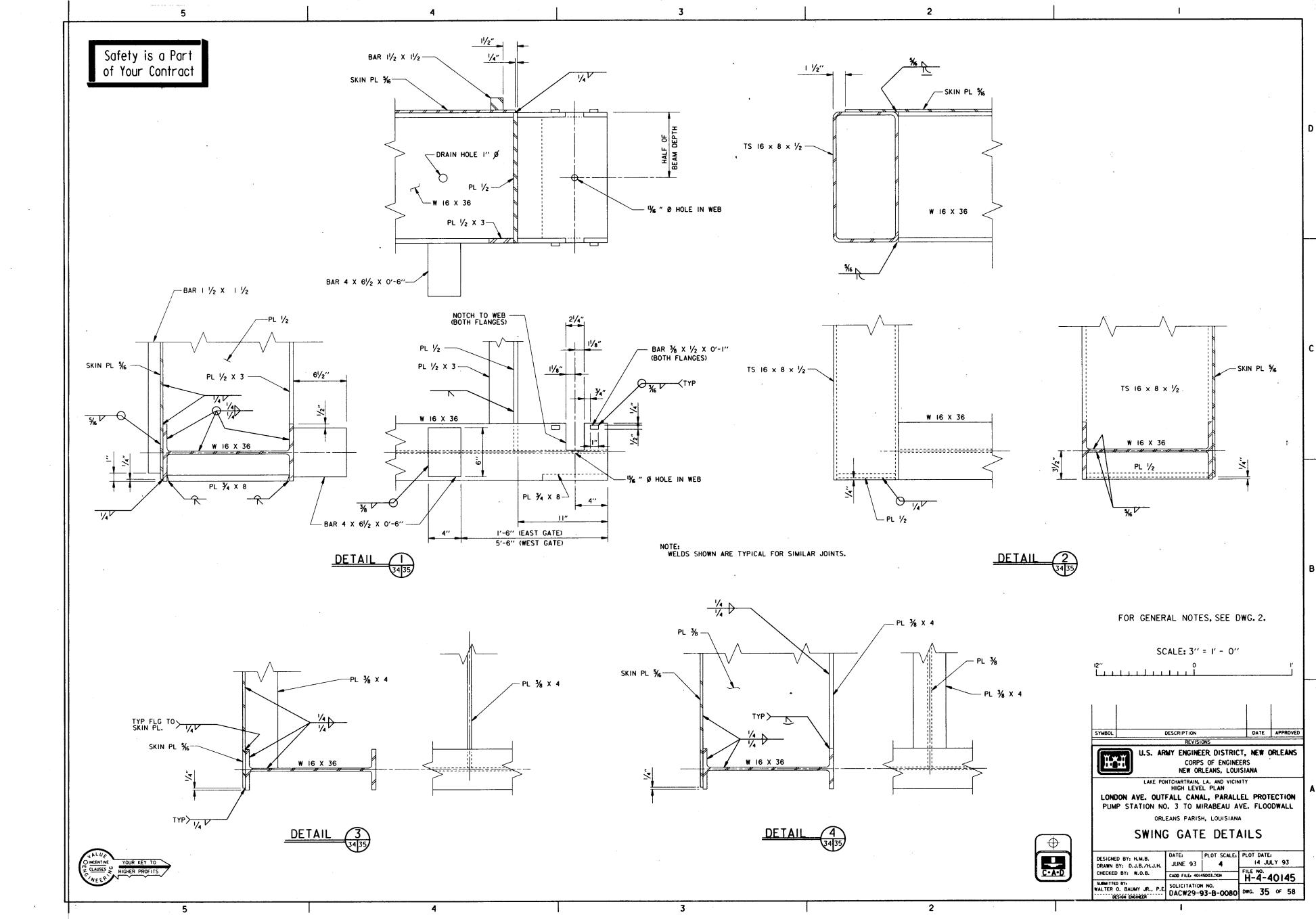


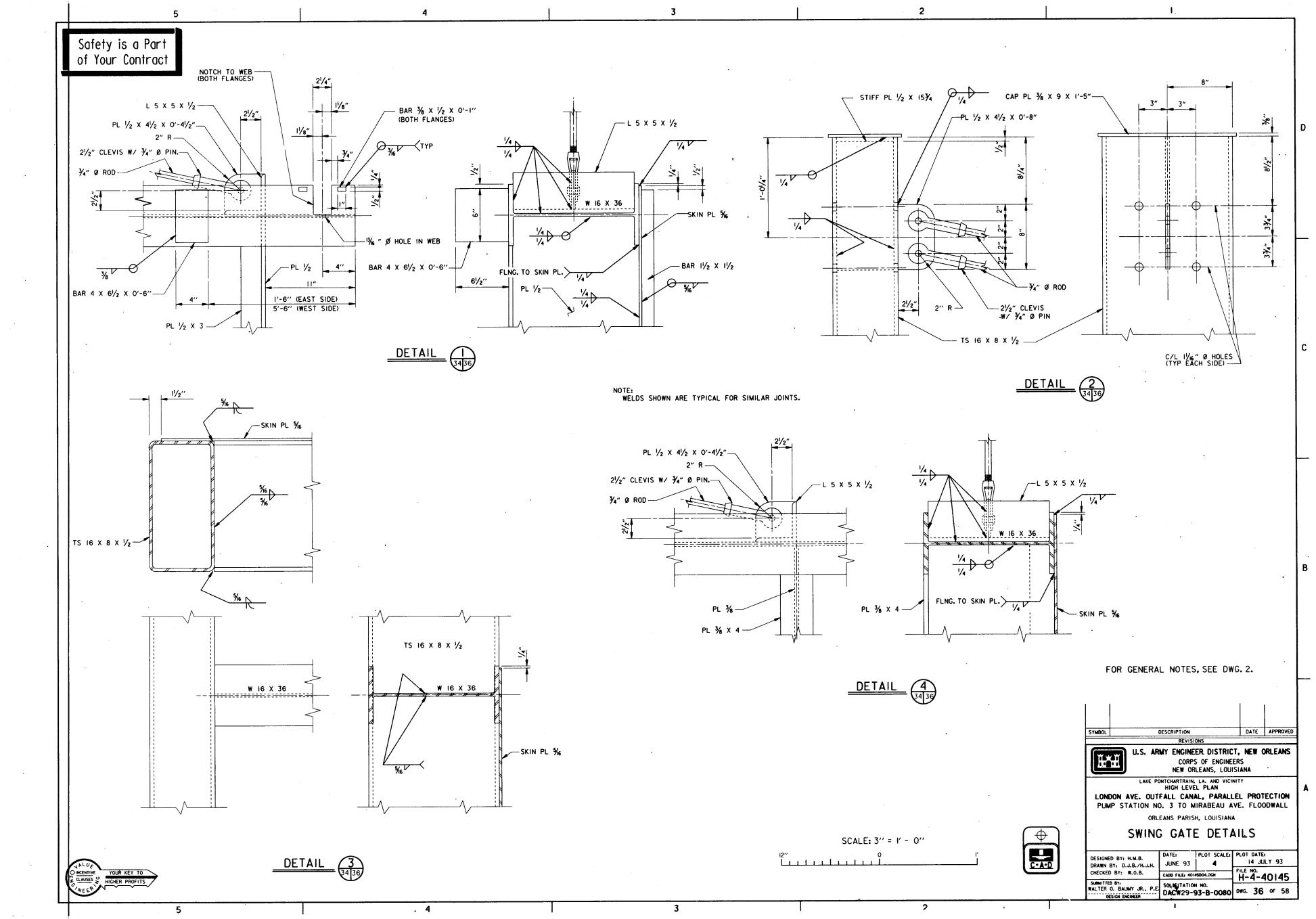


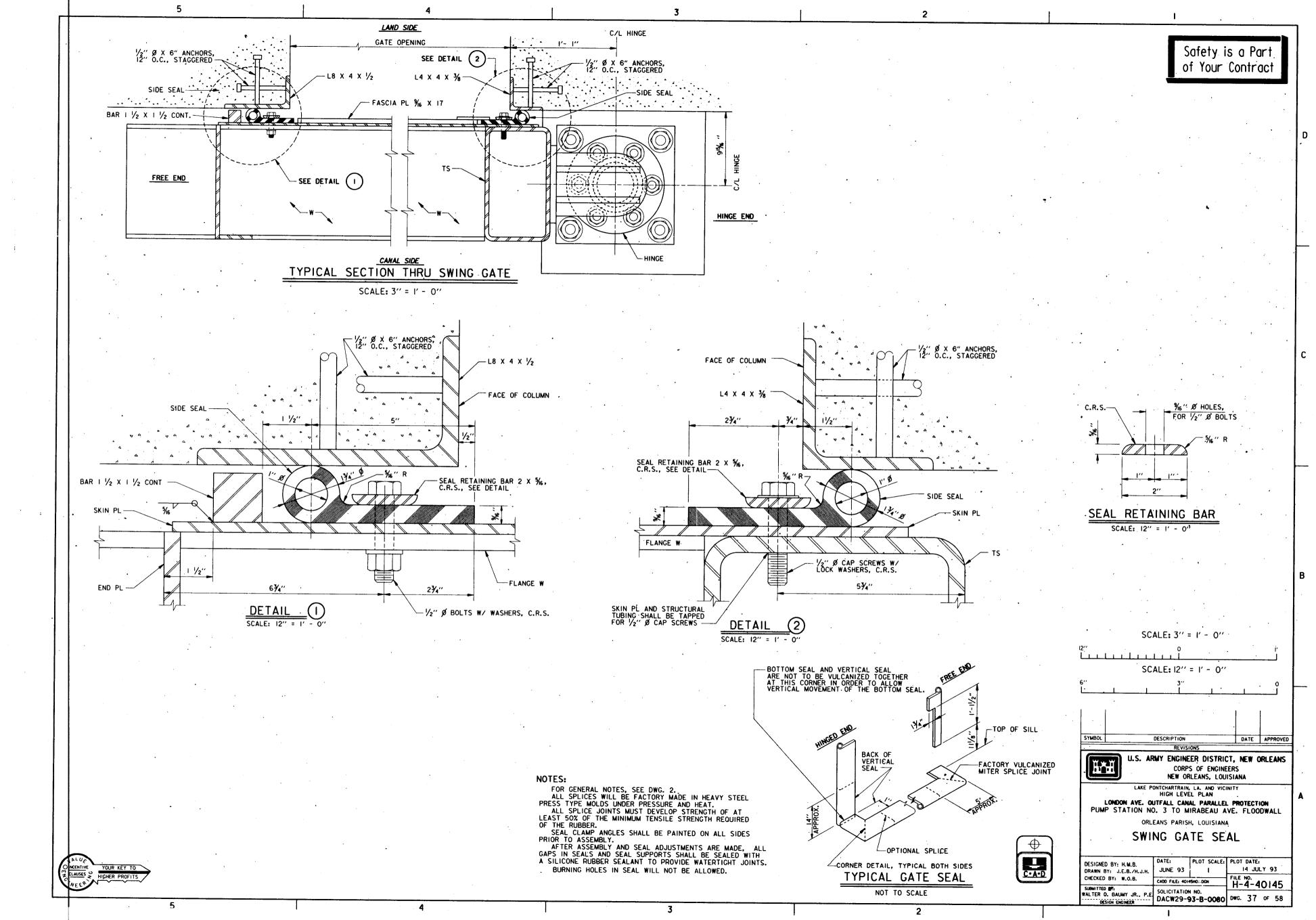


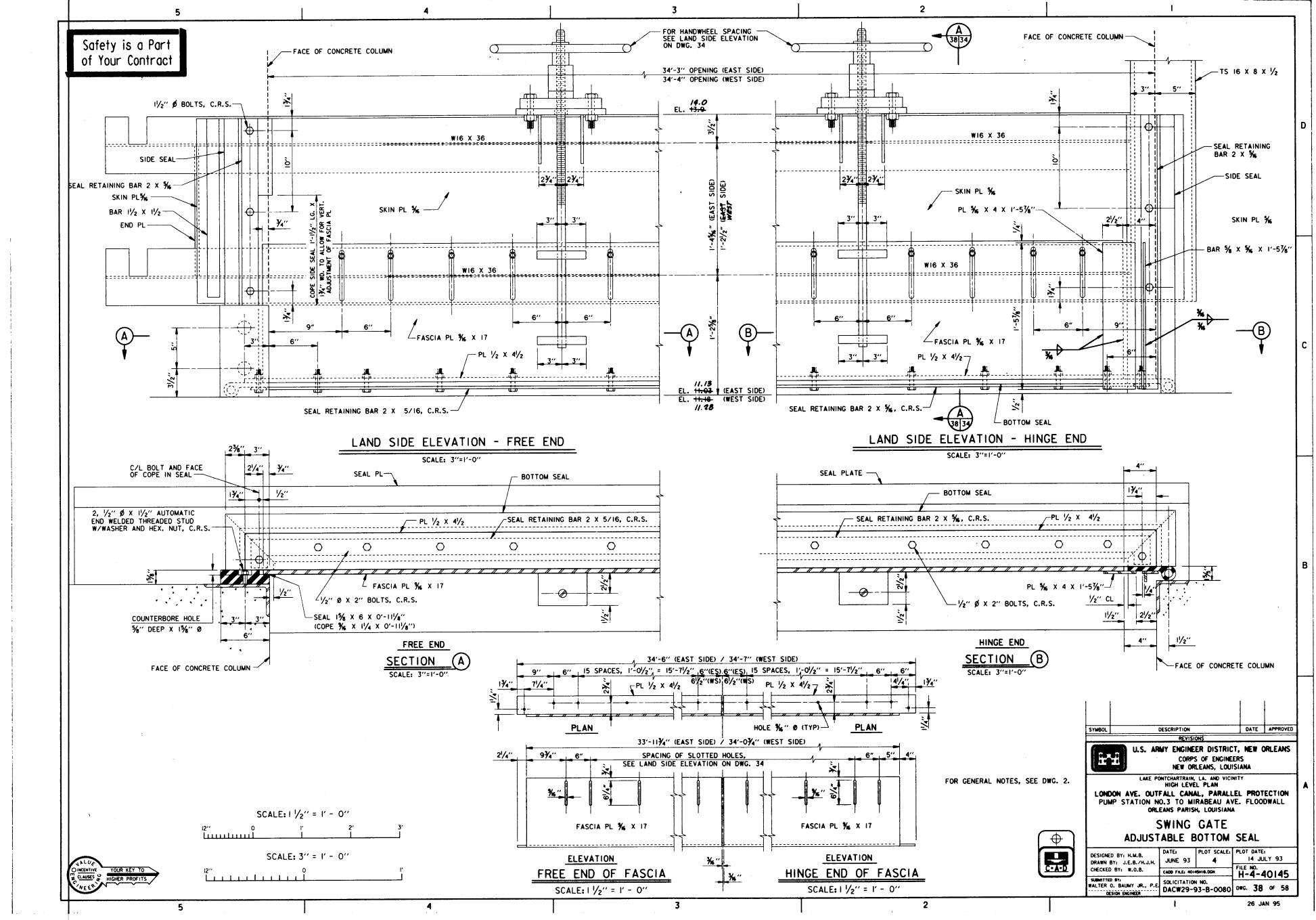


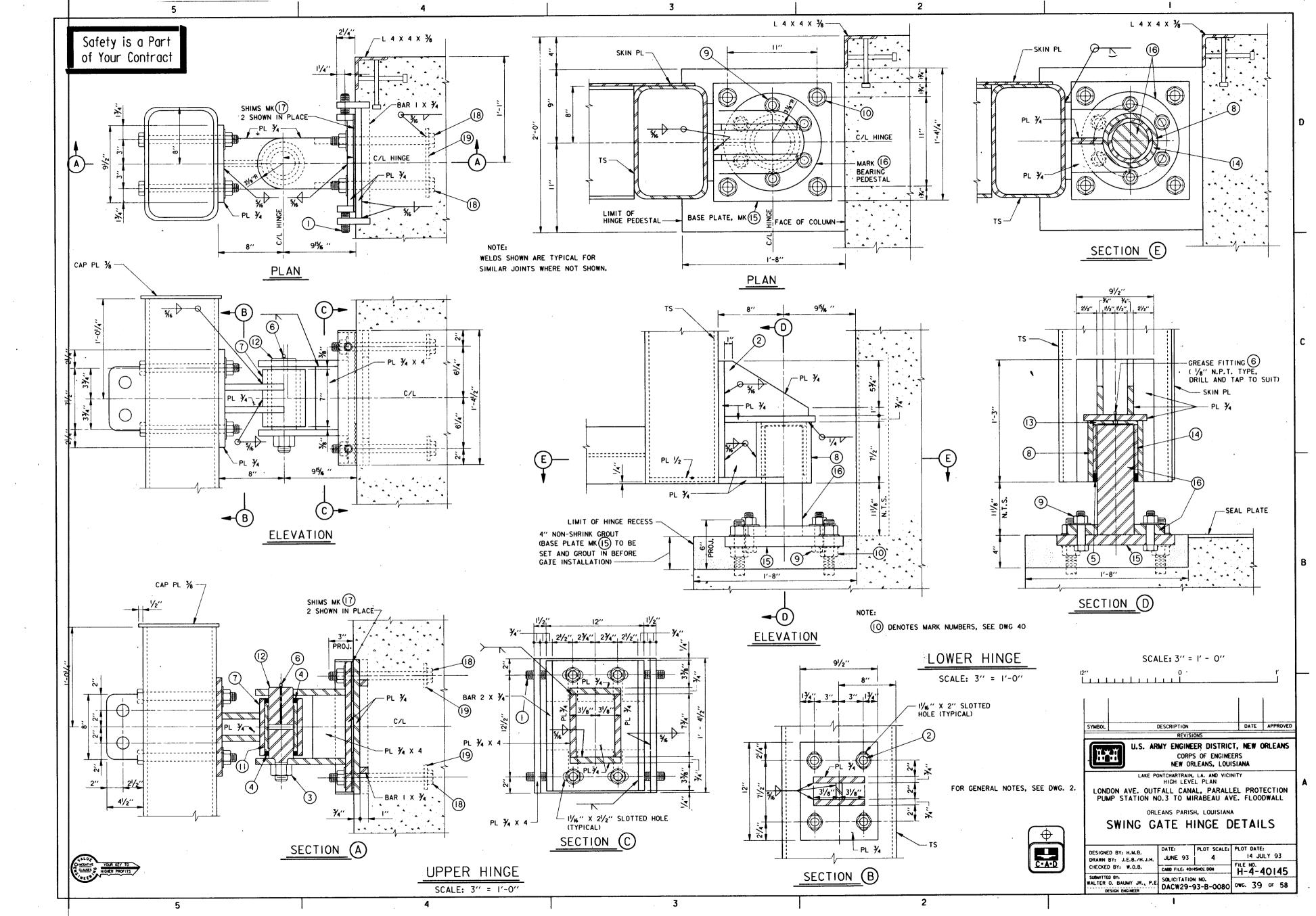


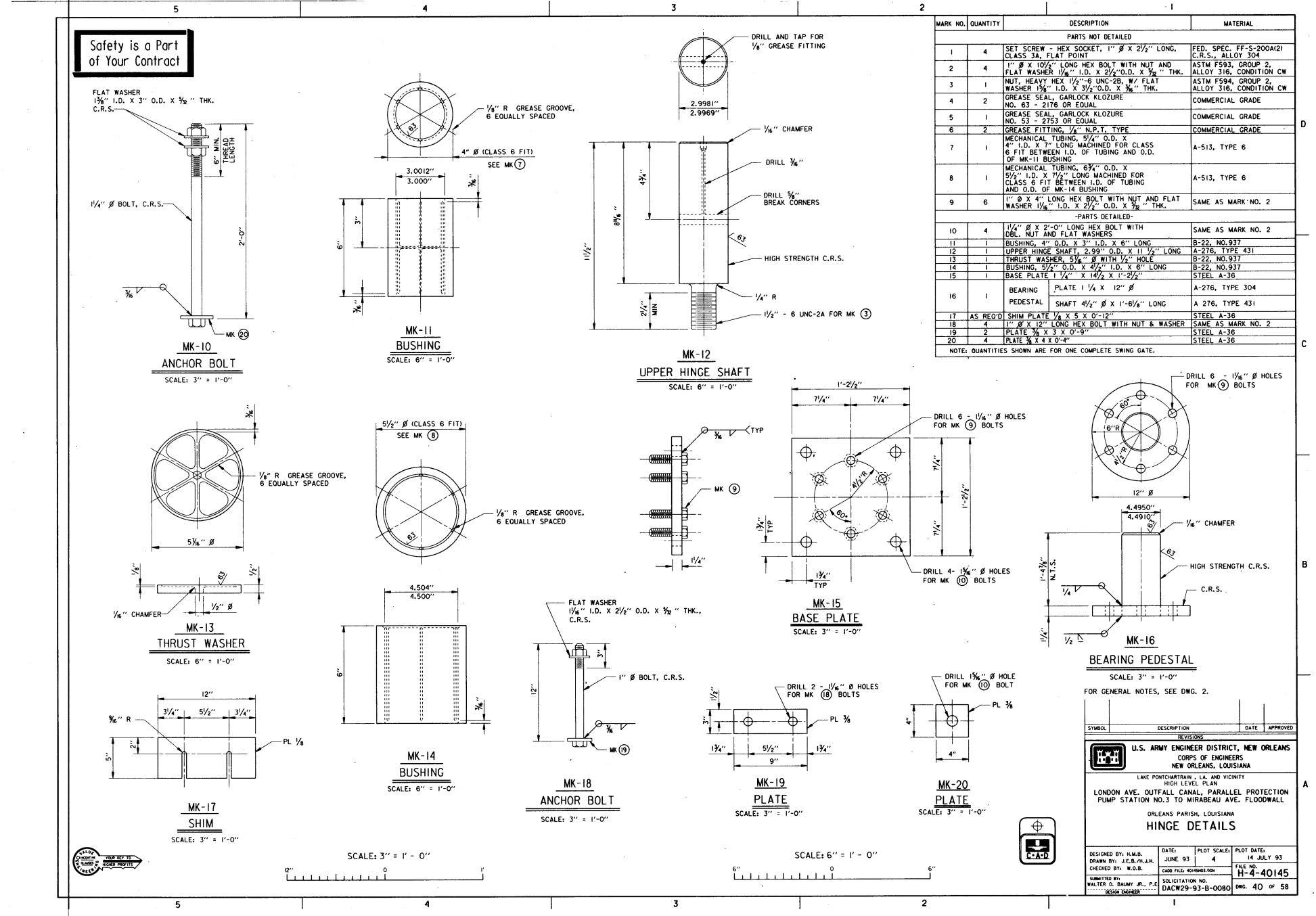


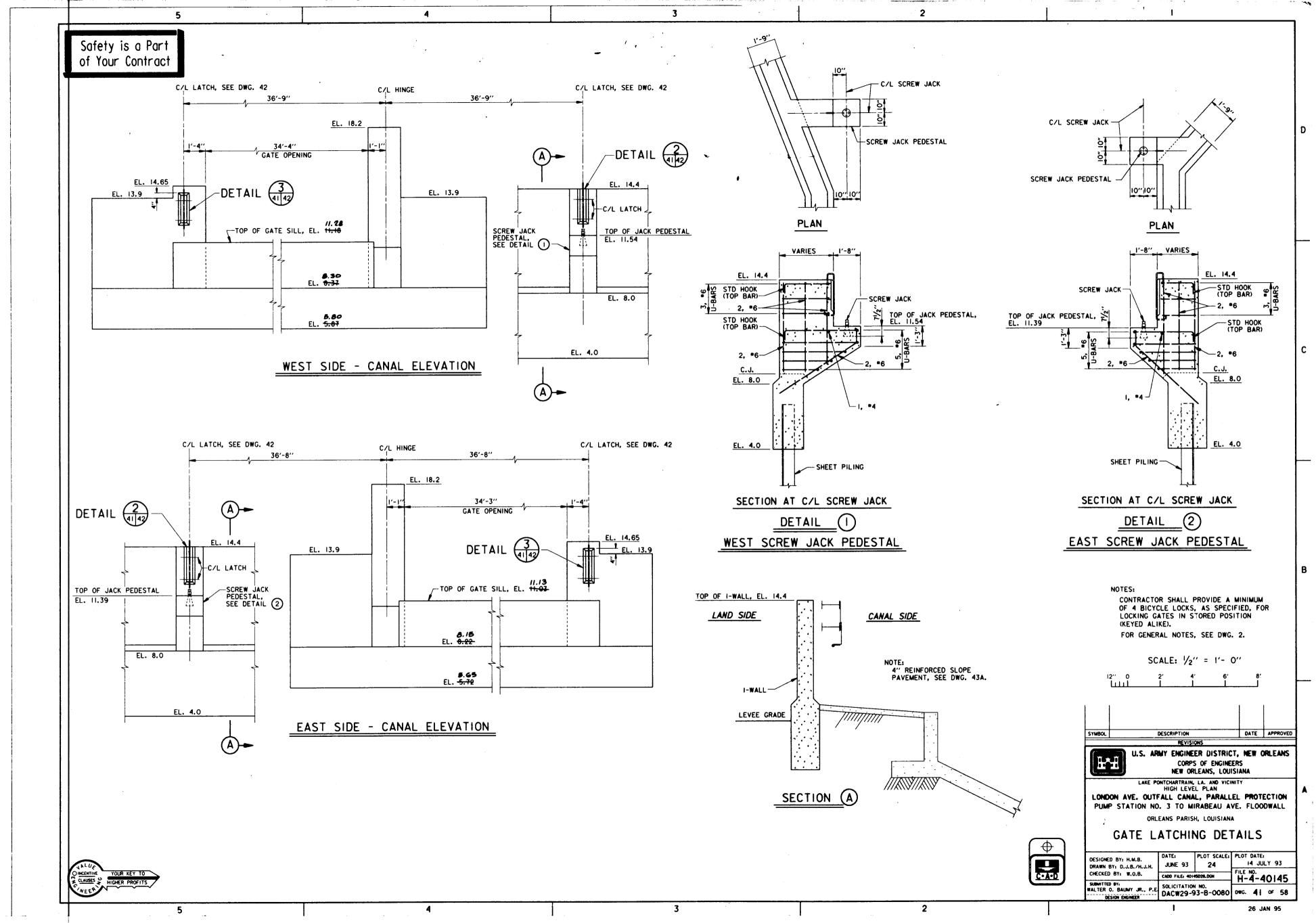


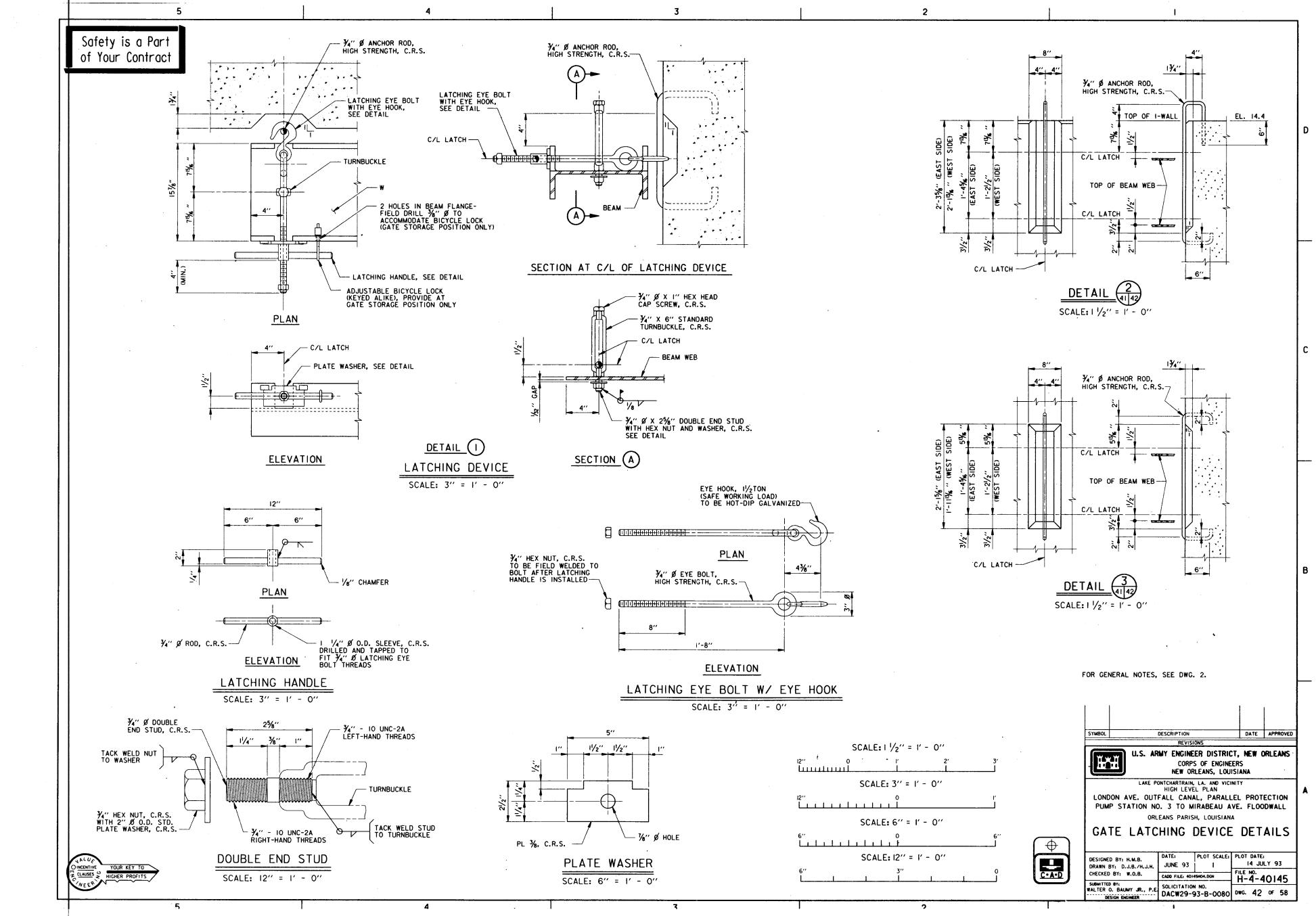


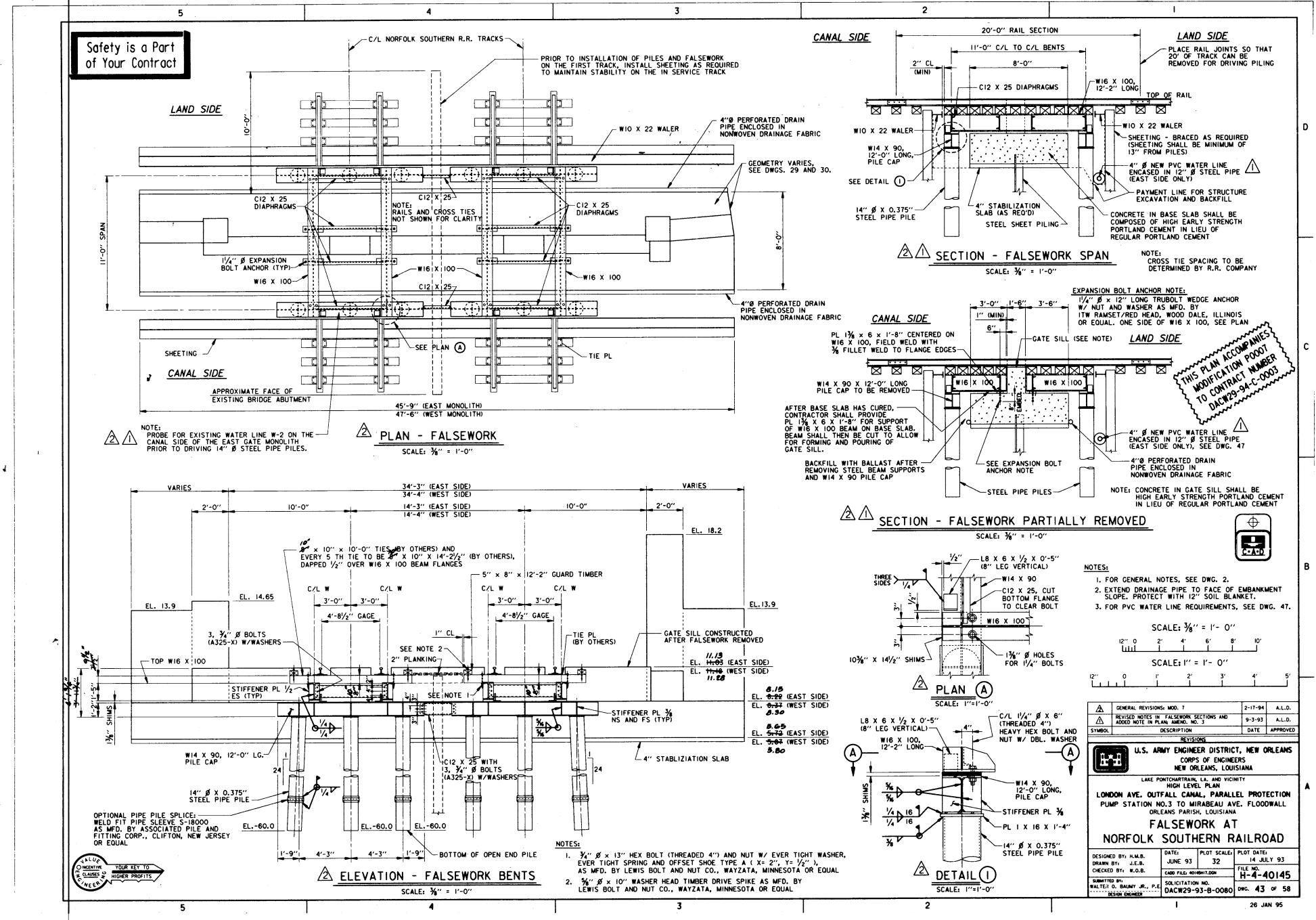


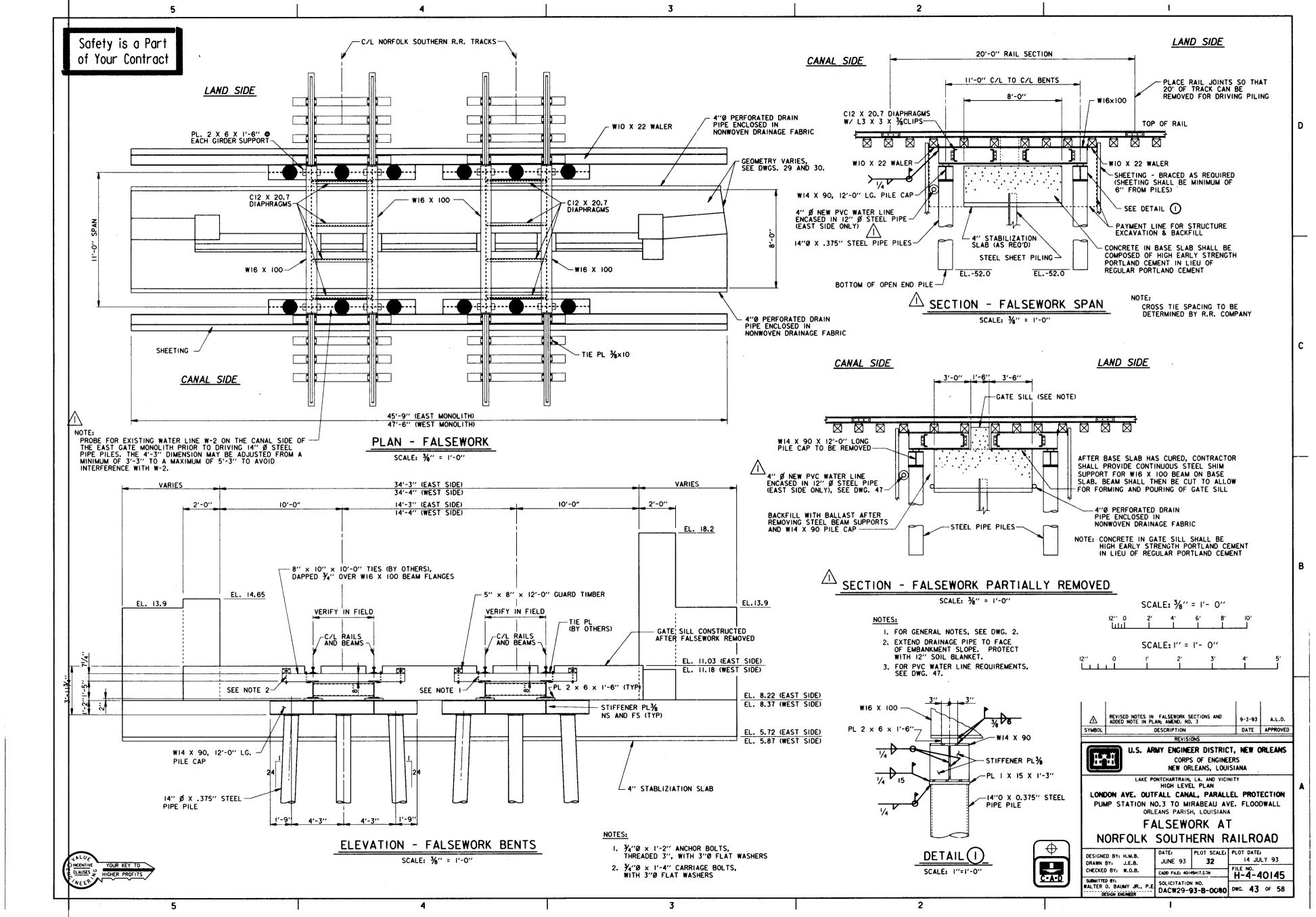


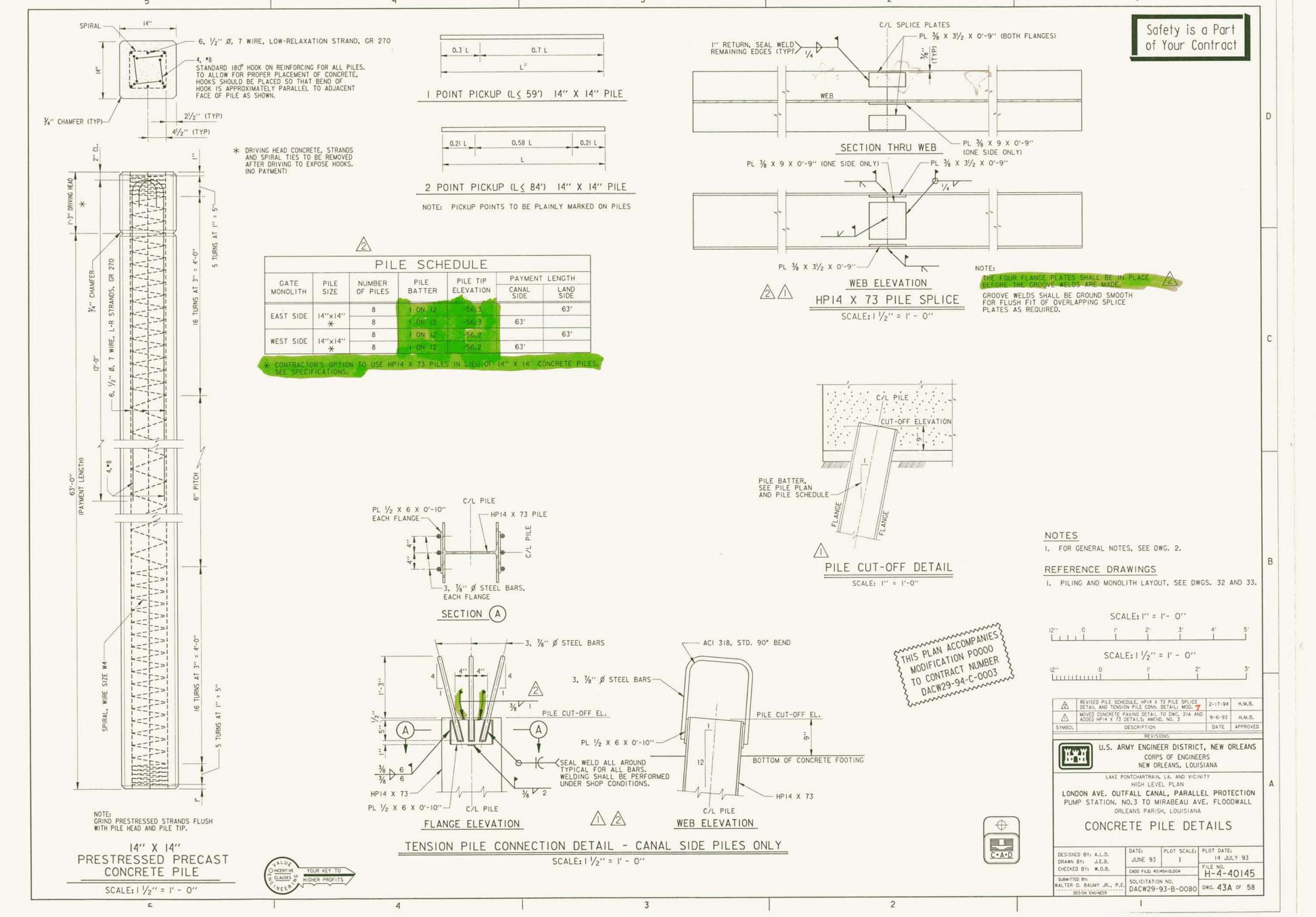


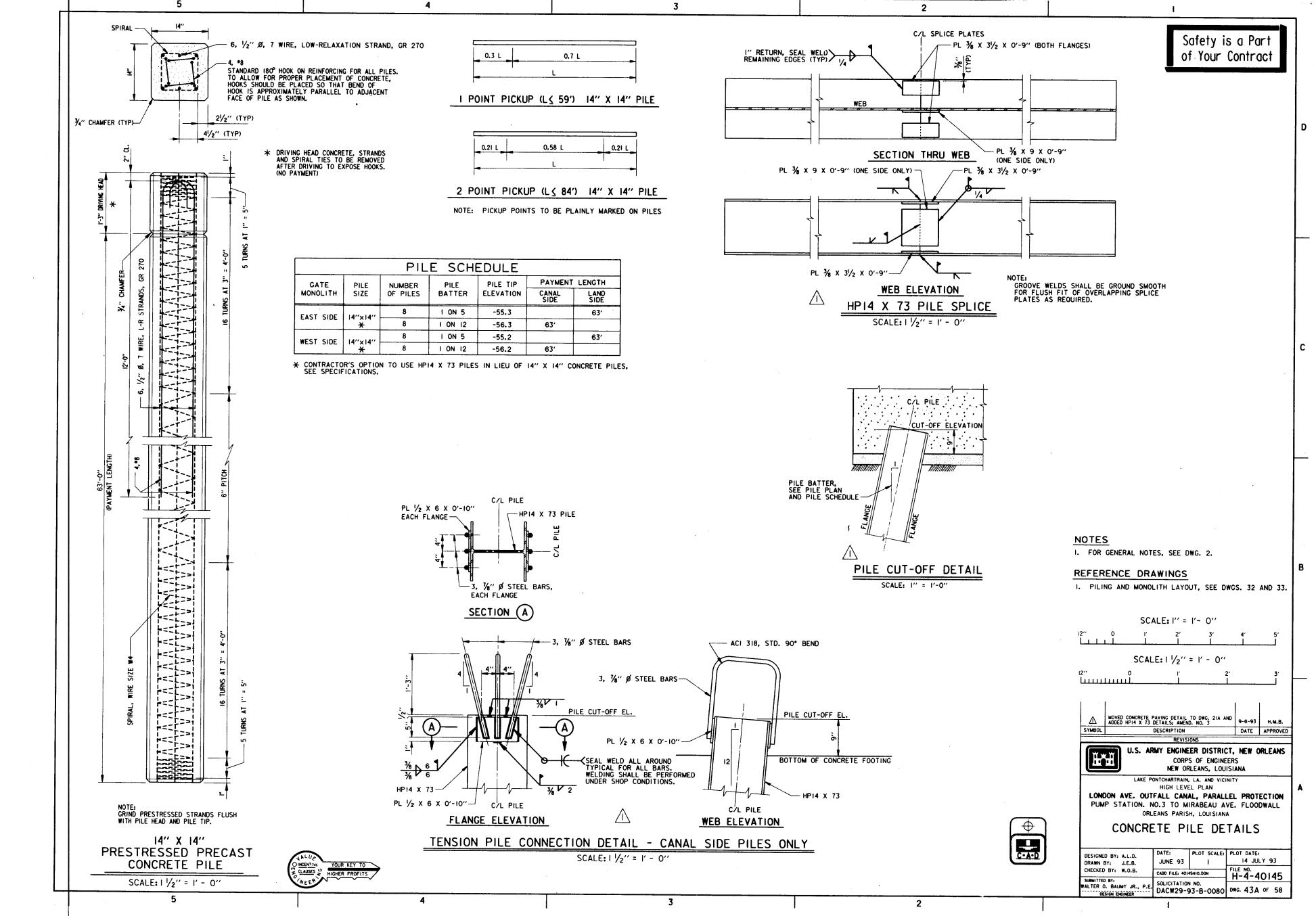


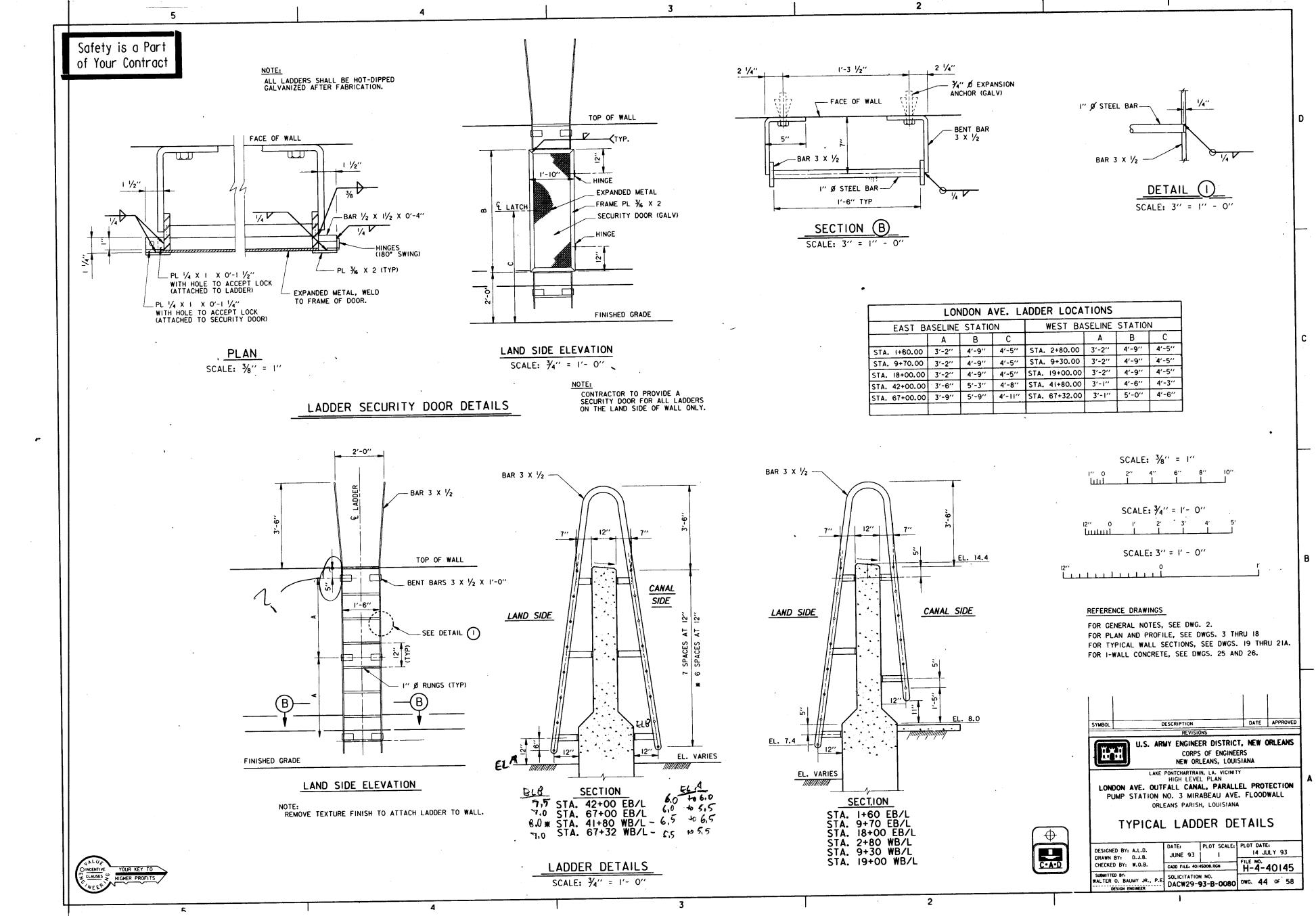


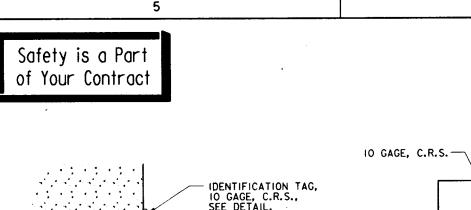


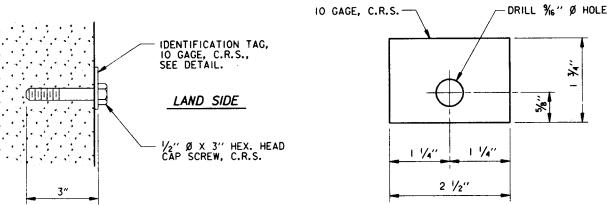






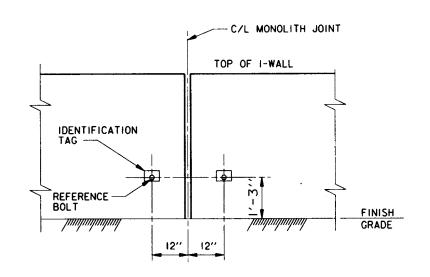






REFERENCE BOLT SCALE: 6" = 1' - 0"

IDENTIFICATION TAG SCALE: 12" = 1' - 0"



LAND SIDE

TYPICAL ELEVATION OF SETTLEMENT REFERENCE MARKER

SCALE: 3/4" = 1'- 0"

SETTLEMENT REFERENCE MARKER SCHEDULE

	EAST SIDE			WEST SIDE	
S.R.M. NO.	W/L STATION * (C/L MONOLITH JOINT)	ELEVATION **	S.R.M. NO.	W/L STATION * (C/L MONOLITH JOINT)	ELEVATION
1	1 + 20.00		1	2 + 30.00	
2	3 + 00.00		2	4 + 00.00	
3	5 + 00.00		3	6 + 00.00	
4	7 + 00.00		4	8 + 00.00	
5	9 + 00.00		5	10 + 00.00	
6	11 + 00.00		6	12 + 00.00	
7	13 + 00.00		7	12 + 80.00	
8	15 + 00.00		8	14 + 15.00	
9	17 + 00.00		9	16 + 00.00	<u> </u>
10	19 + 00.00		10	18 + 00.00	<u> </u>
Ш	21 + 00.00		- 11	20 + 00.00	
12	23 + 00.00		12	24 + 00.00	1
13	25 + 00.00		13	26 + 00.00	
14	27 + 00.00	ļ	14	28 + 00.00	
15	29 + 00.00		15	30 + 00.00	<u> </u>
16 .	31 + 00.00		16	32 + 00.00	ļ <u>-</u> -
17	33 + 00.00		17	34 + 00.00	
18	35 + 00.00		18	36 + 00.00 38 + 00.00	
19 20	37 + 00.00 39 + 00.00		20	40 + 00.00	
21	41 + 00.00		21	42 + 00.00	1
22	43 + 00.00	1	22	44 + 00.00	
23	45 + 00.00	ļ	23	46 + 00.00	
24	47 + 00.00		24	48 + 00.00	
25	49 + 00.00	-	25	50 + 00.00	
26	51 + 00.00	 	26	52 + 00.00	T
27	53 + 00.00	 	27	54 + 00.00	
28	55 + 00.00		28	56 + 00.00	
29	57 + 00.00		29	58 + 00.00	
30	59 + 00.00		30	60 + 00.00	
31	61 + 00.00		31	62 + 00.00	
32	63 + 00.00		32	64 + 00.00	
33	65 + 00.00		33	66 + 00.00	
34	67 + 00.00		34	68 + 85.00	
35	68 + 50.00				
		 	 		-
			 		
		1	t		1

NOTES:

- * W/L STATIONS ARE APPROXIMATE. LOCATE REFERENCE BOLTS AT NEAREST MONOLITH JOINT TO THOSE STATIONS SHOWN. THE CONTRACTOR SHALL ESTABLISH WALL LINE STATIONING, BEGINNING WITH STA. 0+00 AT THE PUMP STATION.
- ** THE CONTRACTOR SHALL TAKE FINAL ELEVATIONS OF ALL SETTLEMENT REFERENCE MARKERS AND SHALL SUBMIT THIS DATA TO THE CONTRACTING OFFICER REPRESENTATIVE (COR). THE COR WILL FURNISH THIS DATA TO ENGINEERING DIVISION, ATTENTION OF: CELMN-ED-DD.

THE FIRST SETTLEMENT REFERENCE BOLT IS LOCATED NORTH OF THE RAILROAD GATE MONOLITH. REFERENCE MARKER I.D. TAGS SHALL BE STAMPED WITH THE APPLICABLE W/L STATION NUMBER.

FOR GENERAL NOTES, SEE DWG. 2.



U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS HAH CORPS OF ENGINEERS

NEW ORLEANS, LOUISIANA

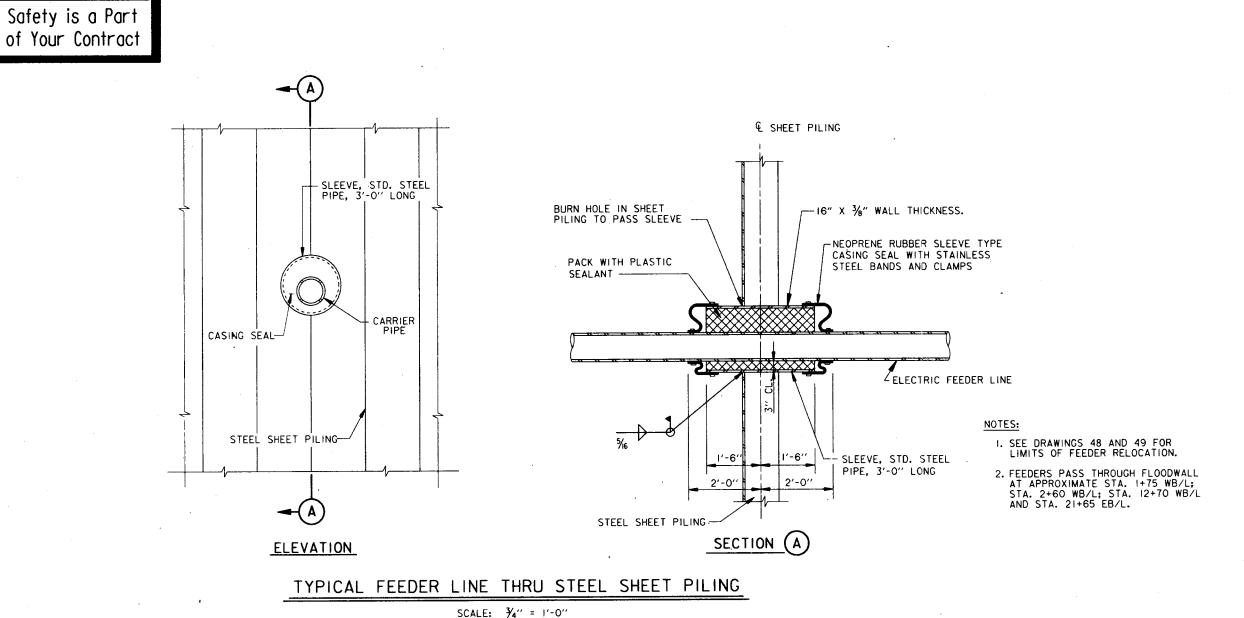
DESCRIPTION

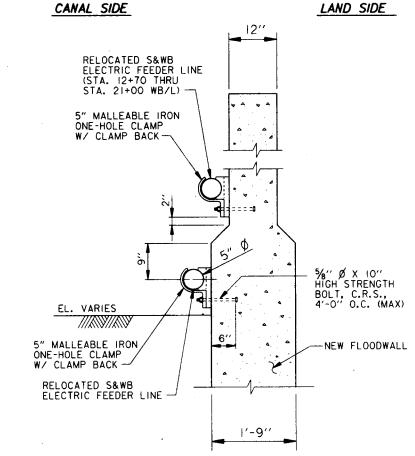
LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN
LONDON AVE. OUTFALL CANAL, PARALLEL PROTECTION PUMP STATION NO. 3 TO MIRABEAU AVE. FLOODWALL ORLEANS PARISH, LOUISIANA

REFERENCE BOLT DETAILS

DESIGNED BY: H.M.B. DRAWN BY: D.J.B. CHECKED BY: W.O.B.	DATE: JUNE 93 CADO FILE: 401	PLOT SCALE:	21 JULY 93 FILE NO. H-4-40145
SUBMITTED BY: WALTER O. BAUMY JR., P.E.	SOLICITATION	N NO. 93-B-0080	45

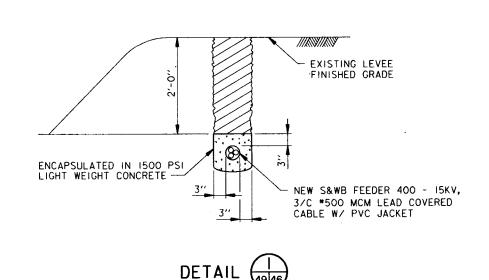
SCALE: 3/4" = 1'- 0" SCALE: 6" = 1' - 0" SCALE: 12" = 1' - 0"





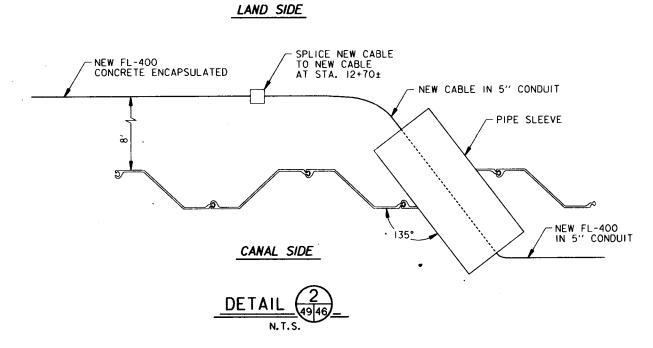
ELECTRIC FEEDER LINE RELOCATED

SCALE: I" = I'- 0"



SCALE: I" = 1'-0"

NOTE: BURY CABLE 4' DEEP BETWEEN STA. 6+76.21 AND STA. 9+97.00 WB/L

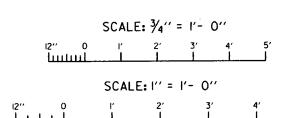


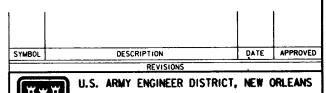
3

2

IF CONDITIONS PERMIT, AN ALTERNATE METHOD OF PASSING A UTILITY LINE THROUGH SHEET PILE CAN BE ACCOMPLISHED WITHOUT CUTTING THE UTILITY LINE. THIS METHOD CONSISTS OF LATERALLY DISPLACING THE UTILITY LINE, DRIVING THE SHEET PILING, NOTCHING THE SHEET PILE AND INSTALLING SLEEVES IN HALVES.

SLEEVE INSTALLATION IN HALVES





D

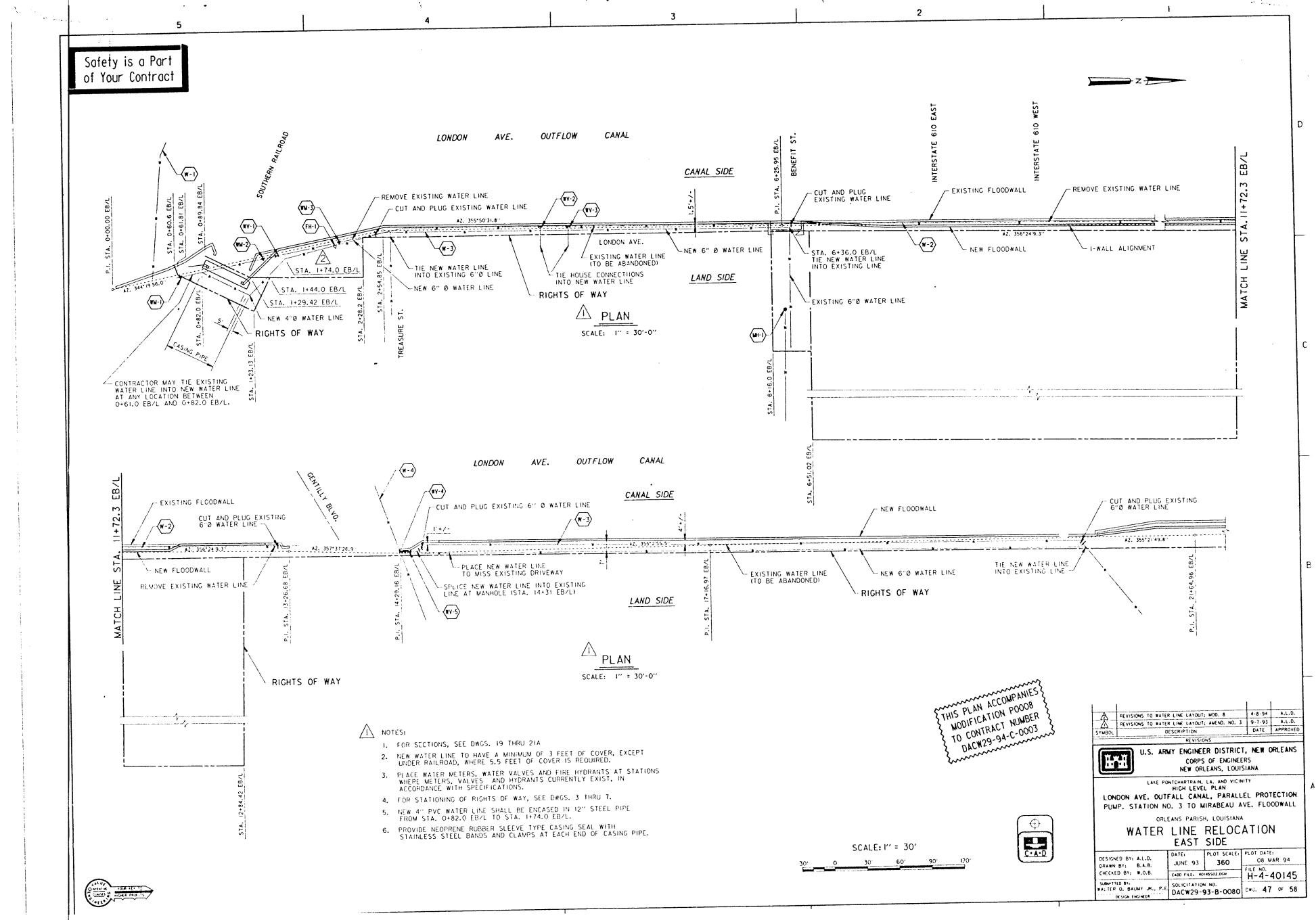
CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA

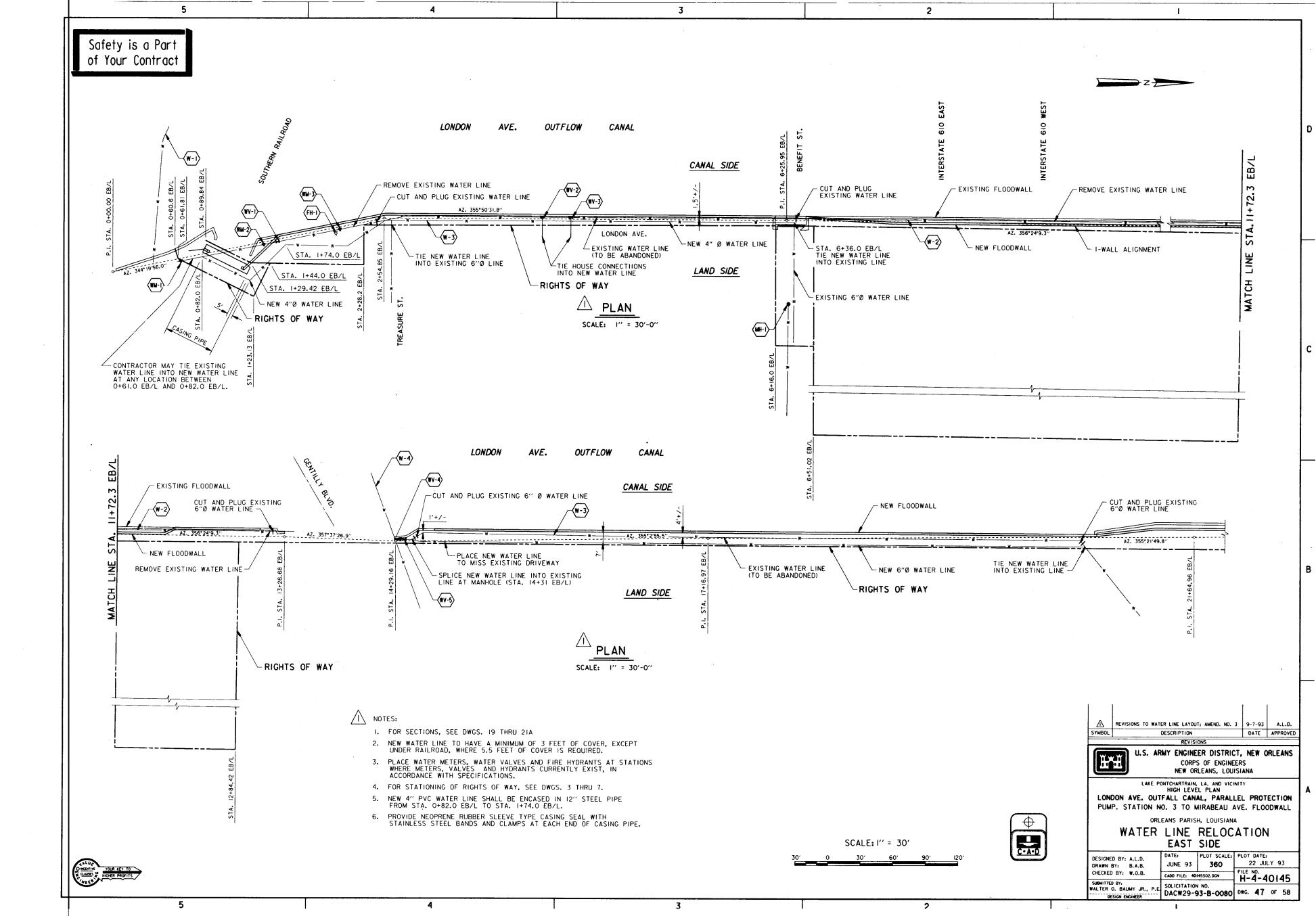
LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN LONDON AVE. OUTFALL CANAL, PARALLEL PROTECTION PUMP STATION NO.3 TO MIRABEAU AVE. FLOODWALL

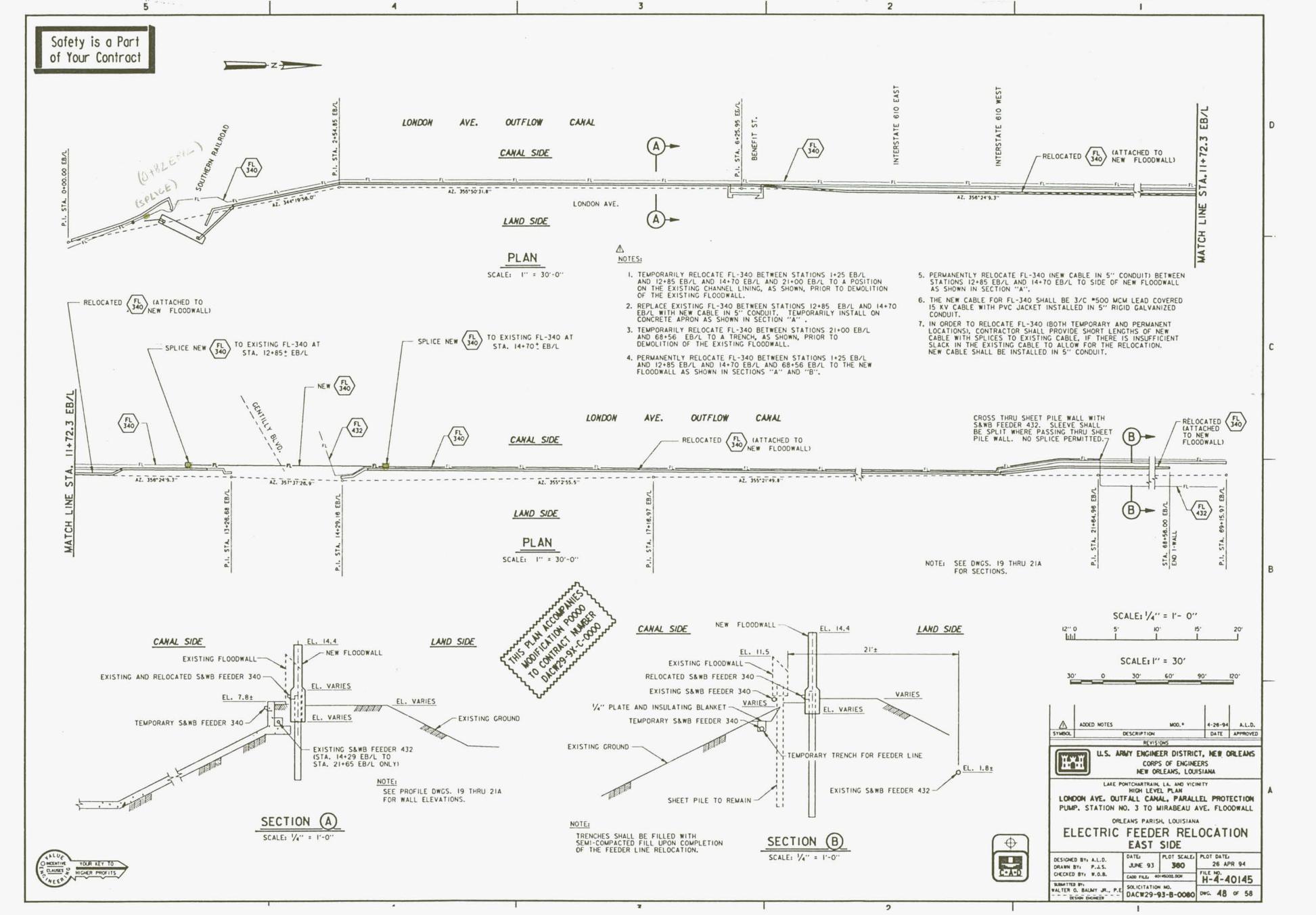
> ORLEANS PARISH, LOUISIANA UTILITIES

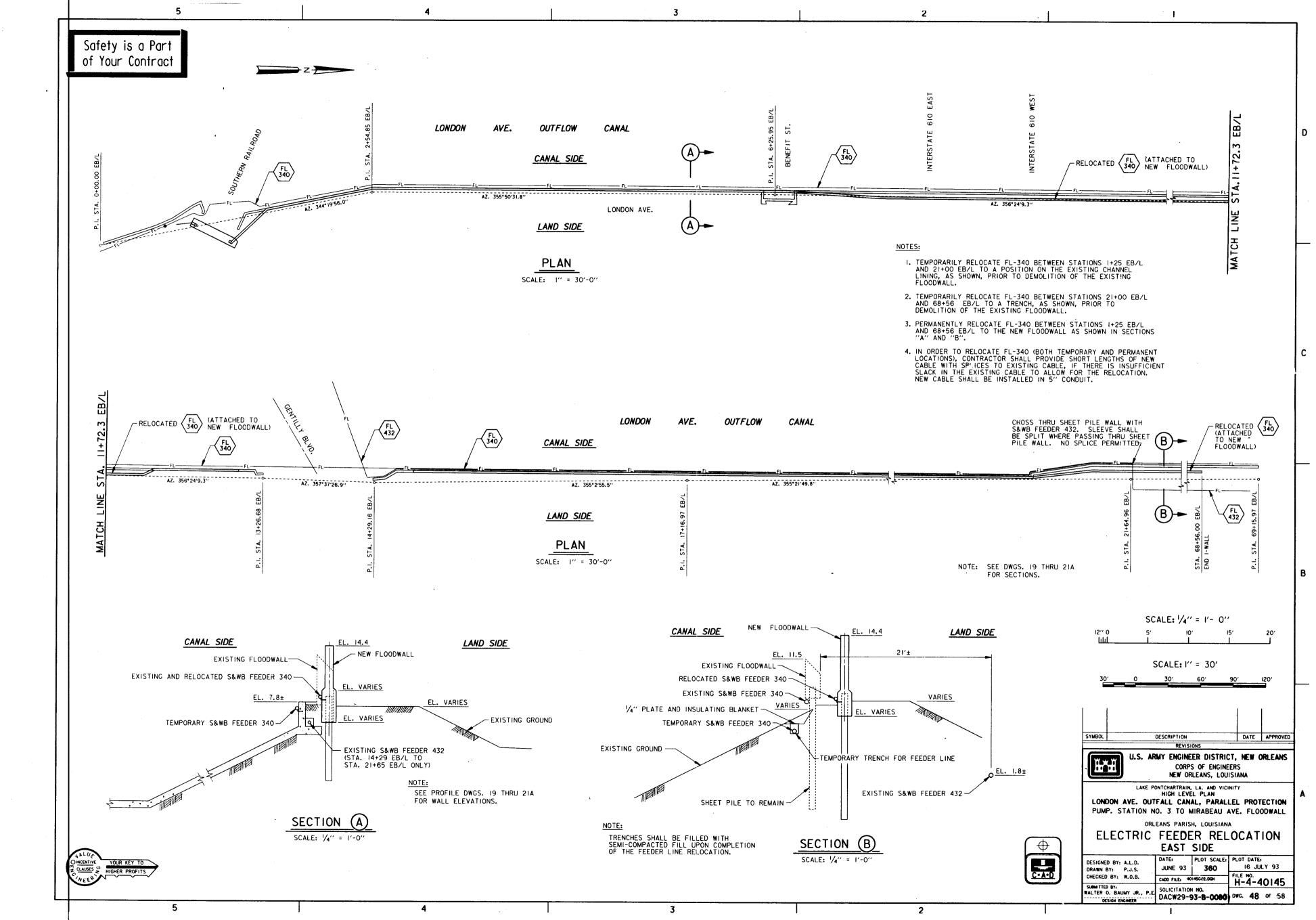


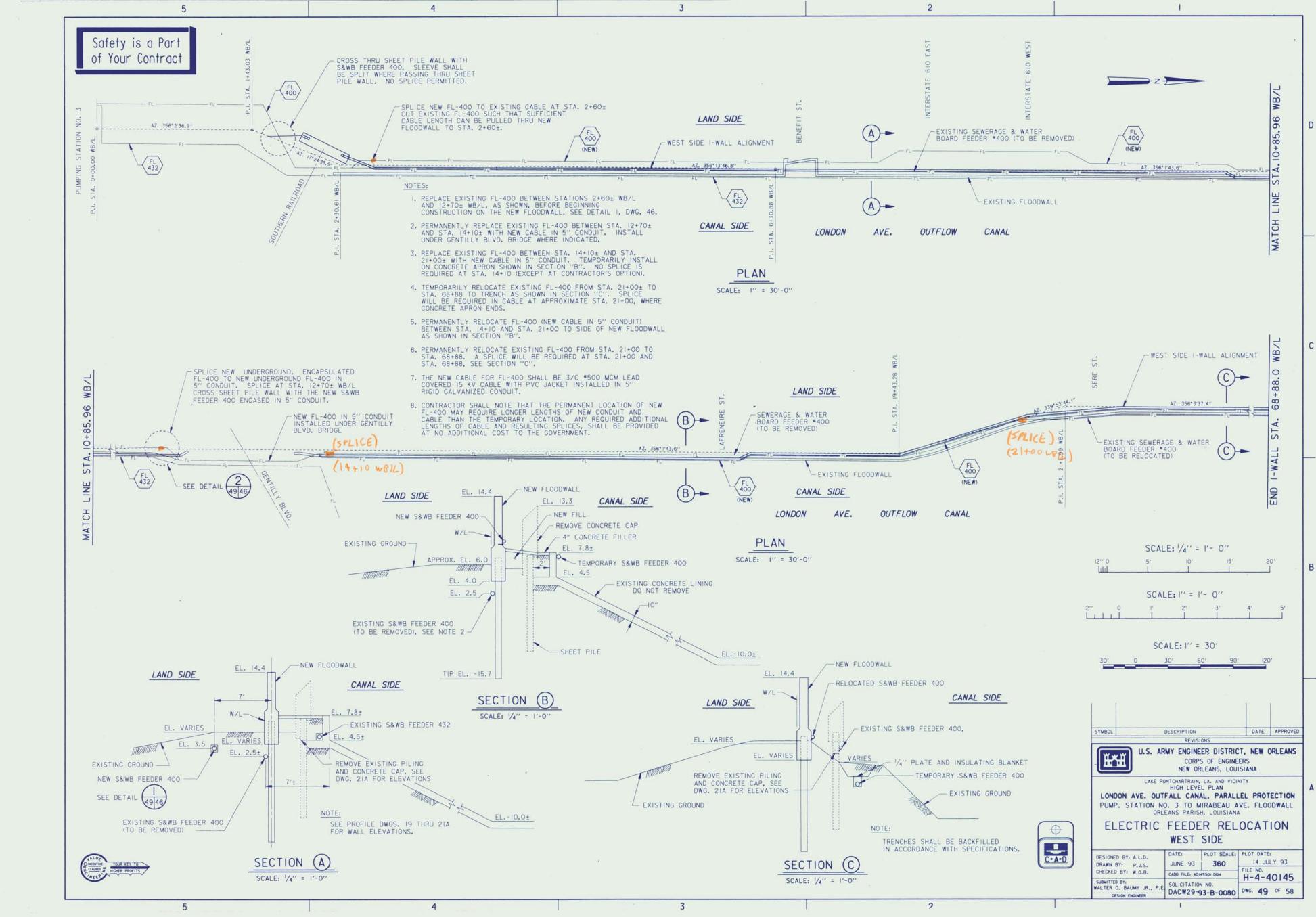
DESIGNED BY: A.L.O. DRAWN BY: J.E.B. CHECKED BY: W.O.B. 14 JULY 93 JUNE 93 16 FILE NO. H-4-40145 SOLICITATION NO. DACW29-93-B-0080 DWG. 46 OF 58 SUBMITTED BY: WALTER O. BAUMY JR., P.

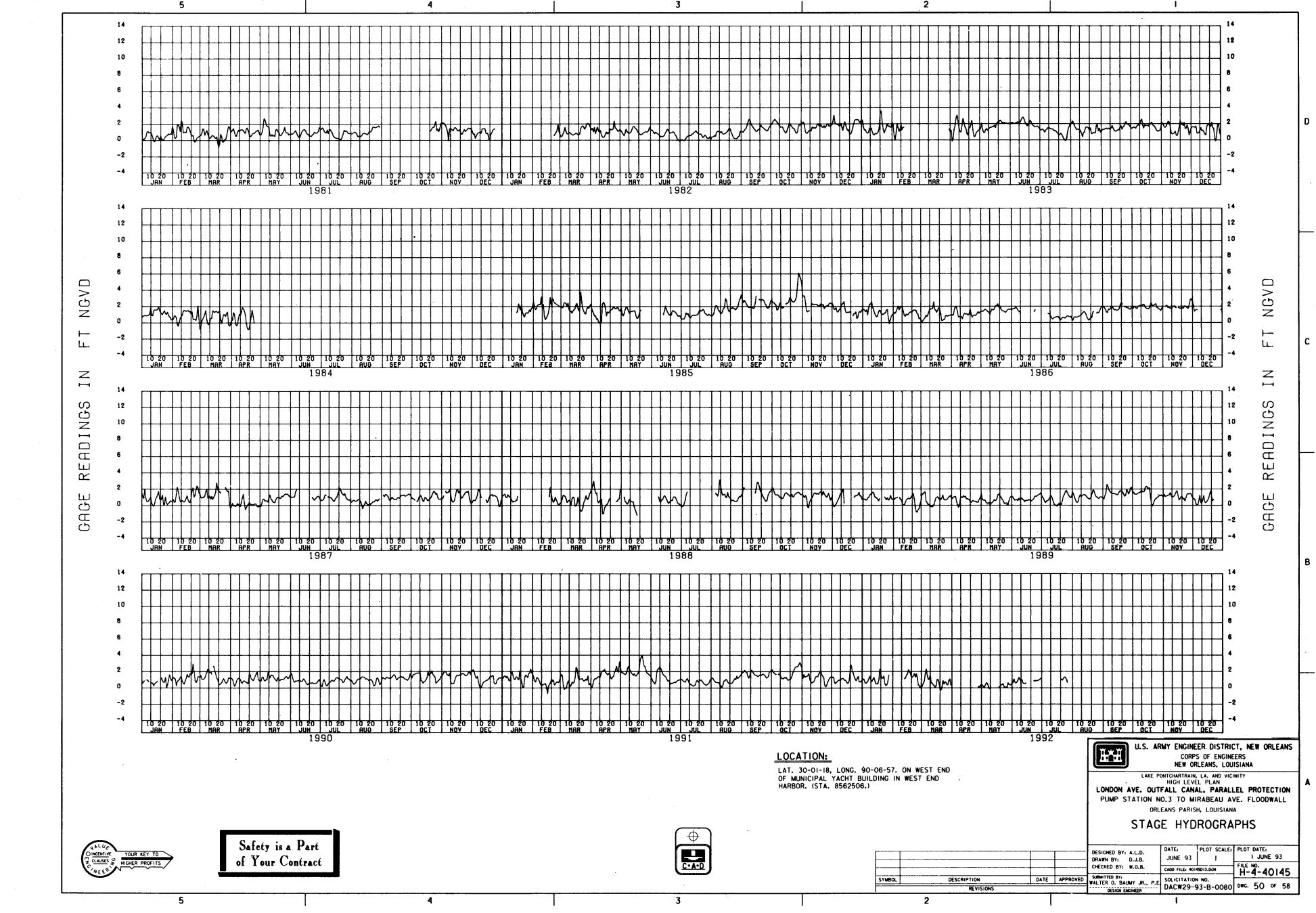


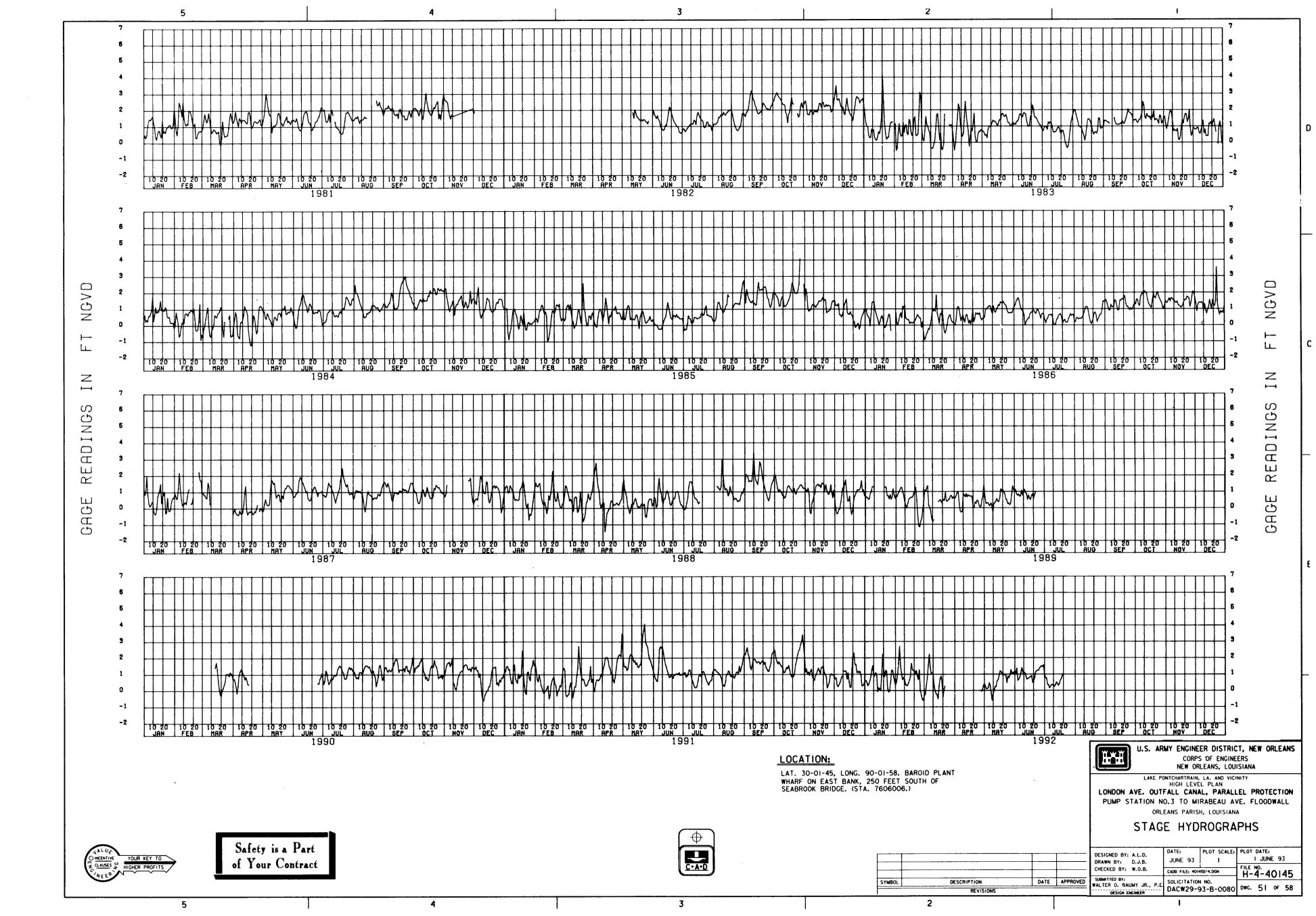












5

	Į	JNI	FI	ED SOIL CLASSIFICATION
MAJOR DIVISION	TYPE	LETTER SYMBOL		TYPICAL NAMES
N P P P P P P P P P P P P P P P P P P P	CLEAN GRAVEL	GW	000	GRAVEL, Well Graded, gravel-sand mixtures, little or no fines
AINED SOILS size. GRAVELS More than half of coorse fraction is increased.	(Little or No fines)	GP	33	GRAVEL, Poorly Graded, gravel-sand mixtures, little or no fines
A par tris	GRAVEL WITH FINES	GM		SILTY GRAVEL,gravel-sand-silt mixtures
Mor coor coor ior sieve	(Appreciable Amount of Fines)	GC		CLAYEY GRAVEL,gravel-sand-clay mixtures
alf of r sieve	CLEAN SAND	SW	0000	SAND, Well-Graded, gravelly sands
DARSE - e than ha no. 200 SANDS than half than than ke traction er than ke size.	(Little or No Fines)	SP		SAND,Poorly-Graded,gravelly sands
COARSE - GR More than half of than No. 200 sieve SANDS SANDS More than half of coorse froction is smaller than No. 4 smaller than No. 4	SANDS WITH FINES	SM	0000	SILTY SAND, sand-silt mixtures
More sewal	(Appreciable Amount of Fines)	SC		CLAYEY SAND, sand-clay mixtures
SOILS	SILTS AND	ML	\prod	SILT & very fine sand, silty or clayey fine sand or clayey silt with slight plasticity
_ ¥ %	CLAY	CL		LEAN CLAY, Sandy Clay, Silty Clay, of low to medium plasticity
GRAINED half the m than No. 24	< 50)	OL		ORGANIC SILTS and organic silty clays of low plasticity
	SILTS AND	МН		SILT, fine sandy or silty soil with high plasticity
NE - re than smaller ve size.	CLAY	CH		FAT CLAY, inorganic clay of high plasticity
FINE More to Is smooth sieve a	> 50)	OH		ORGANIC CLAYS of medium to high plasticity,organic silts
HIGHLY ORGANIC	SOILS	Pt		PEAT, and other highly organic soil
WOOD		Wd		W00D
SHELLS		SI	:::::	SHELLS
NO SAMPL	Ε			
NOTE: Soils p	ossessin	ig char	acte	ristics of two groups are designated by combinations of group symbols

COLOR		,	CONSISTENCY		MODIFICATIONS	
COLOR	SYMBOL		FOR COHESIVE SOILS		MODIFICATION	SYMBOL
TAN	Т	CONSISTENCY	COHESION IN LBS./SQ.FT. FROM	SYMBOL	Traces	īr-
YELLOW	Y		UNCONFINED COMPRESSION TEST		Fine	F
RED	R	VERY SOFT	< 250	vSo	Medium	М
BLACK	ВК	SOFT	250-500	So	Coorse	С
GRAY	Gr	MEDIUM	500-1000	м	Concretions	СС
LIGHT GRAY	IGR	STIFF	1000-2000	St	Rootlets	rt
DARK GRAY	dGr	VERY STIFF	2000-4000	vSt	Lignite fragments	Ig
BROWN	Br	HARD	> 4000	н	Shale fragments	sh
LIGHT BROWN	IBr				Sandstone fragments	sds
DARK BROWN	dBr	× 60 F		1	Shell fragments	sif
BROWNISH-GRAY	br Gr	INDEX			Organic matter	0
GRAYISH-BROWN	gy Br		CH		Clay strata or lenses	, cs
GREENISH-GRAY	gn Gr	PLASTICITY	CL LYNE		Silt strata or lenses	SIS
GRAYISH-GREEN	gy Gn	ST	14.		Sand strata or lenses	SS
GREEN	Gn	PLA	OH		Sandy	S
BLUE	Ві	_ ·	ML- OL MH		Gravelly	G
BLUE-GREEN	BI Gn	<i>-</i> - 1	**************************************		Boulders	В
WHITE	Wh	ا م ا	20 40 60 80 II	 	Slickensides	SL
MOTTLED	Mot	0	L.LLIQUID LIMIT	00	Wood	Wd
			E.C. LIGOTO LIMIT		0xidized	0x
			PLASTICITY CHART			
	+		For classification of fine-grained soils			<u> </u>

	(
BLACK	ВК	SOFT	250-500	So	Coarse	С
GRAY	Gr	MEDIUM	500-1000	М	Concretions	cc
LIGHT GRAY	IGR	STIFF	1000-2000	St	Rootlets	rt
DARK GRAY	dGr	VERY STIFF	2000-4000	vSt	Lignite fragments	Ig
BROWN	Br	HARD	> 4000	Н	Shale fragments	sh
LIGHT BROWN	IBr			,	Sandstone fragments	sds
DARK BROWN	dBr	× 60 F			Shell fragments	sif
BROWNISH-GRAY	br Gr	INDEX			Organic matter	0
GRAYISH-BROWN	gy Br	1	CH		Clay strata or lenses	, cs
GREENISH-GRAY	gn Gr	PLASTICITY 00 04 04 04 04 04 04 04 04 04 04 04 04	CL LIME		Silt strata or lenses	SIS
GRAYISH-GREEN	gy Gn	ST			Sand strata or lenses	SS
GREEN	Gn	PLA	OH		Sandy	S
BLUE	ВІ	1	-ML OL MH		Gravelly	G
BLUE-GREEN	BI Gn	-	Minus &		Boulders	В
WHITE	Wh	g o .	20 40 60 80 IO	_	Slickensides	SL
MOTTLED	Mot		L.LLIQUID LIMIT		Wood	Wd
			E.CCIGOID LIMIT		0xidized	0×
			PLASTICITY CHART			
			For classification of fine-grained soils			
		l	For classification of fine-grained soils			•
	ļ				i	

FIGU	RES TO LEFT OF BORING UNDER COLUMN "W OR DIO"
Are n	atural water contents in percent dry weight
When	underlined denotes D (o size in mm *
FIGURE	S TO LEFT OF BORING UNDER COLUMNS "LL" AND "PL"
Are I	iquid and plastic limits, respectively
SYMBO	LS TO LEFT OF BORING
	Ground-water surface and date observed
©	Denotes location of consolidation test **
<u>\$</u>	Denotes location of consolidation-drained direct shear test **
_	
®	Denotes location of consolidation-undrained triaxial compression test
® ©	Denotes location of consolidation-undrained triaxial compression test Denotes location of unconsolidated-undrained triaxial compression test
0	Denotes location of unconsolidated-undrained triaxial compression test Denotes location of sample subjected to consolidation test and each of

2

In parenthesis are driving resistances in blows per foot determined with a standard split spoon sampler (1 $\frac{1}{8}$ " I.D., 2" 0.D.) and a 140 lb. driving hammer with a 30" drop

Where underlined with a solid line denotes laboratory permeability in centimeters per second of undisturbed sample

Where underlined with a dashed line denotes laboratory permeability in centimeters per second of sample remoulded to the estimated natural void ratio.

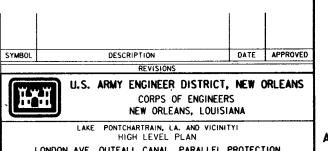
- * The D_{IO} size of a soil is the grain diameter in millimeters of which 10% of the soil is finer, and 90% coarser than D_{10}
- *Results of these tests are available for inspection in the U.S. Army Engineer District Office, if these symbols appear beside the boring logs on the drawings

TYPICAL NOTES:

While the borings are representative of subsurface conditions at their respective locations and for their respective vertical reaches, local variations characteristic of the subsuface materials of the region are anticipated and, if encountered, such variations will not be considered as differing materially within the purview of the contract clause entitled "Differing Site Conditions."

Ground-water elevations shown on the boring logs represents ground-water surfaces encountered in such borings on the dates shown. Absence of water surface data on certain borings indicates that no ground-water data are available from the boring but does not necessarily mean that ground-water will not be encountered at the locations or within the vertical reaches of such borings.

Consistency of cohesive soils shown on the boring logs is based on driller's log and visual examination and is approximate, except within those vertical reaches of the borings where shear strengths from unconfined compression tests are shown.

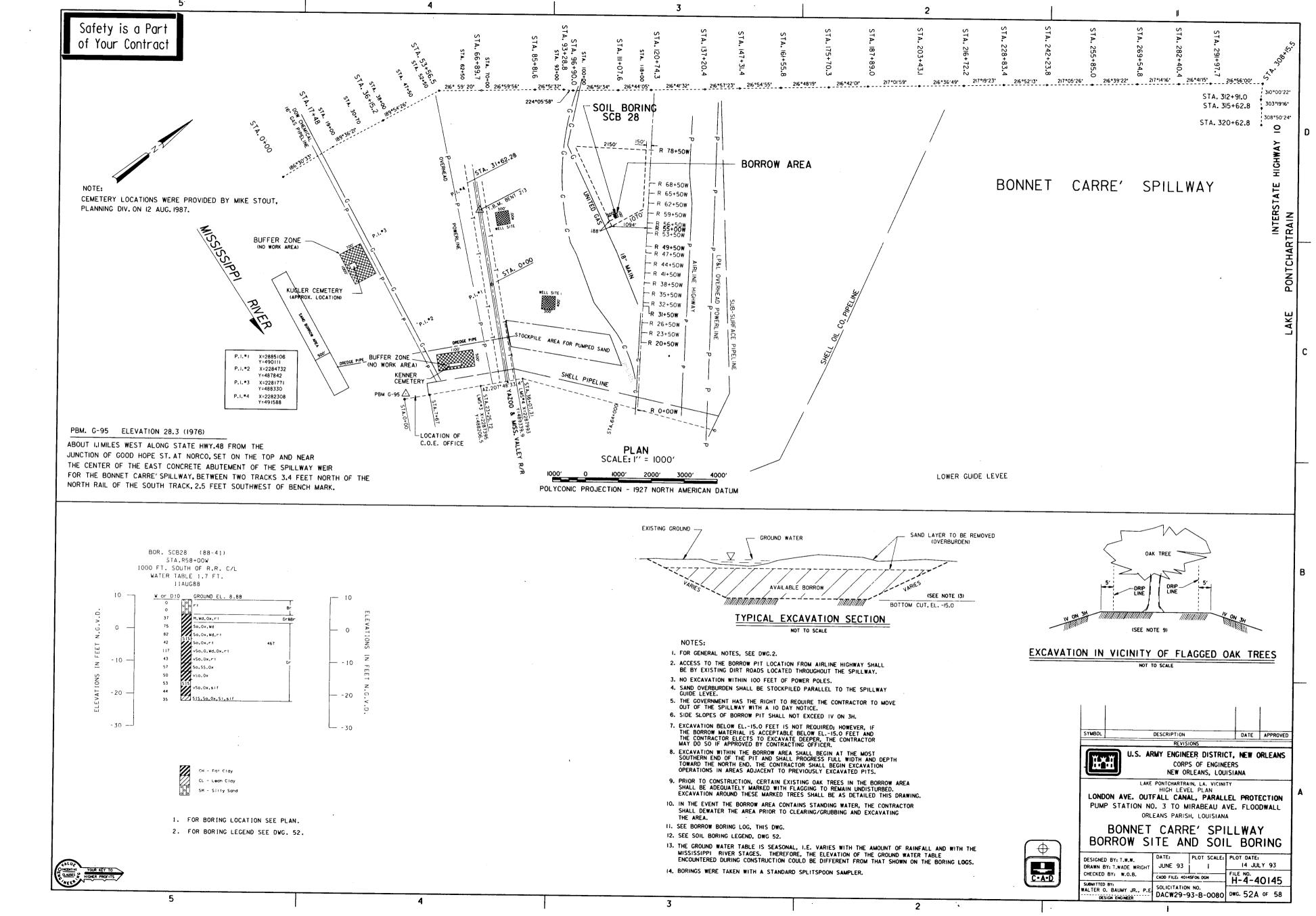


LONDON AVE. OUTFALL CANAL, PARALLEL PROTECTION PUMP STATION NO.3 TO MIRABEAU AVE. FLOODWALL

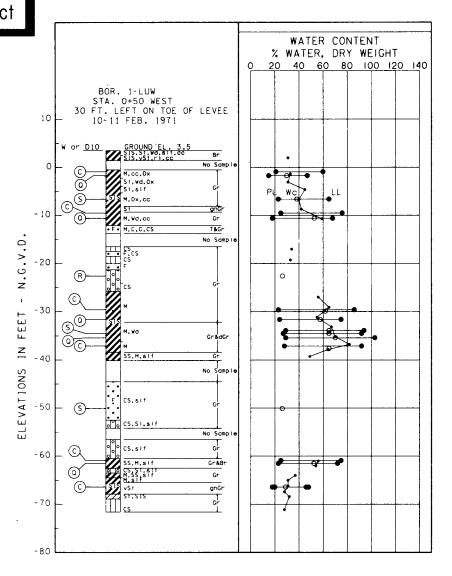


	SOIL	BORIN	IG LEG	END
SIGNED BY	A L D	DATE:	PLOT SCALE:	PLOT DATE

14 JULY 93 JUNE 93 DRAWN BY: J.E.B. CHECKED BY: W.O.B. H-4-40145 CADD FILE: 40145HI- DGN SUBMITTED BY:
WALTER O. BAUMY JR., P.E. DACW29-93-B-0080 DWG. 52 OF 58 DESIGN ENGINEER



Safety is a Part of Your Contract



BOR. 4-LUE STA. 49+75 38 FT. RT. OF SEAWALL TOE OF LEVEE 23 NOV-10 DEC 70

(3)

(\$)-

Ò

0

- 70

R

. S

(34)

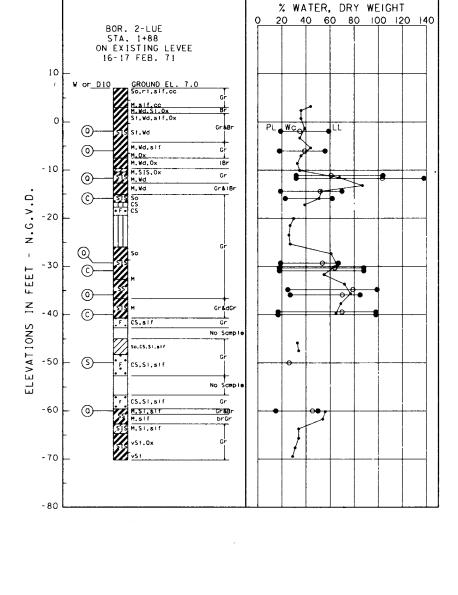
(51) (101)

(12) Sar

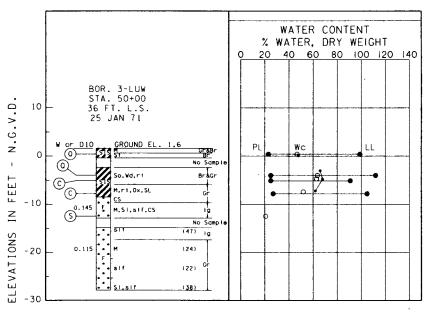
WATER CONTENT

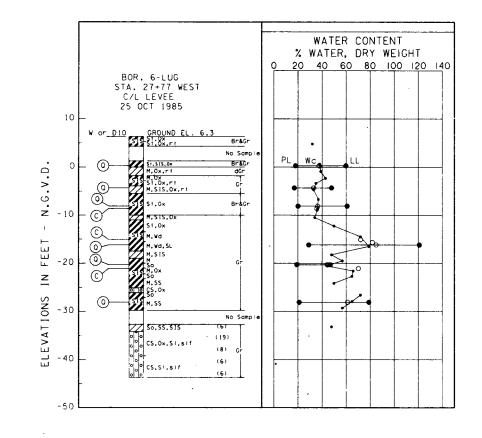
% WATER, DRY WEIGHT

PL WC LL



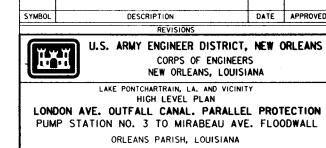
WATER CONTENT





1. FOR BORING LOCATIONS SEE DWGS. 8 THRU 12.

2. FOR BORING LEGEND SEE DWG. 52.



SOIL BORINGS

PLOT SCALE: PLOT DATE: DESIGNED BY: PINNER DRAWN BY: WOODS 21 JULY 93 JUNE 93 24 FILE NO. H-4-40145 CHECKED BY: RICHARDS SUBMITTED BY: WALTER O. BAUMY JR., P.E. SOLICITATION NO.
DACW29-93-B-0080 DWG. 53 OF 58

London Avenue Canal, Levee and Floodwall Improvements Orleans Levee Board Project No. 2049-0269, New Orleans, Louisian For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, 1

loring	No	1 s	oli Tech	nicien _	A. Croml, Jr. Date 3 Octo	obe	1985	
					Deturn Gr. Water Depth_Ser	T	ext	,
-	-	7L	10000		VIDUAL GLADIFEATION	Γ,	TANKA	•
•	-	2	3	70		_		
	1.7	2.5	0.0	2.5	Very stiff gray & brown clay w/shells,	-		
					brick fragments, clayey silt pockets	├-		3
					6 grass roots	-	\vdash	
. 2	4.7		2.5	-	Soft tan & gray clay w/silt pockets	-		
3	7.7	8.5	6.0		Medium stiff gray clay w/silt pockets	├-		
					& few brick fragments	-		4
4	10.7	11.5			Medium stiff gray clay w/sandy silt	<u> </u>		
					pockets, shells & brick fragments	Щ		
5	13.7	14.5			Medium stiff gray clay w/shells, brick	L		
					fragments & organic matter	匚		5
6	18.2	19.0		20.0	Medium stiff to stiff gray clay w/sandy	L		E .
					silt pockets, trace of organic			ž
					matter & brick fragments			Ĺ
7	22.2	23.0	20.0	23.0	Loose gray clayey silt			
8	23.0	24.5	23.0		Medium dense gray silty sand	5	20	•
9		27.0			Ditto .	5	23	
10		29.5		30.0	Ditto	7	24	
11		34.0		_	Medium stiff gray clay w/clayey silt			
	-	- ***			lenees			
12	18 2	39.0			Medium stiff gray clay	Г		
13		44.5		46.5	Medium stiff gray clay w/clayey silt			
• •	77.0	****			& few silty sand lenses			_
14	48.2	49.0	46.5	52.0	Loose gray fine sand w/clay pockets	Г		•
•		12.1			& shell fragments	П		
15	53.2	54.0	52.0	54.5	Loose gray silty sand w/clay pockets,			
	-33.6				lenses & shell fragments			
16	54 5	56.0	54.5	57.0	Medium dense gray silty sand	5	25	•
17					Loose gray silty sand w/clay layers	1	7	
18					Very loose gray clavey sand	2	3	
19					Soft gray sandy clay w/sand nockets,			
19	63.2	04.0	02.5	00.3	thin shell layers & shell fragments			
	(0.0	(0.0		71 5	Medium stiff gray clay w/sandy silt			
20	68.2	69.0	50.5	/1.5				
					pockets & shell fragments			
21	73.2	74.0	71.5		Stiff greenish-gray clay w/clayey slit	Н		Ε.
					nockets & lenses			
22	76.7	77.5		80.0	Stiff greenish-gray clay w/clayey silt	-		Ĕ
					lenses, lavers & sandy silt lavers			•

Name of Project: London Avenue Canal, Levee and Floodwall Improvements Orleans Levee Board Project No. 2049-0269, New Orleans, Louisiana For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, Burk & Associates, Inc., New Orleans, Louisiana Date 5 October 1985 ___ Detum _____ Gr. Water Depth See Text 1 1.5 2.5 0.0 Stiff tan & gray clay w/sandy silt pockets & shells 2 4.51 3.51 6.0 Stiff tan & gray clay w/silt pockets 3 7.5 8.5 6.0 9.0 Medium stiff brown & gray clay w/clayey silt pockets 4 10.5 11.5 9.0 Medium stiff to soft tan & gray silty clay w/clay pockets clay w/clay pockets
4.5 14.0 Soft gray 6 tan silty clay w/clay 5 13.5 14.5 layers 6 18.5 19.5 16.0 20.0 Medium stiff gray clay w/organic matter 6 silty clay layers 7 23.0 24.0 20.0 24.5 Soft gray silty clay w/sandy silt 9 33.5 34.5 29.5 Soft gray clay w/clayey silt lenses & layers 11 43.5 44.5 46.0 Medium stiff gray clay w/clayey silt lenses 6 layers lenses & layers

12 48.5 49.5 46.0 52.5 Loose gray clayey sand w/shells & clay 17 63.5 64.5 68.0 Medium stiff gray sandy clay w/sand pockets & shell fragments 18 68.5 69.5 68.0 72.0 Medium stiff gray 6 brown clay w/sand pockets

19 73.5 74.5 72.0 Very stiff gray 6 tan clay w/clayey
silt pockets

Name of Project: London Avenue Canal, Levee and Floodwall Im Orleans Levee Board Project No. 2049-0269, New Orleans, Louis For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, Le Boring No. 3 Soil Technicien George Hardne

3roun	d Elev				Deturn Gr. Water Depth	See Text	-
_	-		<u></u>		HOUSE GLASSFICETON	T-11040	7
_	~	- * -	~-	2 2		1 100	4
	—	_	0.0	0.7	100/ 0 000 000/ 0.0/	++	1
	- -	-		ļ	w/roots & shells	 	4
1	1.5	2.5	0.7	2.5	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	 	1
				ļ	lenses & pockets	11	-
2	4.5	5.5	2.5	7.0	Stiff tan & gray clay w/clayey silt	11	1
		L			pockets & silty clay layers	↓	1
3	7.5	8.5	7.0		Loose tan & gray clayey silt w/silty		1
					clay layers	<u> </u>	1
4	10.5	11.5		12.0	Loose tan & gray clayey silt]
5	13.5	14.5	12.0		Very loose to loose gray clayey silt		
					w/clay & silty clay lenses		E .
6	18.5	19.5			Very loose to loose gray clayey silt	11	1:
_					w/silty clay layers		1
7	23.5	24.5			Very loose to loose gray clayey silt		1
					w/roots & silty clay layers		1
8	28.5	29.5		30.0	Very loose to loose gray clayey silt		1
					w/silty clay layers	11	1
9	33.5	34.5	30.0	36.0	Soft gray silty clay w/clayey silt		1
					layers		1
10	38.5	39.5	36.0	43.0	Soft gray silty clay w/clayey silt	11	1
				_	layers	1	1
11	43.5	44.5	43.0		Medium compact gray clayey silt	 	1
		1			w/silty clay layers	11	1
12	48 5	49.5		50.0	Ditto	 	1

London Avenue Canal, Levee and Floodwall Improvements Orleans Levee Board Project No. 2049-0269, New Orleans, Louisiana For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, La

lenses, layers & sandy silt layers

Burk & Associates, Inc., New Orleans, Louisiana George Hardee

	i Elev				Deturn Gr. Water Depth_S			7
_	-	Y	-	-	Village, Q. Addardic A Tillan		1	
_	Press	*		200	3-3-3- ()	╁		1
-			0.0	0.2	Asphalt & gravel	+		1
1	1.5	2.5	0.2		Stiff tan & gray silty clay w/clay	+		3
					pockets, brick fragments & clayey	+		1
					silt layers	+	-	1
2	4.5	5.5		6.0	Stiff tan & gray silty clay w/clayey	╁╌		1
_					silt layers & some gravel	+-		•
3	7.5	8.5	6.0	9.0		╀		1
	\vdash				silt pockets	╀╌		1
4	10.5	11.5	9.0		Soft gray & tan silty clay w/clayey	╁		1
					silt layers & lenses	+-		۹ ۾ ا
5	13.5	14.5			Soft gray & tan silty clay w/clayey	+		1:
					silt layers & roots	+-		ŧ
6	17.0	18.0		19.5	Soft gray & tan silty clay w/roots &	+-		┨╸
					clayey silt layers	+-		
7	20.5	21.0	19.5		Soft gray silty clay w/roots, clayey	╁		-
					silt lenses & clay pockets	⊬		4
8	23.5	24.5			Wery soft gray silty clay w/clayey silt	-		1
					& clay lenses	┼-		1 :
9	28.5	29.5		31.5	Soft gray silty clay w/clayey silt &	 		-
					clay layers	+		4
10	33.5	34.5	31.5		Very loose gray clayey silt w/silty	+-		1
					clay lenses & pockets	╀		- 4
11	38.5	39.5			Loose gray clayey silt tycky & silty	┾		i i
					clay layers	╀		-
12	43.5	44.5	L,	46.0	foose gray clayey silt w/clay layers	╁		┨
13	48.5	49.5	46.0		Medium compact gray clayey silt	├		-
		L			w/alternating clay lenses	\vdash		١.
14	53.0	54.0			Ditto	↓_		┦ `
15	57.5	58.5		58.5	Medium compact gray clayey silt w/clay	-	-	-
					pockets & trace of sand	╀		4
16	58.5	60.0	58.5	60.5	Medium compact gray sandy silt	+	22	4
17	61.0	62.5	60.5		Medium stiff gray silty clay	2	6	Ι΄
18	63.5	65.0		65.0	Medium stiff gray silty clay w/clayey	13	14	4
					silt & clay layers	丄		1
19	68.5	69.5	65.0	71.0	Stiff gray clay w/sandy silt layers	١		1
20	74.5	75.0	71.0	75.0	Modium dense gray silty sand w/clay	1		1:
					layers			1.
21	78.5	79.5	75.0	80.0	Medium stiff greenish-gray & tan clay	1_		1
	1	Ī			w/silt lenses	1		

Ditto

23 92.5 93.5 93.5 Stiff greenish-gray & tan clay w/sand layers

25 98.5 100.0 97.5 100.0 Medium compact gray sandy silt w/clayey 6 30

22 88.5 89.5 Innses

24 95.0 96.5 93.5 97.5 Dense gray fine sand

silt s clay layers

Orleans Levee Board Project No. 2049-0269, New Orleans, Louisiana

-	No d Elev		Soil Tect		George Hardee Dess 8 Oc Deturn Gr Water Depth		-
_	-	-V.	•	<u></u>	What Greener was	- Transcep - Transcep	ī
_	~	•	~	-		-	4
1	1.5	2,5	0.0	2.5	Madium stiff tan & gray silty clay	 	┨
2	5.0	5.5	2.5	4.0	w/cincers Medium compact miscellaneous fill	 - 	┨
-	3.0	3.3	4.3	6.0	(cinders, brick fragments & clay	┼┼	-
_		 			pockets)	 - 	┨
3	7.5	8.5	6.0	<u> </u>	Stiff tan 6 gray clay w/clayey silt	 	1
Ť		71.0	7.0		pockets	 	1
4	10.5	11.5		12.0	Stiff tan & gray clay w/silt pockets	 	1
					& silty clay layers	1-1	1
5	13.5	14.5	12.0	_	Soft tan & gray silty clay w/clayey		1
					silt pockets	11	
6	18.5	19.5		20.0	Soft tan & gray silty clay w/clay		1:
					pockets		1
7	23.5	24.5	20.0	27.0	Soft gray silty clay w/clayey silt	Ш <u>.</u>]
_					lenses & roots]
8	28.5	29.5	27.0	31.0	Loose gray sandy silt w/silty clay]
4					& clayey silt layers	<u> </u>	1
9	33.5	34.5	31.0	36.0	Loose gray silty sand w/clayey silt		1
4					layers	L	
익	38.5	39.5	36.0	42.5	Medium stiff dark gray clay w/silt	<u> </u>	-
4					lenses		ļ
4	43.5	44.5	42.5	46.0	Loose to medium dense gray sulty sand	.	1
1				60.6	w/clay layers	 	
2	49.0	49.5	46.0	50.0			į
-	\rightarrow	\longrightarrow			clay pockets		

LCG OF BORING

Name of Project: London Avenue Canal, Levee and Floodwall Improvem Orleans Levee Board Project No. 2049-0269, New Orleans, Louisians For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, Burk & Associates, Inc., New Orleans, Louisiana Soring No. 6 Soil Technician George Hardee Date 7 October 1985

-	-		****	_		7.00
_	~=	•	~-	70		
1	1.5	2.5	0.0	2.5	Stiff tan & gray clay w/clayey silt	
					pockets	
2	4.5	5.5	2.5	6.0	Stiff tan & gray clay w/silt pockets	\Box
3	7.5	8.5	6.0		Medium stiff tan & gray clay w/clayey	
					silt pockets & silty clay layers	
4	10.5	11.5			Medium stiff tan & gray clay w/silt	
_					pockets	
5	13.5	14.5		16.0	Ditto	
6	18.5	19.5	16.0	20.0	Medium stiff tan & gray silty clay	
					w/clay layers	
7	23.5	24.5	20.0		Soft gray clay w/roots & trace of	
					organic matter	
8	28.5	29.5			Soft gray clay w/silt lenses & silty	
					clay layers	
9	33.5	34.5			Soft gray clay w/sandy silt & silty	
_					sand lenses	
0	38.5	39.5	1		Soft gray clay w/silt lenses	
Ц	43.5	44.5		45.0	Soft gray clay w/sand lenses & layers	
2	48.5	49.5	45.0		Loose gray clayey sand w/shells	

Geotechnical Investigation London Avenue Canal Levee and Floodwall Improvement ins Levee Board Project No. 204 New orleans, Louisiana

Inc., Engineers, Planners & Environmental Scientista New Orleans, Louisiana

LOCATION OF BORINGS

	Approx	
Boring	Station	
Number	Number	Location
B-1	0+85	Levee Crown
8-2	7+60	Levee Crown
B-3	11+60	Levee Crown
B-4	14+70	Levee Crown
B-5	19+60	Levee Crown
B-6	24+60	Levee Crown
B-7	29+60	Levee Crown
B-8	34+60	Levee Crown
B-9	39+60	Levee Crown
B-10	44+60	Levee Crown
B-11	50+35	Levee Crown
B-12	55+00	Levee Crown
B-13	60+00	Levee Crown
B-14	65+00	Levee Crown
B-15	69+85	Levee Crown
B-36	1+95	Levee Crown
B-37	7+10	Levee Crown
B-38	11+60	Levee Crown
B-39	13+70	Levee Crown
B-40	21+40	Levee Crown
B-41	24+60	Levee Crown
B-42	29+60	Levee Crown
B-43	34+60	Levee Crown
B-44	39+60	Levee Crown
B-45	44+60	Levee Crown
B-46	50+65	Levee Crown
B-47	55+00	Levee Crown
B-48	60+00	Levee Crown
B-49	65+00	Levee Crown
B-50	68+80	Levee Crown
B-73	19+60	Canal Centerline
B-74	24+60	Canal Centerline
B-75	29+60	Canal Centerline
B-76	34+60	Canal Centerline
B-77	39+60	Canal Centerline
B-78	44+60	Canal Centerline
B-79	50+35	Canal Centerline
B-80	55+00	Canal Centerline
B-81	60+00	Canal Centerline
B-82	65+00 1	Canal Centerline
B-83	69+20	Canal Centerline
	v,	

GENERAL BORING NOTES FOR EUSTIS BORINGS -B-1 THRU B-15, B-36 THRU B-50, AND B-73 THRU B-83

1. Eustis Soil Borings are plotted by depth. 2. For Boring Locations see tabular form above and see dwgs

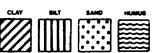
All Eustis undisturbed levee borings were taken with a 3 inch diameter shelby tube sample barrel, except boring B-45, which was taken with a 5 inch diameter shelby tube sample barrel.

4. All canel bottom borings were taken with a 2-inch diameter piston sampler.

5. Standard Penetration Test: Number in first column indicates number of blows of 140 lb. hammer dropped 30-inch required to seat 2-inch 0.D. splitspoon sampler 6-inch. Number in second column indicates number of blows of 140 lb. hammer dropped 30-inch required to drive 2-inch 0.D. splitspoon sampler 1 ft after seating 6-inch.

6. While these logs of borings are considered to be representative of subsurface conditions at its respective location on the date shown, it is not warranted that it is representative of subsurface conditions at other locations and times.

EUSTIS SOIL BORING LEGEND



I. FOR BORING LOCATIONS, SEE DWGS. 8 THRU 12.

2. FOR BORING LEGEND, SEE DWG. 52.

DATE APPROVE SYMBOL DESCRIPTION REVISION

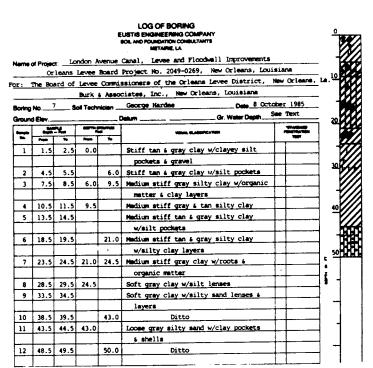
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS CORPS: OF ENGINEERS NEW ORLEANS, LOUISIANA

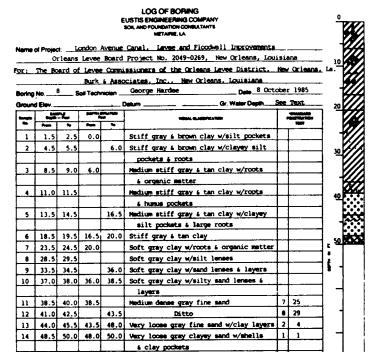
LAKE PONTCHARTRAIN, LA. AND VICINITY

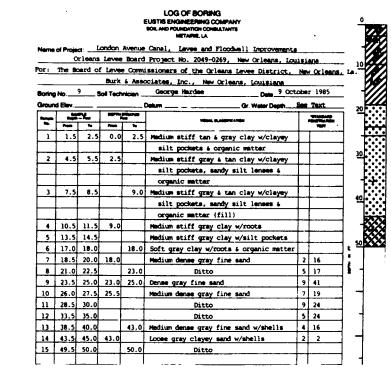
HIGH LEVEL PLAN LONDON AVE. OUTFALL CANAL. PARALLEL PROTECTION PUMP STATION NO. 3 TO MIRABEAU AVE. FLOODWALL ORLEANS PARISH, LOUISIANA

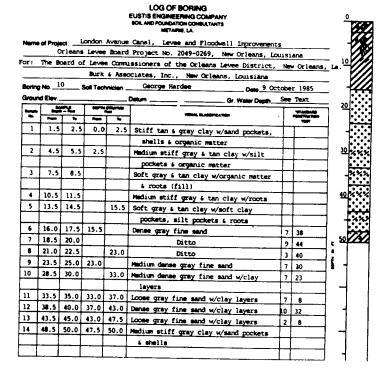
SOIL BORINGS

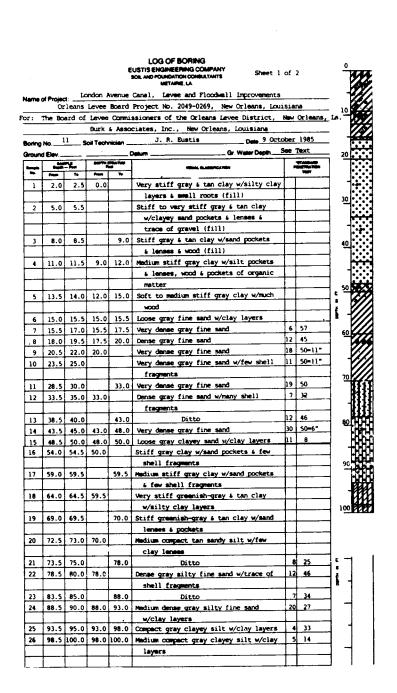
DRAWN BT: WOODS	JUNE 93	PLOT SCALE:		JUN		3		
CHECKED BY: RICHARDSON	CADD FILE: 4014	E023.DGN	H-	1 JUNE 93 TILE NO. H-4-40145				
SUBMITTED BY: WALTER O. BAUMY JR., P.E.	SOLICITATION DACW29-9	но. 93-B-0080	DWG.	54	OF	58		

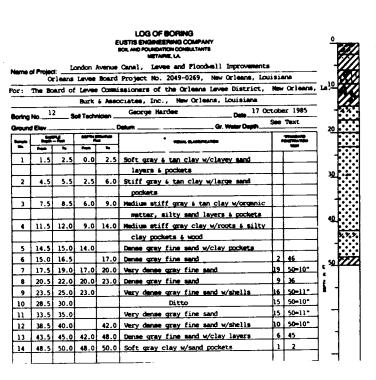












orino	No	3 9	oil Tech	nicien _	George Hardee Dete 17 Oc	tob	r 1985		•
-					Deturn Gr. Water Depth	See	'fext	. ;	0
-	-	<u> </u>			VIQUAL GLASSIFICATION	Γ.	-		
-	****	70		10		╁		1	_
1	1.5	2.5	0.0	3.0	Medium stiff gray & tan clay w/organic	╁╌		1	
_			3.0	6.0	matter & silt & sand pockets Medium stiff gray & tan clay w/roots,	+		1	90
2	4.5	5.5	3.0	6.0	organic matter & sand pockets	+		1	
	7.5	8.5	6.0		Soft gray clay w/roots & organic matter	+		1	
3					Soft dark gray clay w/roots, silt	+		1	
4	10.5	11.5	9.0	13.0	pockets & organic matter	$^{+}$		1	Ю
5		15.0	12.0		Soft gray & tan clay w/roots & wood	+		1	
_	$\overline{}$	17.0	-		Soft gray & tan clay w/roots & wood. Soft gray & tan clay w/large roots &	\vdash		1	
6_	16.0	17.0		17.0	decayed wood	+-		1	
7	17.0	18.5	17.0		Medium dense gray fine sand	5	22	1	50
8		21.5			Ditto	6	·	1 :	
9		25.0		29.0		17		1	-
10	$\overline{}$		_		Dense gray fine sand	1	32	1	
11					Medium dense gray fine sand	+	27	1	-
12		40.0			Dense gray fine sand w/shell fragments	17	39	1	
				_		8		1	-
						13		1	
13 14		45.0 50.0			Dense gray fine sand Loose gray fine sand w/clay layers	+-			

London Avenue Canal, Levee and Floodwall Improvements

:					Project No. 2049-0269, New Orleans, Lou-			_ ,
	The Bo	ard of			ssioners of the Orleans Levee District,	New	Orleans	. ما ي
_					ciates, Inc., New Orleans, Louisiana			_
lorinį	No	14:	Soll Tecl	micien	George Hardee Date 17 Oc	tob	er 1985	-
your	d Elev.				Deturn Gr. Water Depth_S	ee '	rext	- 2
_		-X_		<u></u>	VERMA GLAGOPPEATOR	Ι.	Transacto Contractor	٦ -
	^	Po	~	1 20		╄	~	4
1	2.0	2.5	0.0		Medium stiff gray & tan clay w/organic	1		_
_	ļ	├	<u> </u>	┞	matter & silt pockets	\perp		۱,
2	4.5	5.5		6.0	Medium stiff gray & tan clay w/organic	L] -
	ļ	ļ	<u> </u>		matter, roots & sand pockets			J
3	7.5	8.5	6.0	9.0	Medium stiff dark gray clay w/organic			_
	<u> </u>	<u> </u>			matter & clayey silt layers] .
4	11.5	12.0	9.0	12.0	Medium stiff gray clay w/large roots] "
5	13.5	14.5	12.0		Soft gray & tan clay w/roots			7
6	17.0	18.0		18.0	Soft gray clay w/roots & organic matter	П		7
7	18.0	19.5	18.0		Medium dense gray fine sand	3	11	٦
8	21.0	22.5			Ditto	3	18	ጎ . ጟ
9	23.5	25.0			Ditto	4	19	1:
10	28,5	30.0		33.0	Ditto	6	24	16
11	33.5	35.0	33.0	38.0	Dense gray fine sand	14	36	1
12					Very dense gray fine sand	15	50=9*	1 -
13					Medium dense gray fine sand w/clay	12	24	1
					layers			1 .
14	48.5	50.0	48.0	50.0	Very loose gray clayey sand	1.1	3	1
•	40.2	20.0	40.0	-27,7	very rouse gray crayer sand	+ 4		-

LOG OF BORING

FOR GENERAL NOTES ON EUSTIS' SOIL BORINGS, SEE DWG. 54

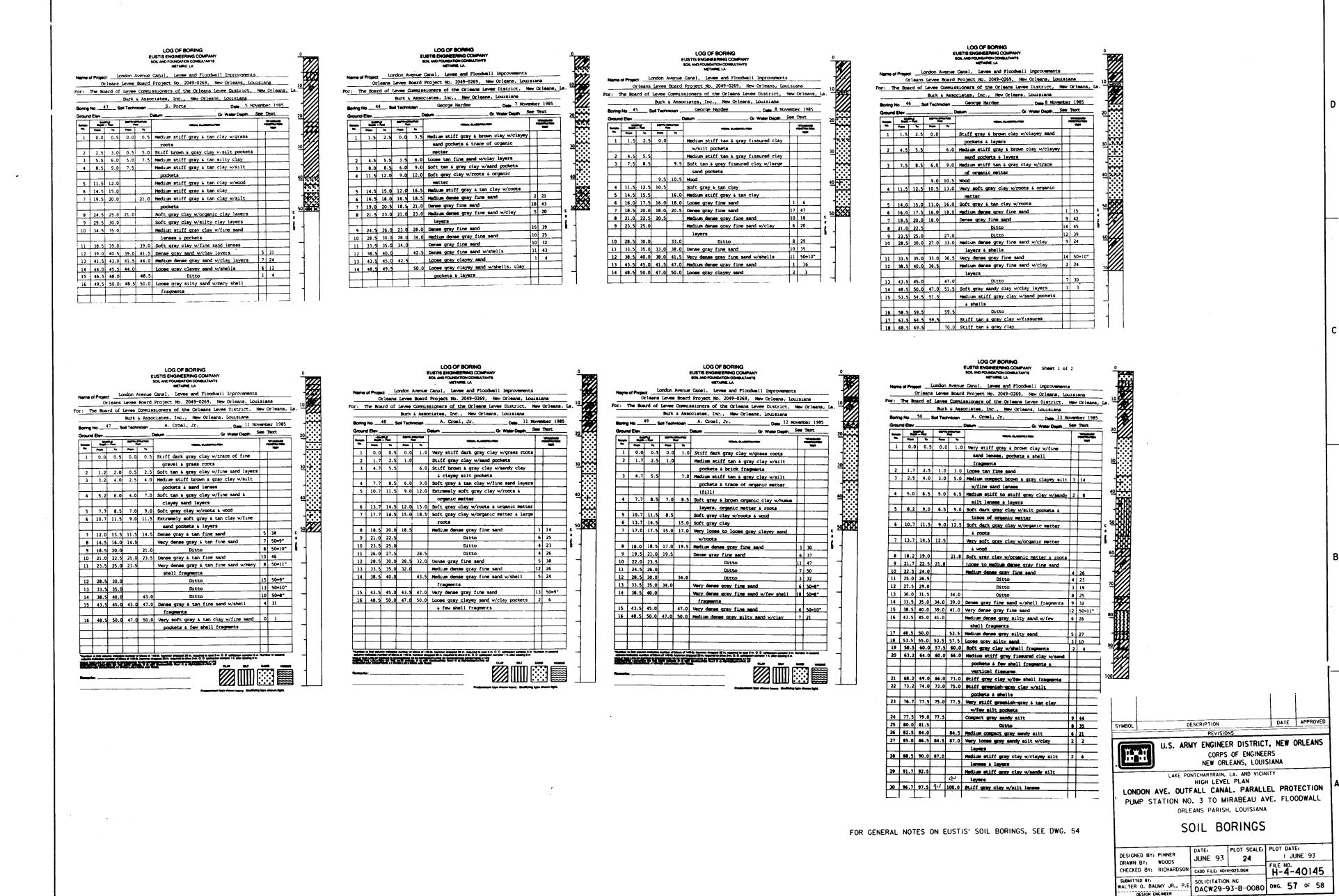
(COME	U.S. ARM	REVISIONS AY ENGINEER DIS	TRICT, NEW	ORLEANS
SYMBOL	Di	ESCRIPTION	DATE	APPROVE

LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN LONDON AVE. OUTFALL CANAL. PARALLEL PROTECTION

PUMP STATION NO. 3 TO MIRABEAU AVE. FLOODWALL ORLEANS PARISH, LOUISIANA

SOIL	BORINGS
90.5	

DESIGNED BY: PINNER DRAWN BY: WOODS	DATE: JUNE 93	PLOT SCALE:		JUN		3
CHECKED BY: RICHARDSON	CADO FILE: 4014	5023.DGN	H-	~- 4	01	45
SUBMITTED BY: WALTER O. BAUMY JR., P.E. DESIGN ENGINEER	SOLICITATION DACW29-9	NO. 93-B-0080	DWG.	55	OF	58



LOG OF BORING 10 LOG OF BORIE Name of Project: London Avenue Canal, Levee and Floodwall Im London Avenue Canal, Levee and Floodwell Orleans Levee Board Project No. 2049-0269, New Orleans, Louisiana Orleans Levee Board Project No. 2049-0269, New Orleans, Louisian London Avenue Canal, Levee and Floodwall Impro Name of Project London Avenue Canal, Levee and Floodwell Improvements
Orleans Levee Board Project No. 2049-0269, New Orleans, Louisiana For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, Le For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, Orleans Levee Board Project No. 2049-0269, New Orleans, Louisiana Burk & Associates, Inc., New Orleans, Louisiana Burk & Associates, Inc., New Orleans, Louisiana For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, La Boring No. 15 Soil Technician A. Crost. Jr. Date 17 October 1985 Soil Technicien A. J. Mayeux Det 26 October 1985 Burk & Associates, Inc., New Orleans, Louisiana Burk & Associates, Inc., New Orleans, Louisians Date 25 October 1985 loring No. 37 Soil Technicien S. Porta **Ground Elev** Boring No. 38 Soll Technician R. Elkins | Commonweal Commonwea 1 1.7 2.5 0.0 2.5 Medium stiff tan & gray clay w/silt 1 2.0 2.5 0.0 4.0 Loose gray & tan clayey silt pockets & grass roots w/miscellansous fill w/miscellaneous fill
2 5.0 5.5 4.0 7.0 Medium stiff tan 6 gray silty clay 1 1.5 2.5 0.0 3.5 Stiff gray & tan clay w/sand, grass 2 4.7 5.5 2.5 5.5 Medium stiff tan & gray clay w/sand 2 2.5 3.0 1.0 4.0 Stiff gray 6 ban clay w/silt pockets
6 shells 2 5.0 5.5 4.0 7.0 Medium stiff tan a gray sitty clay

3 8.0 8.5 7.0 Medium stiff gray sitty clay w/organic
matter

4 11.0 11.5 12.5 Ditto

5 14.0 14.5 12.5 16.0 Stiff tan a gray clay w/silt pockets roots & few brick fragments pockets & roots 2 4.5 5.5 3.5 Soft gray 6 tan silty clay 3 7.7 8.5 5.5 Soft dark gray clay w/silty sand layers, organic matter & roots 3 5.5 6.0 4.0 Medium stiff gray & tan clay w/silt 3 7.5 8.5 9.5 w/concretions 4 10.7 11.5 12.0 Soft dark gray clay w/organic matter Ditto Ditto 4 10.5 11.5 9.5 12.0 Medium stiff gray 6 tan silty clay

w/roots
5 13.5 14.5 12.0 15.0 Loose gray sandy silt w/trace of clay
6 18.0 19.0 15.0 20.0 Medium compact gray sandy silt w/clay & roots 6 19.0 19.5 16.0 21.0 Medium stiff gray 4 tan silty clay
7 24.0 24.5 21.0 26.5 Soft gray clay w/silt pockets 5 11.5 12.0 9.5 14.0 Soft tan 6 gray clay w/vertical sand 5 13.7 14.5 12.0 15.0 Soft gray clay w/roots 6 15.5 17.0 15.0 Medium dense gray fine sand w/clay 3 20 pockets a roots 9 29.0 30.5 29.0 31.0 Medium dense gray silty sand w/clay 7 18.0 19.5 20.5 Medium dense gray fine sand w/clay layers
8 20.5 22.0 20.5 Very dense gray fine sand layers & decayed wood 10 31.5 33.0 31.0 Loose gray silty sand w/clay layers 3 8 7 19.5 20.0 16.0 24.0 Soft gray clay w/clayey silt layers 7 23.0 24.0 20.0 Loose gray clayey silt w/silty clay 8 24.5 25.0 24.0 Loose gray silty fine sand w/clay
1 layers
9 25.0 26.5 28.0 Ditto 8 28.0 29.0 30.0 Loose gray clayey silt w/sand layers 35.5 11 34.0 35.5 9 23.5 25.0 25 50=8" 12 39.0 39.5 35.5 Medium stiff gray clay w/silt lenses Ditto 9 33.0 34.0 30.0 Soft gray 6 tan clay w/sandy silt layers 13 44.0 44.5 Ditto
14 49.0 49.5 50.0 Ditto
15 54.0 54.5 50.0 Soft gray sandy clay w/shell fragments | 10 28.0 29.5 28.0 | Soft gray clay w/silt lenses | 11 34.5 35.0 | Ditto | 12 39.5 40.0 | 41.0 Soft gray clay w/sand lenses a layers Ditto 70 11 33.5 35.0 Very dense gray fine sand w/few shell 18 50m6"

fragments & trace of silt

12 38.5 40.0 Very dense gray fine sand 23 50m6"

13 43.5 45.0 46.0 Very dense gray fine sand w/few shell 2 50m10"

fragments 11 43.0 44.0 41.0 46.0 Medium compact gray clayey silt w/sand lenses 15 54.0 54.5 50.0 56.0 58.5 Very loose gray silty sand w/clay

16 51.5 58.0 56.0 58.5 Very loose gray silty sand w/clay

17 58.5 60.0 58.5 Medium dense gray sand w/shell 3

fragments 13 44.5 45.0 41.0 47.0 Loose gray clayey sand w/clay layers 6 shells 12 48.0 49.0 46.0 50.0 Loose gray clayer silt w/clay layers 14 49.5 50.0 47.0 51.0 Soft gray sandy clay w/large silty sand 6 shell fragments 16 53.0 54.5 56.0 Ditto 4 8 17 56.0 57.5 Loose gray sand 6 shells w/clay layers 3 3 | 18 | 59.5 | 60.0 | 57.5 | 62.0 | Very | loose gray clavey sand w/shells | 19 | 64.5 | 65.0 | 62.0 | 67.0 | Medium stiff gray 5 tan clay w/silt | lenses 6 pockets fragments 22 79.0 79.5 23 84.0 84.5 86.0 20 69.5 70.0 67.0 72.0 Very stiff greenish-gray & tan clay 18 68.2 69.0 66.0 69.0 Stiff gray clay w/shell fragments 4 trace of sand
 24
 89.0
 89.5
 86.0
 Stiff greenish-gray 6 tan clay

 25
 94.0
 94.5
 Very stiff greenish-gray 6 tan clay

 26
 99.0
 99.5
 100.0
 Ditto
 19 69.5 70.0 69.0 70.0 Stiff green clay LOG OF BORING LOG OF BORIN LOG OF BORIN London Avenue Canal, Levee and Floodwall Improvements Name of Project: London Avenue Canal, Levee and Floodwall Improvements London Avenue Canal, Levee and Floodwall Improvements orleans Levee Board Project No. 2049-0269, New Orleans, Louisiana London Avenue Canal, Levee and Floodwall Improvements Orleans Levee Board Project No. 2049-0269, New Orleans, Louisiana Orleans Levee Board Project No. 2049-0269, New Orleans, Louisiana Orleans Levee Board Project No. 2049-0269, New Orleans, Louisian for: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, L Por: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, Le For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, I For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, Burk & Associates, Inc., New Orleans, Louisiana Date 5 November 1985 oring No. 39 Soil Technician S. Porta Boring No. 42 Soll Technician S. Porta Boring No. 41 Soil Technician R. Elkins Water Depth See Text _ Datum _ Gr. Water Depth See Text Ground Elev Der State St Ground Elev. Ground Elev ... De tourn to to face to to face Gr. Water Depth See Text ___ Datum ____ __ Gr. Water Depth See Text Transpersor Particulation Test Transacto Contraction Total 1000 D. AND DA. CO. 1 1.5 2.5 0.0 3.0 Medium stiff ten silty clay w/clay 1 0.0 0.5 0.0 1.0 Medium stiff brown & gray clay w/silt 1 0.0 0.5 0.0 1.0 Medium stiff gray silty clay w/grass Stiff gray & tan clay w/concretions, 2 2.5 3.0 1.0 4.5 Medium stiff gray 5 tan clay w/clayey 2 2.5 3.0 1.0 Stiff gray & tan clay w/roots & trace pockets, organic matter, brick & 2 4.5 5.5 6.0 Medium stiff gray 4 tan clay w/silty
clay layers 5 roots fragments (fill) 2 4.0 5.0 3.0 Stiff tan & gray clay w/cinders, claye 3 5.5 6.0 4.5 7.5 Medium stiff brown & gray clay w/clayey silt & clay pockets (fill)

3 6.5 7.5 8.5 Medium stiff tan & gray clay w/clayey silt & clay pockets (fill) 3 7.5 8.5 6.0 9.0 Very stiff gray 6 tan clay w/roots silt pockets & fill 4 10.5 11.5 9.0 Medium stiff gra 4 8.5 9.0 7.5 11.0 Medium stiff gray 6 tan silty clay 5 11.5 12.0 Medium str silt pockets & roots (fill) Medium stiff gray & tan clay w/silt Medium stiff gray & tan clay 4 9.0 10.0 8.5 11.0 Stiff tan & gray clay w/clayey silt w/roots 6 14.5 15.0 Stiff gray 6 tan clay
7 19.5 20.0 21.0 Stiff tan 6 gray clay w/silt pockets 5 11.5 12.0 11.0 13.5 Wery soft gray silty clay w/clay layers pockets 5 11.5 12.5 11.0 Medium stiff tan 6 gray clay w/clayey 6 14.5 15.0 13.5 17.0 Medium compact gray clayey silt 8 24.5 25.0 21.0 26.0 Medium stiff gray clay w/trace of organic matter
9 29.5 30.0 26.0 Soft gray clay 7 19.5 20.0 17.0 21.0 Loose gray sandy silt w/clay layers silt pockets 7 23.0 24.0 20.0 25.0 Medium stiff gray clay w/organic matter 6 14.0 15.0 8 24.5 25.0 21.0 27.0 Soft gray silty clay w/clay layers 8 28.0 29.0 25.0 30.0 Loose gray sandy silt w/trace of clay 9 29.5 30.0 27.0 Loose gray sandy silt w/silty clay layers 7 16.5 17.5 Ditto Very soft gray clay w/large sandy silt layers & lenses 9 33.0 34.0 30.0 10 34.5 35.0 9 21.5 22.5 20.0 23.0 Medium stiff gray silty clay w/clayey 10 34.5 35.0 11 39.5 40.0 40.0 Soft gray clay w/silty sand lenses 10 38.0 39.0 Ditto silt layers & lense 11 43.0 44.0 45.0 Soft gray clay w/sand lenses, pockets
6 shell fragments 6 layers 11 39.5 40.0 Ditto 10 24.0 25.0 23.0 25.0 Soft gray silty clay w/clayey silt & 12 43.5 45.0 13 45.0 46.5 12 40.0 41.5 40.0 43.0 Soft gray clay clay pockets
11 28.5 29.5 25.0 Loose gray sandy silt w/clay lenses 13 42.5 44.0 43.0 46.0 Soft gray clay w/sand layers Ditto 12 48.0 49.0 45.0 50.0 Loose gray clayey sand w/shell 14 49.5 50.0 46.0 50.0 Loose gray clayey sand w/shells fragments 12 31.5 32.5 33.5 Loose gray sandy silt w/silty clay 6 clay layers Loose gray sandy silt w/clay layers 16 53.5 55.0 13 34.0 35.0 33.5 36.0 Medium stiff gray clay w/clayey silt 6 sandy silt lenses
14 36.9 37.5 36.0 Medium stift gray clay w/sandy silt 18 63.5 65.0 Ditto 20 73.5 75.0 73.5 Medium stiff greenish-gray fissured clay w/clayer silt pockets.

21 79.5 80.0 Medium stiff greenish-gray fissured clay lenses DATE APPROVED SYMBOL 16 41.5 42.5 Soft gray clay w/silty sand lenses 6 layers
17 44.0 45.0 45.0 Soft gray clay w/sand lenses 6 pockets REVISION as.0 Medium stiff greenish-gray fissured

clay w/clayey silt layers

23 89.5 90.0 86.0 Hedium stiff tan & gray fissured clay

24 94.5 95.0 95.0 95.0 Stiff tan & gray fissured clay

25 95.0 96.5 95.0 97.0 Medium dense gray silty sand

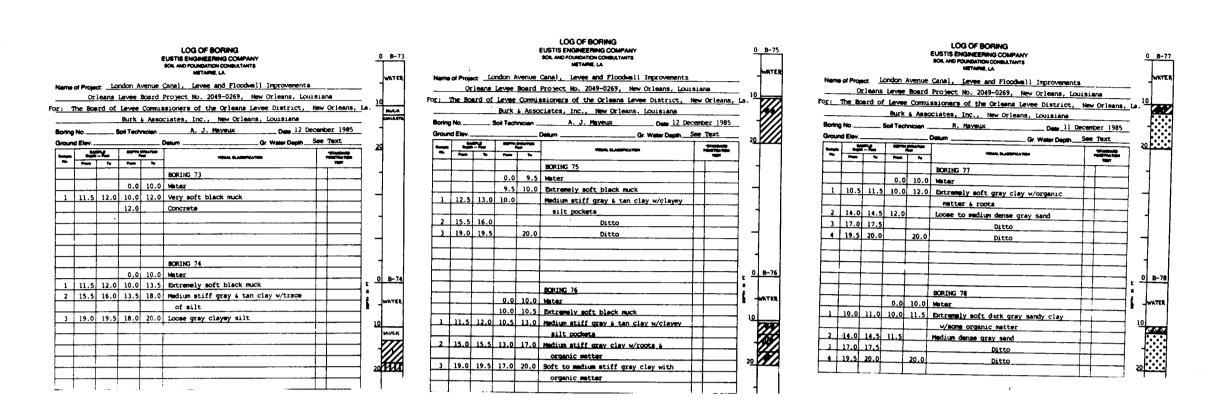
4 29

26 98.5 100.0 97.0 100.0 Hedium dense gray clayey sand w/silty 5 16

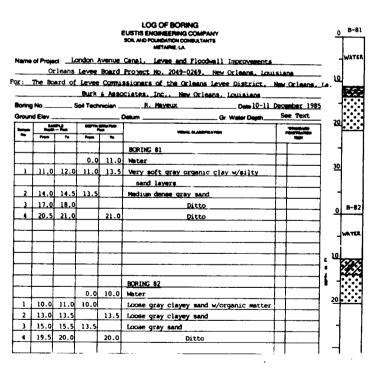
sand layers U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS HAH. 18 45.0 46.5 45.0 47.0 Medium dense gray fine sand CORPS OF ENGINEERS 19 48.5 50.0 47.0 50.0 Loose gray clayey sand w/sandy clay 2 5 NEW ORLEANS, LOUISIANA layers LAKE PONTCHARTRAIN, LA. AND VICINITY HIGH LEVEL PLAN LONDON AVE. OUTFALL CANAL. PARALLEL PROTECTION PUMP STATION NO. 3 TO MIRABEAU AVE. FLOODWALL ORLEANS PARISH, LOUISIANA FOR GENERAL NOTES ON EUSTIS' SOIL BORINGS, SEE DWG. 54 SOIL BORINGS PLOT SCALE: PLOT DATE: DESIGNED BY: PINNER JUNE 93 24 I JUNE 93 WOODS CHECKED BY: RICHARDSON CADD FILE: 40145C23.DCN

H-4-40145

SUBMITTED BY:
WALTER O. BAUMY JR., P.E
DACW29-93-B-0080 DWG. 56 OF 58



Name of Project: London Avenue Canal, Levee and Floodwall Improvements Orleans Levee Board Project No. 2049-0269, New Orleans, Louisiana 20 For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, La Burk & Associates, Inc., New Orleans, Louisiana Soil Technician R. Mayeaux Date 11 December 1985 Gr. Weter Depth See Text 1 10.5 11.0 10.0 11.5 Loose dark gray sand w/organic matter
2 14.5 15.0 11.5 Medium dense gray sand
3 17.0 17.5 Ditto 3 17.0 17.5 4 19.5 20.0 20.0 Ditto BORING 80 0.0 11.0 Water 20 1 11.5 12.0 11.0 12.5 Loose gray sand w/trace of organic matter
2 14.5 15.0 12.5 Loose to medium dense dark gray sand 3 17.5 18.0 Ditto 3 17.5 18.0 4 20.5 21.0 21.0 Ditto



					LOG OF BORING EUSTIS ENGINEERING COMPANY SOIL AND POUNDATION COMBUTANTS METAINE, LA		Ť
Name					Canal, Leves and Flood-mil Improvements		
					Protect No. 2049-0269, New Orleans, Loui		10
: 1	The Box	rd of			ssioners of the Orleans Levee District,	New Orleans, La.	
_					ciates, Inc., New Orleans, Louisiana		K
			Soil Tech	vnicien _	R. Mayesux Dete 10	December 1985	1
							- 1 4
3roun	d Elev.				Deturn Gr. Water Depth	See Text	20
3roun		- L	-	<u></u>	Gr. Water Depth	See Text	20
3rour	d Elev	- X-	Prince	_ ·	VISIAL GLASSIFICA RISH	See Text	20
aroun		- L	~	<u></u>	Deturn Gr. Weter Depth WENNA GLASSIFICATION BORING 83	Text	20
aroun Marie		- L		-	VISIAL GLASSIFICA RISH	- Text	20
3roun			0.0	10.0	WOMA BAGGING 83 Heter	Townson Parketters 1997	20
3roun			0.0	10.0	BORING 83	*** Text	20
1	11.0	12.0	0.0	10.0	BORING 83 Water Very soft gray clay w/sand pockets & roots	** Text	20
1	11.0	12.0	0.0 10.0	10.0	BORING 83 Heter Very soft gray clay w/sand pockets 6	FOR AND	20

FOR GENERAL NOTES ON EUSTIS' SOIL BORINGS, SEE DWG. 54



HXH

U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS
NEW ORLEANS, LOUISIANA

LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN

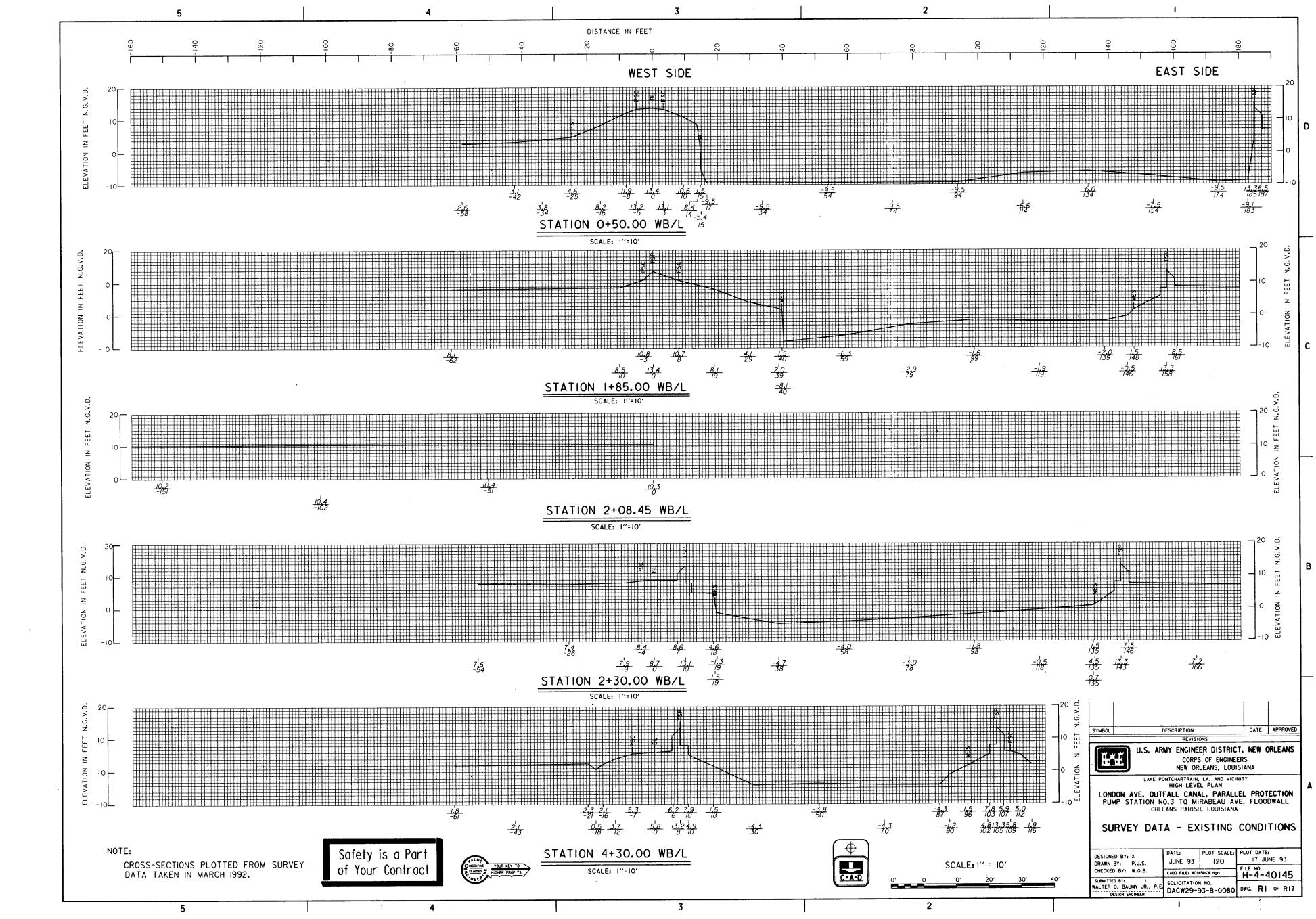
LONDON AVE. OUTFALL CANAL. PARALLEL PROTECTION
PUMP STATION NO. 3 TO MIRABEAU AVE. FLOODWALL
ORLEANS PARISH, LOUISIANA

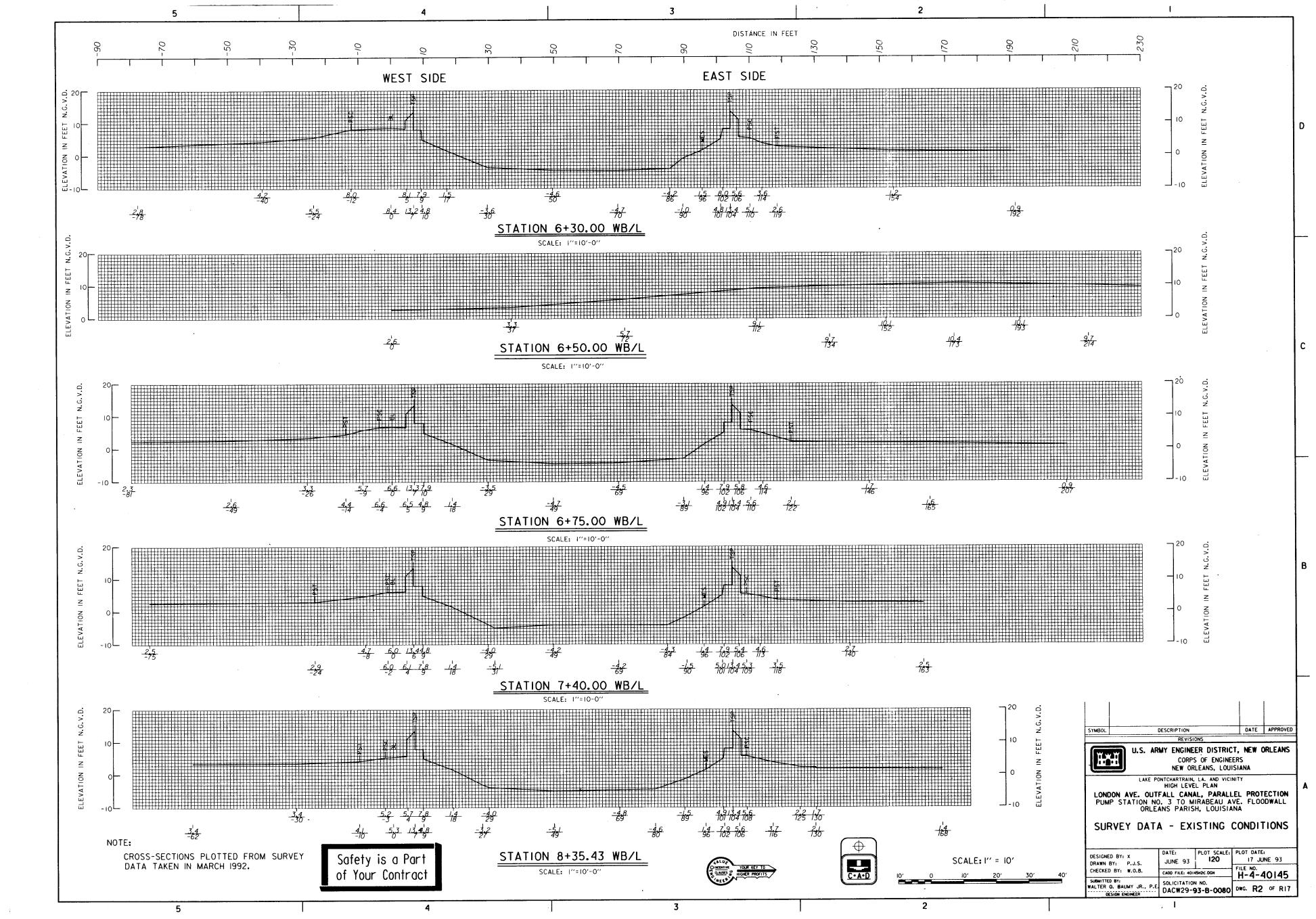
SOIL BORINGS

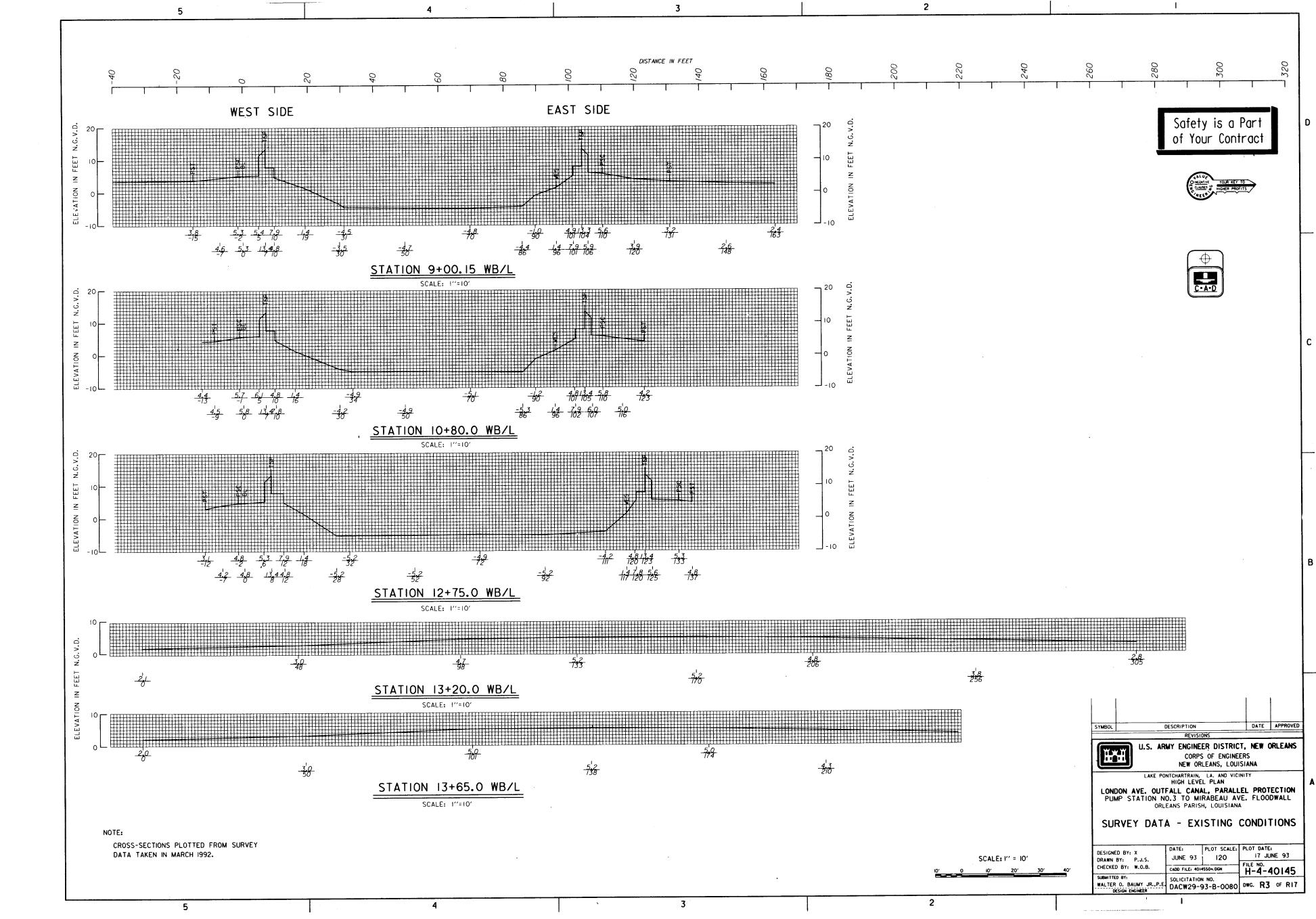
DESIGNED BY: PINNER DRAWN BY: WOODS	DATE: JUNE 93	PLOT SCALE:		I JUNE 9	3
CHECKED BY: RICHARDSON	CADO FILE: 4014	15023.DGN	H-4-4014		
SUBMITTED BY: WALTER O. BAUMY JR., P.E. DESIGN ENGINEER	SOLICITATION DACW29-9	i no. 93-B-0980		58 of	

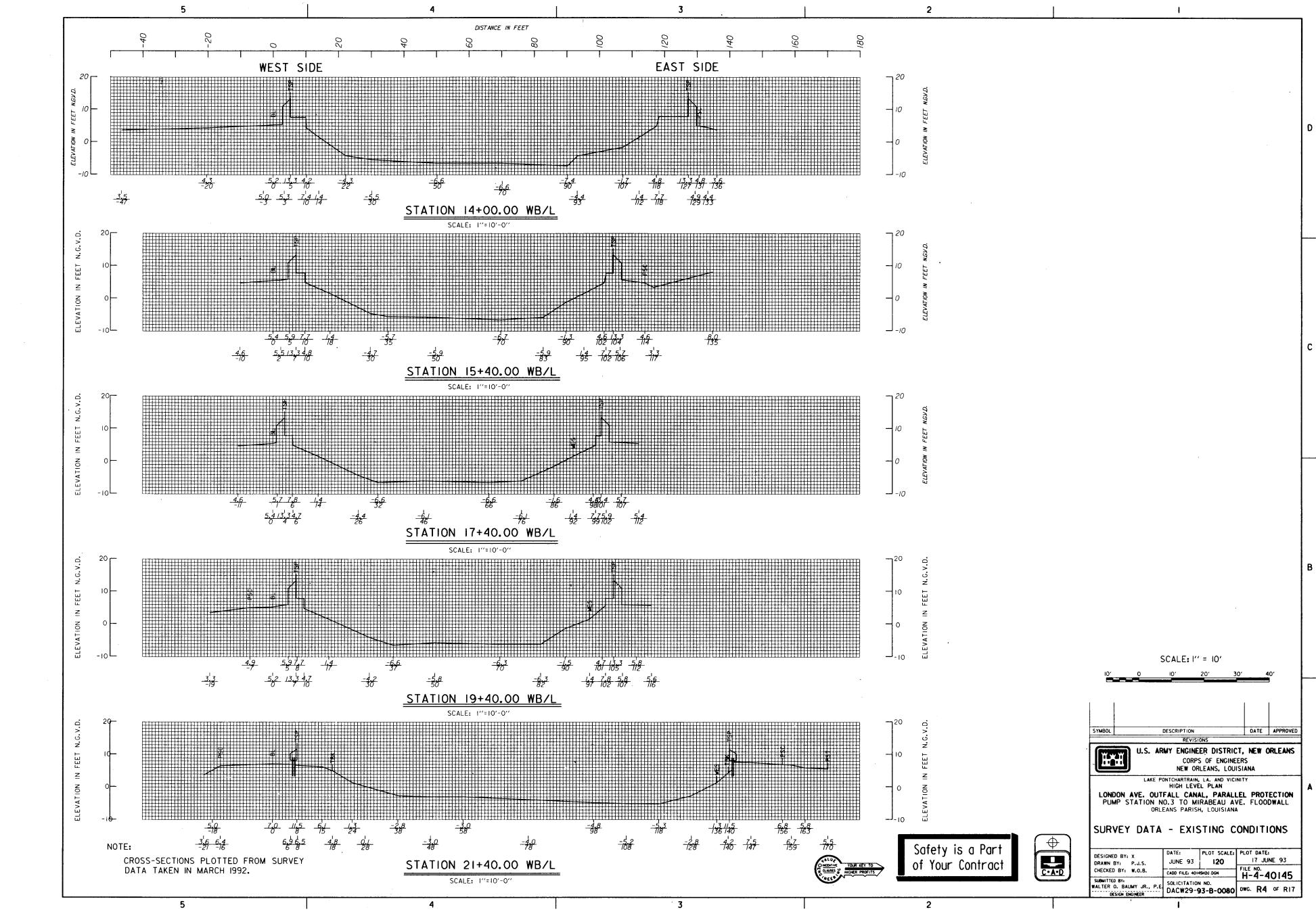
4

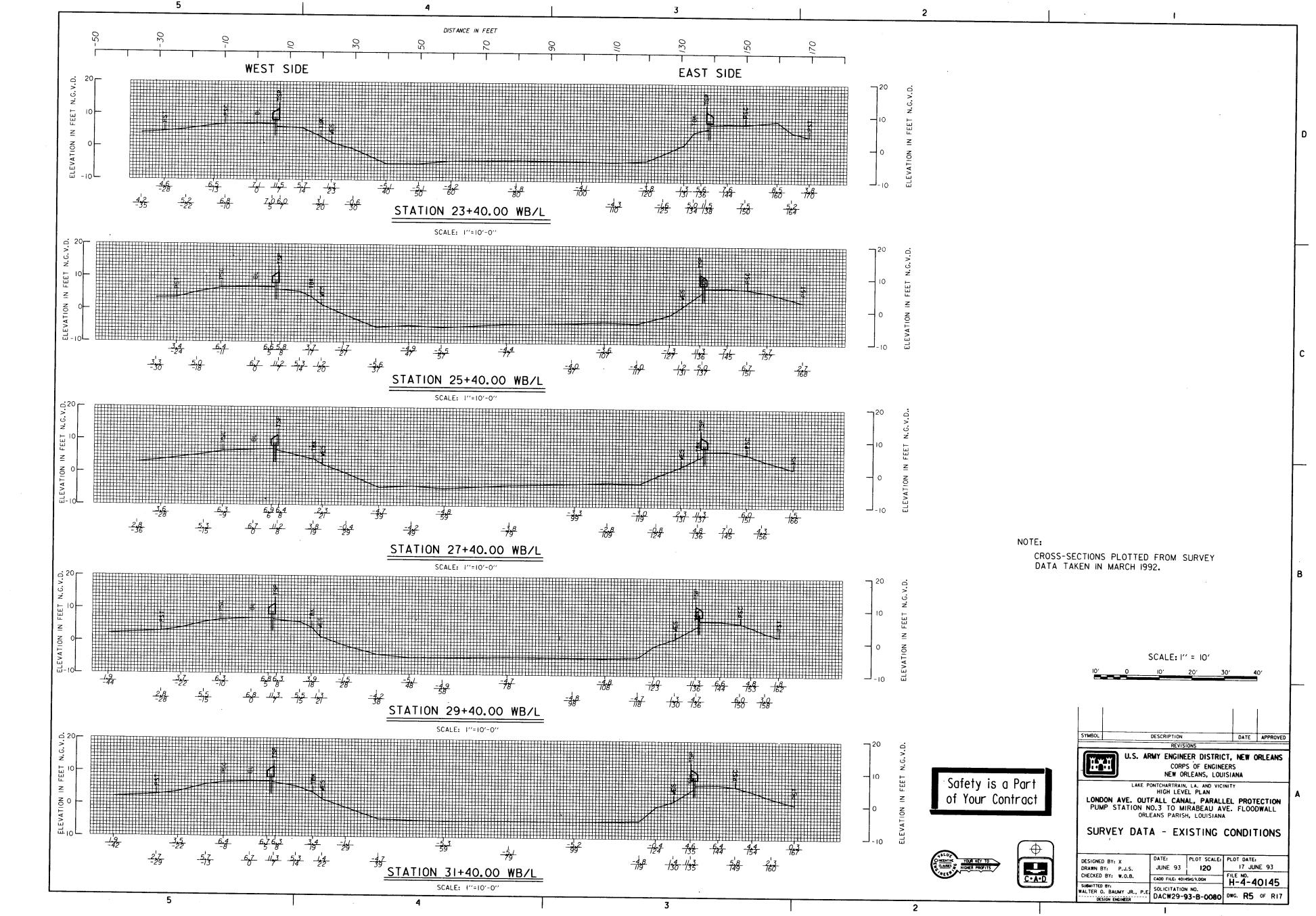
.

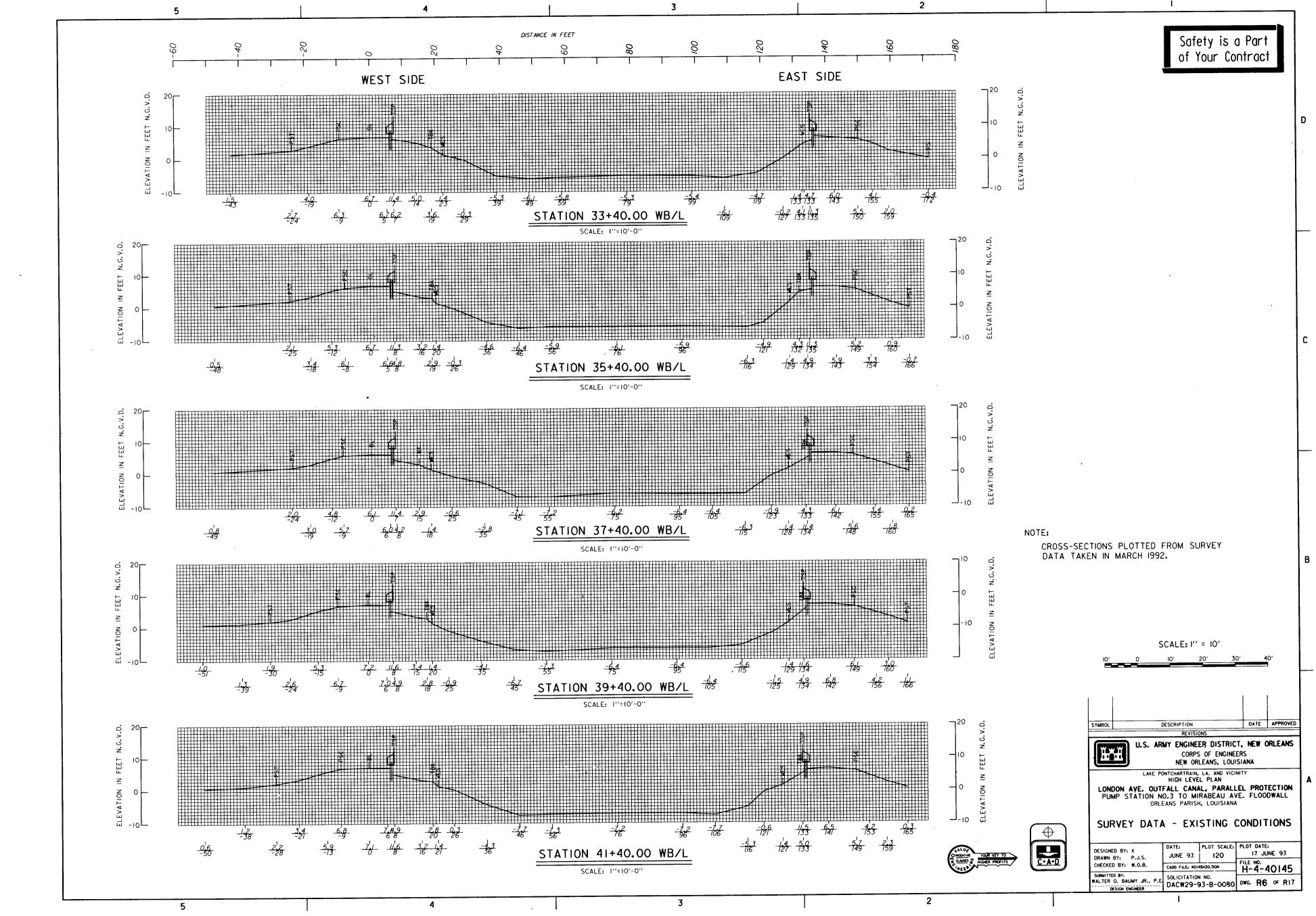


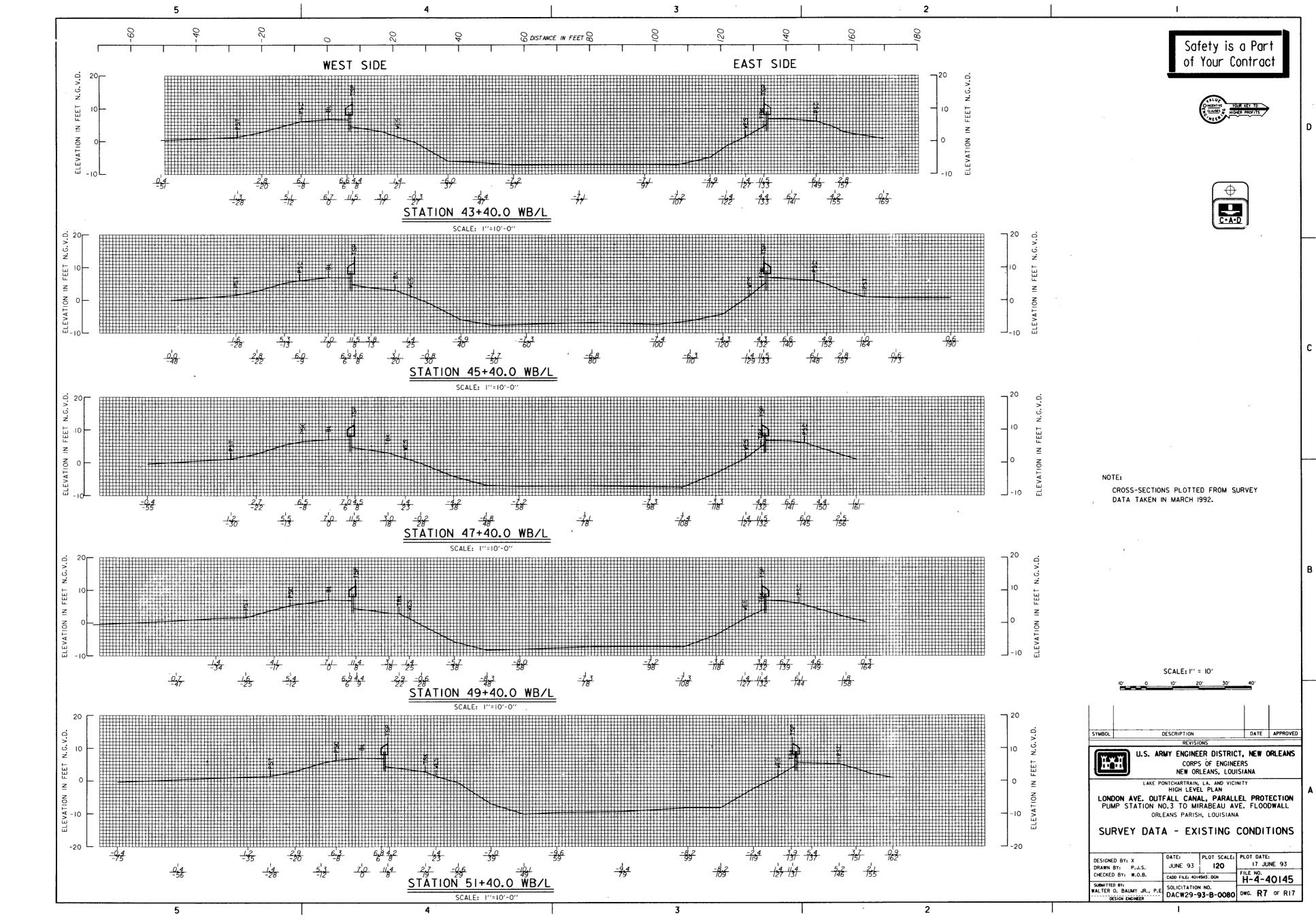


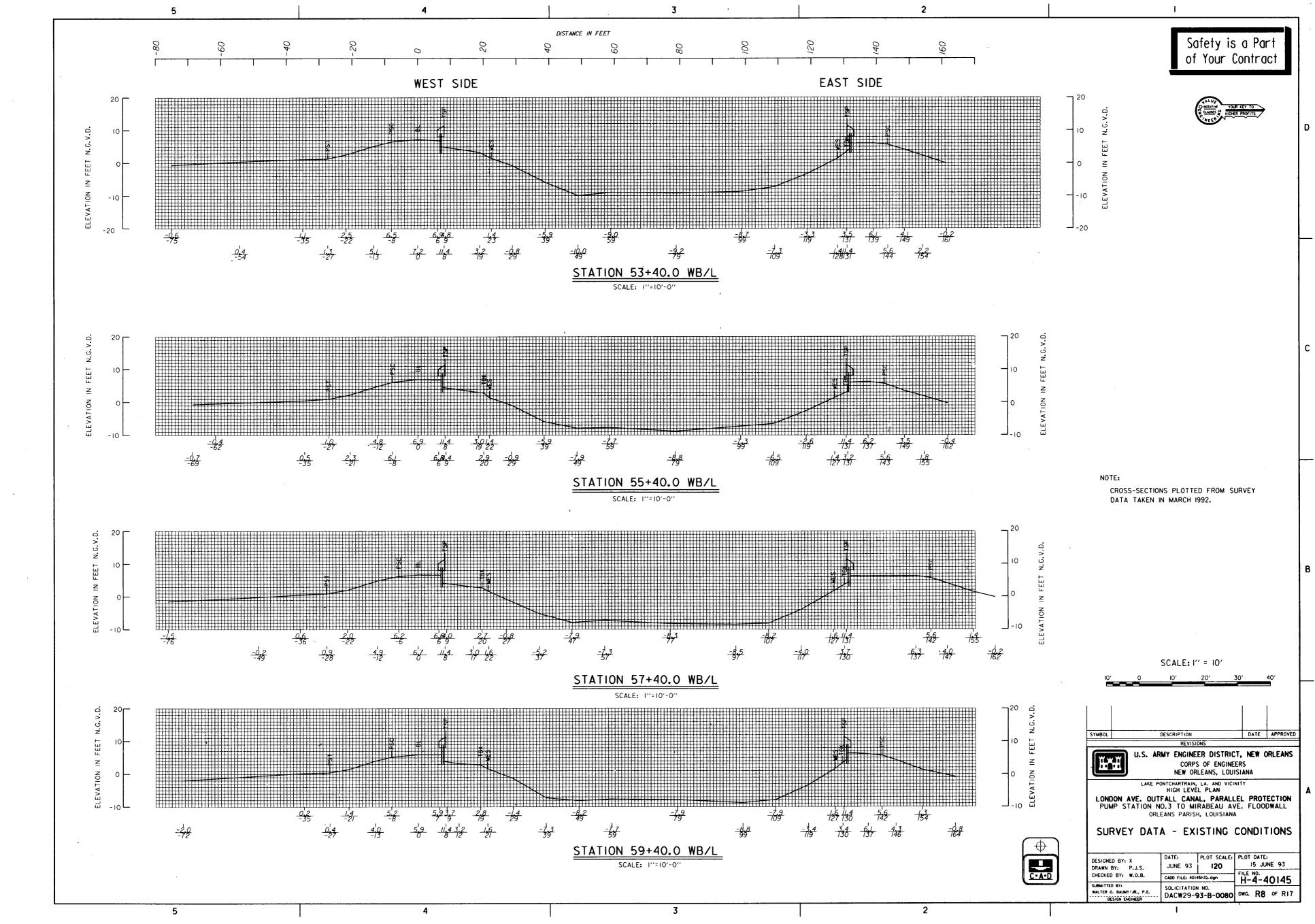


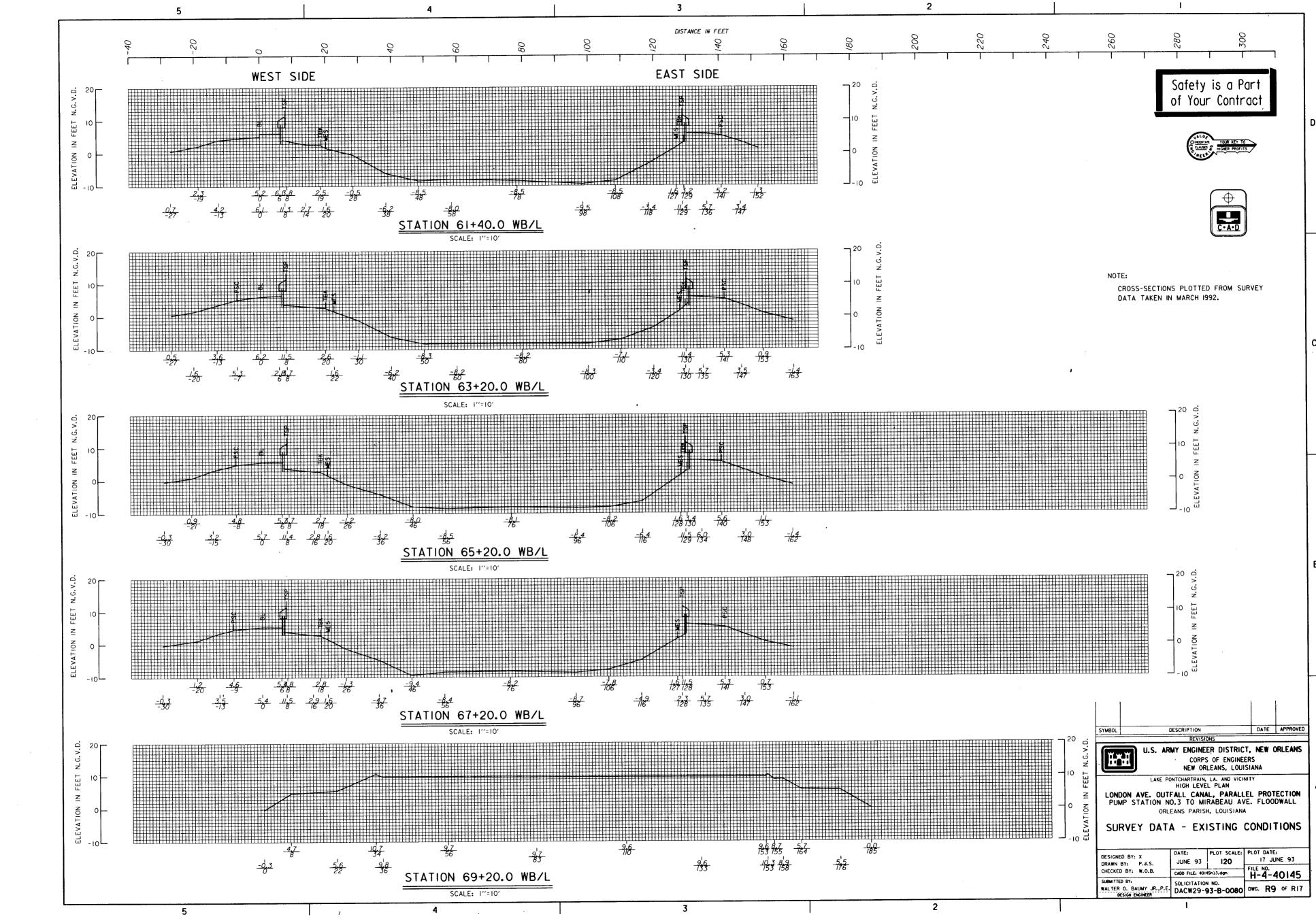


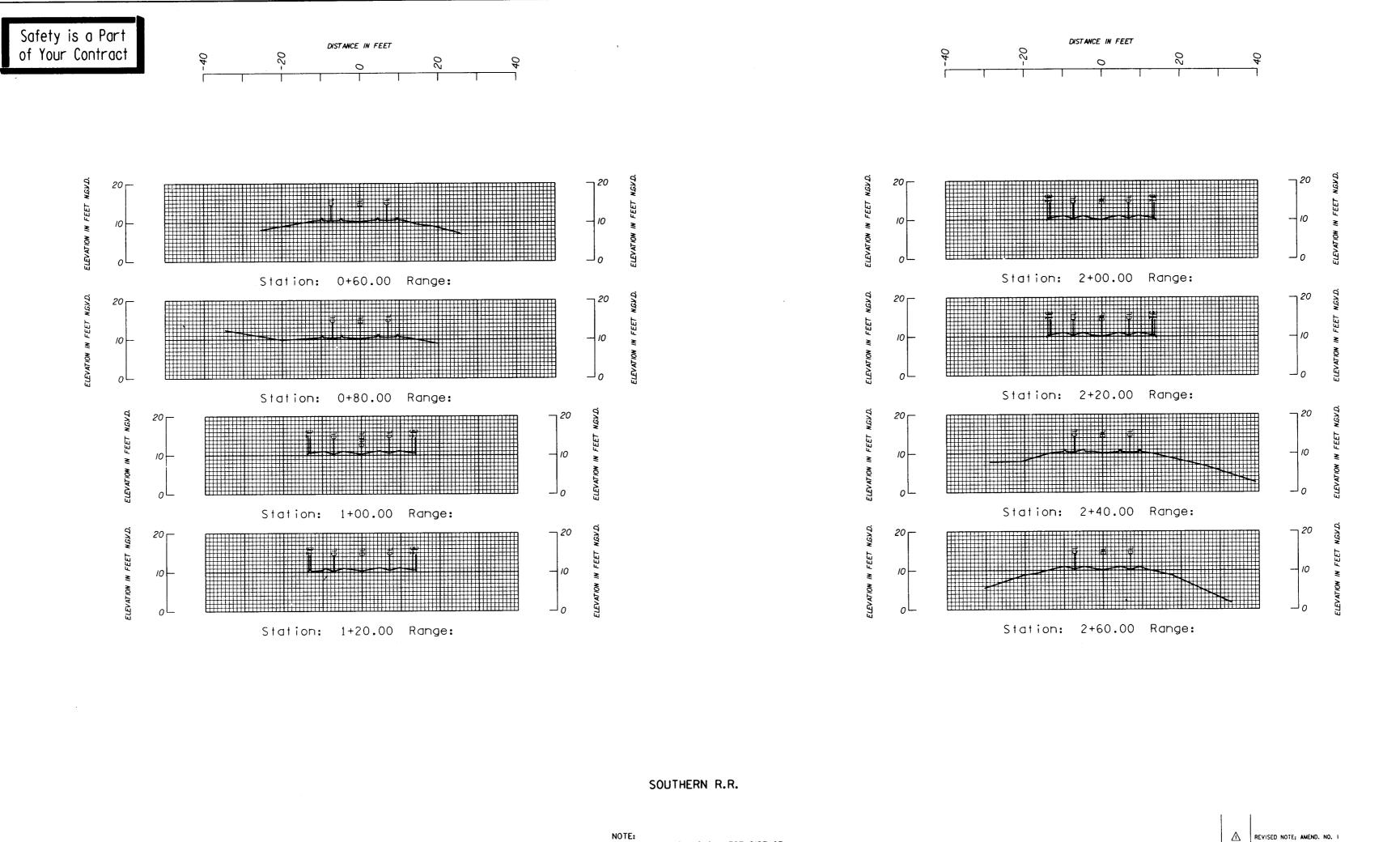












3

STATIONING BEGINS ON WEST SIDE OF CANAL AND CONTINUES ALONG THE CENTERLINE OF THE TWO SETS OF RAILROAD TRACKS FOR THIS DWG. ONLY.

STATION 0+00 IS 80 FEET WEST OF THE BRIDGE ABUTMENT.

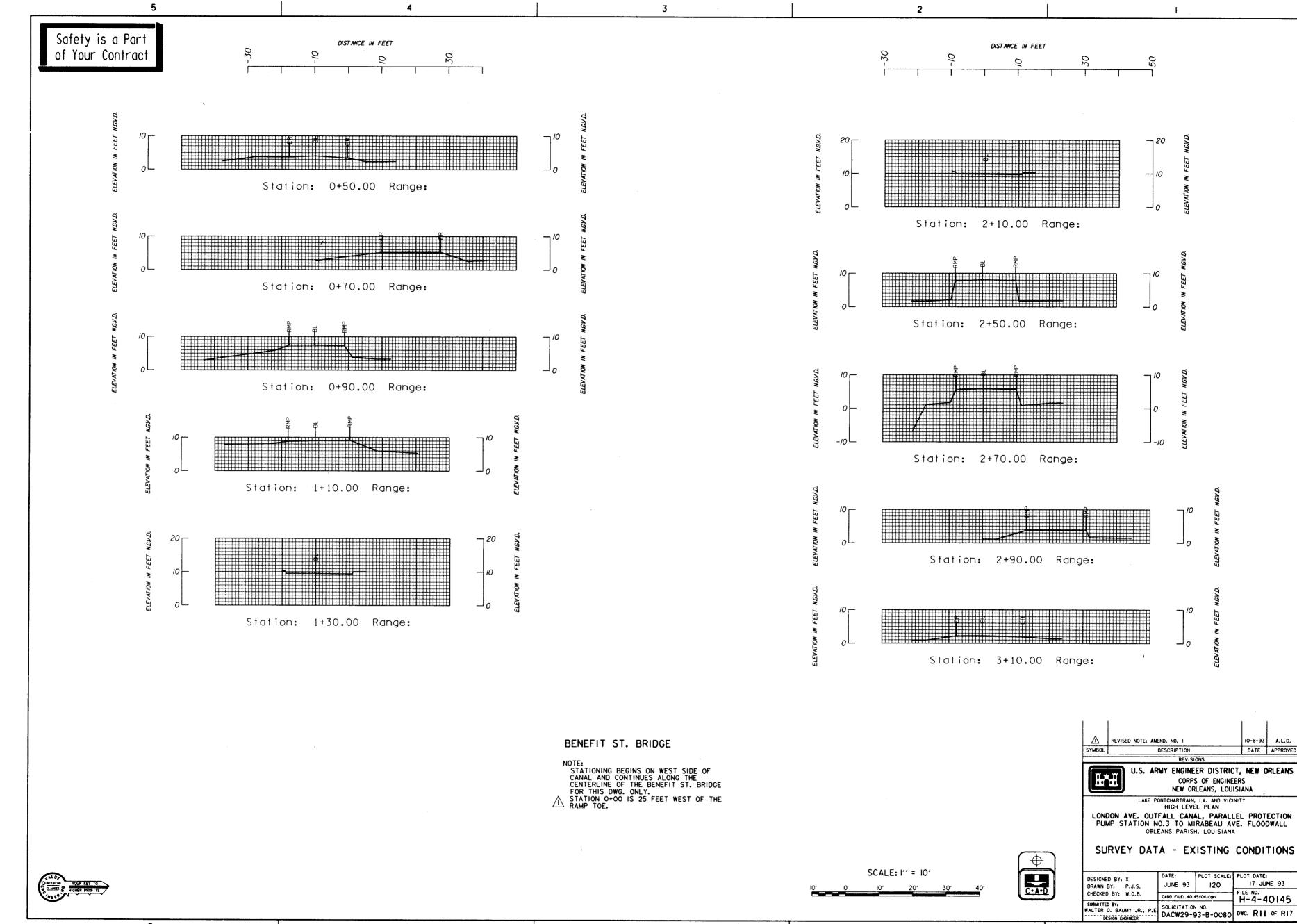
SCALE: 1" = 10"



10-6-93 A.L.D. DESCRIPTION REVISIONS U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN LONDON AVE. OUTFALL CANAL, PARALLEL PROTECTION PUMP STATION NO.3 TO MIRABEAU AVE. FLOODWALL ORLEANS PARISH, LOUISIANA

SURVEY DATA - EXISTING CONDITIONS

DESIGNED BY: X JUNE 93 120 17 JUNE 93 DRAWN BY: P.J.S. CHECKED BY: W.O.B. FILE NO. H-4-40145 CADD FILE: 40145103.cgm SUBMITTED BY:
WALTER O. BAUMY JR., P.E. DACW29-93-B-0080 DWG. RIO OF RI7



Safety is a Part of Your Contract DISTANCE IN FEET DISTANCE IN FEET Station: 1+65.00 EB/L Station: 4+15.00 EB/L Station: 2+15.00 EB/L Station: 4+65.00 EB/L Station: 2+65.00 EB/L Station: 5+15.00 EB/L Station: 3+15.00 EB/L Station: 5+65.00 EB/L Station: 3+65.00 EB/L U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS

CORPS OF ENGINEERS

NEW ORLEANS, LOUISIANA LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN I+65 TO 5+65 EB/L LONDON AVE. OUTFALL CANAL, PARALLEL PROTECTION PUMP STATION NO.3 TO MIRABEAU AVE. FLOODWALL ORLEANS PARISH, LOUISIANA EB/L = EAST BASELINE SURVEY DATA - EXISTING CONDITIONS SCALE: 1" = 10" DESIGNED BY: X
DRAWN BY: P.J.S.
CHECKED BY: W.O.B. JUNE 93 CADD FILE: 40145f05.dgn

H-4-40145 SUBMITTED BY:
VALTER O. BAUMY JR., P.E.
DACW29-93-B-0080
DWG. R12 OF R17

