


**RECORDS TRANSMITTAL AND RECEIPT**

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As shown in FPMR 101-11.410-1  RHA

USAED, New Orleans  
 Structural Design Section  
 Design Branch  
 Engineering Division

**2. AGENCY TRANSFER AUTHORIZATION**  
 TRANSFERRING AGENCY OFFICIAL (Signature and title)  
 Carl R. Guggenheimer  
 Chief, Structural Design Sec  
 DATE  
 Dec 85

**3. AGENCY CONTACT**  
 TRANSFERRING AGENCY LIAISON OFFICIAL (Name, office and telephone No.)  
 Marilyn Sullen, Records Manager, Ext. 2841 RM 141

**4. RECORDS CENTER RECEIPT**  
 RECORDS RECEIVED BY (Signature and title)  
 DATE

Fold line 

**RECORDS DATA**

ACCESSION NUMBER			VOLUME (cu. ft.)	AGENCY BOX NUMBERS	SERIES DESCRIPTION (With inclusive dates of records)	RESTRICTION	DISPOSAL AUTHORITY (Schedule and item number)	DISPOSAL DATE	COMPLETED BY RECORDS CENTER			
RG	FY	NUMBER							LOCATION	SHELF PLAN	CONT. TYPE	AUTO. DISP.
(a)	(b)	(c)	(d)	(e)	(f)	(g)	(h)	(i)	(j)	(k)	(l)	(m)
a			2	1/2	Civil Works Project Files		AR 340-18 1518-01	Perm				
					(1) Fla. Ave Complex, East-T-Wall Monos, Design Comps		"	"				
					(2) Fla. Ave Complex, West-T-Wall Monos, Design Comps		"	"				
					(3) Fla. Ave Complex, West-OVHD Roller Gate Calcs, Vol I		"	"				
					(4) Fla. Ave Complex, West-OVHD Roller Gate Calcs, Vol II		"	"				
					(5) Fla. Ave Complex, West-Vert Lift Gates Calcs Vol I		"	"				
					(6) Fla. Ave Complex, West-Vert Lift Gates Calcs Vol II		"	"				
					(7) Fla. Ave Complex, West-Swing Gates, Walls & Monos Calcs		"	"				

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<b>RECORDS TRANSMITTAL AND RECEIPT (Continuation)</b>	This form is to continue listing of Records Data when space on SF 135 is not adequate. Instructions for completion of SF 135 apply.	<b>TRANSFERRING AGENCY'S NAME</b>	<b>DATE</b>	<b>PAGE</b>	<b>OF</b>
		Carl R. Guggenheimer, Chief, Strucural Design Section	Dec 85	1	2

PAGES

ACCESSION NUMBER			VOLUME (cu. ft.)	AGENCY BOX NUMBERS	SERIES DESCRIPTION (With inclusive dates of records)	RESTRICTION	DISPOSAL AUTHORITY (Schedule and item number)	DISPOSAL DATE	COMPLETED BY RECORDS CENTER			
RG	FY	NUMBER							LOCATION	SHELF PLAN	CONT. TYPE	AUTO. DISP.
(a)	(b)	(c)	(d)	(e)	(f)	(g)	(h)	(i)	(j)	(k)	(l)	(m)
				2/2	(8) Fla. Ave Complex, East & West -Bypass Channel, Des Analysis		AR 340-18 1518-01	Perm				
					(9) Fla. Ave Complex, East & West GDM Cost Est		"	"				
					(10) Fla. Ave Complex, East- OVHD Roller Gate Design		"	"				
					(1) Detailed Design Memorandum No.8 Rigolets Lock, Volume 2		"	"				
					(2) DDM No. 8, Rigolets Lock Vol 1		"	"				
					(3) Back-Up Computations for Vol 1 (2) DDM No.8, Rigolets Lock		"	"				
					(4) Design Memorandum No.2 -General Design Supplement No. 2, Rigolets Lock Preliminary Structural Investi- gations		"	"				
					(5) Addendum No. 1 to Computation for DDM No.8, Rigolets Lock		"	"				
					(6) Computations for Sector Gate Study, Cantilever (2) vs. Roller Support, Rigolets Lock		"	"				
					(7) Design Study, Sector Gate Comparison, Cantilever vs Roller Support, Rigolets Lock		"	"				

BOX #1

13 AUG 85  
TO BE SENT TO HOLDING  
AREA

DESCRIPTION

FLA. AVE. COMPLEX, EAST - T-WALL MONOS., DESIGN COMPS.  
" " " WEST - " " " "  
" " " " - OVHD. ROLLER GATE CALCS., VOL. I  
" " " " - " " " " , " II  
" " " " - VERT. LIFT GATES CALCS., VOL. I  
" " " " - " " " " , " II  
" " " " - SWING GATES, WALLS & MONOS. CALCS.  
" " " , EAST & WEST - BYPASS CHANNEL, DES. ANALYSIS  
" " " , " " - GDM COST EST.  
" " " , EAST - OVHD. ROLLER GATE DESIGN

ESTIMATE OF COST

69. General. Based on September 1976 price levels, the estimated first cost of the Florida Avenue Complex is \$ 9,237,230. This estimate consists of \$2,400 for lands and damages, \$257,440 for relocations, \$7,349,460 for levees and floodwalls, \$882,430 for engineering and design, and \$ 745,500 for supervision and administration. The detailed estimate of first cost is shown in Table 1.

TABLE 1

LAKE PONTCHARTRAIN BARRIER PLAN AND CHALMETTE  
AREA PLAN

FLORIDA  
FLORIDA AVENUE COMPLEX

ESTIMATE OF FIRST COST  
(September 1976 price levels)

Cost Account No.	Item	Estimated Quantity	Unit	Unit Price	Estimated Amount
				\$	\$
I. Floodwall Reach, West IHNC					
CONSTRUCTION					
11 Levees and Floodwalls					
I-walls and T-walls					
	Steel sheet piling, PZ-27	1530	s.f.	8.00	12,240
	Steel sheet piling, PMA-22	16,610	s.f.	8.00	132,880
	Steel sheet piling, PSA-22	350	s.f.	8.00	2,800
	Prestressed conc. piling 12" X 12"	15,280	l.f.	15.00	229,200
	Concrete in stabilization slab	80	c.y.	80.00	6,400
	Concrete in base slab	610	c.y.	80.00	48,800
	Concrete in walls, columns, and beams	410	c.y.	150.00	61,500
	Portland cement	5,100	c.w.t	2.50	12,750
	Steel Reinforcement	882,600	lbs.	0.35	308,910
	Structural steel	46,000	lbs.	1.75	80,500
	Waterstops, L-type	35	l.f.	15.00	525
	Waterstops, 3-bulb type	340	l.f.	5.00	1,700
	Gate seals	130	l.f.	20.00	2,600
	Sack-rubbed finish	16,600	s.f.	0.40	6,640
	Structure excavation	2,300	c.y.	5.00	11,500
	Structure backfill	2,570	c.y.	6.00	15,420
	Expansion joint filler	600	s.f.	1.50	900
	Concrete removal	42	c.y.	30.00	1,260
	Miscellaneous metal	1,000	lbs.	1.50	1,500
	Trolley for overhead roller gates	1	job	500.00	500
	Fertilizing & seeding	1.34	acre	400.00	540
	Clearing and grubbing	2.24	acre	500.00	1,120
	Subtotal, I-wall and T-wall, West IHNC				955,780
	Vertical Lift Gate Structure				
	Steel sheet piling, PMA-22	2,150	s.f.	8.00	17,200
	Prestressed conc. piling 14" X 14"	6,000	l.f.	16.00	96,000
	Concrete in stabilization slab	25	c.y.	80.00	2,000

TABLE 1 (cont'd)

Cost Account No.	Item	Estimated Quantity	Unit	Unit Price	Estimated Amount
✓	Concrete in base slab	300	c.y.	80.00	24,000
	Concrete in walls (El. -18.5 to +2)	251	c.y.	150.00	37,650
	Concrete in deck slab	91	c.y.	180.00	16,380
	Concrete in walls (El. +2 to +27)	147	c.y.	150.00	22,050
	Concrete in floodwall	23	c.y.	150.00	3,450
	Concrete in stairs				
	Concrete in machine room slab	64	c.y.	80.00	5,120
	Concrete in machine room walls	41	c.y.	150.00	6,150
	Concrete in roof slab	17	c.y.	200.00	3,400
	Portland cement	4,850	sq.ft.	2.50	12,130
	Steel reinforcement	161,700	lbs.	0.25	40,425
	Structural steel	70,800	lbs.	1.65	116,820
	Structure excavation	3,250	sq.	2.00	6,500
	Structure backfill	2,250	sq.	4.00	9,000
	Miscellaneous metal	91,440	lbs.	1.50	137,160
	Trolleys	1	ea.	2,500	2,500
	Mechanical equipment	1	ea.	2,500	2,500
	Electrical equipment and generator	1	job	33,000.00	33,000
Subtotal, Vertical Lift Gate Structure					654,590
✓	Vertical Lift Gate structure				
offset	Structure approaches				
	Prestressed conc. piling, 14" X 14"	3,424	l.f.	16.00	54,784
	Concrete in stabilization slab	57	sq.	70.00	3,990
	Concrete in base slab	57	sq.	70.00	3,990
	Concrete in walls	251	c.y.	150.00	37,650
	Portland cement	4,850	sq.ft.	2.50	12,130
	Steel reinforcement	161,700	lbs.	0.25	40,425
	Waterstops, L-type	150	l.f.	20.00	3,000
	Waterstops, 3-bulb type	150	l.f.	10.00	1,500
	Sack-rubbed finish	3,250	sq.	0.40	1,300
	Expansion joint filler	700	sq.	1.50	1,050
Subtotal, Approaches, West IHNC					377,590
offset	Cofferdam, West IHNC				
	Steel sheet piling, PS-32	40,350	sq.	8.50	342,975
	Structure excavation	2,700	sq.	6.50	17,550
	Structure backfill (shell)	4,550	c.y.	10.00	45,500
	Dewatering system	1	job	50,000.00	50,000

Cost Account No.	Item	Estimated Quantity	Unit	Unit Price \$	Estimated Amount \$
	6" dia. waterline thru T-wall	1	each	1,500	1,500
	5" dia. powerline thru T-wall	2	each	5,600	11,200
	54" dia. sewer force main thru T-wall	1	each	25,000	25,000
	48" dia. waterline thru T-wall	1	each	21,000	21,000
Subtotal, Pipelines and Powerlines					73,700
	Railroad Falsework for Swing Gate	1	job	16,000	16,000
Subtotal, RELOCATIONS					89,700
	Contingencies 20%				17,940
Subtotal					107,640
30	Engineering and design 11.6% † (based on estimate of actual E&D required)				12,490
31	Supervision and administration 98% → 9.8% ± (based on estimate of the actual S&A required)				10,550
TOTAL, RELOCATIONS FOR FLOODWALL REACH, WEST IHNC					130,680

I. TOTAL, FLOODWALL REACH, WEST IHNC  
 II. FLOODWALL REACH, EAST IHNC

4,863,340

CONSTRUCTION

11 Levees and Floodwalls, East IHNC

I-walls and T-walls

Steel sheet piling, FZ-27	4,450	s.f.	6.00	75,600
Steel sheet piling, PMA-22	7,100	s.f.	8.00	72,800
Steel sheet piling, PSA-22	500	s.f.	8.00	4,000
Prestressed conc. piling, 12" X 12"	14,500	l.f.	15.00	217,500
Concrete in stabilization slab	70	c.y.	80.00	5,600
Concrete in base slab	540	c.y.	80.00	43,200
Concrete in walls, columns, and beams	285	c.y.	150.00	42,750
Portland cement	4,580	c.w.t	2.50	11,450
Steel reinforcement	367,400	lbs.	0.35	136,220
Structural steel	32,500	lbs.	1.75	56,880

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Cost Account No.	Item	Estimated Quantity	Unit	Unit Price	Estimated Amount
				\$	\$
	Pulling steel sheet piling	40,350	S.F.	1.50	40,350
	Salvage steel	650	tons	30.00	-(19,500)
Subtotal, Cofferdam, West IHNC					476,800
<i>offset</i>	Bypass Channel, West IHNC				
	Steel sheet piling, PZ-38 (USS <del>XXXXXX</del> EX-TEN SS)	48,600	S.F.	9.40	456,840
	Structural steel	3,234	S.F.	14.00	45,280
	Steel H-piles, HP 14 X 73	48,600	S.F.	1.00	48,600
	Pulling steel sheet piling	3,234	S.F.	3.00	9,700
	Pulling steel H-piles			6.00	4,800
	Concrete in deadmen	11,037	lbs.	0.35	3,870
	Steel reinforcement	2,700	C.Y.	6.50	17,550
	Channel excavation	4,500	C.Y.	3.50	15,750
	Structure backfill	.80	C.Y.	50.00	40.00
	Concrete removal	1,200	Tons	30.00	-(36,000)
	Salvage steel				
Subtotal, Bypass Channel, West IHNC					767,300
Subtotal, Vertical Lift Gate Structure, Approaches, Cofferdam, Bypass Channel West, IHNC					2,276,500
Subtotal, Levees and Floodwalls, West, IHNC					3,231,640
Environmental protection (0.5%)					16,200
Subtotal					3,217,000
Contingencies 20% <sup>±</sup>					647,570
Subtotal					3,897,410
30	Engineering and <i>design</i> <del>design</del> 11.6% <sup>±</sup> (based on estimate of actual E&D required)				452,100
31	Supervision and administration 9.9% <sup>±</sup> (based on estimate of the actual S&A required)				381,950
TOTAL, CONSTRUCTION FOR FLOODWALL REACH, West, IHNC					4,731,460
01	LANDS AND DAMAGES				1,200
RELOCATIONS					
02	Relocations				
<i>offset</i>	Pipelines and Powerlines				
	Overhead Powerlines	3	each	5,000	15,000



TABLE 1 (cont'd)

Cost Account No.	Item	Estimated Quantity	Unit	Unit Price	Estimated Amount
	Waterstops, L-type	45	l.f.	15.00	680
	Waterst ops, 3-bulb type	370	l.f.	5.00	1,950
	Gate seals	100	l.f.	20.00	2,000
	Sack-rubbed finish	18,950	s.f.	0.40	7,580
	Structure excavation	1,480	c.y.	5.00	7,400
	Structure backfill	1,600	c.y.	6.00	9,600
	Expansion joint filler	805	sf	1.50	1,240
	Miscellaneous metal	1,400	sq	1.70	2,400
	Trolleys for overhead roller gates	1	set	500.00	500
	Fertilizing and seeding	1.75	acre	400.00	690
	Clearing and grubbing	2.71	acre	500.00	1,360
	<b>Subtotal, I-wall and T-wall, East IHNC</b>				<b>701,400</b>
	<b>Subtotal, Vertical Lift Gate Structures, East IHNC</b> (Same as West IHNC)				<b>654,590</b>
	<b>offset Vertical Lift Gate Structure Approaches, East IHNC</b>				
	Prestressed conc. piling, 14" X 14"	29,000	l.f.	140.00	4,160,000
	Concrete in stabilization slab	25	c.y.	270.00	6,800
	Concrete in base slab	845	c.y.	80.00	67,600
	Concrete in walls	215	c.y.	150.00	30,100
	Portland cement	5,850	c.b.t.	2.50	14,700
	Steel reinforcement	106,200	lbs.	0.35	37,170
	Waterstops, L-type				2600
	Waterstops, 3-bulb type	200	l.f.	5.00	1,000
	Sack-rubbed finish	2,950	s.f.	0.40	1,180
	Expansion joint filler	775	s.f.	1.50	1,160
	<b>Subtotal, Approaches, East IHNC</b>				<b>202,910</b>
	<b>offset Cofferdam, East IHNC</b>				
	Steel sheet piling, PS-32	46,650	s.f.	6.50	3,032,250
	Structure excavation	2,100	c.y.	8.50	20,150
	Structure backfill	4,350	c.y.	1.00	4,350
	Dewatering system	1	job	50,000	50,000
	Pulling steel sheet piling	10,150	tons	40.00	406,000
	Salvage steel	650	tons	30.00	(19,500)
	<b>Subtotal, Cofferdam, East IHNC</b>				<b>498,180</b>
	<b>offset Bypass Channel, East IHNC</b>				
	Steel sheet piling, PZ-38 (USS X-ton) (USS LX-TGN-55)	50,000	sf	9.55	477,500
	Structural steel	327,800	lbs.	0.65	213,070
	Steel H-piles, HP 14 X 73	3,381	l.f.	14.00	47,330
	Pulling steel sheet piling	50,000	s.f.	1.00	50,000

TABLE 1 (cont'd)

Cost Account No.	Item	Estimated Quantity	Unit	Unit Price	Estimated Amount
	Pulling steel H-piles	3,381	l.f.	3.00	10,140
	Concrete in deadmen	90	cu. y.	60.00	5,400
	Steel reinforcement	12,420	lbs.	0.35	4,350
	Channel excavation	2,800	c.y.	6.50	18,200
	Structure backfill	3,000	c.y.	3.50	10,500
	Concrete removal	90	cu. y.	50.00	4,500
	Salvage steel	250	tons	20.00	(5,000)
	<b>Subtotal, Bypass Channel, East IHNC</b>				<b>804,310</b>
	<b>Subtotal, Vertical Lift Gate Structure, Approaches, Cofferdam, Bypass Channel, East IHNC</b>				<b>3,160,990</b>
	<b>Subtotal, Levees and Floodwalls</b>				<b>2,862,390</b>
	Environmental protection (0.5%)				14,320
	Subtotal				2,876,710
	Contingencies 20% †				575,340
	Subtotal				3,452,050
30	Engineering and design 11.6% † (based on estimate of actual E&D required)				400,410
31	Supervision and administration 9.8% † (based on estimate of actual S&A required)				338,300
	<b>TOTAL, CONSTRUCTION FOR FLOODWALL REACH, EAST IHNC</b>				<b>4,190,790</b>
01	LANDS AND DAMAGES, EAST IHNC	1	job	1200	1200
	RELOCATIONS, EAST IHNC				
02	Relocations, EAST IHNC				
	<del>offset</del> Pipelines and Powerlines				
	Overhead telephone line	1	each	5,000	5,000
	Overhead powerlines	4	each	5,000	20,000
	6" dia. water main thru T-wall	1	each	1,500	1,500
	5" dia. powerline thru T-wall	3	each	5,600	16,800
	54" dia. sewer force main thru T-wall	1	each	25,000	25,000
	36" dia. water main thru T-wall	1	each	15,000	15,000
	66" dia. sewer force main thru T-wall	1	each	3,000	3,000
	8" dia. gas line - relocate	1	each	2,500	2,500
	<b>Subtotal, Pipelines and Powerlines</b>				<b>116,200</b>
	Railroad Falsework for Swing Gate	1	job	2,000	2,000

TABLE 1 (cont'd)

Cost Account No.	Item	Estimated Quantity	Unit	Unit Price	Estimated Amount
	Subtotal, <del>Relocations</del> <sup>RELOCATIONS, EAST, IHNC</sup>				124,800
	Contingencies 20%†				25,000
	Subtotal				149,800
30	Engineering and design 11.6% (based on estimate of actual E&D required)				17,400
31	Supervision and administration 9.8% (based on estimate of actual S&A required)				14,700
	TOTAL, RELOCATIONS FOR FLOODWALL REACH, EAST, IHNC				181,900
	II. TOTAL, FLOODWALL REACH, EAST IHNC				4,373,890
	TOTAL BOTH FLOODWALL REACHES, WEST AND EAST IHNC (I & II)				<del>9,236,030</del> 9,237,230

I-Wall, T-Wall, Harbor Rd Gate  
 North Railroad tracks (Sta +0+00 to Sta 4+16.47)

	Est Quantity	Unit Price	Est Amount
Steel sheet piling, P2-27	800	8	6400
Steel sheet piling, PMA-22	8000	8	64000
Steel Sheet piling, PSA-22	150	8	1200
Conc Piling	7710	15	115650
Conc slab slab	40	80	3200
Conc base slab	290	80	23200
Conc walls	190	150	28500
Cement	2750	2.50	6890
Rebar	61900	0.35	21670
Structural steel	10800	1.75	18900
Waterstops, L-type	25	15.0	375
Waterstops, 3-Bulb	120	5.0	600
Gate seals	45	20.	900
Sack-rubble	6800	0.4	2720
Excav.	2150	5.0	10750
Fill	720	6.0	4320
1/4" Matl	210	1.5	315
Misc. Metal	600	15	900
Fert./seeding	0.4	400	160
Clearing/grubbing	0.7	500	350
Concrete removal	42	50	2100
		Σ	313,100
Environmental Prot 0.5%			1570
			314,670
Contingencies 20%			62,940
			377,610
E/D 11.6%			43,810
S/A 9.8%			37010
			\$ 458,430

I-Wall - Conc Vol = 20.37 CY (Walls)  
 Cement = 105.31 cwt  
 Rebar = 625.65 #  
 Pz-27 = 800 sf  
 'L' = 23.5'  
 '3'-B = 9'  
 Jt matl = 41.25 sf

T-Wall - Conc Vol = 252 CY (Base Slab)  
 Conc Vol = 185.6 CY (Walls)  
 Conc Vol = 33.55 CY (Stab. Slab)  
 Cement = 2435.85 cwt  
 PMA-22 = 6938.92 #  
 Conc piling = ~~5633~~ 1. ft. 6687 eff  
 '3'-B = 107.75 ft  
 Jt matl = 167.6 sf  
 Rebar = 56460 #

106.71  
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 2742

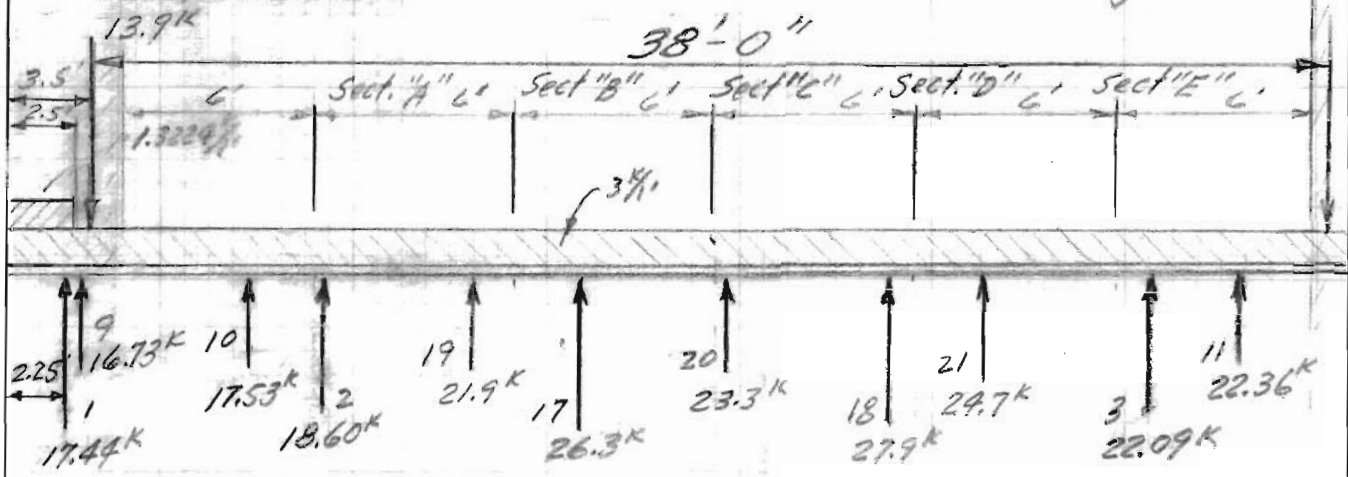
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252	80	1050	1019
<u>240</u>	<u>34</u>	<u>6939</u>	<u>6687</u>
492	114	7989	7706

20.37  
 185.60

36'-0" OVERHEAD ROLLER GATE @ FLORIDA AVENUE

3. Longitudinal Steel (CASE III Loading) Sect "F"



T-Wall Weight

$$(7.6)(1)(1)(0.15) = 1.14 \text{ k/ft}$$

$$\frac{1}{2}(7.6)(0.32)(1)(0.15) = 0.1824 \text{ k/ft}$$

$$\hline 1.3224 \text{ k/ft}$$

Moment @ Sect. "A"

Slab: $3 \times 10.5 = -31.5 \text{ k}$	$\times 5.25$	$= -165.38 \text{ k}$
Wall: $1.3224 \times 2.5 = -3.306 \text{ k}$	$\times 9.25$	$= -30.58 \text{ k}$
Pile #1	$= 17.44 \text{ k} \times 8.25$	$= 143.88 \text{ k}$
Pile #9	$= 16.73 \text{ k} \times 8.00$	$= 133.84 \text{ k}$
Column #1	$= -13.9 \text{ k} \times 7.00$	$= -97.30 \text{ k}$
Pile #10	$= 17.53 \text{ k} \times 3.00$	$= 52.59 \text{ k}$

$\Sigma V = 2.994 \text{ k} \uparrow$        $\Sigma M = -37.05 \text{ k}$

Moment @ Sect. "B"

Slab: $3 \times 16.5 = -49.50 \text{ k}$	$\times 8.25$	$= -408.38 \text{ k}$
Wall:	$= -3.306 \text{ k} \times 15.25$	$= -50.42 \text{ k}$
Pile #1	$= 17.44 \text{ k} \times 14.25$	$= 248.52 \text{ k}$
Pile #9	$= 16.73 \text{ k} \times 14.00$	$= 234.22 \text{ k}$
Column #1	$= -13.9 \text{ k} \times 13.00$	$= -180.70 \text{ k}$
Pile #10	$= 17.53 \text{ k} \times 9.00$	$= 157.77 \text{ k}$
Pile #2	$= 18.60 \text{ k} \times 3.75$	$= 106.95 \text{ k}$
Pile #19	$= 21.90 \text{ k} \times 1.25$	$= 27.38 \text{ k}$

$\Sigma V = 25.494 \text{ k} \uparrow$        $\Sigma M = 135.34 \text{ k}$

(for Item No., see UTILITIES SHEETS)

REASONABLE CONTRACT ESTIMATE					SHEET	OF
PROJECT Florida Avenue Complex, Utility Relocations					INVITATION NO.	
ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT	
WEST SIDE						
P-1	OH Power				5,000.00	
W-1	Water Main 6" $\phi$				1,500.00	
P-2	OH Power				5,000.00	
E-1	Power 5" $\phi$				5,600.00	
E-2	Power 5" $\phi$				5,600.00	
S-1	Sewer Force Main 54" $\phi$				25,000.00	
W-2	Water Main 48" $\phi$				21,000.00	
P-3	OH Power				5,000.00	
				TOTAL	73,700.00 ✓	
EAST SIDE						
T-1	OH Telephone				5,000.00	
S-2	Sewer Force Main 66" $\phi$				30,000.00	
P-4	OH Power				5,000.00	
W-3	Water Main 6" $\phi$				1,500.00	
P-5	OH Power				5,000.00	
W-4	Water Main 36" $\phi$				16,000.00	
S-3	Sewer Force Main 54" $\phi$				25,000.00	
G-1	Gas 8" $\phi$				2,500.00	
E-3	Power 5" $\phi$				5,600.00	
E-4	Power 5" $\phi$				5,600.00	
P-6	OH Power				5,000.00	
E-5	Power 5" $\phi$				5,600.00	
P-7	OH Power				3,000.00	
				TOTAL	116,800.00 ✓	

(CRS 14 Sent 26)







PROJECT PRELIMINARY - FLORIDA AVE. WEST	Page — of —	COMPUTED BY HMB	DATE Mar. 77
SUBJECT ESTIMATE CONCRETE, CEMENT ( 0+40 to 3+79.71 )		CHECKED BY	DATE

CONCRETE:

Base Slab =  $2.5(8)(263.71) = 5274.2 \text{ cf} = 195.34 \text{ cy}$

( Use 200 c y )

Walls:

W/L sta 0+40. to sta 1+19.09

$(1(11) + 11^2/24(\frac{1}{2}))79.09 = 1069.3 \text{ cf} = 39.6 \text{ cy}$

W/L sta 1+19.09 to sta 3+79.71

$(1(12) + 12^2/24(\frac{1}{2}))184.62 = 2769.3 \text{ cf} = 102.6 \text{ cy}$

Total = 142.2 cy

(Use 150 cy )

Stabilization Slab =  $8(263.71)(4/12) = 703.2 \text{ cf} = 26.1 \text{ cy}$

(Use 27 cy )

Cement =  $363.64 \times 5.17 = 1880.02 \text{ cwt.}$

(Use 1890 cwt. )

PROJECT FLORIDA AVENUS - WEST, PRELIMINARY	Page — of —	COMPUTED BY HMB	DATE Mar. 77
SUBJECT ESTIMATE STEEL SHEET PILING ( 0+40 to 3+79.71, W/L)		CHECKED BY	DATE

W/L sta. 0+40. to sta. 1+19.09:

Top sheet piling = EL. +1.25

Bottom sheet piling = EL. -20.

$$A = 79.09 \times 21.25 = 1680.66 \text{ sf}$$

W/L Sta. 1+95.09 to sta. 3+79.71:

Top sheet piling = EL. +0.25

Bottom sheet piling = EL. -20

$$A = 184.62 \times 20.25 = 3738.56 \text{ sf}$$

$$\text{Total} = 5419.22 \text{ sf}$$

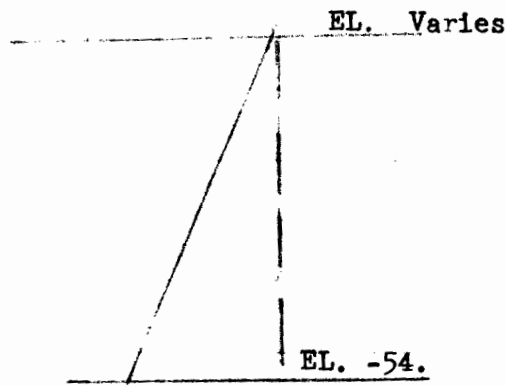
(Use 5500 sf)

PROJECT	Page <u>  </u> of <u>  </u>	COMPUTED BY	DATE
SUBJECT		CHECKED BY	DATE
ESTIMATE T-WALL REINFORCEMENT (0+40 to 3+79.72, W/L)			

Reinforcement:

		Mono. 3			
<i>Long stem</i>	NO. 5	22 x 77.83	x 1.043	=	1786 lb.
		Monm. 6,7,8			
		24 x 180.87	x 1.043	=	4528
		Mono. 3			
	No. 6	234 x 13.5'	x 1.502	=	4745
		<i>Base slab long</i> 30 x 77.83	x 1.502	=	3508
		Mono. 6,7,8			
	<i>all parts</i>	540 x 14.5'	x 1.502	=	11761
		<i>Base slab</i> 30 x 137.12'	x 1.502	=	6179
		<i>long</i> 16 x 48.75'	x 1.502	=	<u>1172</u>
		Mono. 3			
	No. 8	160 x 10'	x 2.670	=	4272
		Mono. 6,7,8			
		374 x 10'	x 2.670	=	<u>9986</u>
					47937

PROJECT	Page <u>  </u> of <u>  </u>	COMPUTED BY HMB	DATE Mar. 77
SUBJECT ESTIMATE PRESTRESSED CONCRETE PILE - (0+40 to 3+79.71)		CHECKED BY	DATE



$$\text{Length} = \frac{\Delta \text{Elev}}{\sqrt{1 + 1/n^2}}$$

MONOLITH	$\Delta$ ELEV	NO. PILES	BATTER	PILE LENGTH	LENGTH
3	55.5	16	2:1	62'	992'
3	55.5	1	4:1	57'	57'
	54.5	8	2:1	61'	488'
	54.5	8	2:1	61'	488'
6	54.5	18	2:1	61'	1098'
6	54.5	2	0	54.5'	109'
7	54.5	14	2:1	61'	854'
7	54.5	4	0	54.5'	218'
8	54.5'	16	2:1	61'	976'
8	54.5'	3	4:1	56'	168'

TOTAL = 5448'

(Use 5500' )

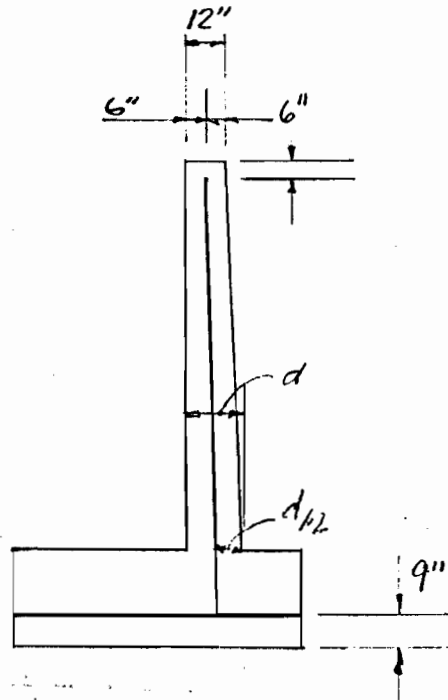
PROJECT	Page — of —	COMPUTED BY	DATE
SUBJECT		CHECKED BY	DATE
ESTIMATE PRESTRESSED CONCRETE PILE - (0+40 to 3+79.71 )			

Length of 12"X12" Prestressed Concrete Piles.

MONOLITH	NO. PILES	PILE LENGTH	LENGTH
3	17	60	1020'
	8	60	480'
5	8	60	480'
6	20	60	1200'
7	18	60	1080'
8	19	60	<u>1140'</u>
		Total	= 5400'

(Use 5500' )

F Wall - Waterstops, 3-Bulb Type (0:40 to 31 1972)



Typical, 3-Bulb Type  
Waterstop Detail

$$\text{Length} = 6(12 + 2.5 - 0.75 + 8) = 130.5 \text{ Lf}$$

(Use 140 Lf )

Construction Joint Material (0.40 to 177.72)

Page — of —

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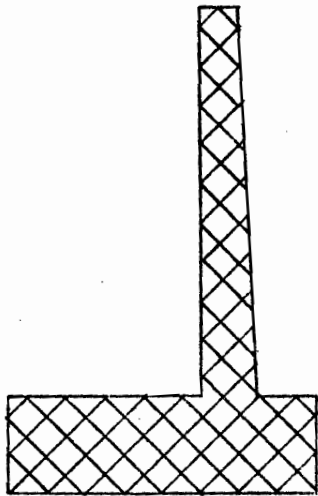
Mar. 77

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DATE

Construction Joint Material

Typical Area



$$A = 5 \left[ \left( \frac{1}{2} \right) (12) \left( 2 + \frac{12}{24} \right) + 20 \right] = 175 \text{ sf}$$

(Use 180 sf)



REASONABLE ESTIMATE

SHEET OF

PROJECT

Florida Avenue Complex - West

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
1	<u>Floodwalls, Levees, Gated Structure</u>				
	Structure Excavation	2,300	CY	5.00	11,500.00
	Structure Backfill	2,870	CY	6.00	17,220.00
	Backfill Fertilization and seeding	1.34	acre	400	536.00
	Clearing and Grubbing	2.34	acre	500	1170.00
	Steel Sheet Piling, PMA-22	16,600	SF	8.00	132,800.00
	Steel Sheet Piling, PZ-27	1,530	SF	8.00	12,240.00
	12" X 12" Prestressed Concrete Piling	15,820	LF	15.00	237,300.00
	Concrete in Stabilization Slab	80	CY	80.00	6,400.00
	Concrete in Base Slab	610	CY	80.00	48,800.00
	Concrete in Walls, Columns, Beams	410	CY	150.00	61,500.00
	Portland Cement	5700	CWT	2.50	14,250.00
	Steel Reinforcement	882,600	Lb	0.35	308,910.00
	Bulb-type Waterstops	340	LF	5.00	1,700.00
	L-type Waterstops	35	LF	15.00	525.00
	Expansion Joint Filler	600	SF	1.50	900.00
	Structural Steel	46,000	Lb	1.75	80,500.00
	Miscellaneous Metal	6,600	Lb	1.50	9,900.00
	Gate Seals	130	LF	20.00	2,600.00
	Overhead Trolleys	1	Job	500.00	500.00
	Sackrub Finish	16,600	SF	0.40	6,640.00
	Subtotal				955,891.00

CBS 7 Sept. 76

REASONABLE ESTIMATE

SHEET OF

PROJECT

Florida Avenue Complex - West

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
2.	<u>Bypass Channel</u>				
	Structure Excavation	2,700	CY	2.50	6,750.00
	Structure Backfill	4,500	CY	3.50	15,750.00
	Steel Sheet Pile, PZ-32 (USS X-Ten '55)	48,600	SF	8.50	413,100.00
	Steel Sheet Pile Cover Plates	5,500	Lb	0.75	4,125.00
	Wale Channels, C15 X40	1,434	FT	8.00	11,472.00
	Standard Pipe, 4" d	6,045	Lb	0.50	3,022.50
	Tie Rods, 3/4" φ	26,400	Lb	0.75	19,800.00
	Steel H-Piles, HP14 X73	3,224	FT	14.00	45,276.00
	Concrete in Deadmen	80	CY	60.00	4,800.00
	Steel Reinforcement	11,037	Lb	0.35	3,862.95
	Pulling Steel Sheet Piling	48,600	SF	1.00	48,600.00
	Pulling H-Piling	3,224	FT	3.00	9,702.00
	Salvage Steel	1,260	Ton	30.00	37,800.00
	Subtotal				624,060.45



REASONABLE ESTIMATE

SHEET OF

PROJECT

Florida Avenue Complex - West

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
4	<u>Vertical Lift Gate Structure</u>				
	Structure Excavation	3,250	CY	3.00	9,750.00
	Structure Backfill	2,250	CY	4.00	9,000.00
	Steel Sheet Piling, PMA-22	2,150	SF	8.00	17,200.00
	Concrete in Base Slab	300	CY	80.00	24,000.00
	Concrete in Walls (E1+18.5 to +2)	251	CY	140.00	35,140.00
	Concrete in Deckslab	91	CY	180.00	16,380.00
	Concrete in Walls (E1+2 to +27)	147	CY	140.00	20,580.00
	Concrete in Floodwalls	23	CY	140.00	3,220.00
	Concrete in Stairs	4	CY	200.00	800.00
	Concrete in Machine Room Slab	64	CY	80.00	5,120.00
	Concrete in Machine Room Walls	41	CY	140.00	5,740.00
	Concrete in Roof Slab	17	CY	200.00	3,400.00
	Portland Cement	4,850	cwt	2.50	12,125.00
	Steel Reinforcement	181,700	lb	0.35	63,595.00
	14" x 14" Prestressed Concrete Piling	6,000	LF	16.00	96,000.00
	Structural Steel	72,600	lb	1.75	127,050.00
	Miscellaneous Metal	91,440	lb	1.50	137,160.00
	Sackrub Finish	5,750	SF	0.40	2,300.00
	Overhead Trolleys	4	Job	500.00	2,000.00
	Subtotal				590,560.00

REASONABLE ██████████ ESTIMATE

SHEET OF

PROJECT

Florida Avenue Complex - West

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
5	<u>Approach Structures</u>				
	Concrete in Stabilization Slab	52	CY	80.00	4,160.00
	Concrete in Base Slab	546	CY	80.00	43,680.00
	Concrete in Walls	260	CY	140.00	36,400.00
	Portland Cement	4,425	CWT	2.50	11,062.50
	14"X14" Prestressed Concrete Piles	3,424	LF	16.00	54,784.00
	Steel Reinforcement	627,100	LB	0.35	219,485.00
	Sackrub Finish	3,500	SF	0.40	1,400.00
	Expansion Joint Filler	700	SF	1.50	1,050.00
	Bulb-type Waterstops	190	LF	5.00	950.00
	L-type Waterstops	100	LF	20.00	2,000.00
	Subtotal				374,971.50

PROJECT Florida Avenue Complex

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
	<u>Florida Avenue Complex-West</u>				
1.	Floodwalls, Levees, Gated Structures				
2.	Bypass Channel				
3.	Cofferdam System				
4.	Vertical Lift Gate Structure				
5.	Approach Structures				
	Subtotal				
	<u>Florida Avenue Complex-East</u>				
1.	Floodwalls, Levees, Gated Structures				
2.	Bypass Channels				
3.	Cofferdam System				
4.	Vertical Lift Gate Structure				
5.	Approach Structures				
	Subtotal				

Item	Estimated Quantity	Unit	Unit Price	Estimated Amount
<del>Compression pile test (12"x12")</del>	<del>2</del>	<del>ea</del>		
<del>Additional comp. pile test (14"x14")</del>	<del>2</del>	<del>ea</del>		
<del>Tension pile test</del>	<del>4</del>	<del>ea</del>		
Steel sheet piling, PZ-27	1530	sf	8.00	12240
Steel sheet piling, PMA-22	16610	sf	8.00	132800
Steel sheet piling, PSA-22	3500	sf	8.00	28000
Prestressed conc. piling, 12"x12"	15280	lf	15.00	229200
Concrete in stabilization slab	80	cy	80.00	6400
Concrete in base slab	610	cy	80.00	48800
Concrete in walls, columns, and beams	410	cy	150.00	61500
Portland cement	5700	c.wt	2.50	14250
Steel reinforcement	882600	lbs.	0.35	308710
Structural steel	46000	lbs.	1.75	80500
Waterstops, L-type	35	lf	15.00	525
Waterstops, S-Bulb type	340	lf	5.00	1700
Gate seals	120	lf	20.00	2400
Sack-rubbed finish	2200	sf	0.40	880
Structural excavation	2810	cy	5.00	14050
Structural backfill	1600	cy	6.00	9600
Expansion joint filler	600	sf	1.50	900
<del>Concrete removal</del>	<del>92</del>	<del>cy</del>	<del>1.50</del>	<del>138</del>
Miscellaneous metal	6600	job	1.50	9900
Trolleys, overhead roller gates	1	job	500.00	500
Fertilizing and seeding	1.24	acre	400.00	496
Clearing and grubbing	2.24	acre	500.00	1120
Subtotal, Floodwall Reaches, East				

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PROJECT	Florida Avenue Complex - <del>W-101</del>	Page 2 of —	COMPUTED BY	DATE
SUBJECT	Levees and Floodwalls		CHECKED BY	DATE

Environmental protection ( )

Subtotal

Contingencies 25% ±

Subtotal

Engineering and design 10.8% ±  
 (based on estimated of actual  
 E&D required)

Supervision and administration 9.0% ±  
 (based on estimated of the actual  
 S&A on E&D required)



	Item	Estimated Quantity	Units	Unit Price	Estimated Quantity
02	Highlines and Powerlines Overhead power line 6" φ water line thru T-Wall	2	Ea <del>Job</del>	5000	10000
	Overhead power line 5" φ power line thru T-Wall	1	Ea	1500	1500
	Overhead power line 5" φ power line thru T-Wall	1	Ea	5000	5000
	54" φ sewer force main thru T-Wall	2	Ea	5600	11200
	54" φ sewer force main thru T-Wall	1	Ea	25000	25000
	48" φ water main thru T-Wall	1	Ea	21000	21000
	Subtotal				73700
	Miscellaneous structures Railroad falsework	1	job	L.S.	15000
	<del>Modification to existing road ways</del>				
	5				
	Subtotal RELOCATIONS				

PROJECT		COMPUTED BY		DATE	
Florida Avenue Complex - <del>East</del> <sup>West</sup>		Page ___ of ___			
SUBJECT		CHECKED BY		DATE	
Relocations					
Item	Estimated Quantity	Unit	Unit Price	Estimated Amount	
Overhead telephone line	1	Ea	5000	5000	
66" $\phi$ sewer force main, thru T-wall	1	Ea	20000	20000	
Overhead power line	4	<del>Ea</del>	5000	<del>20000</del>	
Power line in 5" $\phi$ pipe thru T-wall	3	Ea	5600	16800	
Relocated 6" $\phi$ water main	1	Ea	1500	1500	
Relocated 36" $\phi$ water main	1	Ea	16000	16000	
Relocated 54" $\phi$ sewer force main	1	Ea	25000	25000	
Relocated 8" $\phi$ gas line	1	Ea	2500	2500	
Subtotal				116800	
Miscellaneous Structures					
Railroad falsework	1	job			
Relocate power <sup>transformer</sup> substation	1	job			
Relocate power poles	1	job			
Relocate manhole	1	job			
Subtotal					
Subtotal, Relocations					
Contingencies 25%					
Engineering and design 10.8% $\pm$ (based on estimate of actual E;D required)					
Supervision and administration % $\pm$ (based on estimate of actual S;A on E;D required)					
Total, Relocations for Floodwall Reaches					
Total, Floodwall Reaches					

PROJECT		Florida Avenue Complex - <del>Est</del> <sup>est</sup> Page <u>25</u> of <u>    </u>		COMPUTED BY	DATE
SUBJECT		Vertical Lift Gate Structure		CHECKED BY	DATE
Item	Estimated Quantity	Unit	Unit Price	Estimated Amount	
Steel sheet piling PMA-22	2150	sf	8.00	17200	
Prestressed conc. piling, 14"x14"	6000	lf	16.00	96000	
Concrete in base slab	300	cy	80.00	24000	
Concrete in wall (El. +18.5 to +2)	251	cy	140.00	35140	
Concrete in deck slab	91	cy	180.00	16380	
Concrete in wall (El. +2 to +27)	147	cy	140.00	20580	
Concrete in floodwalls	23	cy	140.00	3220	
Concrete in stairs	4	cy	200.00	800	
Concrete in machine room slab	64	cy	80.00	5120	
Concrete in machine room walls	41	cy	140.00	5740	
Concrete in roof slab	17	cy	200.00	3400	
Portland cement	4850	cwt	2.50	12125	
Steel reinforcement	181700	lbs	0.35	63595	
Structural steel	72600	lbs.	1.75	127050	
Sack-rubbed finish	5750	sf.	0.40	2300	
Structural excavation	<del>2300</del> 2250	cy	3.00	<del>6900</del> 6750	
Structural backfill	<del>3300</del> 3250	cy	4.00	<del>13200</del> 13000	
Miscellaneous metal	91440	lbs.	1.50	137160	
Trolleys, overhead roller gate	4	Ea	500.00	2000	
Machinery assembly electrical	2	Ea	13350	26700	
Subtotal				<del>617240</del> 617260	

Item	Estimated Quantity	Unit	Unit Price	Estimated Amount
Prestressed conc. piling, 14" x 14"	<del>3424</del>	lf	16.00	<del>54784</del>
Concrete in stabilization slab	<del>52</del>	cy	80.00	<del>4160</del>
Concrete in base slab	<del>546</del>	cy	80.00	<del>43680</del>
Concrete in walls	<del>260</del>	cy	140.00	<del>36400</del>
Portland cement	<del>4425</del>	cwt	2.50	<del>11063</del>
Steel Reinforcement	<del>627100</del>	lbs	0.35	<del>219485</del>
Waterstops, L-type	<del>100</del>	lf	20.00	<del>2000</del>
Waterstops, 3-Bulb type	<del>190</del>	lf	5.00	<del>950</del>
Sack-rubbed finish	<del>3500</del>	sf	0.40	<del>1400</del>
Expansion joint filler	<del>700</del>	sf	1.50	<del>1050</del>
Subtotal				<del>85975</del>
<u>Cofferdam System</u>				<u>374972</u>
Steel sheet piling, <del>PS-32</del> <sup>PS-32</sup>	<del>40350</del>	sf	8.50	<del>342975</del>
Structure excavation	<del>2700</del>	cy	3.00	<del>8100</del>
Structure backfill	<del>4550</del>	cy	10.00	<del>45500</del>
Dewatering system	1	job	50000	50000
Subtotal				<u>746575</u>

Item	Estimated Quantity	Unit	Unit Price	Estimated Amount
Steel sheet piling, PZ-32 (USS x-Ten 55)	48600	sf	8.50	413100
Steel sheet piling cover plates	5500	lbs.	0.75	4125
Wale channels, C15x40	1434	lf	8.00	11472
Standard pipe, 4" $\phi$	6045	lbs.	0.50	3023
Tie rods, 3/4" $\phi$	26400	lbs.	0.75	19800
Steel H-piling, HPX73	3234	lf	14.00	45276
Pulling steel sheet piling	48600	sf	1.00	48600
Pulling steel H-piling	3234	lf	3.00	9702
Concrete in deadman	80	cy	60.00	4800
Steel reinforcement	11037	lbs.	0.35	3863
Structure excavation	<del>2700</del> 2700	cy	2.50	<del>6750</del> 6750
Structure backfill	<del>4500</del> 4500	cy	3.50	<del>15750</del> 15750
Salvage steel	1260	Ton	30.00	(37800)
Subtotal				<del>512111</del> 548461
Subtotal, Vertical Lift Gate Structure and adjoining structures				1,987,268
Environmental protection (0.005)				19,873
Subtotal				2,007,141
Contingencies 25% $\pm$				501,786
Subtotal				2,508,927
Engineering and design 10.5% $\pm$ (based on estimate of actual E&D required)				258,437
Supervision and administration % $\pm$ (based on estimate of the actual E&D required)				25,804
Total, Construction for Vertical Lift Gate Structure and adjoining structures				3,005,696

## Approach Structures - West

Bulb-type Waterstops,

$$L_1 = 45' + 1' + 1' + 1.75' + 1.75' + 20.5' + 20.5'$$

$$= 91.5 \text{ lf (2 structures)}$$

$$\text{Total Length} = 183 \text{ lf (Use 190 lf)}$$

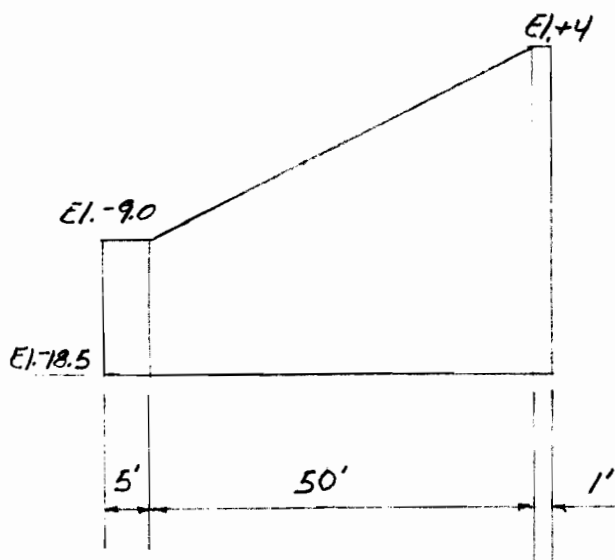
L-type Waterstops,

$$L_2 = 26' + 1' + (1.5 + 9.5) \times 2$$

$$= 49 \text{ lf (2 structures)}$$

$$\text{Total Length} = 98 \text{ lf (Use 100 lf)}$$

Sack rub Finish—  
Approach Structures—West IHNC



$$A = (1' \times 22.5') + \frac{1}{2}(22.5' + 9.5')(50') + (9.5' \times 5')$$

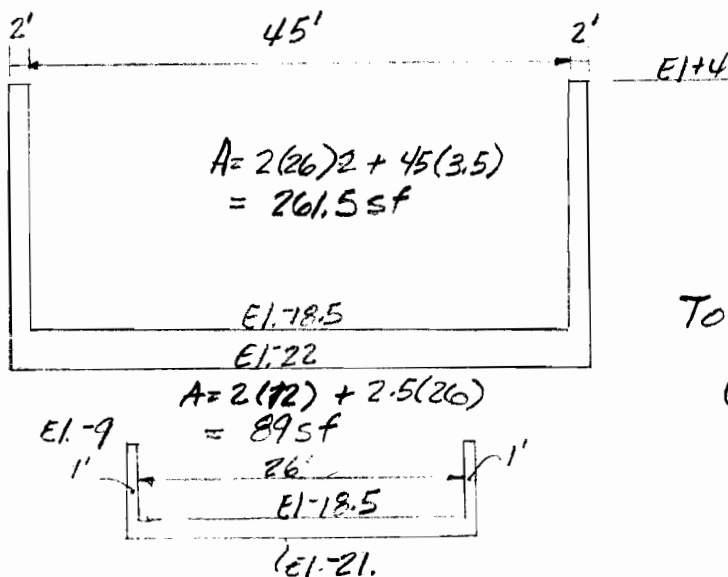
$$= 870 \text{ sf}$$

$$\text{Total Area} = 4 \times 870 \text{ sf}$$

$$= 3480 \text{ sf}$$

(Use 3500 sf)

Expansion Joint Filler - (2 structures)



$$A = 2(26)2 + 45(3.5)$$

$$= 261.5 \text{ sf}$$

$$A = 2(12) + 2.5(26)$$

$$= 89 \text{ sf}$$

$$\text{Total } A = 701 \text{ sf}$$

(Use 700 sf)

PROJECT	Page — of —	COMPUTED BY	DATE
SUBJECT		CHECKED BY	DATE

Sackrub Finish

W/L sta. 0+11.03 to W/L sta 0+40,

$$A = 51.03' \times 6' \times 2 = 612.36 \text{ sf}$$

W/L sta. 0+40 to W/L sta 3+71,

$$A = 332' \times 12' \times 2 = 7992 \text{ sf}$$

Swing Gates,

$$\text{Wall Area} = 10.75' \times 10' \times 2 = 215 \text{ sf}$$

$$\begin{aligned} \text{Column Area} &= 2(7.2)(8) + 14.2(8) + 17(8) \\ &= 379.2 \text{ sf} \end{aligned}$$

W/L sta 4+36.46 to W/L sta 5+55.46,

$$A = 10' \times 119' \times 2 = 2380 \text{ sf}$$

W/L sta 5+55.46 to W/L sta. 6+02.71,

$$A = 47.25 \times 13' \times 2 = 1228.5 \text{ sf}$$

W/L sta 6+02.71 to W/L sta 6+97.5

$$A = 32.27' \times 13 \times 2 = 735.54 \text{ sf}$$



PROJECT	Page <u>  </u> of <u>  </u>	COMPUTED BY	DATE
SUBJECT		CHECKED BY	DATE

Sackrub Finishes (Vertical Lift Gate)

El. 4 to El. 10 -

$$A_2 (36' \times 665' \times 2) + (36' \times 13' \times 2)$$

$$= 5724 \text{ sf}$$

(Use 5750 sf)

3075.65

## Overhead Roller Gate

$$\text{Wall Area} = 40' \times 12' \times 2 = 960 \text{ sf}$$

$$\text{Column Area} = 31' \times 2275' = 705.25 \text{ sf}$$

$$\text{Beam Area} = 2.5' \times 4 \times 78' = 780 \text{ sf}$$

W/L sta. 7+84.5 to W/L sta. 8+09.50,

$$A = 25' \times 10' \times 2 = 500 \text{ sf}$$

W/L sta. 8+09.5 to W/L sta. 8+33.7,

$$A = 24.2' \times 6' \times 2 = 290.4 \text{ sf}$$

$$\text{Total Area} = 16563.25 \text{ sf}$$

(Use 16600 sf)

1. Label all Computations.

9 Sep 76 2. Summary table for pile loads for T-Walls, 161  
Gates

20 Aug 76 3. Reinforcement/Jt Matl for West T-Wall

23 Aug 76 4. Check T-Wall comps (Est.)  
<sup>Rebar</sup>

16 Aug 76 5. (16 Aug 76) - East; Fla Ave. Gate -  
1. relocate rd  
2. give south side larger shoulder  
3. Redo Mono 8, 6

1 Sep 76 6. Estimate VLG Excav (Fill),  
cofferdam syst.,  
Bypass channel  
(Joe Cali - F+M)

19 Aug 76 7. Estimate Utilities (to Est/Spec)

30 Aug 76 8. Row/const Easement (to Real Estate) 16B

19 Aug 76 9. Bypass Channel (to Hugh Howat)

20 Aug 76 10. ~~Summa~~ Check pile tip Elev

23 Aug 76 11. Check sheet pile tip <sup>Elev</sup> - West

12. Check sheet pile tip Elev - East (Cali)

13. Write-up. -

1. called 23 Aug 76  
haven't done  
have changed

PROJECT		Page <u>1</u> of <u>    </u>		COMPUTED BY	DATE
SUBJECT				CHECKED BY	DATE
Floodwall Reaches, East					
Levees and Floodwalls					
Item	Estimated Quantity	Unit	Unit Price	Estimated Amount	
Compression pile test (12"x12")	2	Ea			
Additional comp. pile test (14"x14")	2	Ea			
Tension pile test	4	Ea			
Steel sheet piling, P2-27	9450	SF	8.00	75600	
Steel sheet piling, PMA-22	9100	SF	8.00	72800	
Steel sheet piling, PSA-22	500	SF		4000	
Prestressed conc. piling, 12"x12"	14500	lf	15.00	217500	
Concrete in stabilization slab	70	CY	80.00	5600	
Concrete in base slab	540	CY	80.00	43200	
Concrete in walls, columns, and beams	285	CY	150.00	42750	
Portland Cement	4580	C.WT	2.50	11450	
Steel reinforcement	389200	LBS.	0.35	136220	
Structural steel	32500	LBS.	1.75	56875	
Waterstops, L-type	45	lf	15.00	675	
Waterstops, S-Bulb type	390	lf	5.00	1950	
Gate seals	100	lf	20.00	2000	
Sack-rubbed finish	18950	SF	0.40	7580	
Structural excavation	1480	CY	5.00	7400	
Structural backfill	1600	CY	6.00	9600	
Expansion joint filler	825	SF	1.50	1240	
Miscellaneous metal	1600	JOB	1.50	2400	
Trolleys, overhead roller gates	1	JOB	500.00	500	
Fertilizing and seeding	1.73	acre	400.00	692	
Clearing and grubbing	2.71	acre	500.00	1355	
Subtotal, Floodwall Reaches, East					

PROJECT	Florida Avenue Complex - East	Page 2 of —	COMPUTED BY	DATE
SUBJECT	Leaves and Floodwalls		CHECKED BY	DATE

Environmental protection ( )

Subtotal

Contingencies 25% ±

Subtotal

Engineering and design 10.8% ±  
 (based on estimated of actual  
 E; D required)

Supervision and administration 9.0% ±  
 (based on estimated of the actual  
 S; A on E; D required)

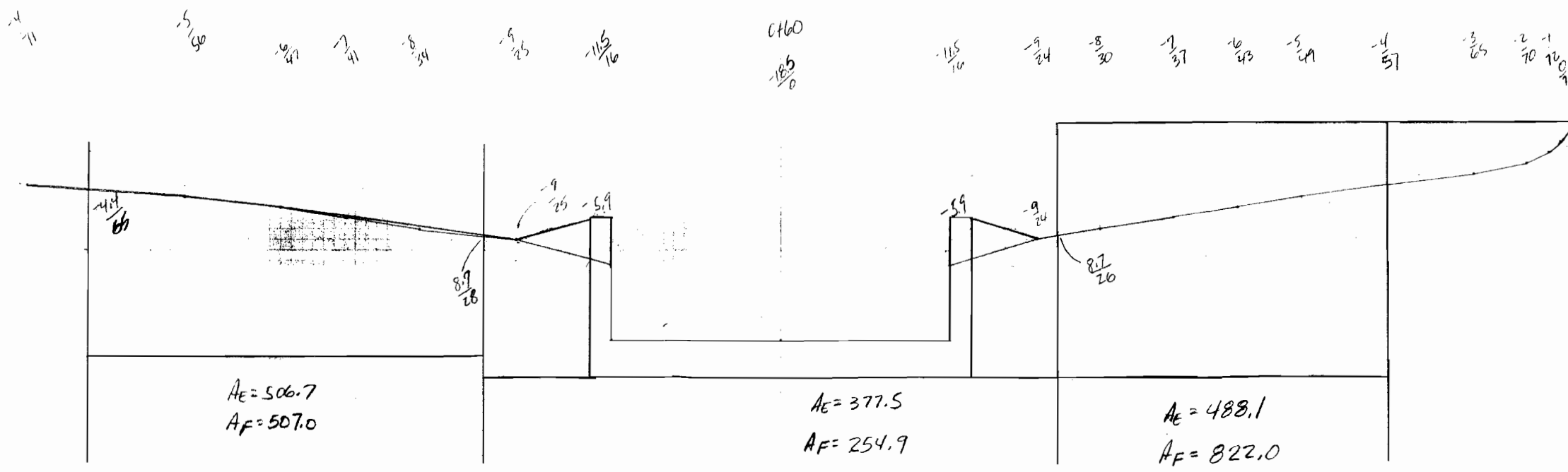
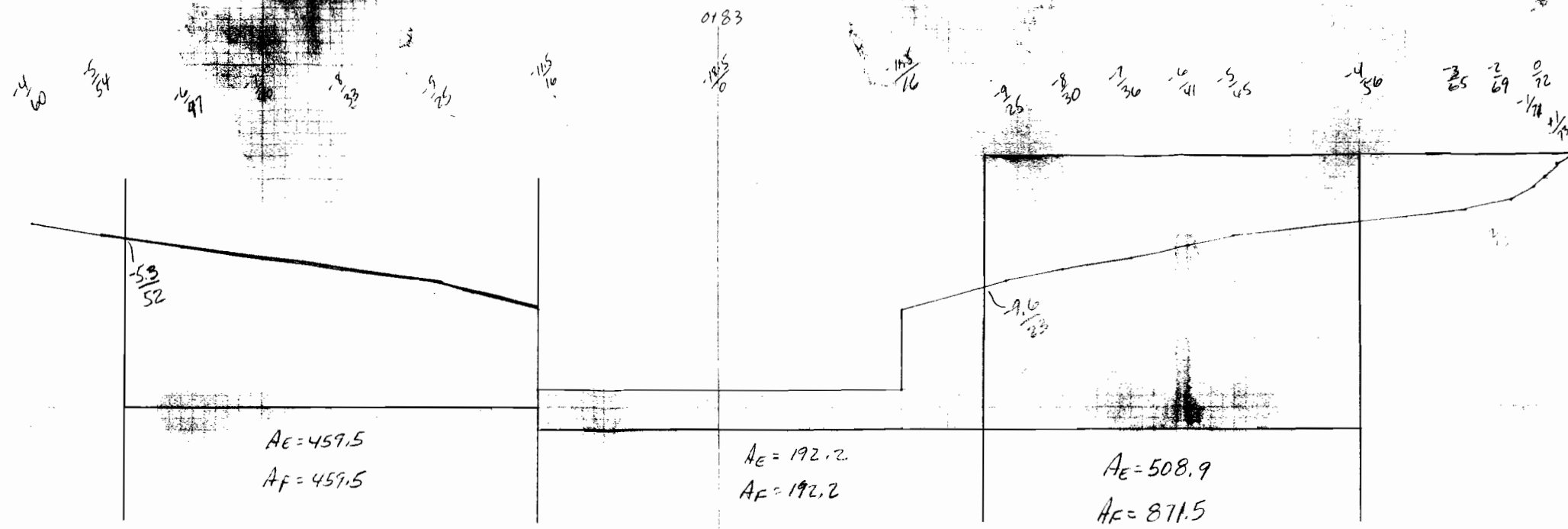
PROJECT		Page ___ of ___		COMPUTED BY	DATE
SUBJECT				CHECKED BY	DATE
Item	Estimated Quantity	Unit	Unit Price	Estimated Amount	
Overhead telephone line	1	Ea	5000	5000	
66" $\phi$ sewer force main, thru T-wall	1	Ea	30000	30000	
Overhead power line	4	Ea	5000	20000	
Power line in 5" $\phi$ pipe thru T-Wall	3	Ea	5600	16800	
Relocated 6" $\phi$ water main	1	Ea	1500	1500	
Relocated 36" $\phi$ water main	1	Ea	16000	16000	
Relocated 54" $\phi$ sewer force main	1	Ea	25000	25000	
Relocated 8" $\phi$ gas line	1	Ea	2500	2500	
Subtotal				116800	
Miscellaneous Structures					
Railroad falsework	1	job			
Relocate power <sup>transformer</sup> substation	1	job			
Relocate power poles	1	job			
Relocate manhole	1	job			
Subtotal					
Subtotal, Relocations					
Contingencies 25%					
Engineering and design 10.8% $\pm$ (based on estimate of actual E;D required)					
Supervision and administration % $\pm$ (based on estimate of actual S;A on E;D required)					
Total, Relocations for Floodwall Reaches					
Total, Floodwall Reaches					

PROJECT		Page ___ of ___		COMPUTED BY	DATE
SUBJECT				CHECKED BY	DATE
Item	Estimated Quantity	Unit	Unit Price	Estimated Amount	
Steel sheet piling PMA-22	2150	sf	8.00	17200	
Prestressed conc. piling, 14"x14"	6000	lf	16.00	96000	
Concrete in base slab	300	cy	80.00	24000	
Concrete in wall (El. -18.5 to +2)	251	cy	140.00	35140	
Concrete in deck slab	91	cy	180.00	16380	
Concrete in wall (El. +2 to +27)	147	cy	140.00	20580	
Concrete in floodwalks	23	cy	140.00	3220	
Concrete in stairs	4	cy	200.00	800	
Concrete in machine room slab	64	cy	80.00	5120.	
Concrete in machine room walls	41	cy	140.00	5740.	
Concrete in roof slab	17	cy	200.00	3400	
Portland cement	4850	cwt	2.50	12125	
Steel reinforcement	181700	lbs	0.35	63595	
Structural steel	72600	lbs.	1.75	127050	
Sack-rubbed finish	5750	sf.	0.40	2300	
Structural excavation	3300	cy	3.00	9900	
Structural backfill	2700	cy	4.00	10800	
Miscellaneous metal	91440	lbs.	1.50	137160	
Trolleys, overhead roller gate	4	Ea	500.00	2000	
Machinery assembly	2	Ea	13350	26700	
Subtotal				619210	

PROJECT		Page — of —		COMPUTED BY	DATE
SUBJECT				CHECKED BY	DATE
Florida Avenue Complex - East					
Vertical Lift Gate Structure - Approach Structures					
Item	Estimated Quantity	Unit	Unit Price	Estimated Amount	
Prestressed conc. piling, 14"x14"	2600	lf	16.00	41600	
Concrete in stabilization slab	85	cy	80.00	6800	
Concrete in base slab	845	cy	80.00	67600	
Concrete in walls	215	cy	140.00	30100	
Portland cement	5880	cwt	2.50	14700	
Steel Reinforcement	106200	lbs	0.35	37170	
Waterstops, L-type	130	lf	20.00	2600	
Waterstops, 3-Bulb type	200	lf	5.00	1000	
Sack-rubbed finish	2950	sf	0.40	1180	
Expansion joint filler	775	sf	1.50	1165	
Subtotal				203915	
<u>Cofferdam System</u>					
Steel sheet piling, PZ-32	46650	sf	8.50	396525	
Structure excavation	3100	cy	3.00	9300	
Structure backfill	4350	cy	10.00	43500	
Dewatering system	1	job	50000	50000	
Subtotal				499325	



PROJECT Florida Avenue Complex - East		Page ___ of ___	COMPUTED BY	DATE
SUBJECT Vertical Lift Gate Structure - Bypass Channel			CHECKED BY	DATE
Item	Estimated Quantity	Unit	Unit Price	Estimated Amount
Steel sheet piling, P2-32 (USS x-Ten 55)	48600	sf	8.50	413100
Steel sheet piling cover plates	5500	lbs.	0.75	4125
Wale channels, C15x40	1434	lf	8.00	11472
Standard pipe, 4" $\phi$	6045	lbs.	0.50	3023
Tie rods, 3/4" $\phi$	26400	lbs.	0.75	19800
Steel H-piling, HPx73	3234	lf	14.00	45276
Pulling steel sheet piling	48600	sf	1.00	48600
Pulling steel H-piling	3234	lf	3.00	9702
Concrete in deadman	80	cy	60.00	4800
Steel reinforcement	11037	lbs.	0.35	3863
Structure excavation	2800	cy	2.50	7000
Structure backfill	3000	cy	3.50	10500
Salvage steel	1260	Ton	30.00	(37800)
Subtotal				543461
Subtotal, Vertical Lift Gate Structure and adjoining structures				1,865,911
Environmental protection (0.005)				9330
Subtotal				1,875,241
Contingencies 25% $\pm$				468,811
Subtotal				2,344,052
Engineering and design 10.8% $\pm$ (based on estimate of actual E&D required)				253,158
Supervision and administration 9% $\pm$ (based on estimate of the actual E&D required)				210,965
Total, Construction for Vertical Lift Gate Structure and adjoining structures				2,808,175



$\frac{4}{100}$

$-\frac{5}{41}$

$-\frac{1}{31}$

$-\frac{5}{20}$   
 $-\frac{11.5}{26}$

$-0.65$

$-\frac{18.5}{0}$

$-\frac{11.5}{26}$

$-\frac{7}{23}$

$-\frac{4}{32}$

$-\frac{2}{32}$

$-\frac{1}{99}$

$\frac{0}{25}$

$1140$

$+\frac{2}{74}$

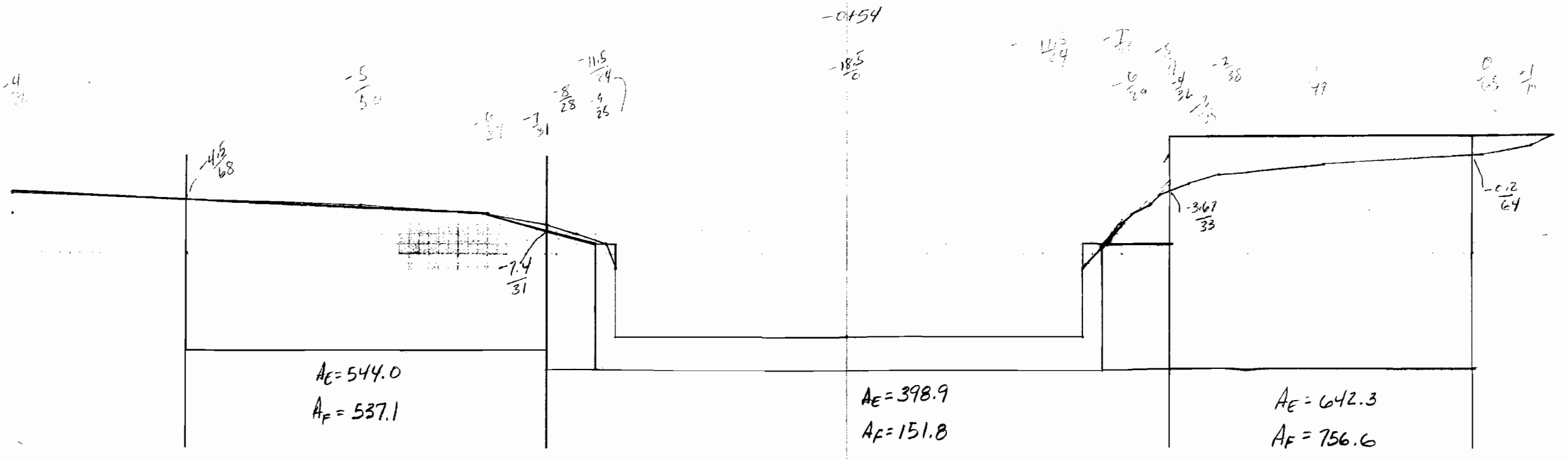
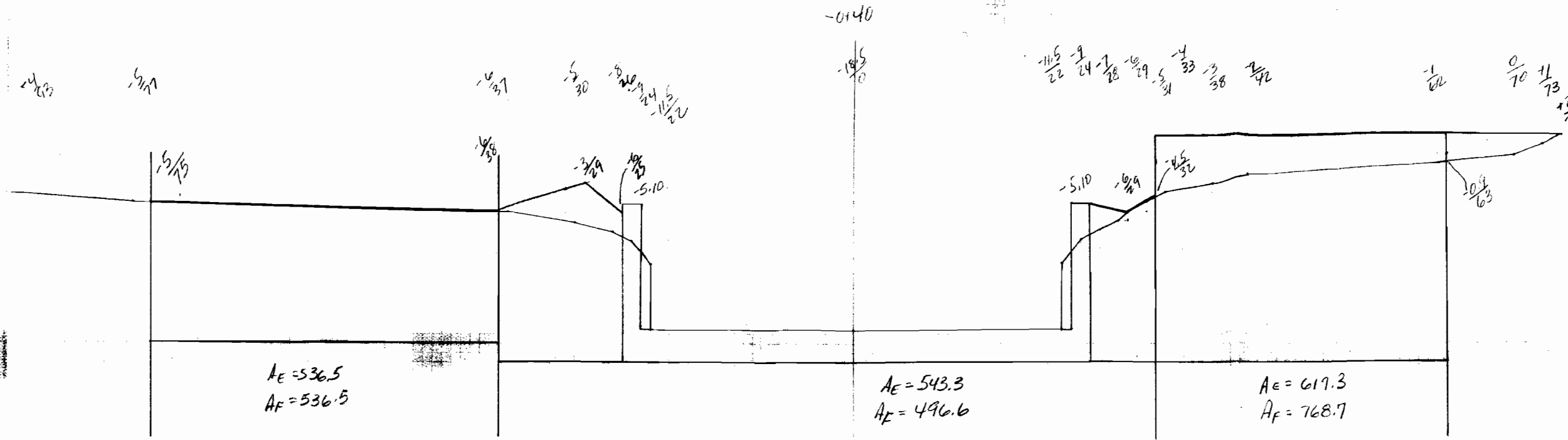
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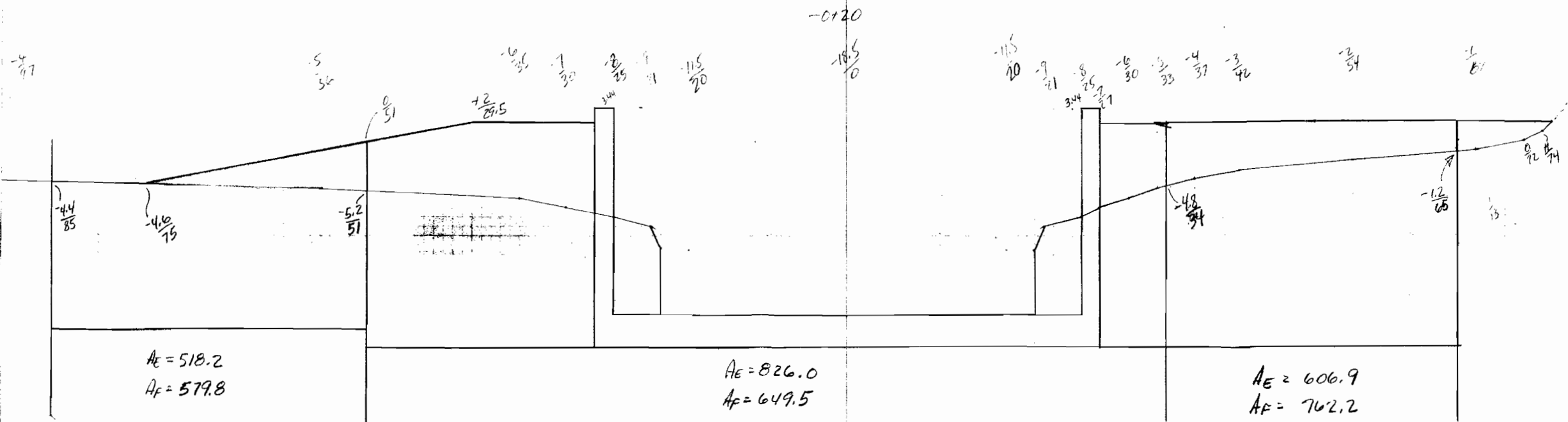
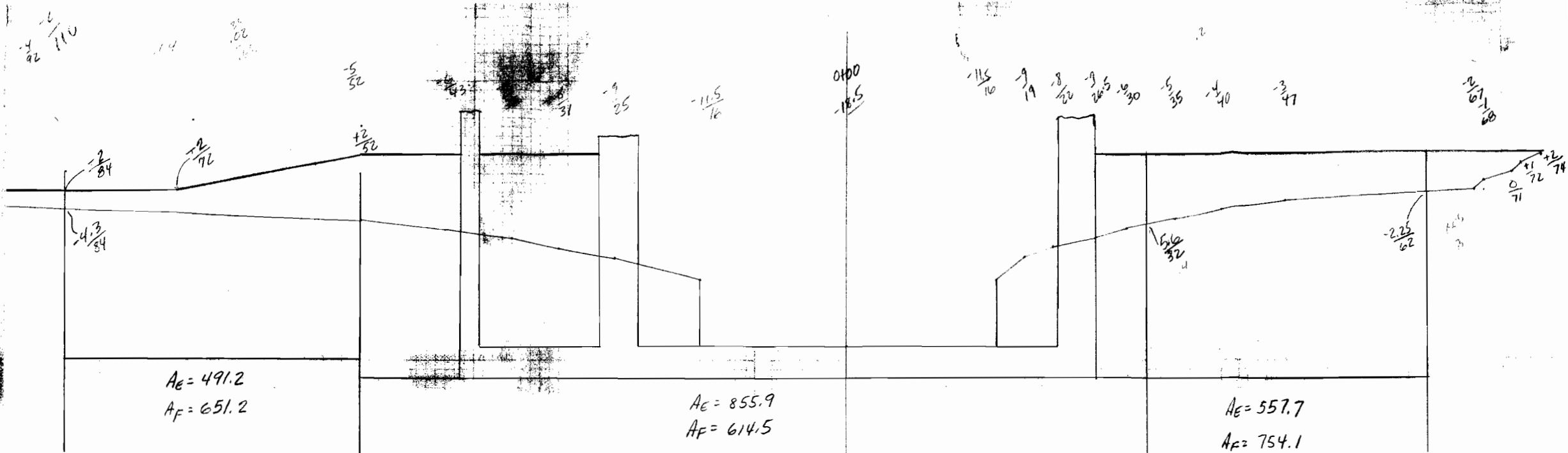
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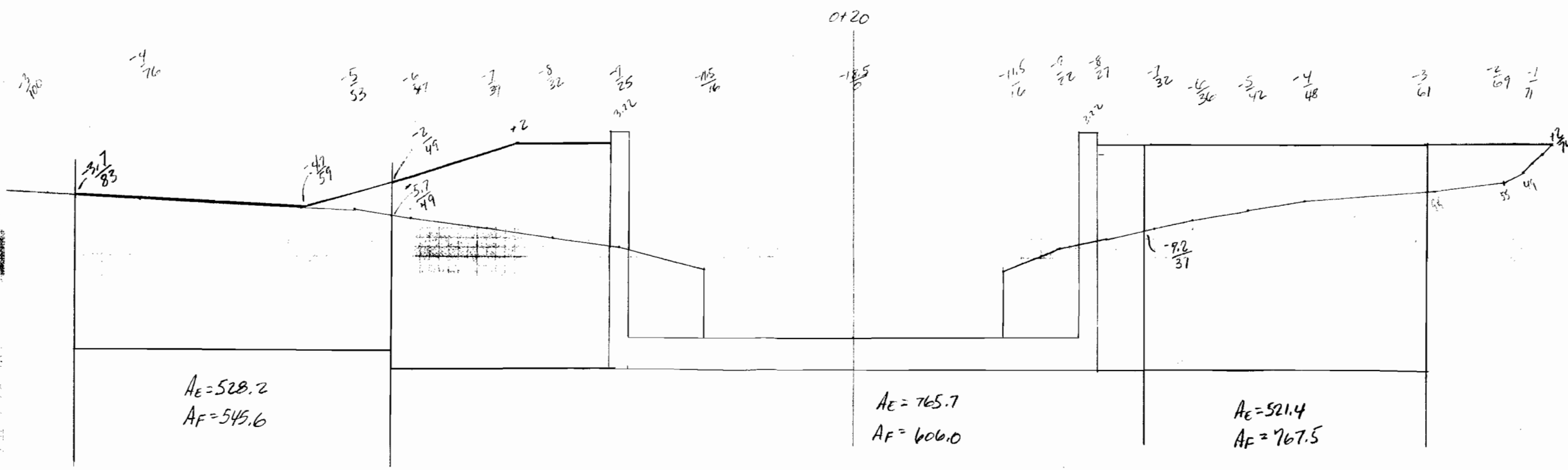
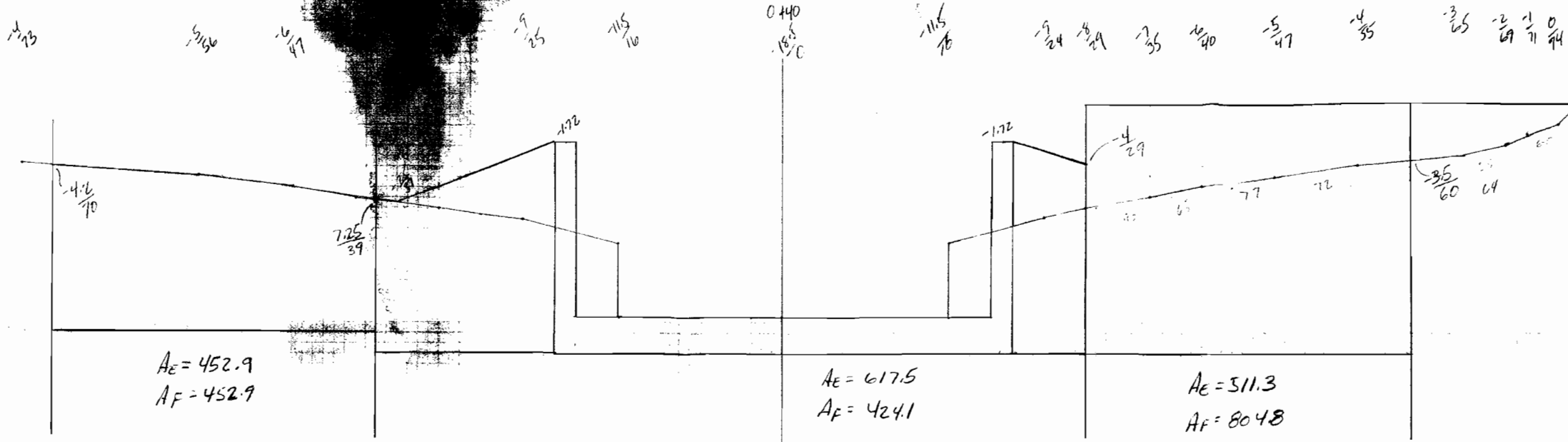
$A_E = 510.8$   
 $A_F = 510.8$

$A_E = 252.5$   
 $A_F = 252.5$

$A_E = 635.2$   
 $A_F = 775.6$







REASONABLE ~~ESTIMATE~~ ESTIMATE

SHEET OF

PROJECT Florida Avenue Complex - East

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
1	<u>Floodwalls, Levees, Gated Structures</u>				
	Structure Excavation	1480	CY	5.00	7400.00
	Structure Backfill	1600	CY	6.00	9600.00
	Backfill Fertilization and Seeding	1.73	acre	400	692.00
	Clearing and Grubbing	2.71	acre	500	1355.00
	Steel Sheet Piling, PMA-22	9100	s.f.	8.00	72,800.00
	Steel Sheet Piling, P2-27	9450	sf.	8.00	75,600.00
	12" X 12" Prestressed Concrete Piling	14500	l.f.	15.00	217,500.00
	Concrete in Stabilization Slab	70	CY	80.00	5600.00
	Concrete in Base Slab	540	CY	80.00	43,200.00
	Concrete in Walls, Columns, Beams	285	CY	150.00	42,750.00
	Portland Cement	4580	cwt	2.50	11,450.00
	Steel Reinforcement	389200	lbs.	0.35	136,220.00
	Bulb-Type Waterstops	390	l.f.	5.00	1950.00
	L-Type Waterstops	45	l.f.	15.00	675.00
	Expansion Joint Filter	825	sf.	1.50	1237.50
	Structural Steel	32500	lbs.	1.75	56,875.00
	Miscellaneous Metal	1600	lbs.	1.50	2400.00
	Gate Seals - J-Type	100	l.f.	20.00	2000.00
	Overhead Trolleys	1	L.S.	500.00	500.00
	Sackrub finish	18950	sf	0.40	7580.00
	Subtotal				\$ 697,384.50

CRS 7 Sent 76

REASONABLE ESTIMATE

SHEET OF

PROJECT

Florida Avenue Complex - East

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
2.	<u>Bypass Channel</u>				
	Structure Excavation	2800	CY	2.50	7000.00
	Structure Backfill	3000	CY	3.50	10500.00
	Steel Sheet Pile, PZ-32 (USS X-Ten '55)	48600	SF	8.50	413100.00
	Steel Sheet Pile Cover Plates	5500	Lb	0.75	4125.00
	Wale Channels, C15 X40	1434	FT	8.00	11472.00
	Standard Pipe, 4" $\phi$	6045	Lb	0.50	3022.50
	Tie Rods, 3/4" $\phi$	26400	Lb	0.75	19800.00
	Steel H-Piles, HP14 X73	3234	FT	14.00	45276.00
	Concrete in Deadmen	80	CY	60.00	4800.00
	Steel Reinforcement	11037	Lb	0.35	3862.95
	Pulling Steel Sheet Piling	48600	SF	1.00	48600.00
	Pulling H-Piling	3234	FT	3.00	9702.00
	Salvage Steel	1260	Ton	30.00	37800.00
	Subtotal				619060.45



PROJECT

Florida Avenue Complex - East

INVITATION NO.

ITEM NO.

DESCRIPTION

ESTIMATED QUANTITY

UNIT

UNIT PRICE

ESTIMATED AMOUNT

3

Cofferdam System

Structure Excavation

3100

CY

3.00

9300.00

Structure Backfill(Shell)

4350

CY

10.00

43500.00

Steel Sheet Pile, PZ-32

46650

SF

8.50

396,525.00

Dewatering system

1

Job

50,000.00

Subtotal

499,325.00

PROJECT

Florida Avenue Complex - East

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
4	<u>Vertical Lift Gate Structure</u>				
	Structure Excavation	3300	CY	3.00	9900.00
	Structure Backfill	2700	CY	4.00	10,800.00
	Steel Sheet Piling, PMA-22	2,150	SF	8.00	17,200.00
	Concrete in Base Slab	300	CY	80.00	24,000.00
	Concrete in Walls (E1-18.5 to +2)	251	CY	140.00	35,140.00
	Concrete in Deck Slab	91	CY	180.00	16,380.00
	Concrete in Walls (E1 +2 to +27)	147	CY	140.00	20,580.00
	Concrete in Floodwalls	23	CY	140.00	3,220.00
	Concrete in Stairs	4	CY	200.00	800.00
	Concrete in Machine Room Slab	64	CY	80.00	5,120.00
	Concrete in Machine Room Walls	41	CY	140.00	5,740.00
	Concrete in Roof Slab	17	CY	200.00	3,400.00
	Portland Cement	4850	CWT	2.50	12,125.00
	Steel Reinforcement	181,700	Lb	0.35	63,595.00
	14" x 14" Prestressed Concrete Piling	6,000	LF	16.00	96,000.00
	Structural Steel	72,600	Lb	1.75	127,050.00
	Miscellaneous Metal	91,440	Lb	1.50	137,160.00
	Sackrub Finish	5,750	SF	0.40	2,300.00
	Overhead Trolleys	4	Lb	500.00	2,000.00
	Subtotal				595,165.00

PROJECT

Florida Avenue Complex - East

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
5	<u>Approach Structures</u>				
	Concrete in Stabilization Slab	85	CY	80.00	6,800.00
	Concrete in Base Slab	845	CY	80.00	67,600.00
	Concrete in Walls	215	CY	140.00	30,100.00
	Portland Cement	5880	CWT	2.50	14,700.00
	14" x 14" Prestressed Concrete Piles	2600	LF	16.00	41,600.00
	Steel Reinforcement	106200	LB	0.35	37,170.00
	Sackrub Finish	2950	SF	0.40	1,180.00
	Expansion Joint Filler	775	SF	0.50	1,162.50
	Bulb-type Waterstops	200	LF	5.00	1,000.00
	L-type Waterstops	130	LF	20.00	2,600.00
	Subtotal				203,912.50

Florida Avenue Complex  
 For 1 - Machinery Assembly

Price List (operating Machinery)

1 - Magnetic Shoe Brake	-----	\$ 400. <sup>00</sup>
2 - Electric motor 10HP	-----	450. <sup>00</sup>
3 - Load Brake	-----	1400. <sup>00</sup>
4 - Speed Reducer	-----	5000. <sup>00</sup>
5 - Drum	-----	2500. <sup>00</sup>
6 - wire rope	-----	250. <sup>00</sup>
7 - Couplings	-----	950. <sup>00</sup>
8 - Bearings & shafting	-----	550. <sup>00</sup>
9 - Pulleys	-----	800. <sup>00</sup>
10 - Air Cylinders and bearings	-----	600. <sup>00</sup>
11 - Air Compressor Unit	-----	450. <sup>00</sup>

\$ 13,350.<sup>00</sup>

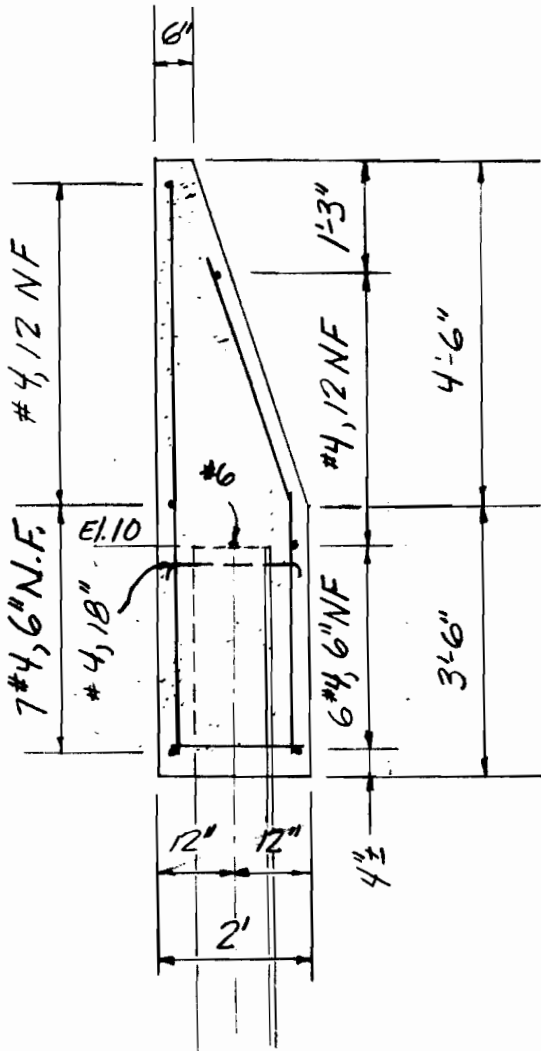
25% Contingency 3,338.<sup>00</sup>

Total Cost \$ 16,688.<sup>00</sup>  
 excluding labor for  
 installation

prices do not include labor  
 100% Labor

10:8 (S-D)  
 9:0 - 5-A





Length = 64'-2"  
 ~ 5 joints, 3 transitions  
 from I-Wall to T-Wall ~

Concrete ~

$$\begin{aligned}
 \text{Volume} &= [0.5(4.5) + \frac{1}{2}(1.5)(4.5) \\
 &\quad + 3.5(2)] 60.2 \\
 &= (13.75 \text{ sf}) 60.2 \\
 &= 827.8 \text{ cf} = 30.66 \text{ CY} \\
 &\quad (\text{Use } 32 \text{ CY})
 \end{aligned}$$

Cement ~

$$\begin{aligned}
 \text{Volume} \times 5.17 \frac{\text{cwt}}{\text{CY}} &= \\
 &= 158.5 \text{ cwt} \quad (\text{Use } 160 \text{ cwt})
 \end{aligned}$$

Reinforcement ~

$$\begin{aligned}
 &(3 + 6 + 7 + 3)(60.2)(.668) + 1.5(60.2)(.668) \\
 &\quad \text{No. bars/ft} \times \text{Length} \times \text{WT/ft} \\
 &+ \frac{2}{3}(60.2)(.668) + 60.2(1.502) = 941.6 \text{ lb} \\
 &\quad (\text{Use } 950 \text{ lb.})
 \end{aligned}$$

PROJECT	Florida Avenue Complex - West	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	I-Wall-Sheet Piling, Waterstops		RJR	9 Aug 76
			CHECKED BY	DATE

Steel Sheet Piling ~

w/L sta 0+00 to sta 0+40 -

$$40 \times 20' = 800 \text{ sf}$$

w/L sta 8+09.5 to sta 8+33.7 -

$$24.2 \times 30 = 726 \text{ sf} , \quad \text{Total} = 1526 \text{ sf}$$

(Use 1530 sf)

Waterstops, L-type ~

w/L sta 0+00 to sta 0+40 -

$$\text{El. 15 to El. 6 (sta. 0+00)} = 9'$$

$$\text{El. 15 to El. 0.5 (sta 0+40)} = 14.5'$$

w/L sta 8+09.5 to sta 8+33.7 -

$$\text{El. 15 to El. 7 (sta 8+09.5)} = 8'$$

$$\text{Total} = 31.5' \text{ (Use 35')}$$

Waterstops, 3-bulb type -

w/L sta 0+00 to sta 0+40 -

$$\text{El. 14.5 to El. 7.25 (sta. 0+20)} = 7.25'$$

$$\text{Total} = 7.25' \text{ (Use 9')}$$

PROJECT	Florida Avenue Complex - West	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	I-Wall - Expansion joint Material		KJR	9 Aug 76
			CHECKED BY	DATE

Expansion joint material -

$$(13.75 \text{ sf}) 5 = 68.75 \text{ sq ft (Use 70 sf)}$$



PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Steel Sheet Piling - East		RJR	15 Sep 76
			CHECKED BY	DATE

Steel Sheet Piling - East

Station	Tip Elevation	Top Elevation	Area
0+00 - 0+60 (P2-27)	(Existing)	—	—
0+60 - 1+40.35	-25	+2.25	1788
1+40.35 - 2+62.5	-25	-1.50	2871
2+62.5 - 4+73.81	-25	-4.75	4279
4+63.81 - 5+51.81	-40	-21.25	1650
5+40.31 - 5+77.49	-25	-4.75	753
5+77.49 - 5+94.99	-25	-1.75	407
5+94.99 - 6+50.49	-25	+1.25	1457
		Total =	13205 sf
			(Use 13250 sf)
P2-27			
6+50.49 - 8+77	-26	+10.0	8154
			(Use 8200 sf)

**REASONABLE CONTRACT ESTIMATE**

SHEET OF

PROJECT **T-Wall - West**

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
	<u>Concrete:</u>				
	Wall	300	cy		
	Base Slab	420	cy		
	Stabilization Slab	60	cy		
	<u>Reinforcement</u>	84000	lb		
	Concrete Prestressed Piles	11400	lf		
	<del>Sheet Piling, PMA-22</del>	<del>12000</del>	<del>sf</del>		
	Cement	4035	cwt		
	Water stops, 3-bulb type	330	lf		
	Construction It. Matl	530	sf		

PROJECT	Florida Avenue Complex-West	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	T-Wall - Concrete		RJR	9 Aug 76
			CHECKED BY	DATE

Base Slab ~

w/L sta 0+40 to sta 3+79.71:

$$2.5(8)(337.71) = 6794.2 \text{ cf} = 251.6 \text{ CY}$$

w/L sta 4+60.46 to sta 5+55.99:

$$2.5(8)(95.53) = 1910.6 \text{ cf} = 70.8 \text{ CY}$$

w/L sta 6+67.21 to sta 6+97.5:

$$2.5(8)(28.29) = 565.8 \text{ cf} = 21.0 \text{ CY} \times$$

w/L sta 7+84.5 to sta. 8+09.5:

$$2.5(8)(25) + 2(2.5)(8)(2) = 580 \text{ cf} = 21.5 \text{ CY}$$

w/L sta. 5+55.99 to sta 6+02.71:

$$11(2.5)(46.72) = 1284 \text{ cf} = 47.6 \text{ CY} \times$$

$$\text{Total} = 412.5 \text{ CY}$$

(Use 420 CY)

Walls ~

w/L sta 0+40 to sta 0+98.09:

$$\left[ 1(11) + \frac{11^2}{24} (1/2) \right] (58.09) = 785.4 \text{ cf} = 29.1 \text{ CY}$$

w/L sta 0+98.09 to sta 3+79.71:

$$\left[ 1(12) + \frac{12^2}{24} (1/2) \right] (281.62) = 4224.3 \text{ cf} = 156.5 \text{ CY}$$

w/L sta 4+60.46 to sta. 5+57.99:

$$\left[ 1(10) + \frac{10^2}{24} (1/2) \right] (97.53) = 1178.5 \text{ cf} = 43.6 \text{ CY}$$

x w/L sta 5+57.99 to 6+02.71:

$$\left[ 1(13) + \frac{13^2}{24} (1/2) \right] (44.72) = 738.8 \text{ cf} = 27.4 \text{ CY}$$

PROJECT	Florida Avenue Complex-West	Page — of —	COMPUTED BY	DATE
SUBJECT	T-Walls - Concrete		RJgr	9 Aug 76
			CHECKED BY	DATE

Walls ~

f w/L sta. 6+69.21 to sta 6+97.5:

$$\left[ 1(13) + \frac{13^2}{24} \left(\frac{1}{2}\right) \right] 28.29 = 467.4 \text{ cf} = 17.3 \text{ CY}$$

w/L sta. 7+84.5 to sta 7+97:

$$\left[ 1(10) + \frac{10^2}{24} \left(\frac{1}{2}\right) \right] 12.5 = 151.0 \text{ cf} = 5.6 \text{ CY}$$

w/L sta. 7+97 to sta. 8+09.5:

$$\left[ 1(7.5) + \frac{7.5^2}{24} \left(\frac{1}{2}\right) \right] 12.5 = 108.4 \text{ cf} = 4.0 \text{ CY}$$

Total = 283.5 CY  
(Use 300 CY)

Stabilization slab ~

w/L sta. 0+40 to sta 5+55.99

and 6+97.5

w/L sta 6+69.21 to sta 8+09.5:

$$\frac{4}{12} (8)(488.53) = 1302.75 \text{ cf} = 48.24 \text{ CY}$$

+ w/L sta 5+55.99 to sta 6+02.71:

$$\frac{4}{12} (11)(46.72) = 171.3 \text{ cf} = 6.3 \text{ CY}$$

Total = 54.6 CY  
(Use 60 CY)

PROJECT	Florida Avenue Complex-West	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	T-Wall-Cement		RJR	9 Aug 76
			CHECKED BY	DATE

Cement ~

$$\text{Vol. of concrete} = 780 \text{ CY}$$

$$\text{Volume of cement} = 780 \times 5.17 = 4032.6 \text{ cwt}$$

Use 4035 cwt

PROJECT	Florida Avenue Complex - West	Page — of —	COMPUTED BY	DATE
SUBJECT	PMA-22 Steel Sheet Piling		RJGr	23 Aug 76
			CHECKED BY	DATE

w/l sta 0+40 to w/l sta 1+00 :

Top sheet piling = El. +1.25

Bottom sheet piling = El. -20.

$$A = 60 \times 21.25 = 1275 \text{ sq ft}$$

w/l sta 1+00 to w/l sta 3+79.7 :

Top sheet piling = El. +0.25

Bottom sheet piling = El. -20

$$A = 279.7 \times 20.25 = 5663.92 \text{ sq ft}$$

w/l sta 4+60.46 to w/l sta 5+55.47:

Top sheet piling = El. +2.25

Bottom sheet piling = El. -25

$$A = 95.01 \times 27.25 = 2589.02 \text{ sq ft}$$

x w/l sta 5+55.47 to w/l sta 6+02.71:

Top sheet piling = El. -0.75

Bottom sheet piling = El. -25

$$A = 47.24 \times 24.25 = 1145.57$$

x w/l sta 6+69.21 to w/l sta 6+97.50:

Top sheet piling = El. -0.75

Bottom sheet piling = El. -25

$$A = 28.29 \times 24.25 = 686.03 \text{ sq ft}$$

PROJECT Florida Avenue Complex - West	Page ___ of ___	COMPUTED BY RJR	DATE 23 Aug 76
SUBJECT PMA-22 Steel Sheet Piling		CHECKED BY	DATE

Continued —

W/L sta 7+84.50 to W/L sta. 7+99.50:

Top sheet piling = El. 2.25

Bottom sheet piling = El. -20.

$$A = 22.25 \times 15 = 333.75 \text{ sq ft}$$

W/L sta 7+99.5 to W/L sta 8+09.50:

Top sheet piling = El. 4.75

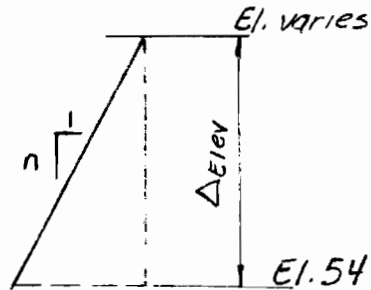
Bottom sheet piling = El. -20

$$A = 24.75 \times 10 = 247.5 \text{ sq ft}$$

Total Area = 11940.79 sq ft

(Use 12000 sq ft)

Concrete Prestressed Piling



$$\text{Pile Length} = \Delta_{\text{Elev}} \sqrt{1 + 1/n^2}$$

Monolith	Δ Elev	No Piles	Batter	Pile Length	Total
3'	54.5	15	2:1	61'	915'
		1	4:1	56'	56'
4.5	53.5	32	2:1	60'	1920'
		8	0	53.5'	428'
6'	53.5	18	2:1	60'	1080'
		2	0	53.5	107'
8	53.5	16	2:1	60'	960'
		3	4:1	55.5'	167'
11	55.5	8	2:1	62'	496'
		4	2.5:1	60'	240'
12	55.5	17	2:1	62'	1054'
		1	2.5:1	60'	60'
+13	52.5	10	2:1	59'	590'
		1	2.5:1	57'	57'
		1	3:1	55'	55'
7	53.5	14	2:1	60'	840'
		4	0	53.5	214'



PROJECT Florida Avenue Complex - West

Page \_\_\_ of \_\_\_

COMPUTED BY

DATE

RJR

9 Aug 76

SUBJECT T-Wall - Concrete Piling

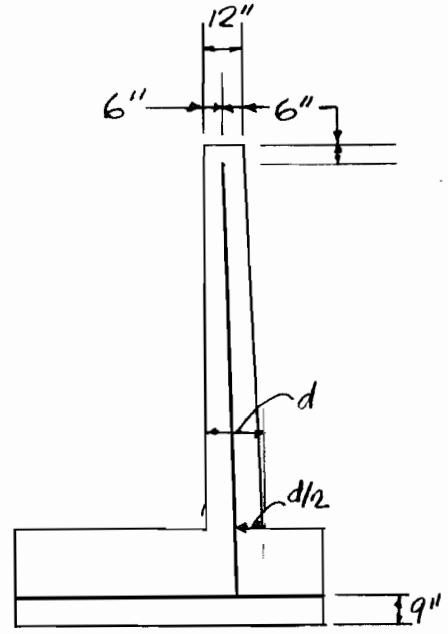
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Monolith	Area	No Piles	Batter	Pile Length	Total
x 14	52.5	14	2:1	59'	826'
x 16	52.5	6	2:1	59'	354'
		6	4:1	54'	324'
18	55.5	6	2:1	62'	372'
19	58	4	2:1	65'	260'

Total = 11375 lf

Waterstops, 3-Bulb Type 2



Typical 3-Bulb Waterstop Detail

w/L sta 0+95.59;  $L = 11' + 2.5' - 0.75' + 8' = 20.75'$

w/L sta 1+57.09, w/L sta. 2+18.59,

w/L sta 2+74.09, w/L sta. 3+79.72:

$$4L = 4(12 + 2.5 - 0.75 + 8) = 87.0'$$

w/L sta. 4+16.47, w/L sta. 4+63.47,

w/L sta. 5+10.97, w/L sta 5+55.47:

*L-type*

$$4L = 4(10 + 2.5 - 0.75 + 8) = 79.0'$$

x w/L sta. 5+77.74, w/L sta 6+02.74:

$$2L = 2(13 + 2.5 - 0.75 + 11) = 51.5'$$

PROJECT	Florida Avenue Complex - West	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	T-Wall - Waterstops		RJR	9 Aug 76
			CHECKED BY	DATE

$$\times \text{W/L sta } 6+69.21: L = 13 + 1.25 + 8 = 22.25'$$

$$\text{W/L sta } 6+97.50: L = 12 + 2.25 + 11 = 25.25'$$

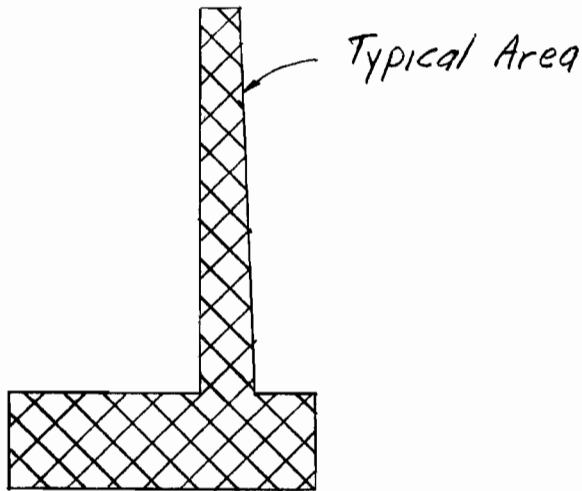
$$\text{W/L sta } 7+84.5: L = 12 + 2.25 + 11 = 25.25'$$

$$\text{W/L sta } 7+99.5: L = 7.5 + 1.75 + 8 = 17.25'$$

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$$\text{Total} = 328.25'$$

(Use 330')

Construction Joint Material ~

$$\text{w/L sta } 0+95.59: A = \frac{1}{2}(11)\left[2 + \frac{11}{24}\right] + 8(2.5) = 33.5 \text{ sf}$$

$$\text{w/L sta. } 1+57.09, \text{ w/L sta } 2+18.59, \text{ w/L sta } 2+74.09:$$

$$3A = \left[\frac{1}{2}(12)\left(2 + \frac{12}{24}\right) + 20\right]3 = 105 \text{ sf}$$

$$\text{w/L sta } 3+79.72: A = \frac{1}{2}(7.8)\left(2 + \frac{7.8}{24}\right) + 20 = 29.1 \text{ sf}$$

$$\text{w/L sta } 4+16.47, \text{ w/L sta } 4+63.47, \text{ w/L sta } 5+10.97,$$

$$\text{w/L sta } 5+55.47: 4A = \left[\frac{1}{2}(10)\left(2 + \frac{10}{24}\right) + 20\right]4 = 128.3 \text{ sf}$$

$$\times \text{ w/L sta } 5+77.74, \text{ w/L sta } 6+02.74:$$

$$2A = \left[\frac{1}{2}(13)\left(2 + \frac{13}{24}\right) + 11(2.5)\right]2 = 88.1 \text{ sf}$$

$$\times \text{ w/L sta } 6+69.21, \text{ w/L sta } 6+97.5:$$

$$\vee 2A = \left[\frac{1}{2}(13)\left(2 + \frac{13}{24}\right) + 8(2.5)\right]2 = 73.1 \text{ sf}$$

PROJECT	Florida Avenue Complex-West	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	T-Wall-Construction Joint Material		RJR	10 Aug 76
			CHECKED BY	DATE

$$w/l \text{ sta } 7+84.5 : A = \frac{1}{2}(12)(2 + \frac{12}{24}) + 8(2.5) = 35 \text{ sf}$$

$$w/l \text{ sta } 7+99.5 : A = \frac{1}{2}(7.5)(2 + \frac{7.5}{24}) + 8(2.5) = \underline{28.7 \text{ sf}}$$

$$\text{Total} = 520.8 \text{ sf}$$

(Use 530 sf)

PROJECT	Florida Avenue Complex.	Page — of —	COMPUTED BY	DATE
SUBJECT	Reinforcement, T-Wall - West		RJA	10 Aug 76
			CHECKED BY	DATE

Monolith No.	Monolith		Transverse		Longitudinal		Vertical		Height	Horizontal	
	L	W	Top	Bottom	Top	Bottom	F.S.	B.S.		F.S.	B.S.
3	56.83	8'	#8,12"	#8,12"	15-#6	15-#6	#6,6"	#6,12"	11'	#5,12	
4	64.5	8'	#8,12"	#8,12"	8-#6	8-#6	#6,6"	#6,12"	12'	#5,12	
5	61.5	8'	#8,12	#8,12"	8-#6	8-#6	#6,6"	#6,12"	12'	#5,12	
6	57.5	8'	#8,12	#8,12"	15-#6	15-#6	#6,6"	#6,12"	12'	#5,12	
7	50.0	8'	#8,12	#8,12"	8-#6	8-#6	#6,6"	#6,12"	12'	#5,12	
8	55.8	8'	#8,12	#8,12"	15-#6	15-#6	#6,6"	#6,12"	12'	#5,12	
11	47.5	8'	#8,12	#8,12"	8-#6	8-#6	#7,12"	#6,12"	10'	#5,12	
12	39.5	8	#8,12	#8,12"	15-#6	15-#6	#7,12"	#6,12"	10'	#6,12	
13	21.75	11	#9,12	#8,12"	10-#6	10-#6	#7,6"	#6,12	13'	#6,12	
14	24	11	#8,12	#9,12	10-#6	10-#6	#7,6"	#6,12	13'	#6,12	
16	28.3	8	#8,12	#8,12	15-#6	15-#6	#7,6"	#6,12	13'	#5,12	
18	15.45	8	#8,12	#8,12	15-#6	15-#6	#7,12"	#6,12	10'	#5,12	
19	10'	8'	#8,12	#8,12	9-#6	9-#6	#6,12"	#6,12	7.5'	#5,12	

53.75

$$\#5 - 22 \times 56.83' \times 1.043 = 1304$$

$$10 \times 12 \times (61.5' + 61.5' + 57.5' + 50' + 55.8') \times 1.043 = 35833$$

$$6 \times 10 \times (47.5' + 39.5' + 15.45') \times 1.043 = 6411$$

$$26 \times 28.3' \times 1.043 = 767$$

$$15 \times 10' \times 1.043 = 156$$

$$\#6 - 272 \times 529.63' \times 1.502 = 216377$$

$$530 \times 180' \times 1.502 = 143291$$

$$687 \times 86' \times 1.502 = 88741$$

$$10 \times 10' \times 1.502 = 150$$

$$72 \times 85.25' \times 1.502 = 9219$$

$$\#7 - 251 \times 84' \times 2.044 = 43096$$

$$\#8 - 484 \times 220' \times 2.67 = 284302$$

$$46 \times 26' \times 2.67 = 3193$$

$$\#9 - 46 \times 26' \times 3.4 = 4066$$

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$$\text{Total} = 836906 \text{ lb.}$$

(Use 840000 lb)

**ESTIMATE**

SHEET OF

PROJECT: Florida Avenue Complex - Swing Gate, Harbor Rd, West

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
	Concrete				
	1 <sup>st</sup> pour - Base Slab	28.0	CY	80	<del>2240</del> 2240
	2 <sup>nd</sup> pour - Base Slab	1.0	CY	80	80
	Step - Base Slab	1.0	CY	80	80
	Stabilization Slab	4.0	CY	80	320
	Columns	4.0	CY	150	600
	Walls	0.5	CY	150	750
	Cement	200.0	CWT	2.50	500
	Reinforcement	4800	lb	0.35	1680
	Structural Steel	—			
	Gates	10800	lb	1.75	18900
	Embedded steel	600	lb	1.75	1050
	Concrete Piling	1019	cf	15	15285
	Steel Sheet Piling, PMA-22	1050	sf	8	8400
					<hr/>
					47210



Concrete ~

$$\begin{aligned} 1^{\text{st}} \text{ pour, Base Slab} &= 2.5 \times 8 \times 36.75 / 27 \\ &= 27.2 \text{ CY} \end{aligned}$$

$$\begin{aligned} 2^{\text{nd}} \text{ pour, Base Slab} &= 0.5 \times 1. \times 36.75 / 27 \\ &= 0.7 \text{ CY} \end{aligned}$$

$$\begin{aligned} \text{Step} &= 22 \times 1.5 \times 8 / 27 \\ &= 1.0 \text{ CY} \end{aligned}$$

$$\begin{aligned} \text{Stabilization Slab} &= 4/2 \times 8 \times 36.75 / 27 \\ &= 3.6 \text{ CY} \end{aligned}$$

$$\begin{aligned} \text{Walls \& Columns} &= \left[ (2 \times 2 \times 8.55) + \right. \\ &\quad (2 \times 2 \times 14.8) + \\ &\quad (0.5 \times 1 \times 7.8) + 0.5 \left( \frac{7.8^2}{48} \right) + \\ &\quad (0.75 \times 1 \times 7.8) + \\ &\quad \left. (0.75) \left( \frac{7.8^2}{48} \right) \right] / 27 \\ &= 3.9 \text{ CY} \end{aligned}$$

$$\text{Cement} = 36.4 \times 5.17 = 188.2 \text{ cwt}$$

Reinforcement ~

Base Slab - Top, Transverse:

36 #8 from 0' - 8'

Base Slab - Bottom, Transverse:

36 #8 from 0' - 8'

Base Slab - Top ; Bottom, Longitudinal:

16 #8 from 0' - 36.75'

Hinged Column:

22 #9 from E16.2 to E114.0

16 #10 from E1.14 to E1.21

Column:

16 #10 from E1.6.2 to E1.14.75

$$\#10 - 16 \times 7 \times 4.303 = 481.9 \text{ lb}$$

$$\#10 - 16 \times 7.8 \times 4.303 = 537.0 \text{ lb}$$

$$\#9 - 22 \times 7.8' \times 3.4 = 583.4 \text{ lb}$$

$$\#8 - 72 (8') \times 2.67 = 1537.9 \text{ lb}$$

$$16 \times 36.75 \times 2.67 = 1570.0 \text{ lb}$$

$$\text{Total} = 4710.2 \text{ lb}$$

(Use 4800 lb)

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Structural Steel, Concrete Piling, Sheet Piling		RJ9r	24 Aug 76
			CHECKED BY	DATE

Structural Steel ~

Gate Weight = 10800 lb  
 Other Steel = 600 lb

Concrete Prestressed Piles ~

12 Battered Piles at 65.4' = 785 lf  
 4 Vertical Piles at 58.5' = 234 lf  
 1019 lf

Steel Sheet Piles ~

El. 3.7 to El. -25 = 28.7 use 28'  
 Length = 36.75'

Total Area = 1029 sf

(Use 1050 sq ft)

**ESTIMATE**

SHEET OF

**PROJECT**

Florida Avenue Complex - Swing Gate, Railroad, West IHNC

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
	<u>Concrete</u>				
	Stabilization Slab	5	CY		
	Base Slab	37	CY		
	Wall	5	CY		
	Columns	84.5	CY		
	Sill	7.5	CY		
	Structural Steel	—			
	Gate	9000	lb		
	Embedded Steel	1000	lb		
	Reinforcement	10800	lb		
	Conc. Prestressed Piles	1250	LF		
	Steel Sheet Piles, PMAD22	1350	SF		
	Cement	305	cwt		
	<u>Support Pedestal</u>				
	Concrete				
	Base Slab	0.4	CY		
	Wall	0.8	CY		
	Reinforcement				
	Precast Concrete Pile	56	LF		
	Jack				

Concrete ~

$$\begin{aligned} \text{Base Slab} &= (49' \times 2.5' \times 8') / 27 \\ &= 36.3 \text{ CY} \end{aligned}$$

$$\begin{aligned} \text{Stabilization Slab} &= (49' \times \frac{4}{12}' \times 8') / 27 \\ &= 4.8 \text{ CY} \end{aligned}$$

$$\begin{aligned} \text{Walls} &= [(6.5' \times 1' \times 10') + (1.5' \times 3' \times 10') + \\ &\quad (0.5' \times 1.5' \times 10') + \frac{1}{2} \left( \frac{10^2}{24} \right) 6.5] / 27 \\ &= 4.9 \text{ CY} \end{aligned}$$

$$\begin{aligned} \text{Hinged Column} &= (2' \times 2' \times 17') / 27 \\ &= 2.5 \text{ CY} \end{aligned}$$

$$\begin{aligned} \text{Column} &= (2' \times 2' \times 10.75') / 27 \\ &= 1.6 \text{ CY} \end{aligned}$$

$$\begin{aligned} \text{Sill} &= (3.53' \times 1.5' \times 34.33') / 27 \\ &= 6.7 \text{ CY} \end{aligned}$$

PROJECT	Florida Avenue Complex	Page — of —	COMPUTED BY	DATE
SUBJECT	Concrete, Cement		RJgr	25 Aug 76
			CHECKED BY	DATE

$$\begin{aligned} \text{Pedestal} &= (1.67' \times 1.67' \times 3.53') / 27 \\ &= 0.4 \text{ CY} \end{aligned}$$

$$\text{Cement} = 57.2 \times 5.17 = 295.7 \text{ cwt}$$

PROJECT Florida Avenue Complex	Page — of —	COMPUTED BY RJgr	DATE 25 Aug 76
SUBJECT Reinforcement		CHECKED BY	DATE

Reinforcement ~

Base Slab, Top; Bottom, Transverse

#9, 12" from 0'-8"

Base Slab, Top and Bottom, Longitudinal

16-#9 from 0'-49'

Columns,

16-#11 from El. 1.5 to El. 21

16-#11 from El. 1.5 to El. 14.75

Walls, Both sides, Horizontal

#6, 12 from El. 4.0 to El. 14.0  
(12' x 2 sides)

Vertical,

#8-12" from 0-12'

#7-12" from 0-12"  
(El. 1.5 to El. 14.0)

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Reinforcement		RJR	25 Aug 76
			CHECKED BY	DATE

Sill,

8-#6 0-36'

#6-12" , at 8.54' each

$$\#6 - 8 \times 36' \times 1.502 = 432.6$$

$$36 \times 8.54' \times 1.502 = 462.4$$

$$20 \times 12' \times 1.502 = 360.5$$

$$\#7 - 12 \times 12.5' \times 2.044 = 306.6$$

$$\#8 - 12 \times 12.5 \times 2.67 = 400.5$$

$$\#9 - 49 \times 20' \times 3.4 = 3332.0$$

$$16 \times 49' \times 3.4 = 2665.6$$

$$\#11 - 16 \times 19.5' \times 5.313 = 1657.9$$

$$16 \times 13.25' \times 5.313 = 1126.4$$

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$$\text{Total} = 10744.3 \text{ lb}$$

(Use 10800 lb)



PROJECT Florida Avenue Complex	Page — of —	COMPUTED BY RJR	DATE 25 Aug 76
SUBJECT Concrete, Cement (Support Pedestal)		CHECKED BY	DATE

Support pedestal ~

$$\begin{aligned} \text{Concrete, Base} &= (2' \times 1.75' \times 2') / 27 \\ &= 0.3 \text{ CY} \quad (\text{Use } 0.3 \text{ CY}) \end{aligned}$$

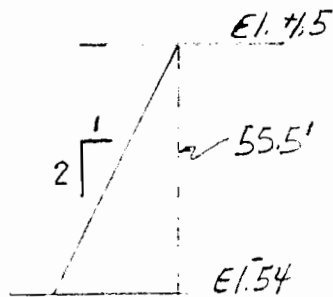
$$\begin{aligned} \text{2nd Pour} &= (1' \times 2' \times 0.5) / 27 \\ &= 0.1 \text{ CY} \quad (\text{Use } 0.1 \text{ CY}) \end{aligned}$$

$$\begin{aligned} \text{Wall} &= (1.25' \times 8.14' \times 2') / 27 \\ &= 0.8 \quad (\text{Use } 0.8 \text{ CY}) \end{aligned}$$

~~Reinforcement =~~

$$\text{Cement} = 1.2 \times 5.17 = 6.2 \text{ cwt (use } 6.5 \text{ cwt)}$$

Concrete Prestressed Piling ~



$$\begin{aligned} \text{Battered Pile Length} &= \sqrt{(55.5)^2 + (27.75)^2} \\ &= 62.05' \text{ (Use } 62') \end{aligned}$$

$$\begin{aligned} 13 \text{ Battered Piles @ } 62' &= 806' \\ 8 \text{ Vertical Piles @ } 55.5' &= \underline{444} \\ &1250 \text{ l.f.} \end{aligned}$$

Steel Sheet Piling:

$$\begin{aligned} \text{Top sheet piling} &= \text{El. } 72.25 \\ \text{Bottom sheet piling} &= \text{El. } 25 \end{aligned}$$

$$\begin{aligned} \text{Area} &= 49 \times 27.25 = 1335.25 \text{ sq ft} \\ &\text{(Use } 1350 \text{ sq ft)} \end{aligned}$$

**~~REASONABLE CONTRACT ESTIMATE~~**

SHEET      OF

PROJECT

Vertical Lift Gate Structure, West

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
	<u>Concrete:</u>				
	Base Slab (-24 to E1-18)	300	CY		
	Walls (E1-18.5 to E1+2)	251	CY		
	Deck Slab (E1 2 to E1 4.0)	91	CY		
	Walls (E1 2 to E1 27)	147	CY		
	Flood Walls (E1 2 to E1 14)	23	CY		
	Stairs	4.0	CY		
	Machine Rm Slab (E1.30)	64	CY		
	Machine Rm Walls	41	CY		
	Roof Slab (E1.40)	17	CY		
	<u>Cement:</u>	4850	CWT		
	Machinery (Hoists - Gate)		2EA		
	Lighting		L.S.		
	Nailing				
	Protective Sealing		L.S.		
	Cathodic Protection		L.S.		
	<u>Structural Steel:</u>				
	Embedded	21300	lb		
	Exposed	70140	lb		
	Gates	72600	lb		
	Reinforcement	181700	lb		
	Concrete Prestressed Piles <sup>14x14</sup>	6000	LF		
	Miscellaneous		L.S.		
	Steel Sheet Piling, PMA-LU	2150	SF		

**REASONABLE CONTRACT ESTIMATE**

SHEET OF

PROJECT

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
----------	-------------	--------------------	------	------------	------------------

	Structure Excavation	3250	CY		
	Structure Fill	2250	CY		
	Cofferdam Excavation	2700	CY		
	Cofferdam Fill (Shield)	4550	CY		
	Cofferdam Sheet Pile (P2-32)	<u>40350</u>	BS		

PROJECT FLORIDA AVENUE COMPLEX	Page <u>  </u> of <u>  </u>	COMPUTED BY HMB	DATE NOV. '75
SUBJECT VER. LIET GATE STR. - TEMP. BYPASS CHANNEL		CHECKED BY	DATE

Contract Estimate - Quantity Takeoff - Base On West Side

DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT COST	COST
Channel Excavation	5,300	cu. yd.	5.00	26,500.00
Steel Sheet Pile Walls PZ 38 USS EX-TEN 55	48,600	Sq. ft.	8.00	388,800.00
Sheet Pile Cover Plates Steel Plate, t=1 5/16"	5,500	Sq. ft.	16.00	88,000.00
" " t= 5/16"	6,070	Sq. ft.	4.00	24,280.00
Wale Channels C 15X40	1,434	ft.	11.00	15,774.00
Standard Pipe 4" 560'	6,045	lb.	0.50	3,022.50
Tie Rod 3 1/4" 935'	26,400	lb.	0.75	19,800.00
Steel H-Piles HP 14X73	3,234	ft.	13.00	42,042.00
Concrete	80	cu. yd.	100.00	8,000.00
Steel Reinforcement	11,037	lb.	.40	4,414.80
Pulling Sheet Pile	48,600	sq. ft.	1.00	48,600.00
Pulling H Piles	3,234	ft.	2.00	6,468.00
Channel Backfill	5,300	cu. yd.	5.00	26,500.00
Salvage Steel (Sheet pile+Plates, H-Piles, Wale, Pipe)	1,260	tons	40.00	-50,400.00
				651,801.30
			10% Profit	65,180.10
			30% Contingencies	195,540.00
			Total Cost =	912,521.40
			One On Each Side	2
			Cost Of Both Bypass Channel =	1,825,042.80

PROJECT Vertical Lift Gate	Page ___ of ___	COMPUTED BY RJR	DATE 26 Aug 76
SUBJECT Concrete, Cement		CHECKED BY	DATE

Concrete ~

(All concrete volumes from concrete weight computations.)

$$\text{Volume} = \frac{\text{Weight}}{0.15(27)}$$

Item	Weight (Kips)	Volume (CY) (Estimated)
Base Slab (El. -22 to El. -18.5)	1213.8	300
Walls (El. -18.5 to El. +2)	1015.89	251
Deck Slab (El. +2 to El. +4)	366.83	91
Walls (El. +2 to El. +27)	594.97	147
Flood walls (El. +2 to El. +4)	90.0	23
Stairs	13.14	4
Machine Room Slab	255.93	64
Machine Room Walls	165.76	41
Roof Slab (El. 40)	68.01	17

$$\begin{aligned} \text{Cement Volume} &= 938 \text{ CY} \times 5.17 \text{ cwt/CY} \\ &= 4849.46 \text{ cwt (Use 4850)} \end{aligned}$$

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Structural Steel, Reinforcement		RSR	26 Aug 76
			CHECKED BY	DATE

Structural Steel ~

All weights obtained from computations

Gate Weight = 36300 lb/Gate

Other Structural Weight = 91440 lb

Reinforcement ~

See Schedule No. 3, Reinforcement  
Design.

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Concrete Piling		RJR	27 Aug 16
			CHECKED BY	DATE

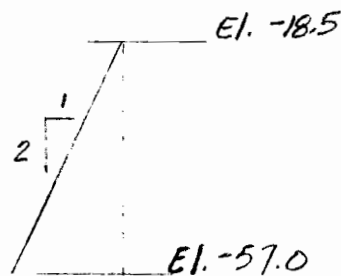
## Concrete Prestressed Piling ~

### Battered Piles,

5 rows with 13 piles/rows = 65 piles

4 rows with 12 piles/rows = 48 piles

113 piles



$$\begin{aligned} \text{Length} &= \sqrt{(38.5)^2 + (19.25)^2} \\ &= 43.04 \text{ (Use 44')} \end{aligned}$$

$$\therefore \text{Total Length} = 4972 \text{ lf}$$

### Vertical Piles,

2 rows with 13 piles/row = 26 piles

Length = 38.5'

$$\therefore \text{Total Length} = 1001 \text{ lf}$$

Total Length Piles = 5973 lf

(Use 6000 lf)



PROJECT Florida Avenue Complex	Page ___ of ___	COMPUTED BY RJG	DATE 27 Aug 76
SUBJECT Steel Sheet Piling		CHECKED BY	DATE

Steel Sheet Piling~

Use #2-27 sheet piling.

From El. -21 to El. -45.0

and w/ Sta 5+71.21 to Sta 6+79.21

∴ Total Sheet Piling = 24' x 88' = 2112.0 sf

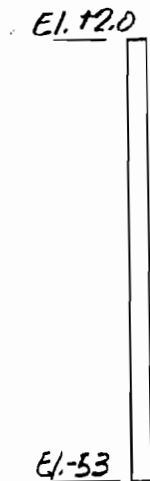
(Use 2150 sq ft)

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Cotterdam Steel Sheet Piling		RJR	27 Sep 76
			CHECKED BY	DATE

### Cotterdam Steel Sheet Piling ~

6 cells, Circumference = 95.03' / cell

5 intermediate cells, Length = 32.52' / cell



Length of piling = 55'

$$\therefore \text{Area} = (6 \times 95.03' \times 55') + (5 \times 32.52' \times 55')$$

$$= 40302.8 \text{ sq ft}$$

(Use 40350 sq ft)

Cofferdam Excavation:

End Area $A_n$ (CY/1')	Average End Area $\frac{1}{2}(A_n + A_{n+1})$ (CY/1')	Length $L_n$ (ft)	Volume $\frac{1}{2}(A_n + A_{n+1})L_n$ (CY)
14.34	14.87	23.0	342.01
15.39	16.27	30.0	488.10
17.14	17.64	30.0	529.20
18.14	17.74	30.0	532.20
17.33	16.46	30.0	493.80
15.58	15.32	18.0	275.76
15.06			

Excavation Volume = 2661.07CY  
(Use 2700CY)

Cofferdam Fill:

29.44	29.16	23.0	670.68
28.87	28.25	30.0	847.50
27.63	27.29	30.0	818.70
26.94	27.12	30.0	813.60
27.29	27.91	30.0	837.30
28.53	28.86	18.0	519.48
29.19			

Fill Volume = 4507.26CY  
(Use 4550CY)

Structure Excavation:

End Area $A_n$ (cy/l)	Average End Area $\frac{1}{2}(A_n + A_{n+1})$ (cy/l)	Length $L_n$ (ft.)	Volume $\frac{1}{2}(A_n + A_{n+1})L_n$ (cy)
5.43	8.67	23	199.41
11.91	18.61	30	558.30
25.30	27.88	30	836.40
30.46	28.05	30	841.50
25.64	19.60	30	588.00
13.56	10.71	18	192.78
7.86			

Structure Excavation Volume = 3216.39 cy  
(Use 3250 cy)

Structure Fill:

5.43	5.79	23	133.17
6.15	10.94	30	328.20
15.73	20.40	30	612.00
25.06	21.21	30	636.30
17.35	12.57	30	377.10
7.79	7.83	18	140.94
7.86			

Structure Fill Volume = 2227.71 cy  
(Use 2250 cy)

Bypass Channel Excavation

End Area $A_n$ (cy/1')	Average End Area $\frac{1}{2}(A_n + A_{n+1})$ (cy/1')	Length $L_n$ (ft)	Volume $\frac{1}{2}(A_n + A_{n+1})L_n$ (cy)
14.3	14.9	23	342.7
15.4	16.2	30	489.0
17.1	17.6	30	528.0
18.1	17.7	30	531.0
17.3	16.5	30	495.0
15.6	15.4	18	277.2
15.1			
			2662.9 (Use 2700 cy)

Bypass Channel Fill

29.4	29.2	23	671.6
28.9	28.3	30	849.0
27.6	27.3	30	819.0
26.9	27.1	30	813.0
27.3	27.9	30	837.0
28.5	28.5	18	504.0
29.2			
			4493.6 (Use 4500 cy)



PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Concrete, Cement		RJgr	1 Sep 76
			CHECKED BY	DATE

Vertical Lift Gates ~

West Approach Basins;

From computations;

$$\text{Base Slab Vol} = 5458 \text{ CY}$$

$$\text{Wall Vol.} = 257.8 \text{ CY}$$

$$\text{Stabilization Slab} = 52.0 \text{ CY}$$

$$\text{Cement Volume} = 855.6 \times 5.17 = 4423.5 \text{ cwt}$$

Concrete Prestressed Piling ~

VLG

Vertical Piles =  $32 \times 2 = 64$  piles

Battered Piles =  $14 \times 2 = 28$  piles

Battered Piles length = 40'

Vertical Piles length = 36'

$\therefore$  Total length =  $2304 + 1120$

= 3424 lf



Reinforcement ~

VLS

Longitudinal Steel - Base Slab

Top - #10 from 0-55', (215 #10)

Bottom - #9 from 0-55', (48 #9)

Transverse Steel - Base Slab

Top and Bottom - #10 from 0-49', (117 #10)

Vertical Steel - Walls

Outside - #8 from 0-20', (38 #8)

- #9 from 20-55', (154 #9)

Inside - #9 from 0-55', (108 #9)

Horizontal Steel - Walls

Outside / Inside - El. -18.5 to El. +4 (88 #7)

$$\#10 - 332 \times 104' \times 4.303 \#/1 = 148574$$

$$\#9 - 310 \times 145' \times 3.40 \#/1 = 152830$$

$$\#8 - 38 \times 20' \times 2.67 \#/1 = 2029$$

$$\#7 - 88 \times 56' \times 2.044 \#/1 = 10073$$

313506 lb

Total = 627012 lb (Use 627100 lb)

REASONABLE CONTRACT ESTIMATE

SHEET OF

PROJECT

Overhead Roller Gate - Fla Ave

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
	Concrete				
	Base Slab	107	CY	80	8560
	Walls	23	CY	150	3450
	Columns	17	CY	150	2550
	Beams (overhead)	23	CY	150	3450
	Stabilization Slab	12	CY	80	960
	Cement	940	cwt	2.50	2350
	Structural Steel	—	#		
	Gate	26000	lb	1.75	45500
	Embedded Steel	5000	lb	1.50	7500
	Reinforcement	26000	lb	0.35	9100
	Steel Sheet Piling, PMA-22	2200	SF	8	17600
	Concrete Prestressed Piling	2020	RF	15	30300
	Water stops				

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Concrete, Cement		RJgr	7 Sep 76
			CHECKED BY	DATE

Concrete -

$$\begin{aligned} \text{Stabilization Slab} &= (87' \times 11' \times \frac{4}{12}) / 27 \\ &= 11.8 \text{ CY (Use 12 CY)} \end{aligned}$$

$$\begin{aligned} \text{Base Slab} &= (87' \times 11' \times 3) / 27 \\ &= 106.3 \text{ CY (Use 107 CY)} \end{aligned}$$

$$\begin{aligned} \text{Walls} &= [(40' \times 1' \times 2.33') + \frac{1}{2} (\frac{12.33^2}{24}) \times 40] / 27 \\ &= 23.0 \text{ CY (Use 23 CY)} \end{aligned}$$

$$\begin{aligned} \text{Columns} &= [2(2.5' \times 3' \times 22.58') + \\ &\quad + 2(2.5 \times 22.58)] / 27 \\ &= 16.7 \text{ CY (Use 17 CY)} \end{aligned}$$

$$\begin{aligned} \text{Overhead Beam} &= [(2.75' \times 2') + (1.5' \times 1.75')] \times 77' / 27 \\ &= 23.2 \text{ CY (Use 24 CY)} \end{aligned}$$

$$\begin{aligned} \text{Cement Volume} &= (181 \text{ CY}) \times 5.17 \text{ cwt/CY} \\ &= 935.8 \text{ cwt (Use 940 cwt)} \end{aligned}$$

PROJECT

Florida Avenue Complex

Page \_\_\_ of \_\_\_

COMPUTED BY

RJR

DATE

7 Sep 76

SUBJECT

Structural Steel

CHECKED BY

DATE

Structural Steel ~

Gate Weight = 26000 lb.

Exposed / or Embedded Steel,

1-S12x50

50 x 77 = 3850 lb

miscellaneous  
steel

1000 lb

Use 5000 lb

## Reinforcement ~

Base Slab, Top, Transverse -

#9, 12" from 0-8'

Base Slab, Bottom, Transverse -

#9, 12" from 0-8'

Base Slab, Top, Longitudinal -

12-#9 from 0-87'

Base Slab, Bottom, Longitudinal -

13-#9 from 0-87'

Walls, Vertical (Both sides) -

#6, 12" from El. 14 to El. 4.67

#9, 12" from El. 6.5 to El. 7.0

#6, 12" from El. 14.0 to El. 2.0

#9, 12" from El. 3.83 to El. 7.0

Walls, Horizontal (Both sides) -

#6, 12" for 40' (El. 14 to El. 2.0)

Beam, Overhead -

14 #11 for 77'

6 #4 for 77'

Columns 1;3, Vertical El. 2.0 to El. 14.0 -  
9 #11's

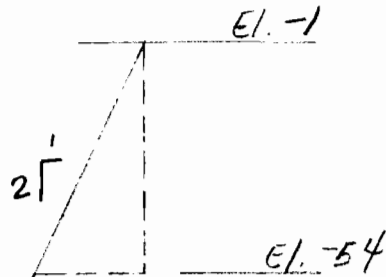
El. 14 to El. 24.5 -  
8 #11's

Column 5, vertical -

El 2.0 to El. 14.0  
6 #11's  
El. 14 to El. 24.5  
5 #11's

- #4 -  $6 \times 77' \times 0.668 = 308.6$
  - #6 -  $40 \times 9.33' \times 1.502 = 560.6$
  - $40 \times 12.0' \times 1.502 = 721.0$
  - $24 \times 40' \times 1.502 = 1441.9$
  - #9 -  $87 \times 8' \times 3.4 = 2366.4$
  - $87 \times 8 \times 3.4 = 2366.4$
  - $12 \times 87' \times 3.4 = 3549.6$
  - $13 \times 87' \times 3.4 = 3845.4$
  - $40 \times 7.5' \times 3.4 = 1020.0$
  - $40 \times 4.82 \times 3.4 = 656.9$
  - #11 -  $14 \times 77' \times 5.313 = 5727.4$
  - $24 \times 12' \times 5.313 = 1530.1$
  - $21 \times 10.5' \times 5.313 = 1171.5$
- $25265.8$   
 ~~$42102.2$~~
- Total = 25265.8  
(Use 26000 lb)

Concrete Prestressed Piling ~



$$\text{Pile length} = \sqrt{(53)^2 + (26.5)^2}$$

$$= 59.25'$$

34 Battered Piles @ 59.25' = 2014.7 lf  
(Use 2020 lf)

Steel Sheet Piling -

Top sheet piling = 0.25

Bottom sheet piling = -25

$$A = 24.75 \times 87 = 2153.25 \text{ sq ft}$$

(Use 2200 sq ft)





PROJECT <i>Florida Avenue Complex</i>	Page <i>—</i> of <i>—</i>	COMPUTED BY <i>RJR</i>	DATE <i>8 Sep 76</i>
SUBJECT <i>Fill Volume, West</i>		CHECKED BY	DATE

*Levee and Floodwalls-Fill Volume*

<i>End Area An (cy/')</i>	<i>Average End Area 1/2 (An + An+1) (cy/')</i>	<i>Length Ln (ft)</i>	<i>Estimated Vol. 1/2 (An + An+1) Ln (cy)</i>
<i>1.45</i>	<i>1.32</i>	<i>10</i>	<i>13.2</i>
<i>1.19</i>	<i>1.95</i>	<i>40</i>	<i>78.0</i> <i>11.2</i>
<i>2.71</i>			
<i>1.39</i>	<i>1.3</i>	<i>15</i>	<i>19.5</i>
<i>1.21</i>	<i>1.27</i>	<i>40</i>	<i>50.8</i>
<i>1.33</i>	<i>1.11</i>	<i>35</i>	<i>38.85</i>
<i>0.88</i>	<i>1.13</i>	<i>35</i>	<i>39.55</i>
<i>1.37</i>	<i>1.35</i>	<i>35</i>	<i>47.25</i>
<i>1.33</i>	<i>1.25</i>	<i>40</i>	<i>50.00</i>
<i>1.17</i>	<i>1.24</i>	<i>31.09</i>	<i>34.95</i>
<i>1.30</i>			<i>76.10</i>
<i>1.24</i>	<i>2.58</i>	<i>20</i>	<i>51.6</i>
<i>3.91</i>	<i>3.15</i>	<i>20</i>	<i>63.0</i>
<i>2.38</i>	<i>3.06</i>	<i>40</i>	<i>122.4</i>
<i>3.74</i>	<i>4.04</i>	<i>14</i>	<i>56.56</i>
<i>4.34</i>			<i>65.16</i>
<i>4.29</i>	<i>2.36</i>	<i>14.4</i>	<i>33.98</i> <i>69.64</i>
<i>0.42</i>			

End Area $A_n$ (cy/1')	Average End Area $\frac{1}{2}(A_n + A_{n+1})$ (cy/1')	Length $L_n$ (ft)	Estimated Vol. $\frac{1}{2}(A_n + A_{n+1})L_n$ (cy)
<i>Continued from previous page.</i>			
	<i>0.52</i>	<i>30</i>	<i>15.60</i>
<i>0.61</i>	<i>0.91</i>	<i>23.66'</i>	<i>21.53</i>
<i>1.20</i>	<i>1.81</i>	<i>12.04</i>	<i>21.79</i>
<i>2.41</i>	<i>3.50</i>	<i>14.46</i>	<i>50.61</i>
<i>4.58</i>			<i>207.17</i>
<i>13.79</i>	<i>13.16'</i>	<i>22</i>	<i>289.52</i>
<i>12.53</i>	<i>12.68</i>	<i>50</i>	<i>634.00</i>
<i>12.82</i>	<i>12.84</i>	<i>18</i>	<i>231.12</i>
<i>12.86</i>			<i>1963.81</i>
<i>4.77</i>	<i>11.22</i>	<i>18</i>	<i>201.96</i>
<i>17.67</i>	<i>20.06</i>	<i>12</i>	<i>240.72</i>
<i>22.45</i>			<i>2406.49</i>
<i>12.63</i>	<i>6.71</i>	<i>65.10</i>	<i>436.82</i>
<i>0.78</i>	<i>0.39</i>	<i>57.45</i>	<i>22.41</i>
<i>0</i>			<i>2865.12</i>

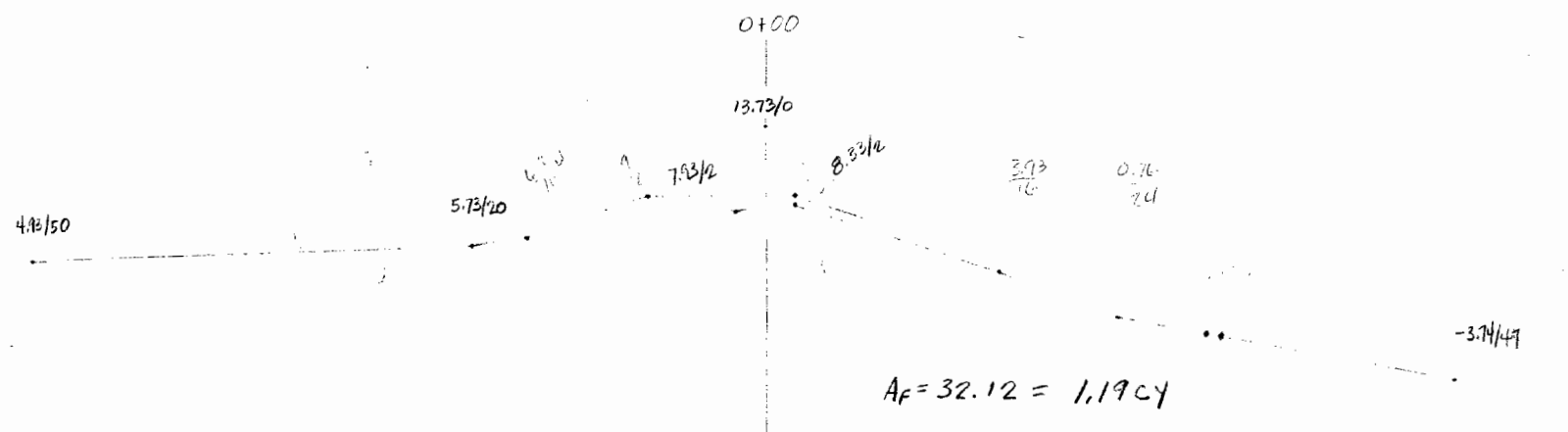
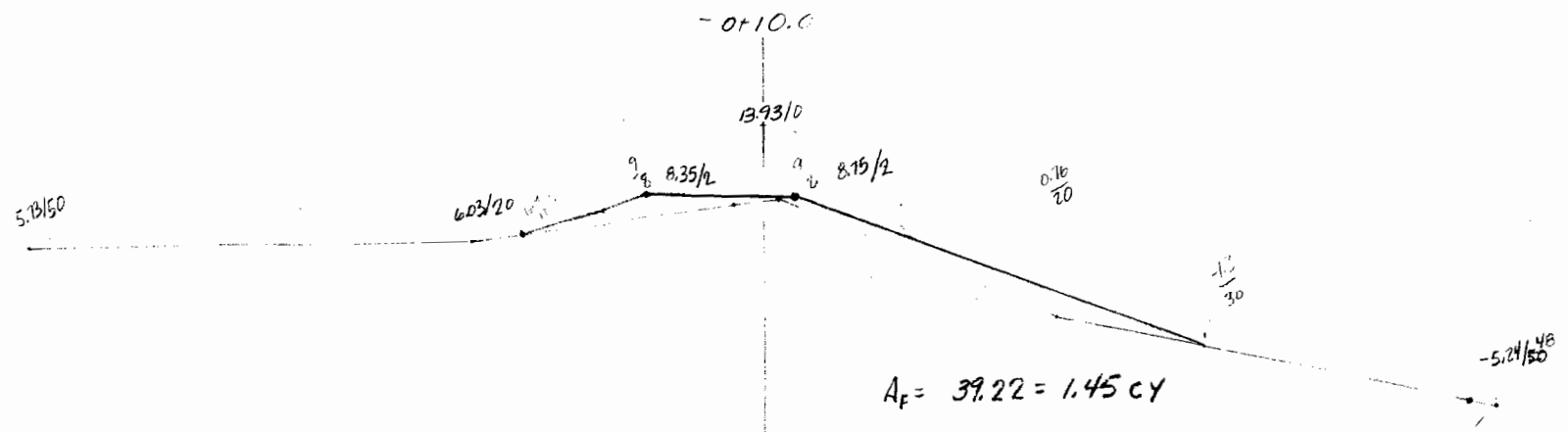
*Estimated Fill Volume = 2865.72  
(Use 2870 CY)*

## Levee and Floodwall - Excavation Volume

End Area $A_n$ (CY/ft)	Average End Area $\frac{1}{2}(A_n + A_{n+1})$ (CY/ft)	Length $L_n$ (ft)	Estimated Vol. $\frac{1}{2}(A_n + A_{n+1})L_n$ (CY)
0	2.55	15	38.25
5.10	3.34	40	133.60
1.58	2.88	35	100.80
1.30	1.09	35	38.15
0.88	1.98	35	69.30
3.08	8.08	40	323.20
13.52	15.42	31.09	479.41 <span style="float:right">1182.71</span>
17.32			
19.15	14.39	20	287.80
9.62	7.25	20	145.00
4.88	6.19	40	247.60
7.49	8.00	14	112.00 <span style="float:right">74.4</span>
8.51			<span style="float:right">1915.11</span>
8.60	5.22	14.4	75.17
1.84	1.03	30	30.90
0.55	1.25	23.66	29.58 <span style="float:right">2140.76</span>
1.94			

End Area $A_n$ (cy/ft)	Average End Area $\frac{1}{2}(A_n + A_{n+1})$ (cy/ft)	Length $L_n$ (ft)	Estimated Vol. $\frac{1}{2}(A_n + A_{n+1})L_n$ (cy)
Continued from previous page.			
3.15	2.55	12.04	30.70
0	1.58	14.46	22.85 2164.31
0	1.0	65.10	65.10
2.0	1.0	57.45	57.45 2296.86
0			

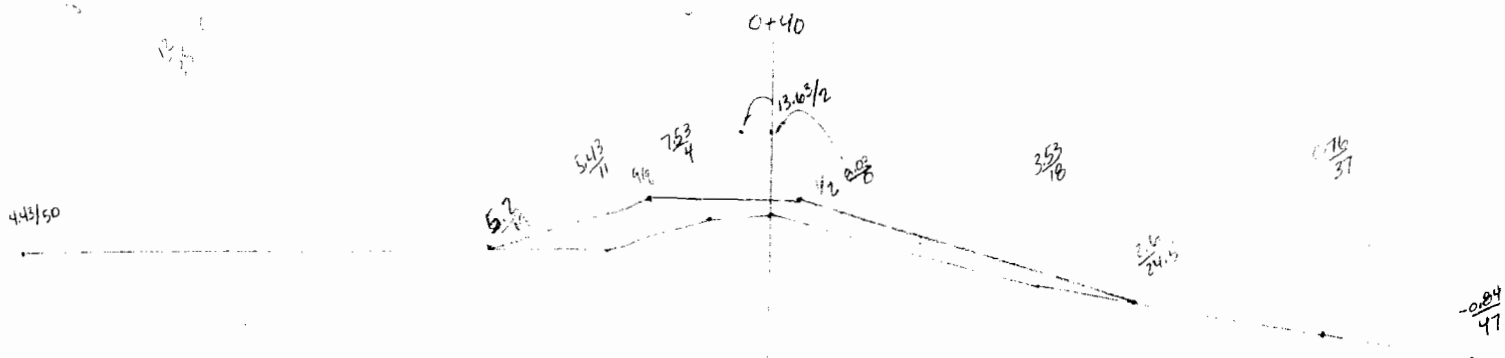
Estimated Excavation Volume = 2286.86  
(Use 2300 cy)



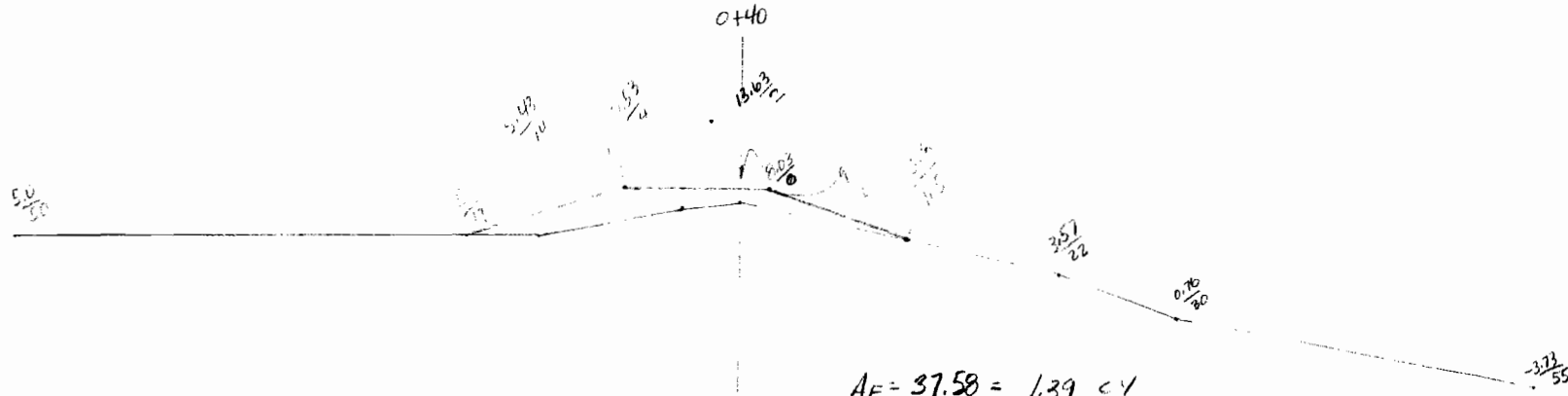
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25  
10

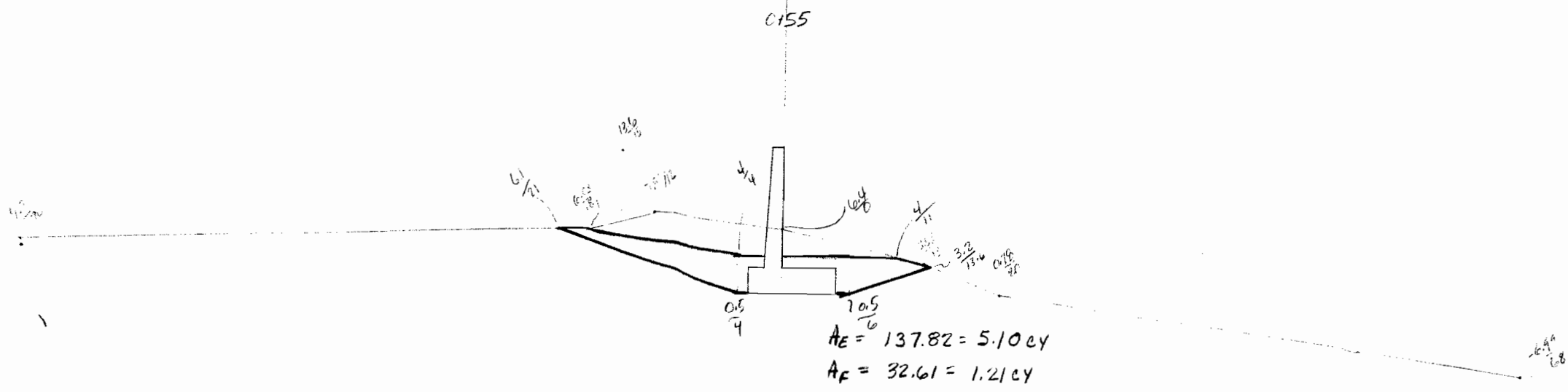
West



$A_F = 73.14 = 2.71 \text{ cy}$

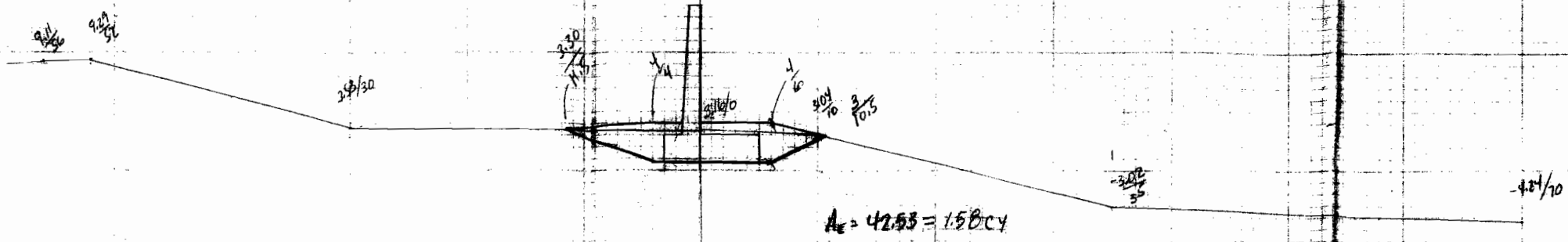


$A_F = 37.58 = 1.39 \text{ cy}$



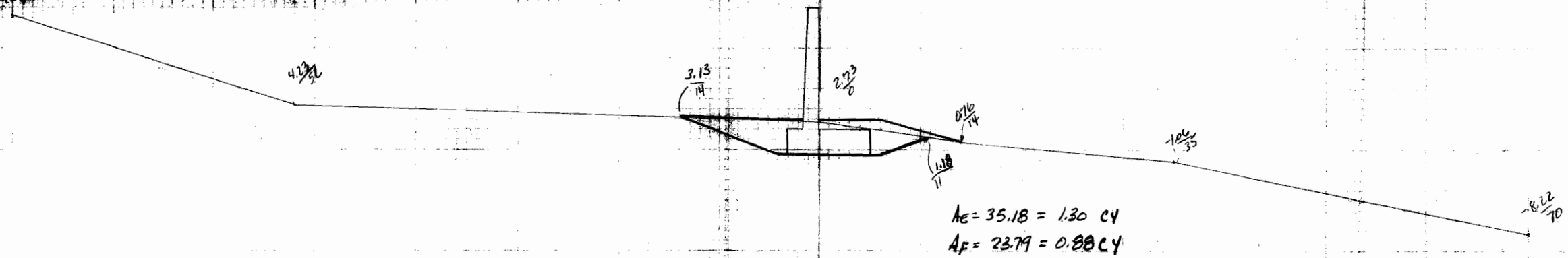
$A_E = 137.82 = 5.10 \text{ cy}$

$A_F = 32.61 = 1.21 \text{ cy}$



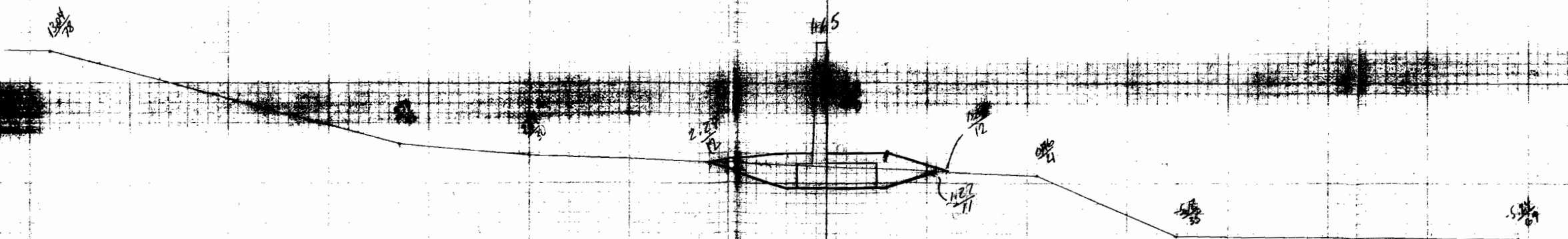
$$A_E = 42.53 = 1.58 \text{ CY}$$

$$A_F = 36.0 = 1.33 \text{ CY}$$



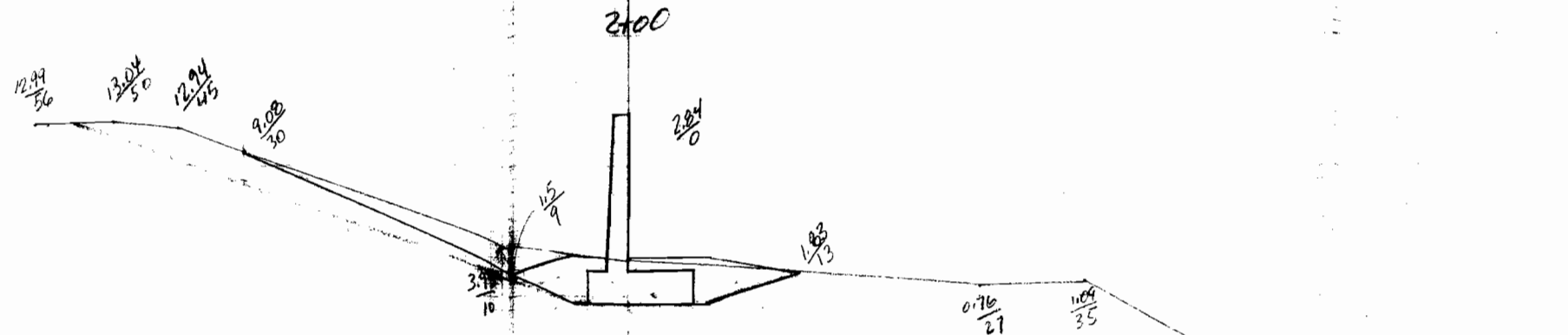
$$A_E = 35.18 = 1.30 \text{ CY}$$

$$A_F = 23.79 = 0.88 \text{ CY}$$



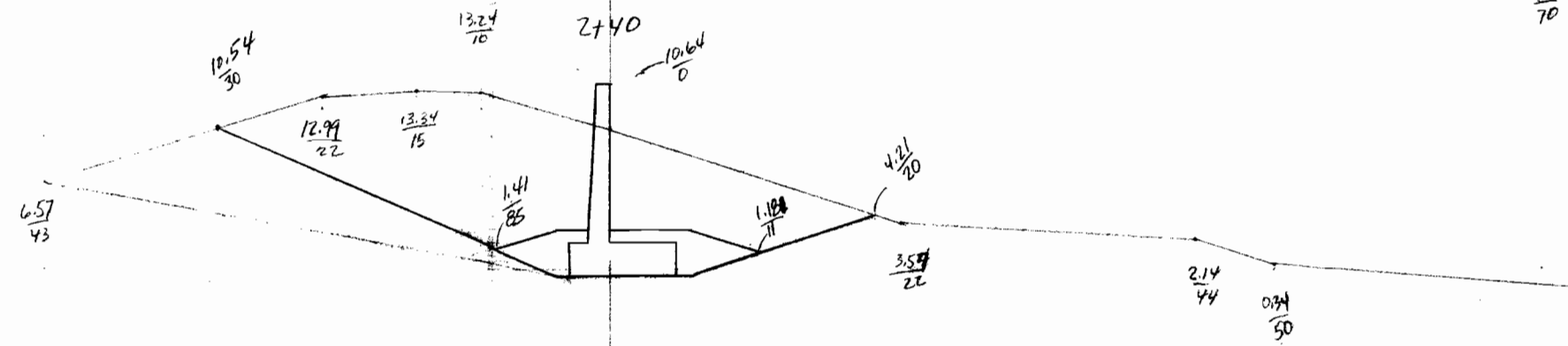
$$A_E = 23.68 = 0.88 \text{ CY}$$

$$A_F = 37.12 = 1.37 \text{ CY}$$



$A_E = 83.11 = 3.08 \text{ cy}$

$A_F = 36.10 = 1.33 \text{ cy}$

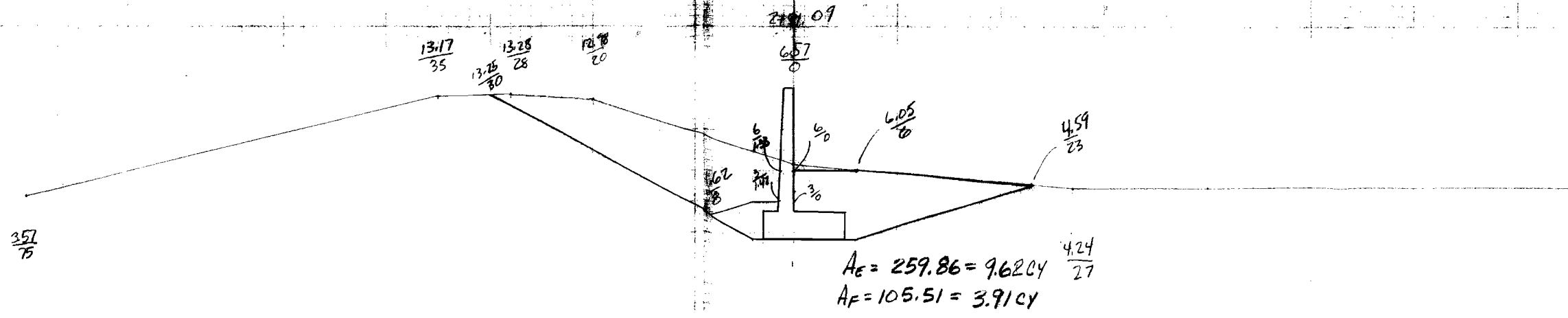
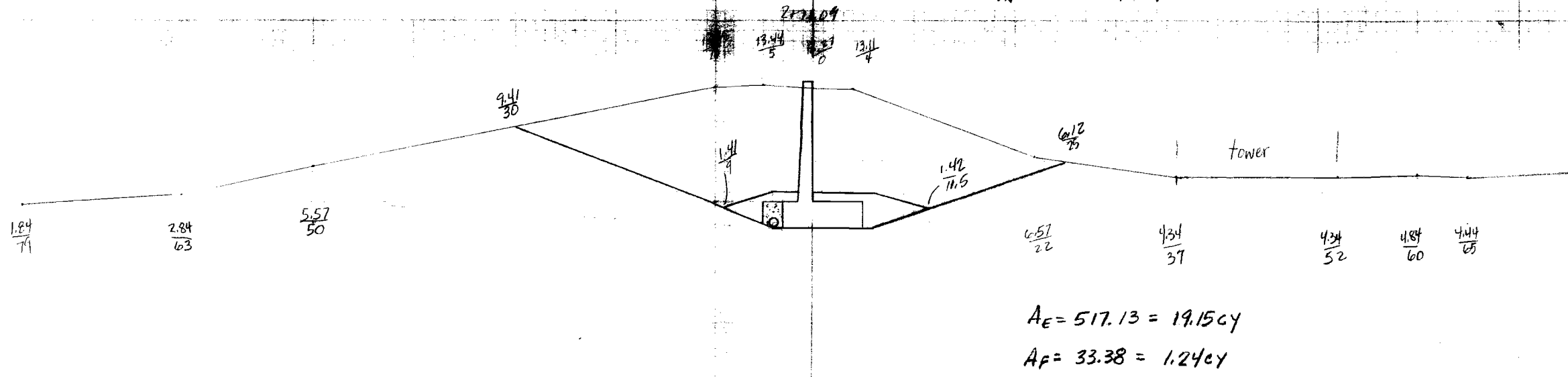
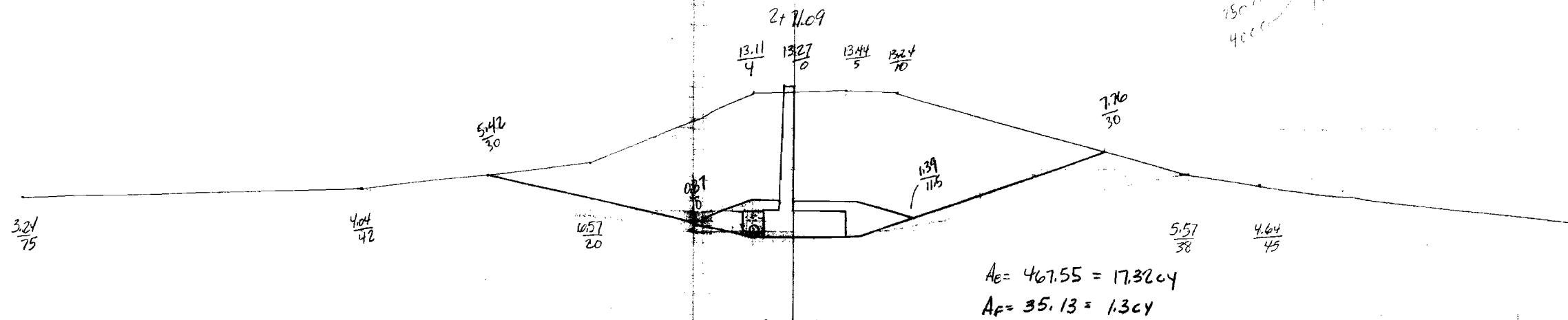


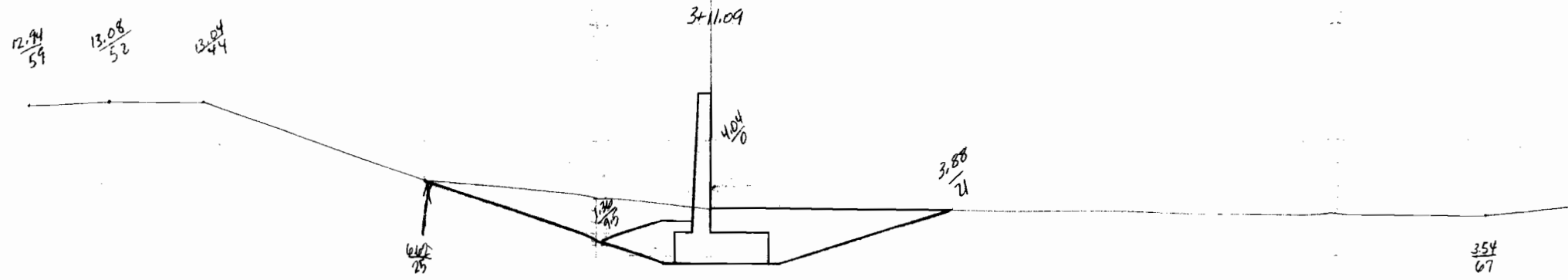
$A_E = 365.14 = 1352 \text{ cy}$

$A_F = 31.63 = 1.17 \text{ cy}$

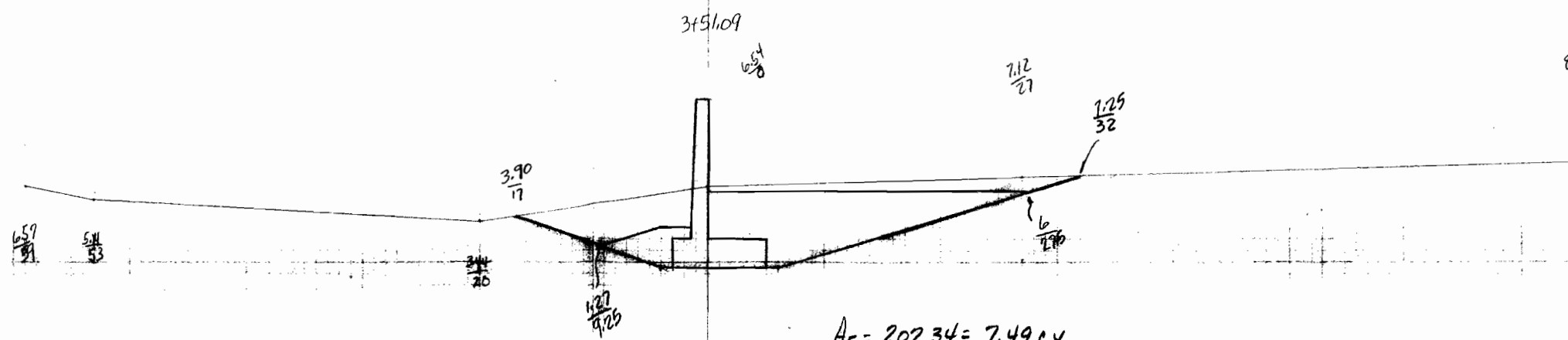


1500  
4000  
8000  
14000

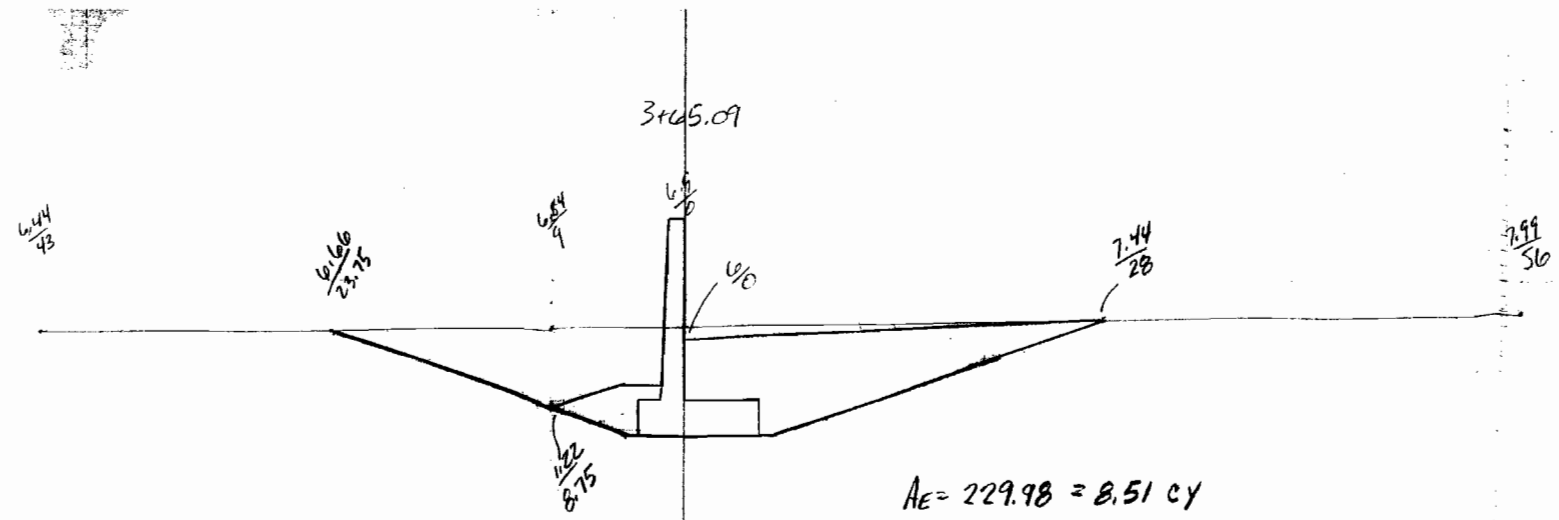




$A_E = 131.8 = 4.88 \text{ CY}$   
 $A_F = 64.38 = 2.38 \text{ CY}$

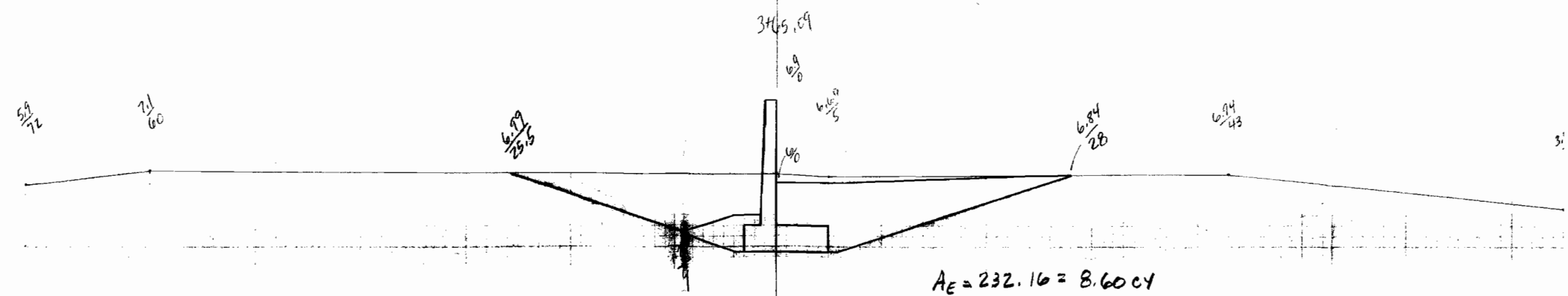


$A_E = 202.34 = 7.49 \text{ CY}$   
 $A_F = 100.85 = 3.74 \text{ CY}$



$$A_E = 229.98 = 8.51 \text{ CY}$$

$$A_F = 117.13 = 4.34 \text{ CY}$$

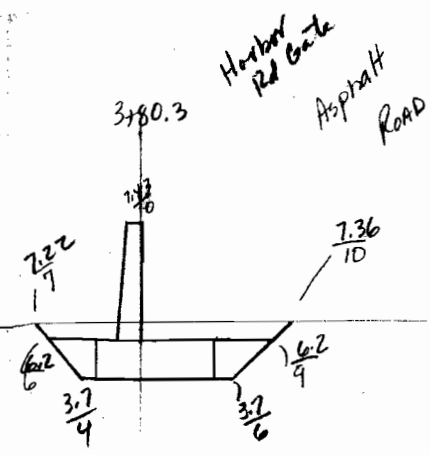


$$A_E = 232.16 = 8.60 \text{ CY}$$

$$A_F = 115.77 = 4.29 \text{ CY}$$

46/72

1/24



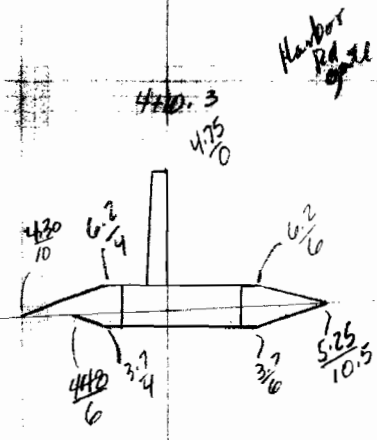
$AE = 49.73 = 1.84 \text{ CY}$   
 $AF = 11.25 = 0.42 \text{ CY}$

714/40

714/70

57/72

38/21

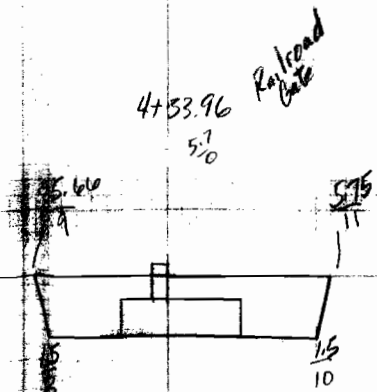


$AE = 14.87 = 0.55 \text{ CY}$   
 $AF = 16.39 = 0.61 \text{ CY}$

433.96  
 410.35  
 23.61

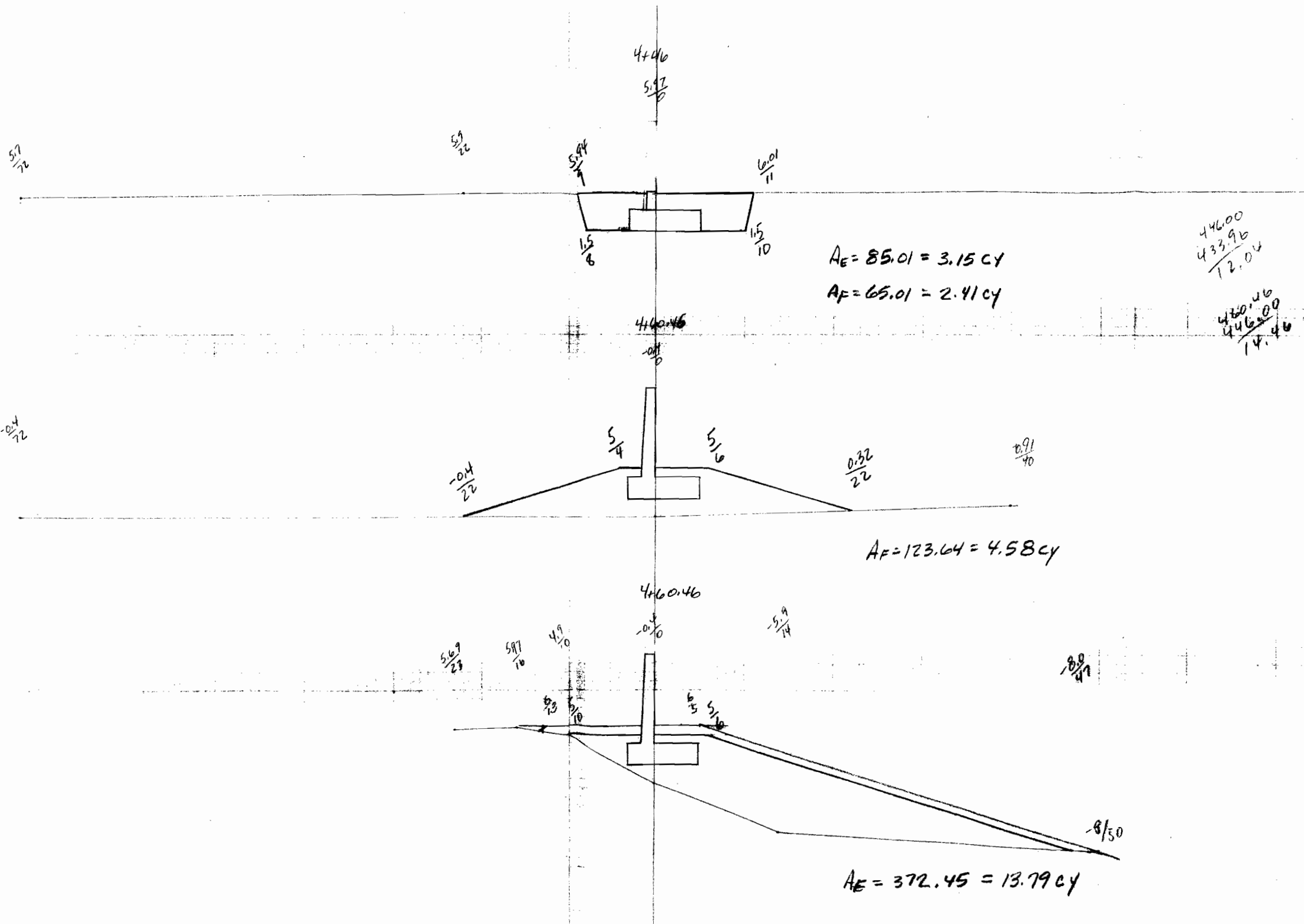
56/72

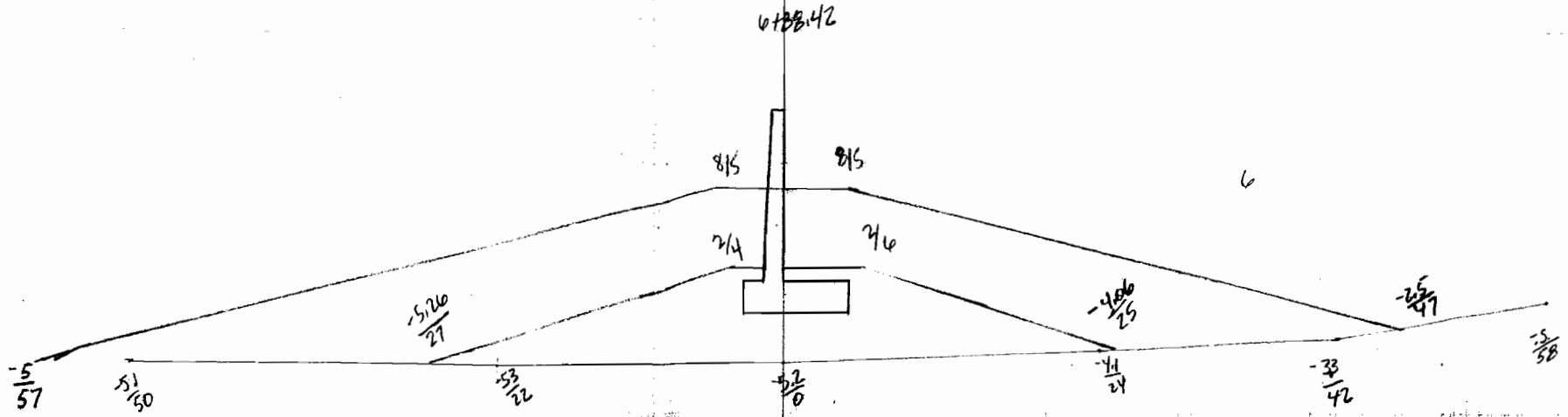
56



$AE = 52.39 = 1.94 \text{ CY}$   
 $AF = 32.39 = 1.20 \text{ CY}$

60/78





$A_F = 341.04 = 12.63 \text{ CY}$

$A_{shell} = 12.6 \text{ CY}$

$7+53.52 \text{ @ Fla Ave } A_{clay} = 15.7 \text{ CY}$

$$\begin{array}{r} 252.54 \\ 588.02 \\ \hline 165.10 \end{array}$$

$\frac{133}{55}$

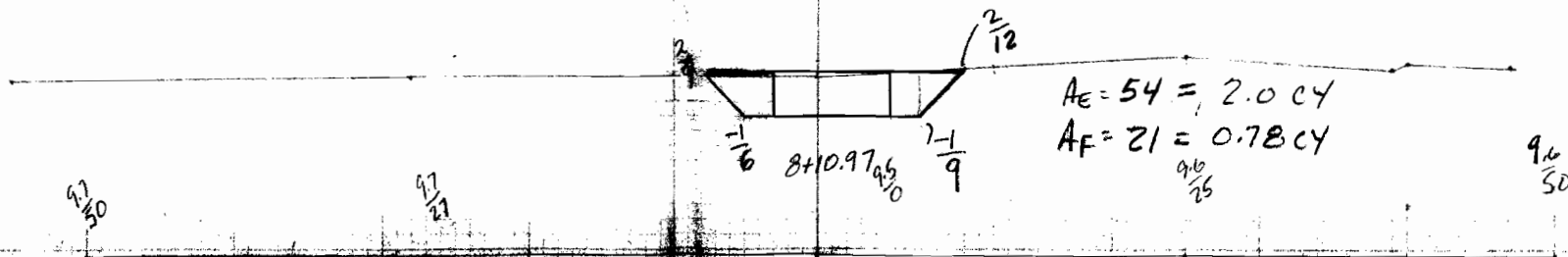
$\frac{140}{27}$

$\frac{135}{0}$

$\frac{1303}{26}$

$\frac{1208}{39} \frac{254}{40}$

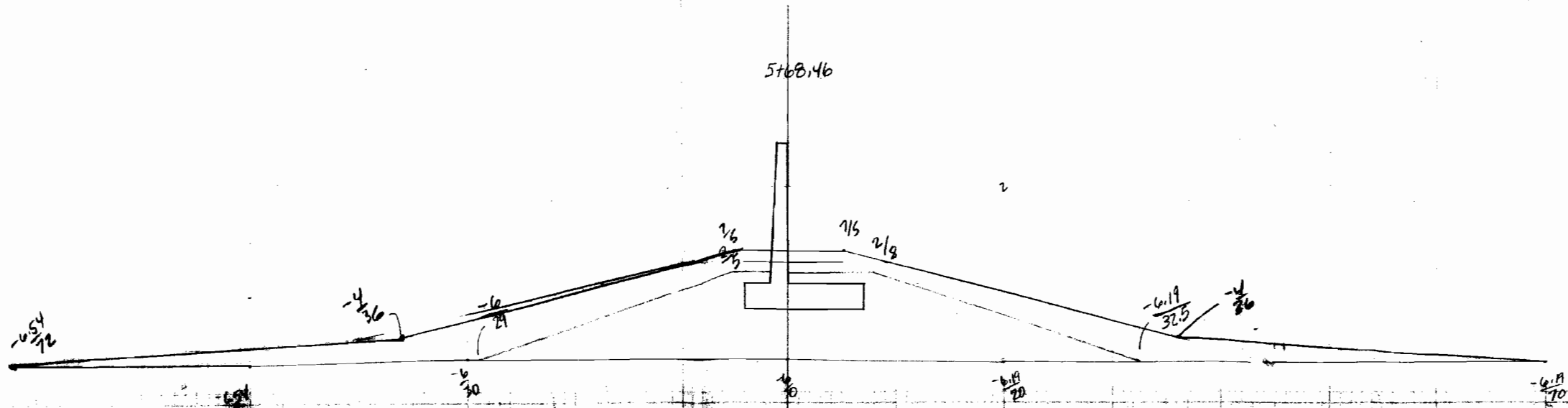
$\frac{23}{47}$



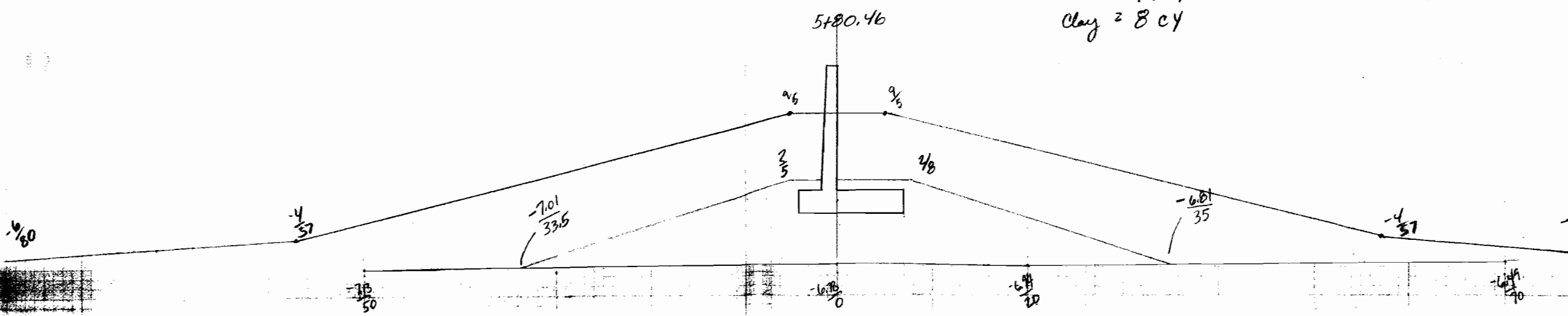
$A_E = 54 = 2.0 \text{ CY}$

$A_F = 21 = 0.78 \text{ CY}$

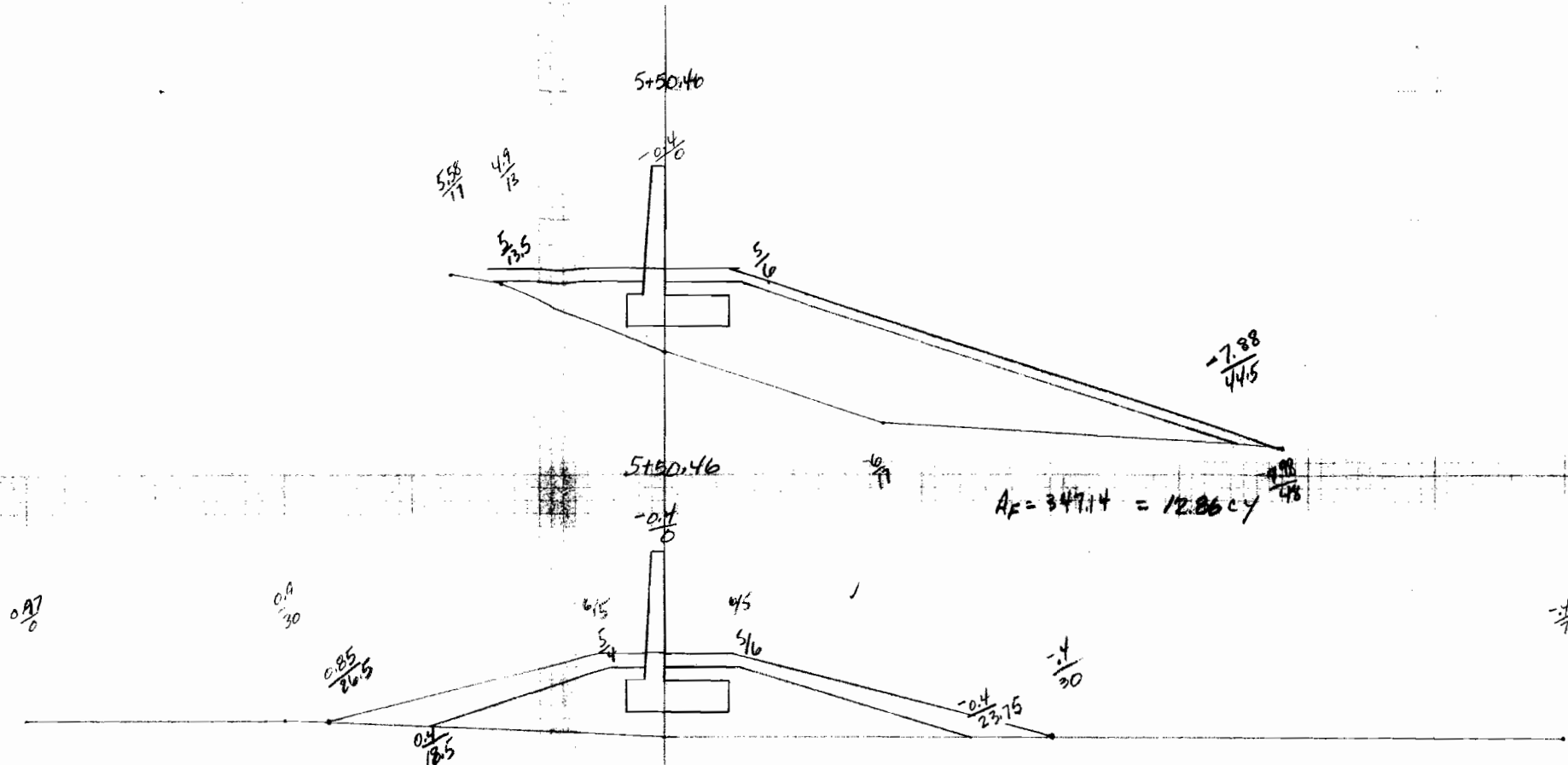
$$\begin{array}{r} 46.48 \\ 10.97 \\ \hline 57.45 \end{array}$$



$A_F = 477.10 = 17.67 \text{ cy}$   
 shell = 19.7 cy  
 clay = 8 cy



$A_F = 606.14 = 22.45 \text{ cy}$   
 shell = 22.5 cy  
 clay = 22 cy

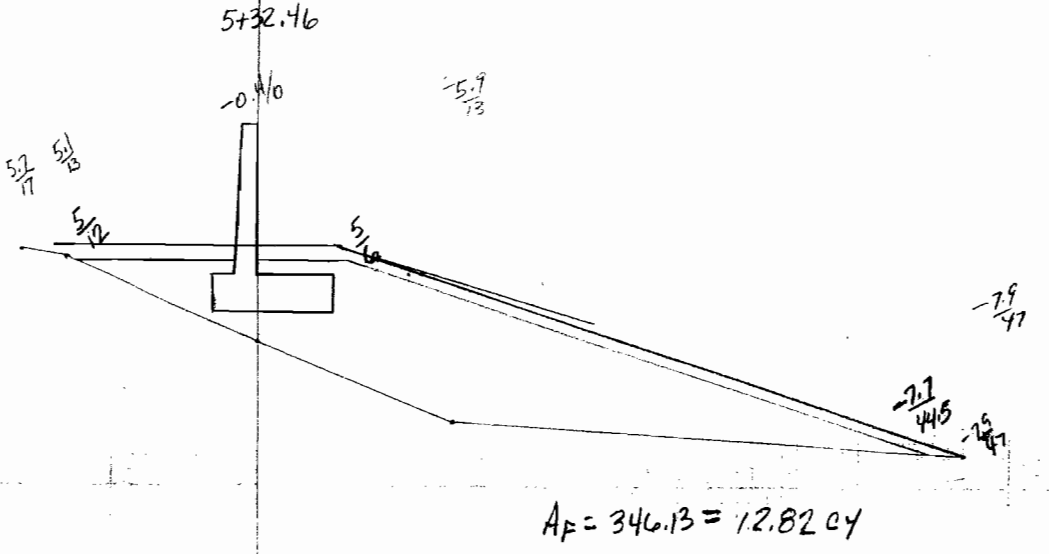
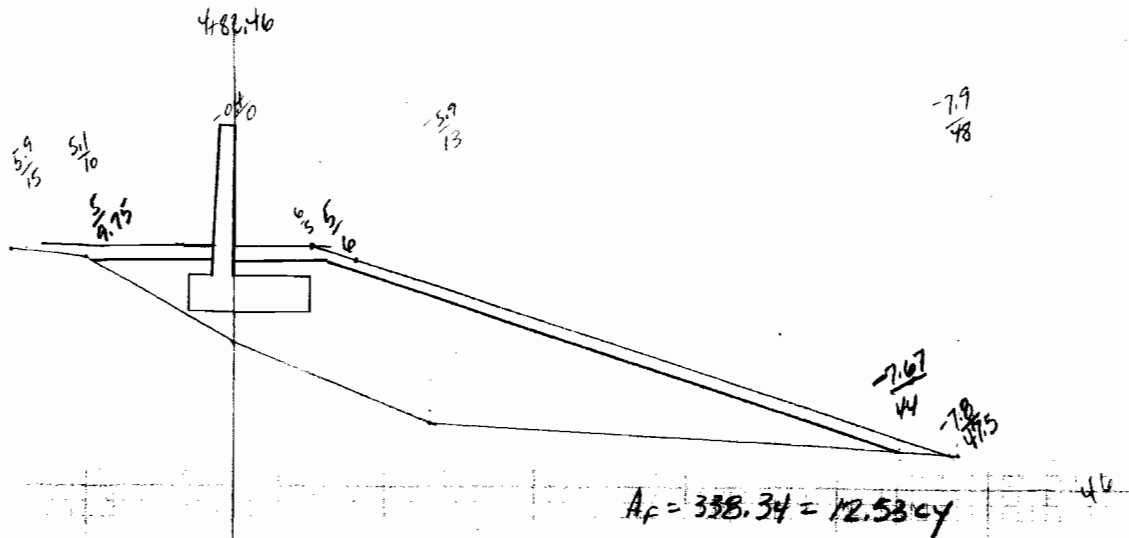


$A_F = 347.14 = 12.86 \text{ cy}$

$A_F = 128.97 = 4.77 \text{ cy}$

shell = 4.77 cy  
 clay = 3.00 cy





$\frac{27}{6}$

$\frac{1.0}{47}$

$\frac{114}{13}$

$\frac{15}{10}$

$\frac{1151}{13}$

$\frac{-5.2}{63}$

west

$\frac{12}{103}$

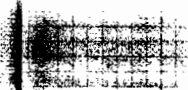
$\frac{-10.3}{18}$

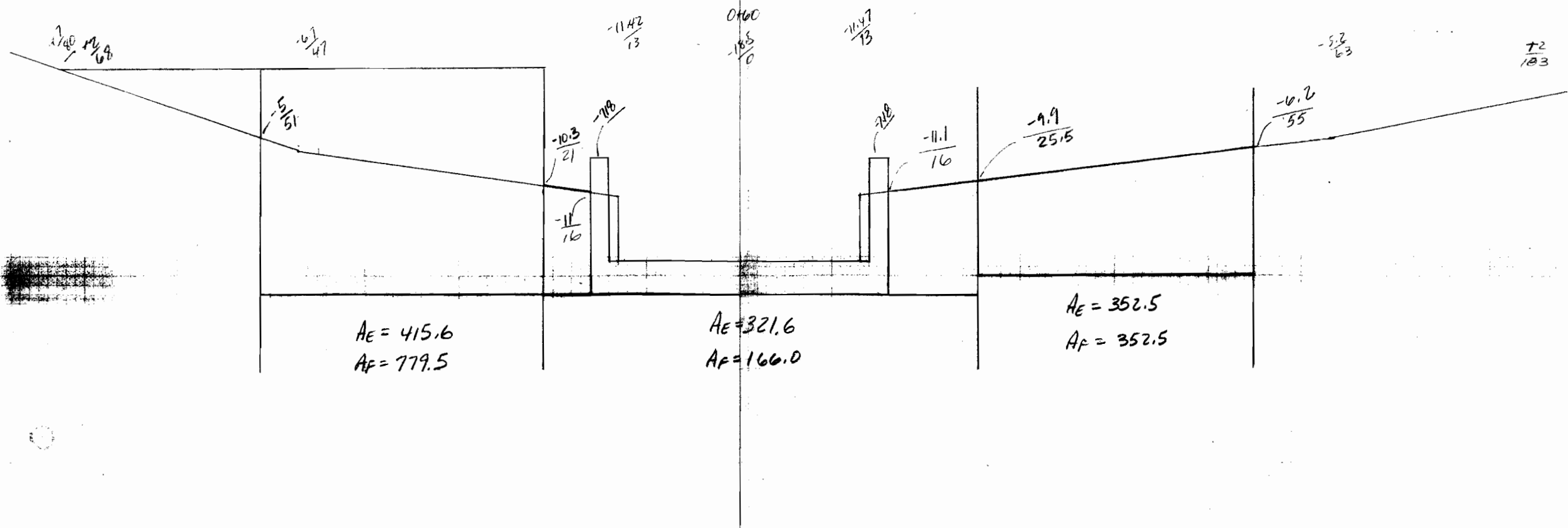
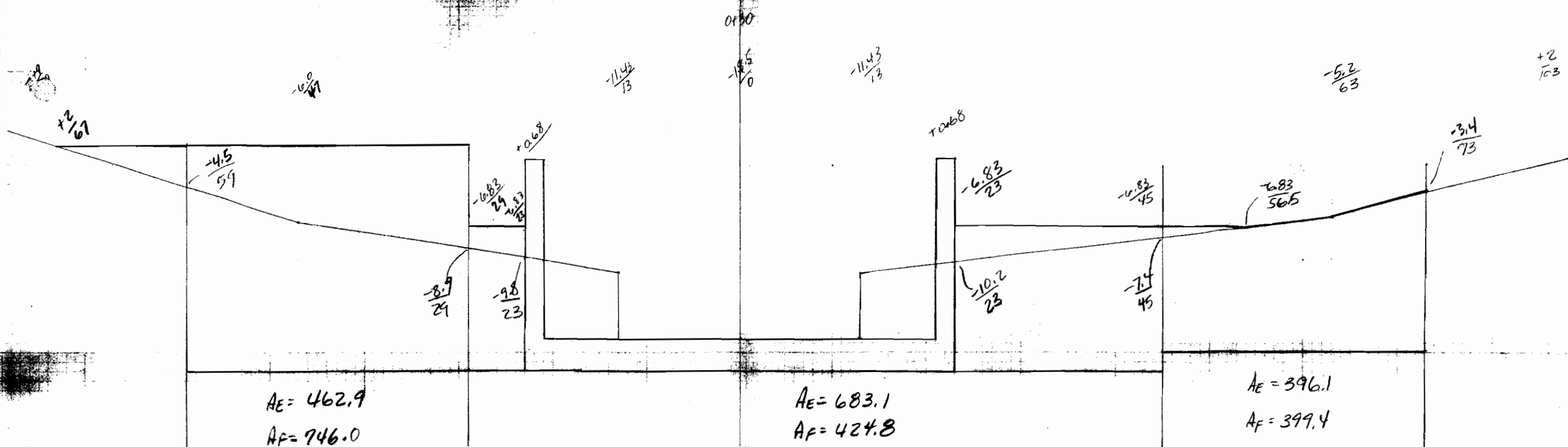
$\frac{-7.72}{48}$

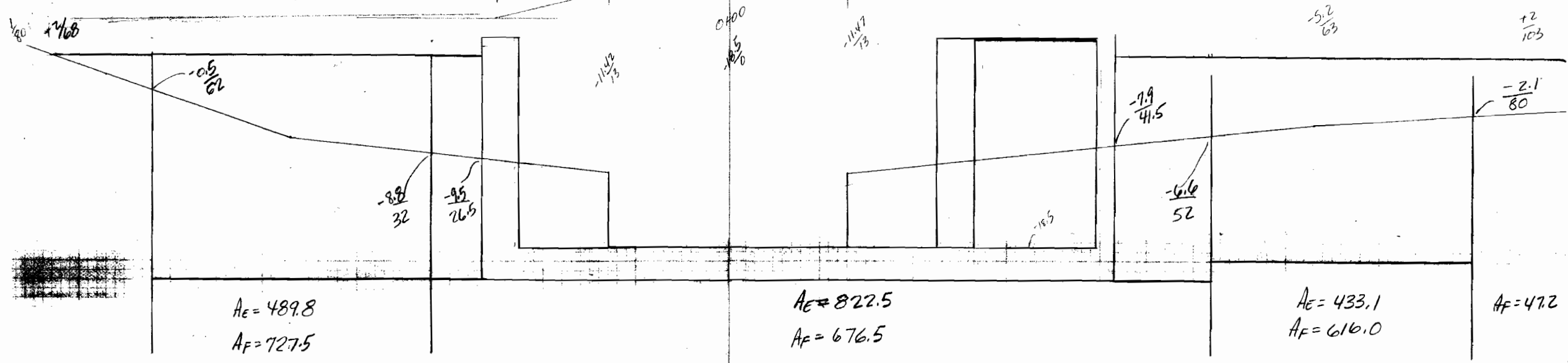
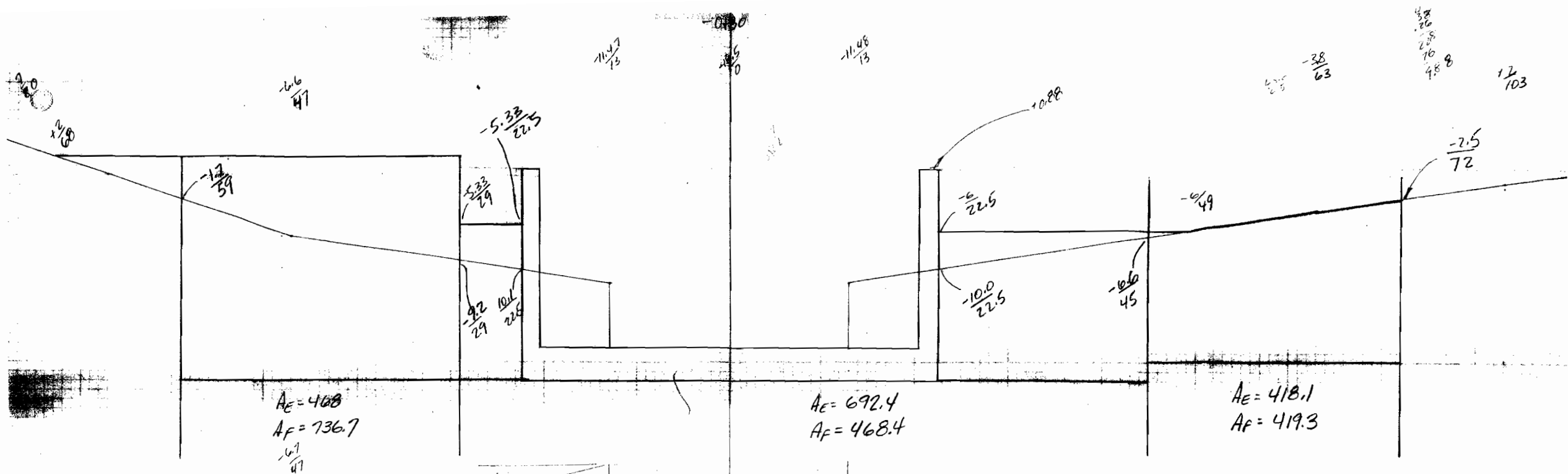
$A_E = 387.2$   
 $A_F = 795.0$

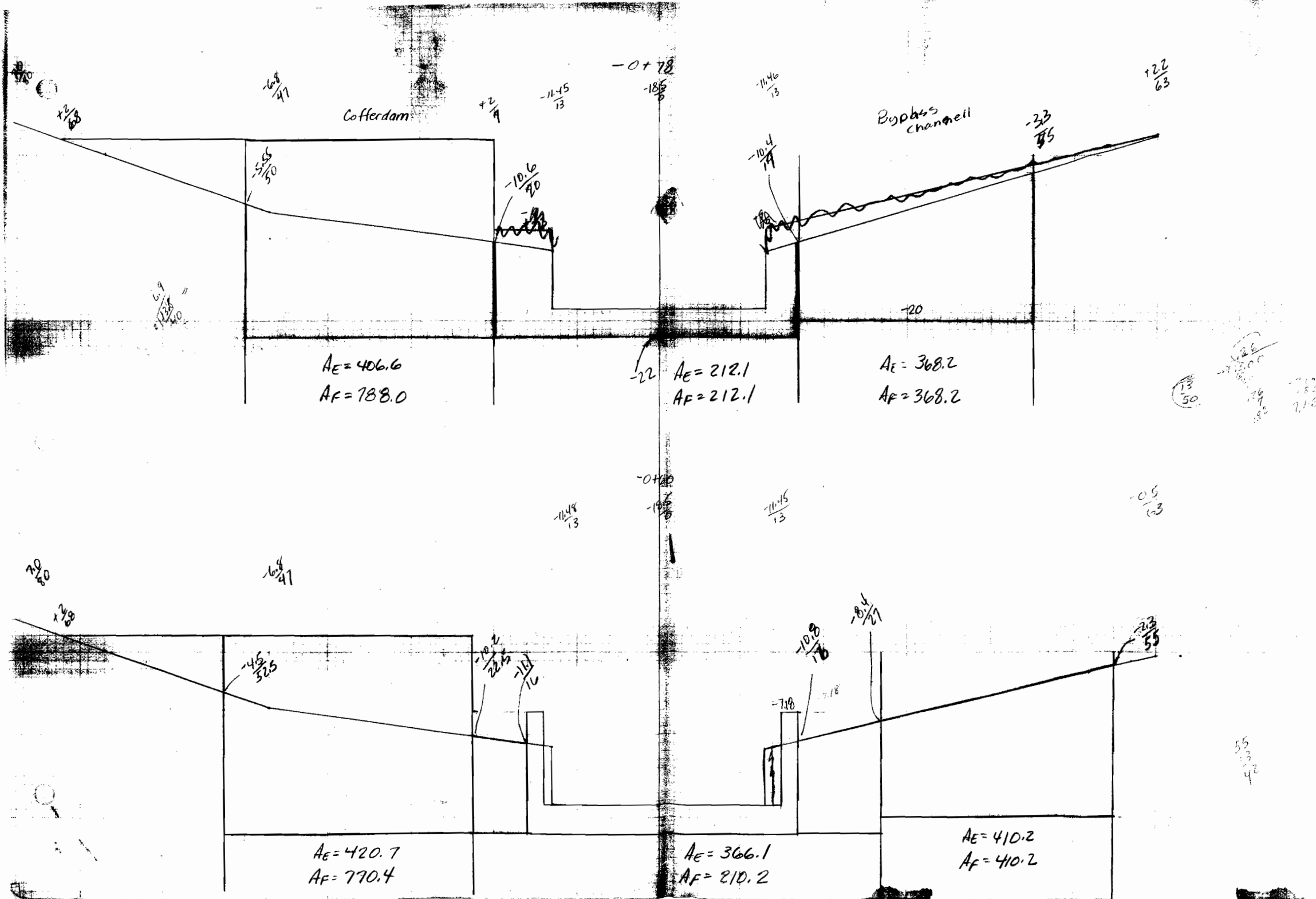
$A_E = 146.7$   
 $A_F = 146.7$

$A_E = 311.6$   
 $A_F = 311.6$









$\frac{26}{50}$   
 $\frac{13}{50}$   
 $\frac{26}{50}$   
 $\frac{13}{50}$

$\frac{5}{13}$

$\frac{55}{3}$   
 $\frac{42}{4}$

~~REASONABLE CONTRACT ESTIMATE~~

SHEET OF

PROJECT

Vertical Lift Gate Structure ~~West~~ East

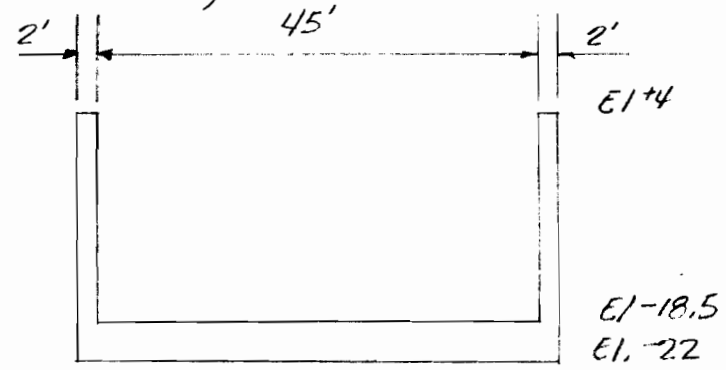
INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
	<u>Concrete:</u>				
	Base Slab (-24' to E1-1E)	300	CY		
	Walls (E1-18.5 to E1+2)	251	CY		
	Deck Slab (E1 <sup>2</sup> to E1 <sup>4.0</sup> )	91	CY		
	Walls (E1 <sup>2</sup> to E1 <sup>27</sup> )	147	CY		
	Flood Walls (E1 <sup>2</sup> to E1 <sup>14</sup> )	23	CY		
	Stairs	4.0	CY		
	Machine Rm Slab	64	CY		
	Machine Rm Walls	41	CY		
	Roof Slab (E1.40)	17	CY		
	<u>Cement:</u>	4850	CWT		
	Machinery (Hoists - Gate)		2EA		
	Lighting		L.S.		
	Railing				
	Protective Sealing		L.S.		
	Cathodic Protection		L.S.		
	<u>Structural Steel:</u>				
	Embedded	21300	lb		
	Exposed	70140	lb		
	Gates	72600	lb		
	Reinforcement	181700	lb		
	Concrete Prestressed Piles 14x14	6000	LF		
	Miscellaneous		L.S.		
	Steel Sheet Piling, P2-27	12320	SF		



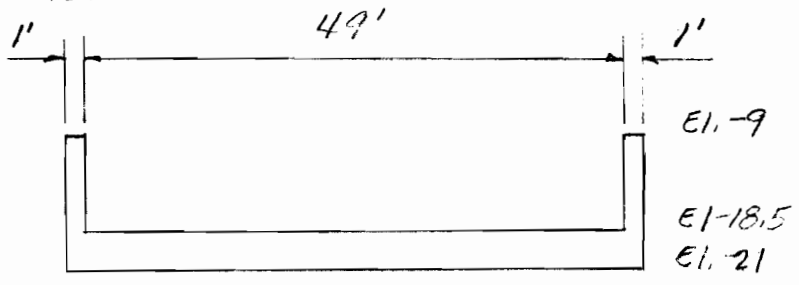
### East Approach Structures - Expansion Joint Filler

East-West,



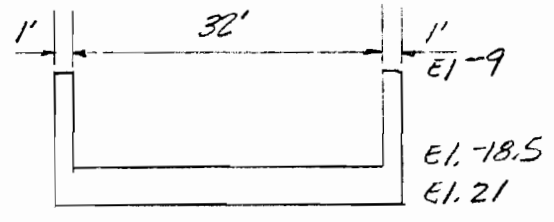
$$A = 2[2(2 \times 26) + 45(2.5)] = 523 \text{ sf}$$

East-



$$A = 2(12) + 49(2.5) = 146.5 \text{ sf}$$

West-



$$A = 2(12) + 32(2.5) = 104.0 \text{ sf}$$

$$\text{Total Area} = 773.5 \text{ sf} \quad (\text{Use } 775 \text{ sf})$$



PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Waterstops		RJgr	8 Sep 76
			CHECKED BY	DATE

East Approach Structures -  
 Bulb-Type Waterstop, East-West

$$L = 2[2(22 + 1.75) + 47] = 189 \text{ lf}$$

(Use 200 lf)

L-Type Waterstops, East

$$L = 50 + 2(7.5 + 1.25) = 71.5 \text{ lf}$$

West -

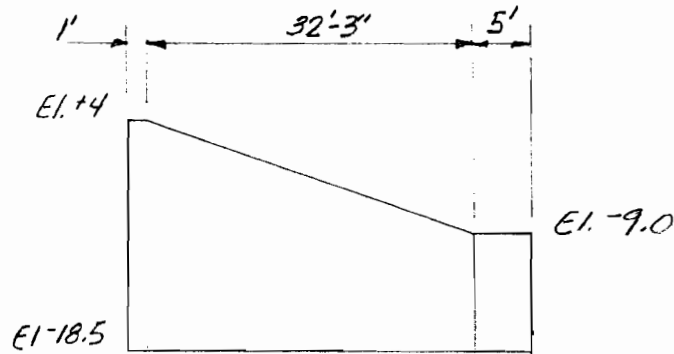
$$L = 33' + 2(9.5 + 1.25) = 54.5$$

Total, L-Type Waterstop = 126 lf

(Use 130 lf)

East Approach Structures  
Sackrub Finish

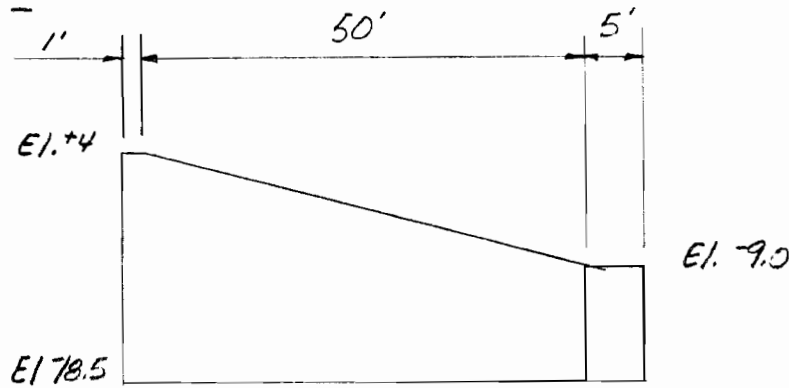
East -



$$A = 2 \left[ (1 \times 22.5') + \frac{1}{2} (22.5 + 9.5) 32.25 + (5 \times 9.5) \right]$$

$$= 1172 \text{ sf}$$

West -



$$A = 2 \left[ (1 \times 22.5) + \frac{1}{2} (22.5 + 9.5) 50 + (5 \times 9.5) \right]$$

$$= 1740 \text{ sf}$$

$$\text{Total} = 2912 \text{ sf}$$

(USE 2950 sf)

PROJECT	Florida Avenue Complex	Page <u>  </u> of <u>  </u>	COMPUTED BY	DATE
SUBJECT	Cofferdam - Sheet Piling		RJG	9 Sep 76
			CHECKED BY	DATE

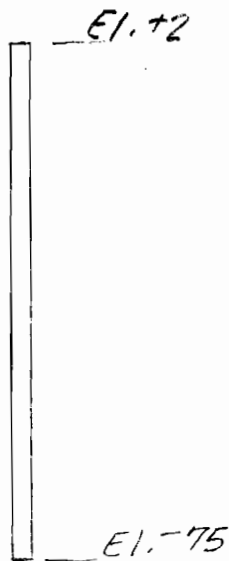
East

Cofferdam System,

Cell perimeter = 95.03' / cell

Intermediate cell length = 32.52' / cell

5 cells, 4 intermediate cells



$$\begin{aligned}
 A &= 5(95.03)(77) \\
 &+ 4(32.52)(77) \\
 &= 46602.71 \text{ sf} \\
 &\text{(Use 46650 sf)}
 \end{aligned}$$

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Sackrub - T-Wall, I-Wall		RJgr	9 Sep 76
			CHECKED BY	DATE

Sackrub Finish - East T-Wall, I-Wall, etc.

w/l sta 0+00 to 0+60,

$$A = 6' \times 60' \times 2 = 720 \text{ sf}$$

w/l sta 0+60 to w/l sta 1+40.35

$$A = 80.35' \times 10' \times 2 = 1607 \text{ sf}$$

w/l sta 1+40.35 to w/l sta 1+79.50,

$$A = 39.15' \times 14' \times 2 = 1096.2 \text{ sf}$$

Overhead Keller Gate,

$$\text{Wall Area} = 41.5' \times 14.0' \times 2 = 1162.0$$

$$\text{Column Area} = (11' \times 22.5' \times 2) + (9' \times 22.5')$$

$$= 697.5 \text{ sf}$$

$$\text{Beam Area} = 11' \times 70' = 770 \text{ sf}$$

w/l sta. 2+62.5 to w/l sta 4+79.81,

$$A = 17' \times 211.31' \times 2 = 7184.54 \text{ sf}$$

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Sackrub Finish -		RJR	10 Sep 76
			CHECKED BY	DATE

Sackrub finish, (East)

W/L sta. 5+40.31 to W/L sta. 5+77.49,

$$A = 37.18' \times 17' \times 2 = 1264.12 \text{ sf}$$

W/L sta 5+77.49 to W/L sta 5+94.99,

$$A = 14.' \times 17.5' \times 2 = 490 \text{ sf}$$

Swing Gate,

$$\text{Wall Area} = 2.' \times 15' \times 2 = 399 \text{ sf}$$

$$\text{Column Area} = 8' \times 10.5' \times 2 = 168 \text{ sf}$$

W/L sta. 6+40.49 to W/L sta 6+50.49,

$$A = 10' \times 9.5' \times 2 = 190 \text{ sf}$$

W/L sta 6+50.49 to W/L sta 8+77,

$$A. = 7' \times 226.51' \times 2 = 3171.14 \text{ sf}$$

$$\text{Total Area} = 18919.5 \text{ sf}$$

(Use 18950 sf)

PROJECT Florida Avenue Complex	Page ___ of ___	COMPUTED BY RJ9r	DATE 10 Sept 76
SUBJECT Gate Seals		CHECKED BY	DATE

Overhead Roller Gate - Fla. Ave. Complex (East)

Gate Seals,

$$\text{Gate height} = 14'-0''$$

$$\text{Gate width} = 36'-0''$$

$$\therefore \text{Length} = 28 + 36 = 64'$$

Swing Gate - Fla Ave. Complex (East)

$$\text{Gate height} = 6.58'$$

$$\text{Gate width} = 21'$$

$$\text{Length} = 21 + (6.58)2 = 34.16'$$

$$\text{Total Length} = 98.16'$$

(Use 100 sf)

PROJECT	Florida Avenue Complex	Page — of —	COMPUTED BY	DATE
SUBJECT	East Approach Structure - Concrete, Ceme.		RJR	10 Sep 76
			CHECKED BY	DATE

Concrete =

East Approach Structure:

$$\text{Base Slab} = 982.53 \text{ K} / (4.05 \text{ K/cy})$$

$$= 242.6 \text{ cy} \quad (\text{Use } 245 \text{ cy})$$

$$\text{Stabilization Slab} = \left[ \left( \frac{4'}{12} \right) \frac{982.53 \text{ K}}{3.5'} \right] / (4.05 \text{ K/cy})$$

$$= 23.1 \text{ cy} \quad (\text{Use } 25 \text{ cy})$$

$$\text{Walls} = 342.96 \text{ K} / (4.05 \text{ K/cy})$$

$$= 84.68 \text{ cy} \quad (\text{Use } 85 \text{ cy})$$

$$\text{Cement} = 350.38 \text{ cy} \times 5.17 \frac{\text{cy}}{\text{cwt}}$$

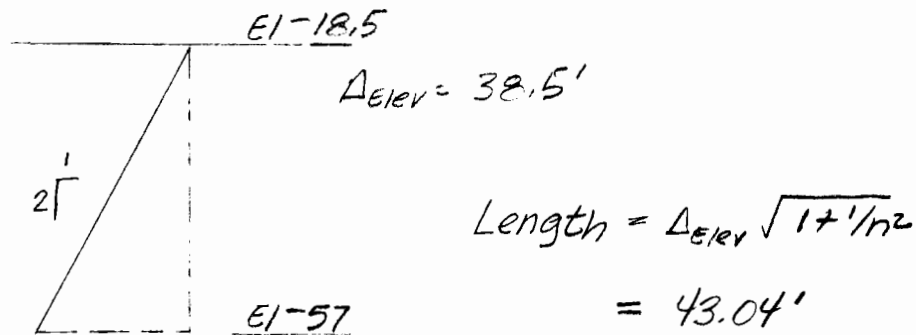
$$= 1811.5 \text{ cwt} \quad (\text{Use } 1820 \text{ cwt})$$

PROJECT	Florida Avenue Complex	COMPUTED BY	DATE
SUBJECT	Concrete Piling	RJgr	10 Sep 76
		CHECKED BY	DATE

Concrete Prestressed Piling ~ (14" x 14")

No. Battered Piles = 0

No. Vertical Piles = 22



∴ Length = 38.5 x 22' = 847 lf  
(Use 850 lf)



PROJECT	Florida Avenue Complex	COMPUTED BY	DATE
SUBJECT	Reinforcement	RJgr	10 Sep 76
		CHECKED BY	DATE

Reinforcement -

$$\#7 - 105 \times 37.25' \times 2.044 = 7995$$

$$74 \times 53' \times 2.044 = 8017$$

$$37 \times 37.25' \times 2.044 = 2817$$

$$44 \times 22.5' \times 2.044 = 2024$$

$$\#8 - 74 \times 53' \times 2.67 = 10472$$

$$4 \times 10' \times 2.67 = 240$$

$$14 \times 10' \times 2.67 = 374$$

$$51 \times 17.25' \times 2.67 = 2349$$

$$\#9 - 52 \times 37.25' \times 3.4 = 6586$$

---


$$\text{Total} = 40874 \text{ lb}$$

(Use 40900 lb)



PROJECT Florida Avenue Complex	Page ___ of ___	COMPUTED BY RJgr	DATE 10 Sep 76
SUBJECT Concrete Piling		CHECKED BY	DATE

Concrete Prestressed Piling ~ (14" x 14")

No. Battered Piles = 10

No. Vertical Piles = 34

Length, Battered Piling =  $\Delta_{\text{elev}} \sqrt{1 + 1/n^2}$

$\Delta_{\text{elev}} = 38.5'$

$n = 2$

$\therefore$  Length = 43.04', Use 44'

Total Length =  $(44' \times 10) + 34 \times (38.5')$

= 1749 l.f.

(Use 1750 l.f.)

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Concrete, Cement		RJgr	10 Sep 76
			CHECKED BY	DATE

Concrete ~

West Approach Structure:

$$\begin{aligned} \text{Stabilization Slab} &= \left(\frac{4}{12}\right)(1213.53)/(3.5)(4.05) \\ &= 57.07 \text{ cy (Use 60 cy)} \end{aligned}$$

$$\begin{aligned} \text{Base Slab} &= 1213.53^4/(4.05^4/\text{cy}) \\ &= 599.27 \text{ cy (Use 600 cy)} \end{aligned}$$

$$\begin{aligned} \text{Walls} &= 517.2^4/(4.05^4/\text{cy}) \\ &= 127.70 \text{ cy (Use 130 cy)} \end{aligned}$$

$$\begin{aligned} \text{Cement} &= 784.04 \times 5.17 \\ &= 4053.49 \text{ cwt} \\ &\text{(Use 4060 cwt)} \end{aligned}$$

Reinforcement ~

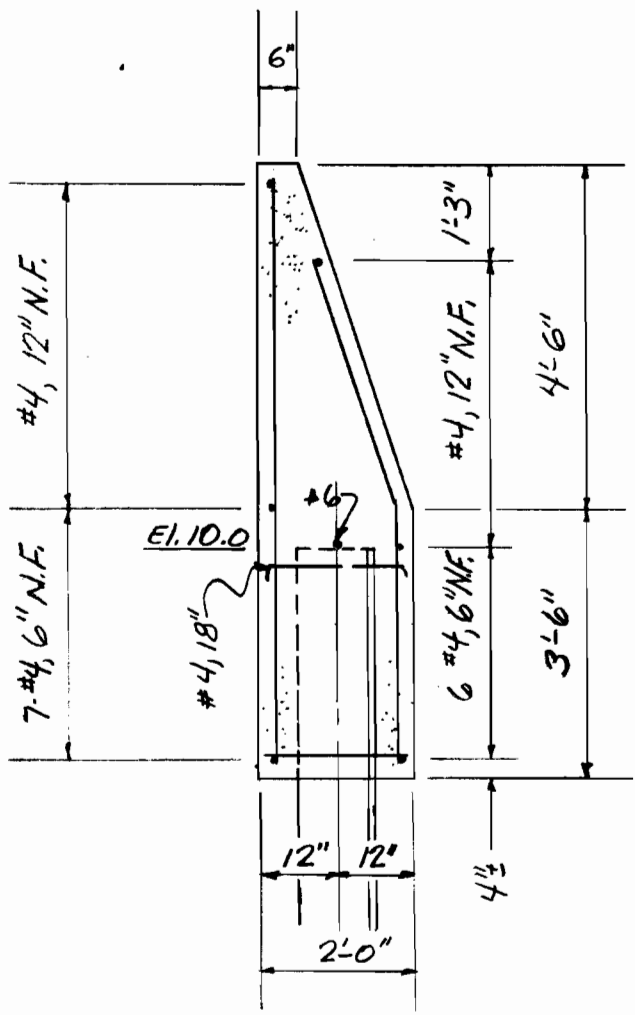
$$\begin{aligned}
 \#7 - 97 \times 49' \times 2.044 &= 9715 \\
 &44 \times 22.5' \times 2.044 = 2024 \\
 \#8 - 19 \times 20' \times 2.67 &= 1015 \\
 \#9 - 38 \times 8' \times 3.4 &= 1036 \\
 &114 \times 33' \times 3.4 = 12791 \\
 &63 \times 14' \times 3.4 = 2999 \\
 &48 \times 55' \times 3.4 = 8976 \\
 &32 \times 10' \times 3.4 = 1088 \\
 &44 \times 15' \times 3.4 = 2244 \\
 &54 \times 54' \times 3.4 = 9914 \\
 \#10 - 64 \times 49' \times 4.303 &= 13494
 \end{aligned}$$

---


$$\text{Total} = 65296 \frac{1}{6}$$

(Use 65300  $\frac{1}{6}$ )





Total Length = 292'

Concrete:

$$\text{Volume} = (13.75 \times 292) / 27$$

$$= 148.7 \text{ cy (Use 150 cy)}$$

Cement:

$$\text{Volume} = 148.7 \times 5.17$$

$$= 768.77 \text{ cwt}$$

(Use 770 cwt)

Reinforcement:

$$\text{Wt} = 15.64 \text{ #/ft} \times 292'$$

$$= 4567.2 \text{ lb.}$$

(Use 4570 lb.)

Steel Sheet Piling; Total = 9432 sf (Use 9450 sf)

w/L sta. 0+00 to w/L sta. 0+60;

$$A = 60' \times 18' = 1080 \text{ sf}$$

w/L sta 6+45 to w/L sta. 8+77;

$$A = 232' \times 36' = 8352.0 \text{ sf}$$

Waterstops, L-type ~

w/L sta 0+60:  $L = 13.5'$

w/L sta 6+45, w/L sta 8+77:  $L = 2 \times 14 = 28$

Total = 41.5' (Use 45')

Waterstops, E-bulb type ~

w/L sta 0+00 to w/L sta 0+60:  $L = 2 \times 8' = 16'$

w/L sta 6+45 to w/L sta 8+77:  $L = 9' \times 7 = \underline{63}$

Total = 79'

(Use 80')

Expansion joint Material:

$A = 13.75 \times 11 = 151.25 \text{ sf}$  (Use 155 sf)



~~REASONABLE CONTRACT~~ ESTIMATE

SHEET OF

PROJECT

T-Wall, East

INVITATION NO.

ITEM NO.

DESCRIPTION

ESTIMATED QUANTITY

UNIT

UNIT PRICE

ESTIMATED AMOUNT

Concrete

Stabilization Slab

Base Slab

Walls

Cement

Reinforcement

Waterstops

Construction Joint Matl

Concrete Prestressed Piling

Steel Sheet Piling PMA-22

— —

51 cy

420 cy

80 cy

2840 cwt

350000 lb

390 lf

670 sf

11013 lf

6500 sf

Wall

No	Monolith		Transverse		Longitudinal	Vertical		Horizontal
	L	W	Top	Bottom	Top & Bottom	F.S.	B.S.	F.S. & B.S.
3	31.0'	8'	#8,12	#8,12	9-#6	#7,12	#6,12	#5,12
4	39.54'	8'	#8,12	#8,12	15-#6	#7,12	#6,12	#5,12
5	21.0'	8'	#8,12	#8,12	9-#6	#7,12	#6,12	#5,12
6	39.15'	11'	#9,12	#8,12	21-#6	1/2 #7,6 1/2 #7,12	#6,12	#5,12
8	14.63'	11'	#8,6	#8,12	21-#6	1/2 #7,6 1/2 #7,12	#6,12	#5,12
9	42.78'	11'	#8,6	#8,12	21-#6	#9,6	#6,12	#5,12
10	45.0'	11'	#8,6	#8,12	12-#6	#9,6	#6,12	#5,12
11	44.0'	14'	#9,6	#8,12	15-#6	#9,6	#6,12	#5,12
12	21.91'	14'	#9,6	#8,12	27-#6	#9,6	#6,12	#5,12
13	44'	14'	#9,6	#8,12	15-#6	#9,6	#6,12	#5,12
15	37.18'	11'	#9,12	#8,12	21-#6	#9,6	#6,12	#5,12
16	17.5'	8'	#8,12	#8,12	15-#6	#7,12	#6,12	#5,12
18	10'	8'	#8,12	#8,12	15-#6	#7,12	#6,12	#5,12

Total = 363,344.5 lb. (Use 365 000 lb) <sup>check</sup>

- #5 - 20 x 91.54' x 1.043 = 1909.5
- 28 x 39.15' x 1.043 = 1143.3
- 34 x 250' x 1.043 = 8865.5
- 28 x 17.5' x 1.043 = 511.1
- 19 x 10' x 1.043 = 198.2
- #6 - 92 x 12.5' x 1.502 = 1727.3
- 40 x 16.5' x 1.502 = 991.3
- 250 x 19.5' x 1.502 = 7322.3
- 18 x 16.5' x 1.502 = 446.1
- 10 x 12.5' x 1.502 = 187.8
- 217 x 816' x 1.502 = 265962.1
- #7 - 92 x 12.5' x 2.044 = 2350.6 - 291615.1
- 40 x 24.75' x 2.044 = 2023.6
- 15 x 29.25' x 2.044 = 896.8
- 18 x 16.5' x 2.044 = 607.1
- 10 x 12' x 2.044 = 245.3 - 295387.5
- #8 - 119 x 20 x 2.67 = 6354.6
- 179 x 13 x 2.67 = 6213.1
- 110 x 16 x 2.67 = 4699.2
- 205 x 10 x 2.67 = 5473.5
- #9 - 110 x 32 x 3.4 = 11968.0
- 77 x 13 x 3.4 = 3403.4 - 333499.7
- 231 x 38 x 3.4 = 29845.20

PROJECT	Florida Avenue Complex	Page — of —	COMPUTED BY	DATE
SUBJECT	Concrete		RJgr	11 Sep 76
			CHECKED BY	DATE

Concrete ~

w/ sta 0+60 to w/ sta 1+40.35:

$$\text{Stabilization Slab} = (4\frac{1}{2} \times 82.35' \times 8') / 27 = 8.13 \text{ cy}$$

$$\text{Base Slab} = (2.5 \times 84.85' \times 8') / 27 = 62.85 \text{ cy}$$

$$\text{Walls} = 82.35 \left[ \left( \frac{1}{2} \times \frac{10^2}{24} \right) + 1.0 \right] / 27 = 9.4 \text{ cy}$$

w/ sta 1+40.35 to sta 3+67.90:

$$\text{Stabilization Slab} = (141.55 \times 4\frac{1}{2} \times 11) / 27$$

$$= 19.22 \text{ cy}$$

(Step = 4.89 cy)

$$\text{Base Slab} = (141.55 \times 2.5 \times 11) / 27$$

$$= 144.17 \text{ cy}$$

$$\text{Walls} = \left[ 39.15 \left[ \left( \frac{1}{2} \times \frac{14^2}{24} \right) + 1.0 \right] + \right.$$

$$\left. 104.40 \left[ \left( \frac{1}{2} \times \frac{16^2}{24} \right) + 1.0 \right] \right] / 27$$

$$= 31.86 \text{ cy}$$

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Concrete		RJgr	11 Sep 76
			CHECKED BY	DATE

w/l sta 3+67.90 to w/l sta 4+73.81:

$$\text{Stabilization Slab} = (109.91 \times \frac{1}{2} \times 14) / 27 = 19.0 \text{ cy}$$

$$\text{Base Slab} = (109.91 \times 2.5 \times 14) / 27 = 142.48 \text{ cy}$$

$$\begin{aligned} \text{Walls} &= 88 \left[ \left( \frac{1}{2} \times \frac{16^2}{24} \right) + 1.0 \right] / 27 + \\ &16.91 \left[ \left( \frac{1}{2} \times \frac{16^2}{24} \right) + 1.0 \right] / 27 \\ &= 24.61 \text{ cy} \end{aligned}$$

w/l sta. 5+40.31 to w/l sta. 5+77.49:

$$\text{Stabilization Slab} = \frac{37.18}{27} \left( \frac{1}{2} \right) (11) = 1.26 \text{ cy}$$

$$\text{(Step} = 3.06 \text{ cy)} \quad \text{Base Slab} = \frac{37.18}{27} (2.5)(11) = 37.87 \text{ cy}$$

$$\text{Walls} = \frac{37.18}{27} \left[ \left( \frac{1}{2} \times \frac{16^2}{24} \right) + 1.0 \right] = 8.72 \text{ cy}$$

w/l sta. 5+77.49 to w/l sta 6+50.49:

$$\text{Stabilization Slab} = \frac{27.5}{27} \left( \frac{1}{2} \right) (8) = 2.72 \text{ cy}$$

(Step = 3.33 cy)

$$\text{Base Slab} = \frac{27.5}{27} (2.5)(8) = 20.37 \text{ cy}$$

$$\begin{aligned} \text{Wall} &= \frac{17.5}{27} \left[ \left( \frac{1}{2} \times \frac{14^2}{24} \right) + 1.0 \right] + \frac{10}{27} \left[ \left( \frac{1}{2} \times \frac{9.5^2}{24} \right) + 1.0 \right] \\ &= 4.36 \text{ cy} \end{aligned}$$

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Concrete, Cement		RJgr	11 Sep 76
			CHECKED BY	DATE

Stabilization Slab = 50.33 cy (use 51 cy)

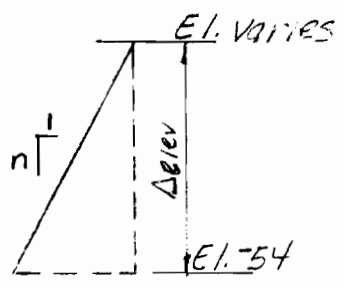
Base Slab = 419.02 cy (use 420 cy)

Walls = 78.95 cy (use 80 cy)

Cement =  $548.3 \times 5.17 = 2835 \text{ cwt}$

Use 2840 cwt

## Concrete Prestressed Piling ~



$$\text{Pile Length} = \Delta \text{elev} \sqrt{1 + 1/n^2}$$

Monolith	Δ elev	No. Piles	Batter	Pile Length	Total
3, 4, 5	55.5	16	2:1	62'	993'
		6	3:1	58.5'	351'
		2	25:1	60'	120'
6	51.5	14	2:1	58'	812'
		8, 9, 10	48.5	59	2:1
11, 12, B	48.5	2	2.5:1	52.5'	105'
		1	3:1	51.5'	51.5'
		2	3.5:1	50.5'	101'
		69	2:1	54.5'	3760.5'
		1	2.5:1	52.5'	52.5'
15	48.5	22	2:1	54.5'	1199'
18	56.0	4	2:1	63'	252'

Total = 11013'

Steel Sheet Pile, PMA-22~

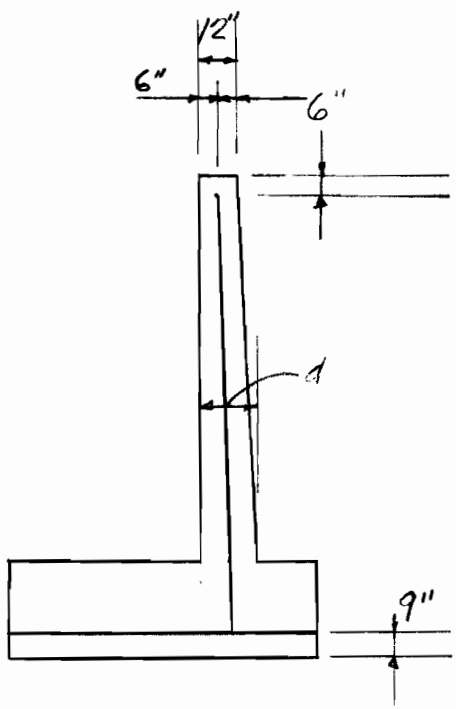
Monolith	Sheet Pile Length, Lp	Δ elev	Lp x Δ elev, sf
3, 4, 5	77.35	21.5	1663.1
6	37.15	17.5	685.1
8, 9, 10, 11, 12, 13	209.31	14.5	3035.0
15	37.18	14.5	539.0
16	17.5	17.5	306.25
18	10	22	220.0

Total = 6448.45

(Use 6500 sf)



Waterstops, 3-Bulb type



Typical 3-Bulb Waterstop Detail

W/L sta 0+91.0, sta. 1+19.35, sta 1+40.35:

$$3L = (11.25' + 8')3 = 57.75'$$

W/L sta 1+79.50, sta 2+62.5 :

$$2L = (15.25 + 11)2 = 52.5'$$

W/L sta 2+77.12, sta. 3+22.9, sta 3+67.9:

$$3L = (18.25 + 11)3 = 87.75'$$

W/L sta 4+11.90, sta. 4+29.81, sta. 4+73.81:

$$3L = (18.25 + 14)3 = 96.75'$$

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	T-wall; Waterstops		RJR	11 Sep 76
			CHECKED BY	DATE

$$w/l \text{ sta. } 5+40.31: L = 18.25 + 11 = 29.25'$$

$$w/l \text{ sta } 5+77.49: L = 15.25 + 8 = 23.25'$$

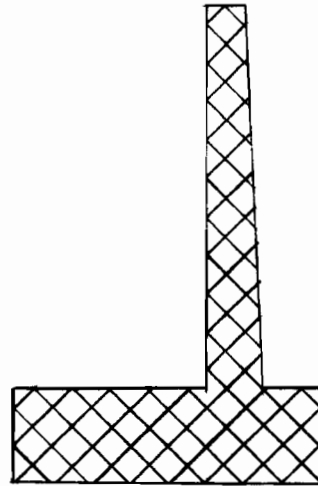
$$w/l \text{ sta } 5+94.99: L = 10.75 + 8 = 18.75'$$

$$w/l \text{ sta } 6+40.49: L = 10.75 + 8 = 18.75'$$

$$\text{Total} = 384.75'$$

(Use 390.2f)

## Expansion Joint Material ~



Typical Area

w/L sta. 0+91.00, sta. 1+19.35, sta. 1+40.35:

$$3A = \left[ \frac{1}{2}(10)\left(2 + \frac{10}{24}\right) + 8(2.5) \right] 3 = 96.25 \text{ sf}$$

w/L sta. 1+79.5, sta. 2+62.5:

$$2A = \left[ \frac{1}{2}(14)\left(2 + \frac{14}{24}\right) + 11(2.5) \right] 2 = 91.2 \text{ sf}$$

w/L sta. 2+77.12, sta. 3+22.90, sta. 3+67.90:

$$3A = \left[ \frac{1}{2}(17)\left(2 + \frac{17}{24}\right) + 11(2.5) \right] 3 = 151.56 \text{ sf}$$

w/L sta. 4+11.90, sta. 4+29.81, sta. 4+73.81:

$$3A = \left[ \frac{1}{2}(17)\left(2 + \frac{17}{24}\right) + 14(2.5) \right] 3 = 174.06 \text{ sf}$$

PROJECT	Florida Avenue Complex	COMPUTED BY	DATE
SUBJECT	Expansion Joint Material	RJar	11 Sep 76
		CHECKED BY	DATE

$$\text{w/L sta } 5+40.31: A = \frac{1}{2}(17)(2 + \frac{17}{24}) + 11(2.5) = 50.52 \text{ sf}$$

$$\text{w/L sta } 5+77.49: A = \frac{1}{2}(14)(2 + \frac{14}{24}) + 8(2.5) = 38.08 \text{ sf}$$

w/L sta 5+94.99, w/L sta 6+40.49:

$$2A = 2[\frac{1}{2}(9.5)(2 + \frac{9.5}{24}) + 8(2.5)] = 62.76 \text{ sf}$$

$$\text{Total} = 664.43 \text{ sf}$$

(Use 670 sf)



Concrete -

$$\text{Stabilization Slab} = (83' \times 11' \times 4\frac{1}{2}') / 27$$

$$= 11.3 \text{ cy (Use 11.3 cy)}$$

$$\text{Base Slab} = (2.5' \times 11' \times 8\frac{1}{2}') / 27$$

$$= 84.5 \text{ cy (Use 85 cy)}$$

$$\text{Walls} = 40 \left[ \left( \frac{1}{2} \times \frac{14.33^2}{24} \right) + 1.7 \right] / 27$$

$$= 7.8 \text{ cy (Use 8.0 cy)}$$

$$\text{Column 1} = (3.5' \times 2' \times 22.83') / 27$$

$$= 5.9 \text{ cy (Use 6.0 cy)}$$

$$\text{Column 2} = 22.5 \left[ \left( \frac{1}{2} \times 2.5' \times 2' \times \frac{14}{22.5} \right) + 2 \times 2.5 \right] / 27$$

$$= 5.2 \text{ cy (Use 5.5 cy)}$$

$$\text{Column 3} = 22.5 \left[ \left( \frac{1}{2} \times 2.5' \times 2' \times \frac{14}{22.5} \right) + 2 \times 2.5 \right] / 27$$

$$= 5.2 \text{ cy (Use 5.5 cy)}$$

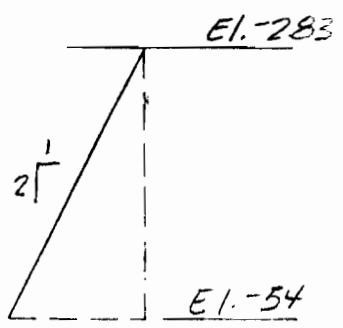
$$\text{Overhead Beam} = [(2.75' \times 2) + (1.5 \times 1.75)] 70.5 / 27$$

$$= 21.2 \text{ cy (Use 21.5 cy)}$$

$$\text{Cement} = 141.1 \times 5.17 = 729.5 \text{ cwt}$$

(Use 730 cwt)

### Concrete Prestressed Piles ~



$$\text{Pile Length} = \Delta \text{elev} \sqrt{1 + 1/n^2}$$

$$\Delta \text{elev} = 56.24', \text{ say } 57'$$

$$\text{Pile Length} = 32 \times 64' = 2048 \text{ lf (Use 2050')}$$

### Steel Sheet Piling ~

$$\text{Depth of Piling} = 18' \times 83' = 1494 \text{ sf (Use 1500 sf)}$$

PROJECT	Florida Avenue Complex	Page ___ of ___	DESIGNED BY	DATE
SUBJECT	Structural Steel		RJ9r	10 Sep 76
			CHECKED BY	DATE

Structural Steel -

$$\text{Gate Weight} = 27000 \text{ lb}$$

$$\begin{aligned} \text{Misc. Metals} &= 50 \times 70.5 + 2 \times 25 \times 70.5 \\ &= 705 \text{ lb} \quad (\text{use } 1000 \text{ lb}) \end{aligned}$$



PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Reinforcement		RJG	10 Sep 76
			CHECKED BY	DATE

Reinforcement ~

Base Slab -

- 12 #8 at 83' - Top, Longitudinal
- 13 #10 at 83' - Bottom, Longitudinal
- #8 at 12" (22') - Transverse

T-Wall - Horizontal

- #5 at 12" (40') - F.S.
- #6 at 12" (40') - P.S.

- Vertical

- #6 at 12" (9.5') - FS
- #10 at 12" (9.0') - FS.
- #6 at 12" (19.0') - PS

Overhead beam -

- 5-#4 at 70.5'
- 11-#11 at 70.5'

Column 1 -

- 9-#11 from El 0.0 to El 14.0 (17'/bar)
- 8-#11 from El 14.0 to El 22.5 (9'/bar)

Column 3; 5 -

- 14-#10 from El 0.0 to El 14.0 (17'/bar)
- 10-#10 from El 14.0 to El 14.0 (9'/bar)

PROJECT	Florida Avenue Complex	COMPUTED BY	DATE
SUBJECT	Reinforcement	RJgr	10 Sep
		CHECKED BY	DATE

$$\#4 - 5 \times 70.5' \times 0.668 = 235.5$$

$$\#5 - 14 \times 40' \times 1.043 = 584.1$$

$$\#6 - 14 \times 40' \times 1.502 = 841.1$$

$$40 \times 9.5' \times 1.502 = 570.8$$

$$40 \times 19' \times 1.502 = 1141.5$$

$$\#8 - 12 \times 83' \times 2.67 = 2659.3$$

$$83 \times 22' \times 2.67 = 4875.4$$

$$\#10 - 13 \times 83' \times 4.303 = 4642.9$$

$$40 \times 9' \times 4.303 = 1549.1$$

$$14 \times 17' \times 4.303 = 1024.1$$

$$10 \times 9' \times 4.303 = 387.3$$

$$\#11 - 11 \times 70.5' \times 5.313 = 4120.2$$

$$9 \times 17' \times 5.313 = 812.9$$

$$8 \times 9' \times 5.313 = 382.5$$

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23826.7

(Use 25000 lb)



PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Concrete, Cement		RJgr	10 Sep
			CHECKED BY	DATE

Concrete ~

$$\begin{aligned} \text{Base Slab} &= (2.5 \times 8 \times 45.5) / 27 \\ &= 33.7 \text{ CY} \quad (\text{Use } 34 \text{ CY}) \end{aligned}$$

$$\begin{aligned} \text{Stabilization Slab} &= (45.5 \times 8 \times 4/12) / 27 \\ &= 4.5 \text{ CY} \quad (\text{Use } 4.5 \text{ CY}) \end{aligned}$$

$$\begin{aligned} \text{Walls} &= 21.5 \left[ \frac{1}{2} \times \frac{9.5^2}{24} + 1.0 \right] / 27 \\ &= 1.5 \text{ CY} \quad (\text{Use } 1.5 \text{ CY}) \end{aligned}$$

$$\begin{aligned} \text{Columns} &= (2 \times 2 \times 10.5) 2 / 27 \\ &= 3.1 \text{ CY} \quad (\text{Use } 3.5 \text{ CY}) \end{aligned}$$

$$\begin{aligned} \text{Sill} &= (1.5 \times 23 \times 2.46) / 27 \\ &= 3.1 \text{ CY} \quad (\text{Use } 3.5 \text{ CY}) \end{aligned}$$

$$\begin{aligned} \text{Cement} &= 45.9 \times 5.17 = 237.3 \text{ cwt} \\ &(\text{Use } 240 \text{ cwt}) \end{aligned}$$

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Reinforcement		RJR	10 Sep 76
			CHECKED BY	DATE

Reinforcement ~

Base slab, Top & Bottom, Transverse

#9, 12" from 0'-8'

Base slab, Top & Bottom, Longitudinal

#9, 12" from 0-45.5'

Columns,

16- #11 from E1.2 to E1.15

Wall, Both sides, Horizontal,

#6, 12" from E1.45 to E1.14.0

, Vertical

#5, 12" for 21.5'

#7, 12" for 21.5'

Sill,

8- #6 for 23'-0"

#6, 12" for 23'-0" (6.5')

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Reinforcement		RJGR	10 Sep 76
			CHECKED BY	DATE

$$\#6 - 8 \times 23' \times 1.502 = 276.4$$

$$22 \times 6.5' \times 1.502 = 224.5$$

$$22 \times 21.5' \times 1.502 = 710.4$$

$$\#7 - 22 \times 12' \times 2.044 = 539.6$$

$$\#8 - 22 \times 12' \times 2.61 = 704.9$$

$$\#9 - 46 \times 20' \times 3.4 = 3128.0$$

$$16 \times 45.5' \times 3.4 = 2475.2$$

$$\#11 - 22 \times 13' \times 5.313 = 1519.5$$

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9578.5 lb.

(Use 9600 lb)

## Steel Sheet Piling ~

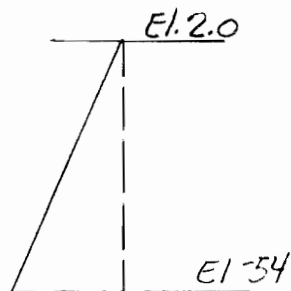
$$\text{Depth} = 22'$$

$$\text{Length} = 45.5'$$

$$A = 45.5' \times 22' = 1001 \text{ sf}$$

Use 1100 sf

## Concrete Prestressed Piling



$$\text{Pile Length} = \Delta \text{Elev} \sqrt{1 + 1/n^2}$$

$$18 \text{ Piles at } 2:1 \text{ Batter: } L_p = 18 \times 63' = 1134'$$

$$4 \text{ Piles at } 4:1 \text{ Batter: } L_p = 4 \times 58' = 232'$$

$$\underline{1366'}$$

(Use 1400')

REASONABLE CONTRACT ESTIMATE

SHEET OF

PROJECT *Fill/Excavation - East*

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
	<i>Fill</i>	<i>1600</i>	<i>CY</i>		
	<i>Excavation</i>	<i>1480</i>	<i>CY</i>		



<i>End Area An (cy/l)</i>	<i>Average End Area 1/2 (An + An+1) (cy/l)</i>	<i>Length Ln (ft)</i>	<i>Estimated Vol. 1/2 (An + An+1) Ln (cy)</i>
<i>1.40</i>	<i>3.81</i>	<i>28.0</i>	<i>106.68</i>
<i>6.22</i>	<i>6.27</i>	<i>29.3</i>	<i>183.71</i>
<i>6.61</i>			<i>290.37</i>
<i>1.34</i>	<i>1.81</i>	<i>42.7</i>	<i>77.29</i>
<i>2.28</i>	<i>2.03</i>	<i>19.43</i>	<i>39.44</i>
<i>1.78</i>			<i>407.12</i>
<i>1.54</i>	<i>3.39</i>	<i>30.57</i>	<i>103.63</i>
<i>5.24</i>	<i>4.91</i>	<i>34.0</i>	<i>166.94</i>
<i>4.58</i>	<i>3.20</i>	<i>60.0</i>	<i>192.0</i>
<i>1.82</i>	<i>1.59</i>	<i>15.64</i>	<i>24.87</i>
<i>1.35</i>	<i>1.39</i>	<i>65.88</i>	<i>91.57</i>
<i>1.42</i>	<i>1.67</i>	<i>50.00</i>	<i>83.50</i>
<i>1.92</i>	<i>1.73</i>	<i>16.90</i>	<i>29.24</i>
<i>1.53</i>			<i>1098.87</i>
<i>1.50</i>	<i>1.70</i>	<i>25.0</i>	<i>42.5</i>
<i>1.89</i>	<i>3.89</i>	<i>20.0</i>	<i>77.8</i>
<i>5.89</i>			<i>1219.17</i>

PROJECT	Florida Avenue Complex	Page <u>    </u> of <u>    </u>	COMPUTED BY	DATE
SUBJECT	Fill Volume - East		RJgr	9 Sep 76
			CHECKED BY	DATE

End Area $A_n$ (cy/l')	Average End Area $\frac{1}{2}(A_n + A_{n+1})$ (cy/l')	Length $L_n$ (ft)	Estimated Vol. $\frac{1}{2}(A_n + A_{n+1})L_n$ (cy)
Continued from the previous page.			
9.93	6.30	37.99	239.34
2.66	2.63	4.98	13.10
2.59	2.30	13.01	29.92
2.01	1.05	40.21	42.22
0			
0	0.15	20.41	2.04
0.3	0.33	50.0	16.50
0.35	0.18	50.0	9.00
0	0.12	89.03	10.68
0.23			

$\frac{224.56}{1548.75}$

1581.97

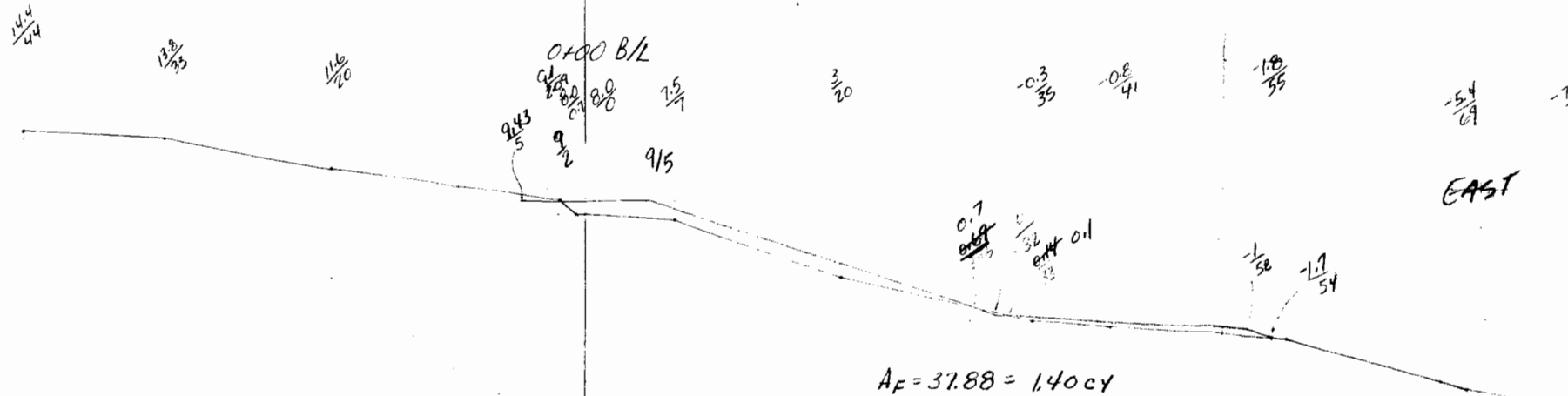
Estimated Fill Volume = 1581.97  
 (Use 1600 cy)

PROJECT		Page	of	COMPUTED BY	DATE
Florida Avenue Complex				RJgr	9 Sep 76
SUBJECT				CHECKED BY	DATE
Excavation Volume - East					
End Area $A_n$ (cy/l')	Average End Area $\frac{1}{2}(A_n + A_{n+1})$ (cy/l')	Length $L_n$ ft	Estimated Vol. $\frac{1}{2}(A_n + A_{n+1})L_n$ (CY)		
0	1.30	42.7	55.51		
2.60	2.88	19.43	55.96	111.47	
3.17					
3.54	4.90	30.57	149.79		
6.26	3.13	34.0	106.42		
0	0.69	15.64	10.79	378.47	
1.37					
1.37	1.54	65.88	101.46		
1.70	1.59	50.00	79.50		
1.47	1.77	16.90	29.91	589.34	
2.06					
2.0	1.35	25.0	33.75		
0.7	0.35	20.0	7.00	630.09	
0					
1.98	2.69	37.99	102.19		
3.40	3.37	4.98	16.78	749.06	
3.33					

PROJECT	Florida Avenue Complex	DATE	9 Sep 76
SUBJECT	Excavation Volume - East	CHECKED BY	RJG

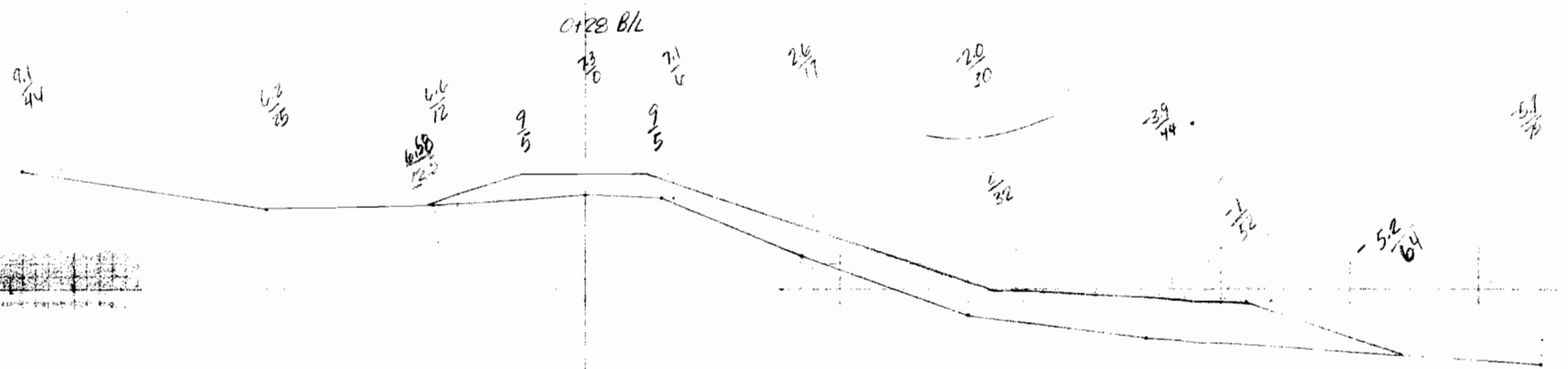
End Area An (cy/l)	Average End Area $\frac{1}{2}(An + Antl)$ (cy/l)	Length Ln (ft)	Estimated Vol. $\frac{1}{2}(An + Antl)Ln$ (cy)
Continued from the previous page.			
1.72	2.53	13.01	32.92
4.6	3.16	40.21	127.06 <sub>909.04</sub>
1.6	1.89	20.41	38.57
2.17	2.25	50.0	112.50
2.33	3.08	50.0	154.00
3.83	2.96	89.03	263.53 <sub>1477.64</sub>
2.08			

Estimated Excavation Volume = 1477.64  
(Use 1480 cy)

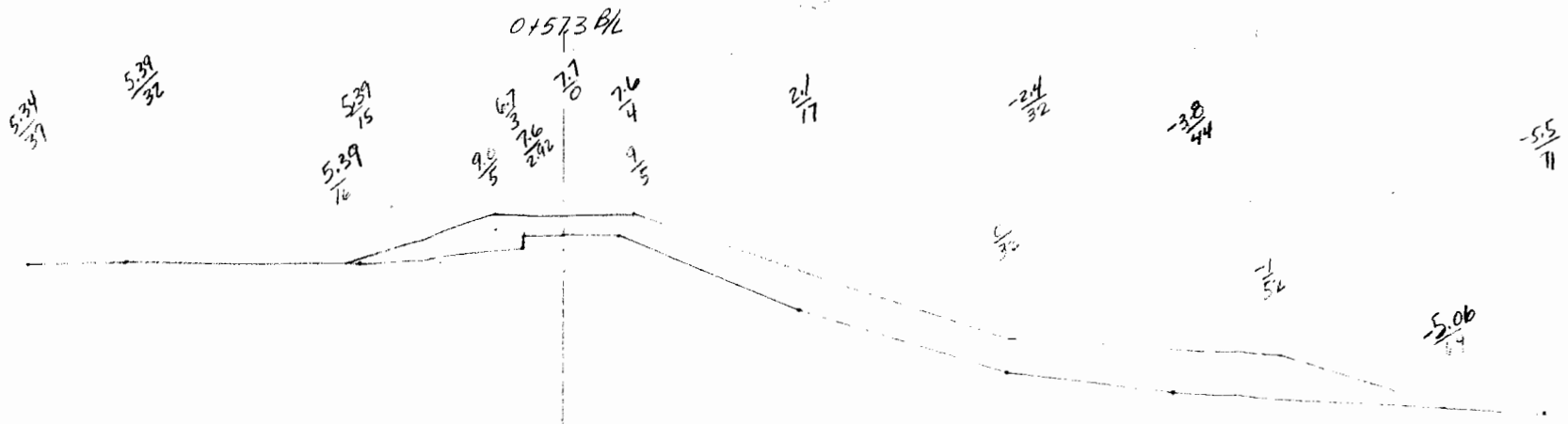


$A_F = 37.88 = 1.40 \text{ CY}$

$A_E = 0$

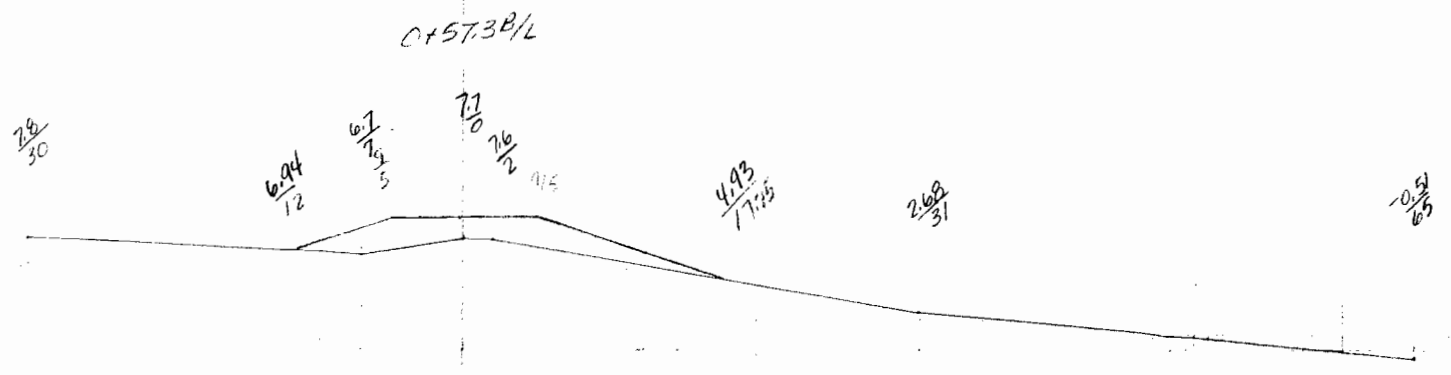


$A_F = 167.88 = 6.22 \text{ CY}$

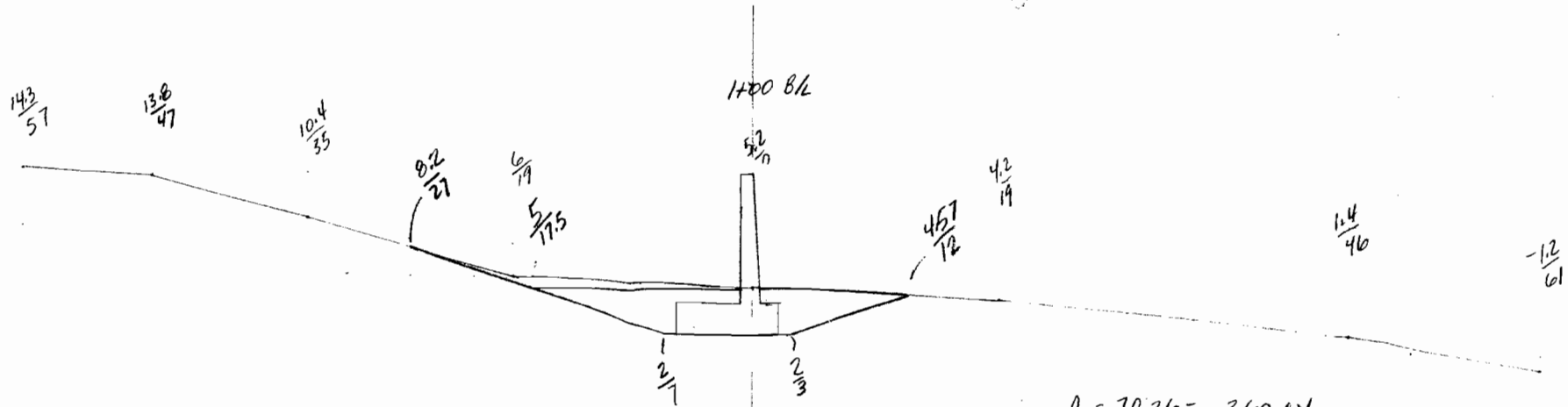


$A_f = 178.42 = 6.61 \text{ cy}$

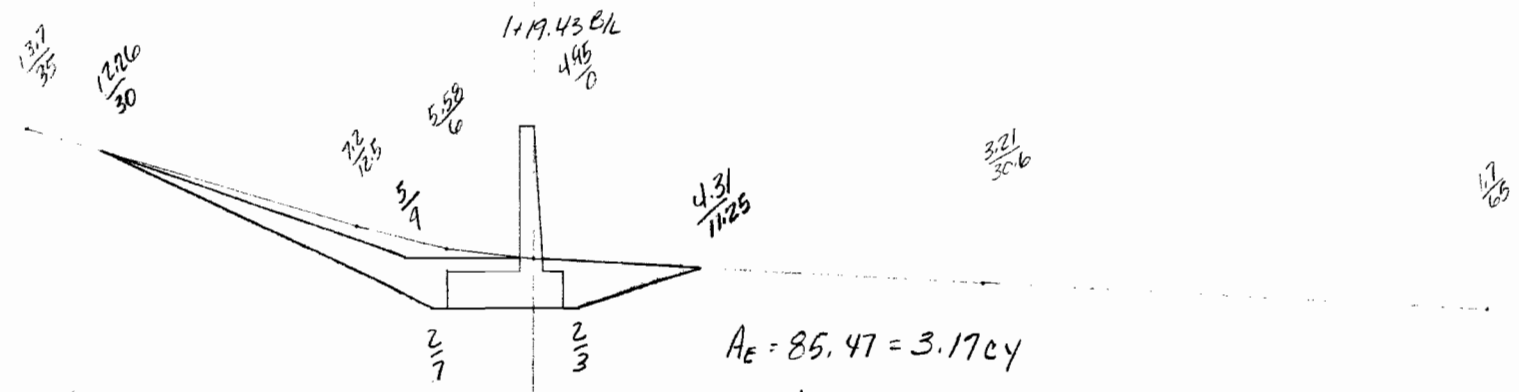
5.13  
2.8  
2.1



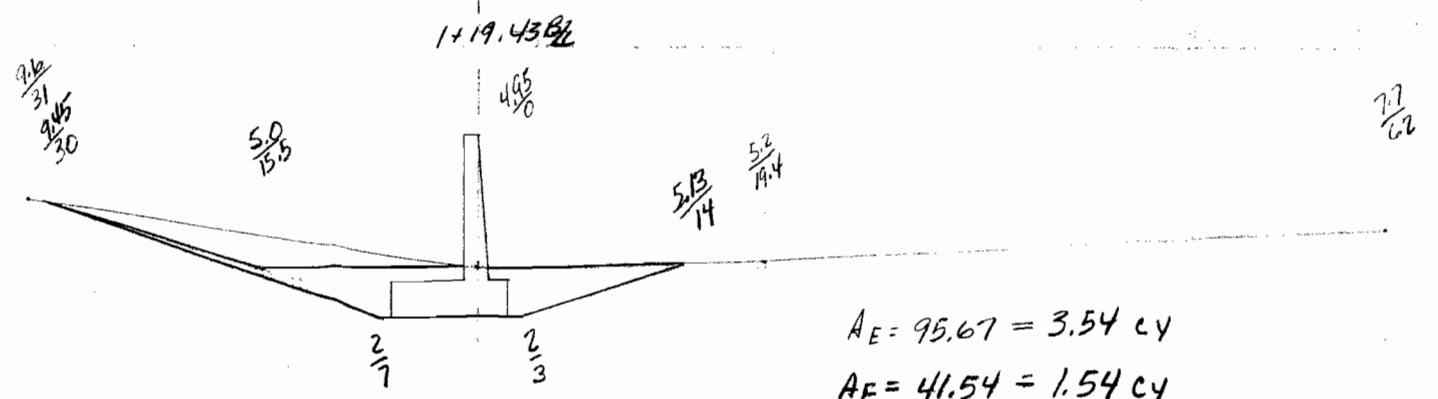
$A_f = 36.12 = 1.34 \text{ cy}$



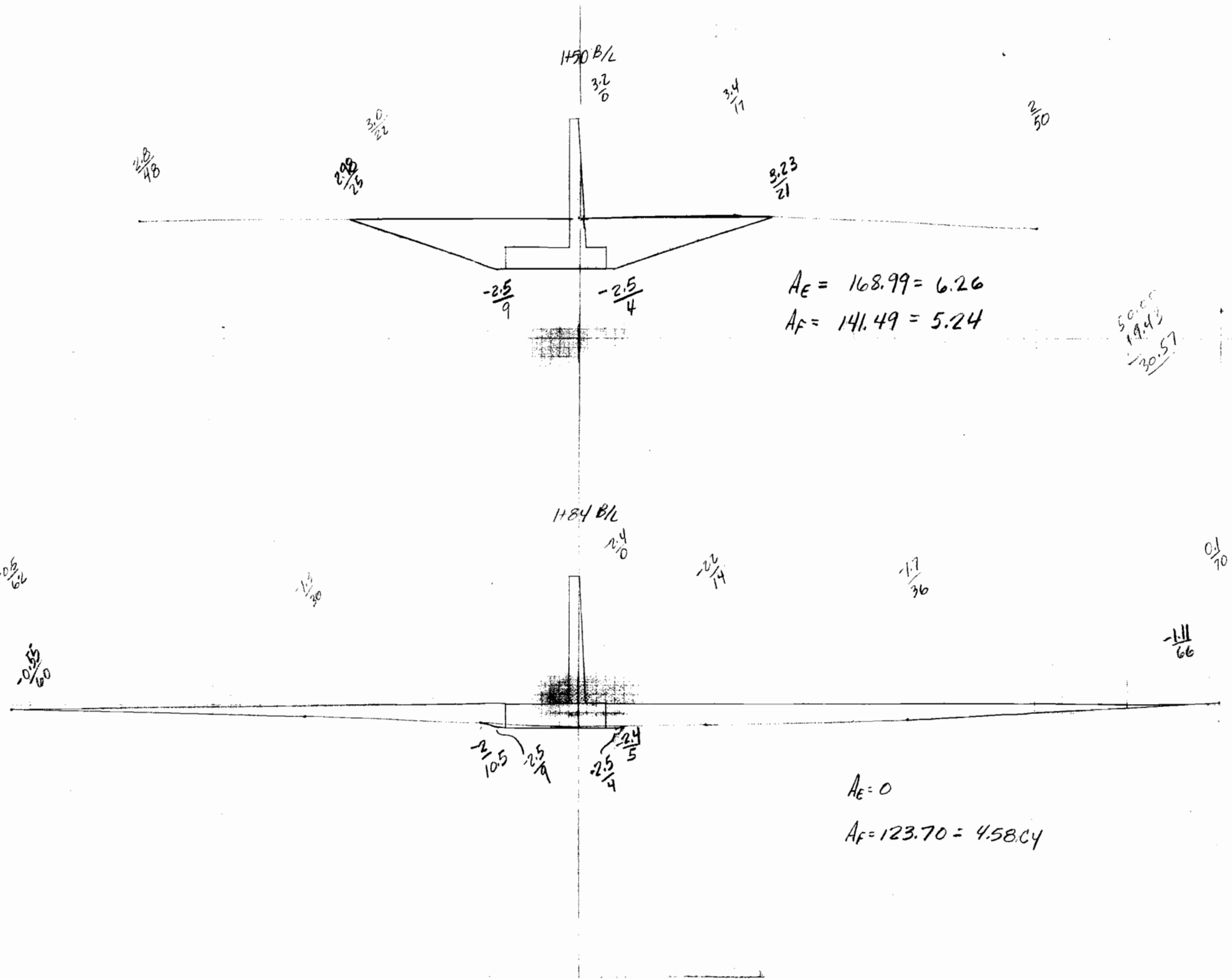
$A_E = 70.26 = 2.60 \text{ cy}$   
 $A_F = 61.51 = 2.20 \text{ cy}$



$A_E = 85.47 = 3.17 \text{ cy}$   
 $A_F = 48.07 = 1.78 \text{ cy}$

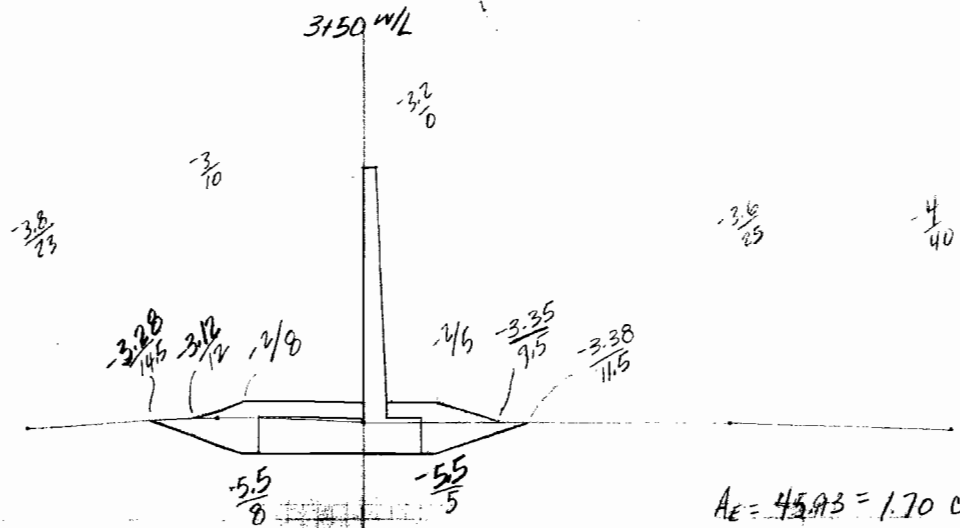


$A_E = 95.67 = 3.54 \text{ cy}$   
 $A_F = 41.54 = 1.54 \text{ cy}$



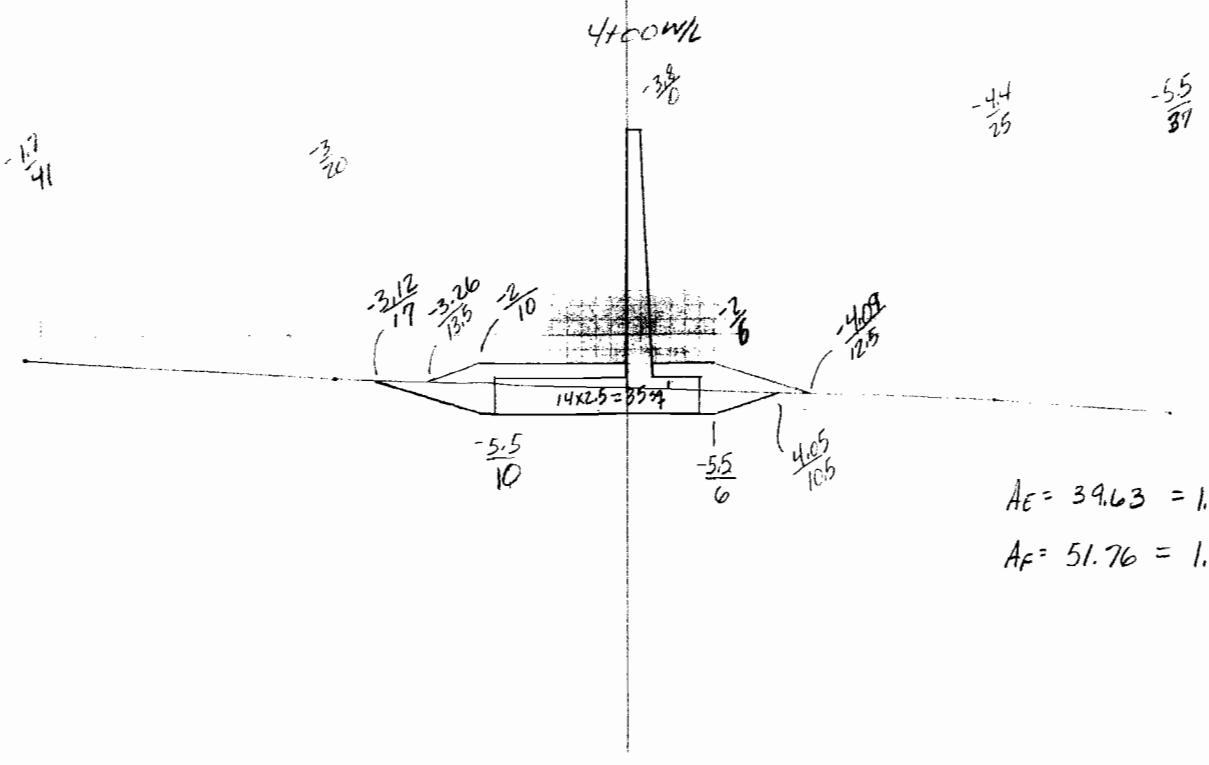




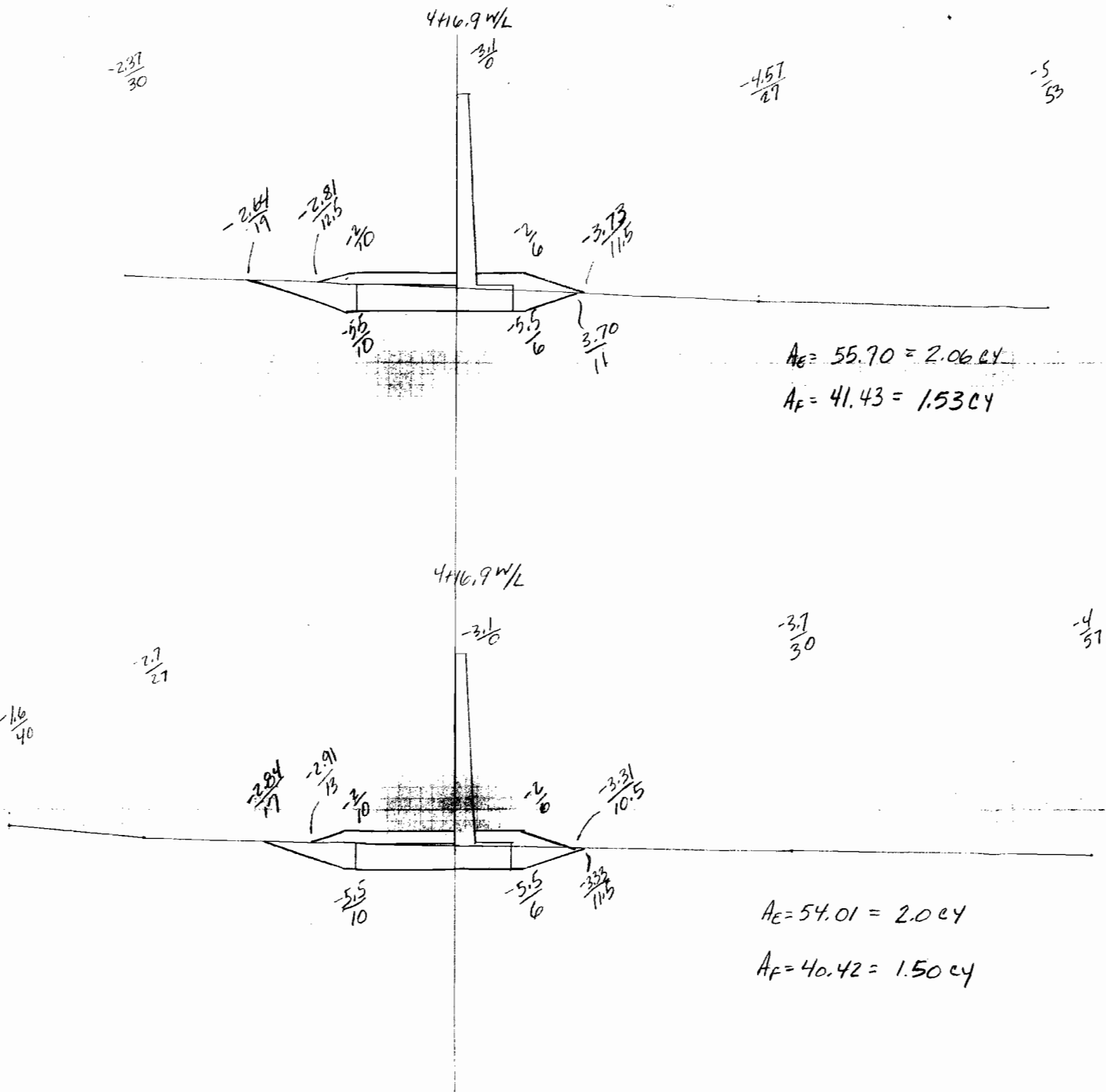


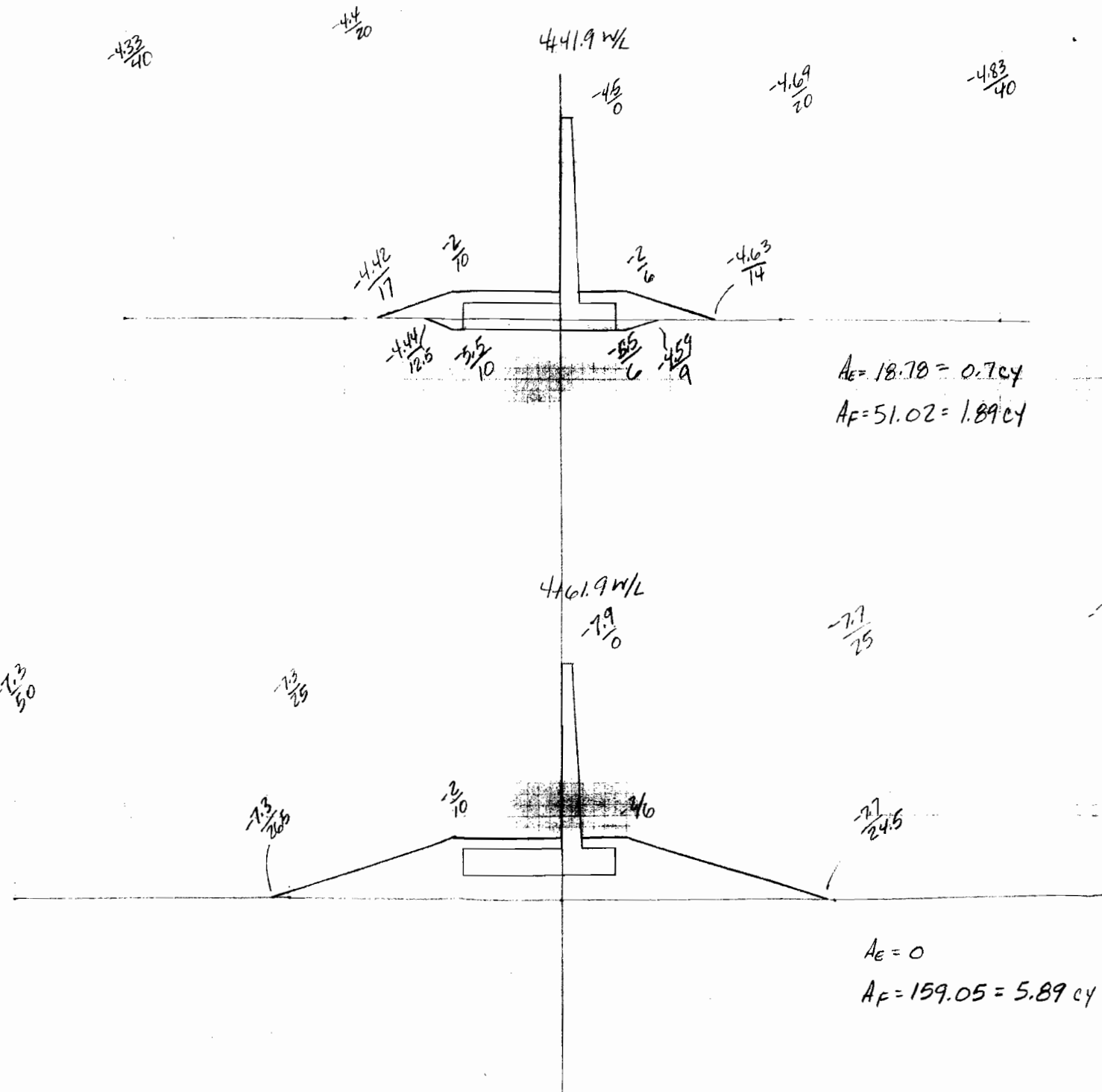
$$\begin{array}{r} 350.70 \\ 218.12 \\ \hline 65.88 \end{array}$$

$A_E = 45.93 = 1.70 \text{ cy}$   
 $A_F = 38.39 = 1.42 \text{ cy}$



$A_E = 39.63 = 1.47 \text{ cy}$   
 $A_F = 51.76 = 1.92 \text{ cy}$





$43.3/40$

$44/20$

$44.9 \text{ W/L}$

$46/0$

$4.69/20$

$4.83/40$

$4.42/17$

$2/10$

$2/10$

$4.63/14$

$4.44/12.5$

$5/10$

$5.5/6$

$4.59/9$

$44.9 \text{ W/L}$   
 $44.0/5.0$

$7.3/50$

$7.3/25$

$46.9 \text{ W/L}$

$7.9/0$

$7.7/25$

$7.2/50$

$7.3/24.5$

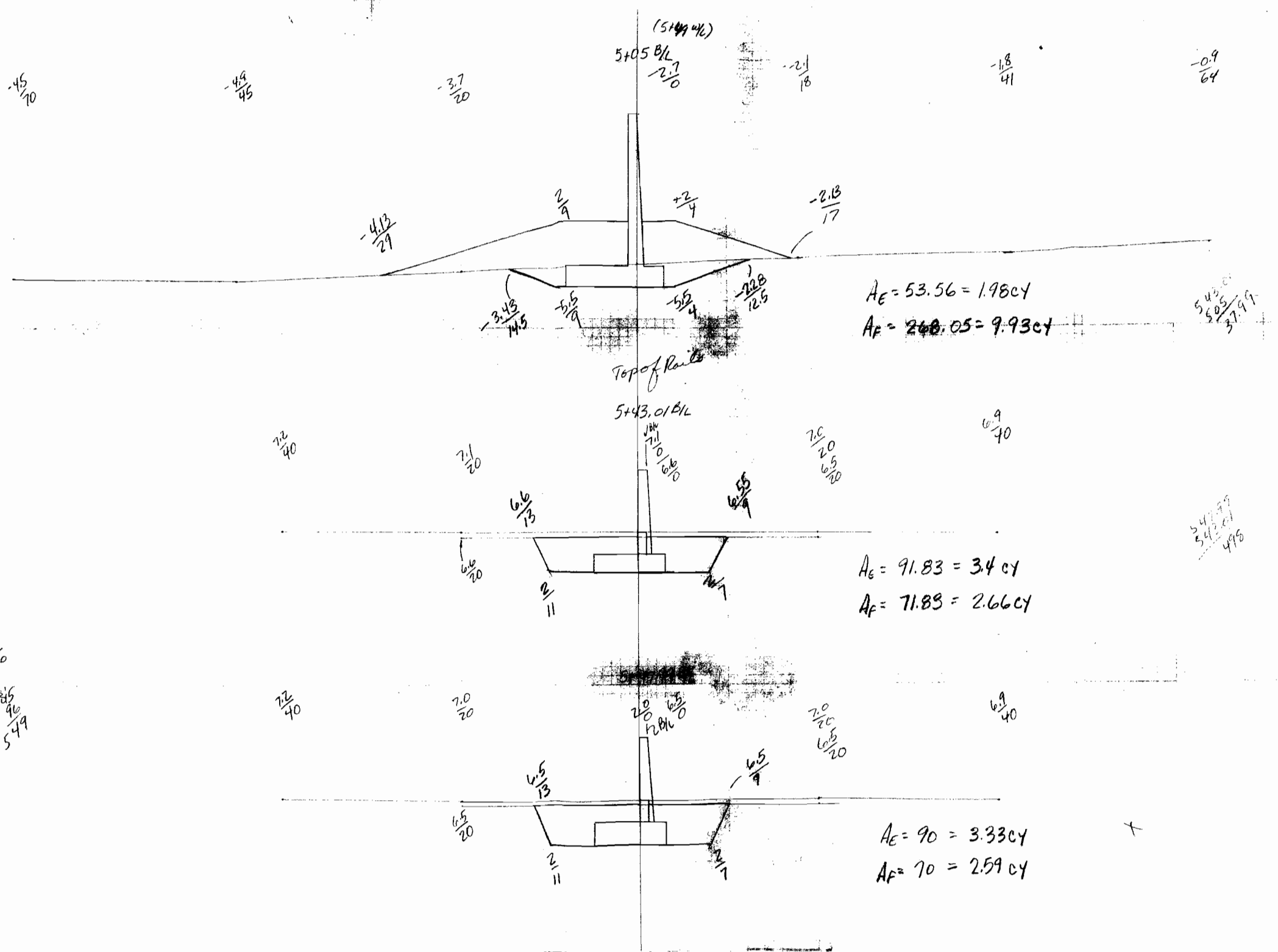
$3/10$

$4/0$

$7.7/24.5$

$A_e = 0$

$A_f = 159.05 = 5.89 \text{ cy}$



(5+44.46)  
5+05 B/L  
2.7  
2.0

$A_e = 53.56 = 1.98 \text{ cy}$   
 $A_f = 268.05 = 9.93 \text{ cy}$

Top of Raft  
5+43.01 B/L

$A_e = 91.83 = 3.4 \text{ cy}$   
 $A_f = 71.83 = 2.66 \text{ cy}$

$A_e = 90 = 3.33 \text{ cy}$   
 $A_f = 70 = 2.59 \text{ cy}$

$4.5/20$

$4.9/45$

$3.7/20$

$2.1/18$

$1.8/41$

$0.9/64$

$4.13/29$

$2/9$

$2/4$

$2.13/17$

$3.43/14.5$

$5/10$

$5.5/4$

$2.28/12.5$

$5.45/37.99$

$7.2/40$

$7.1/20$

$5.6/13$

$2/11$

$6.5/9$

$7.0/20$

$6.9/40$

$5.45/49.6$

$5.01/5.03/9$

$6.85/9.6/5.79$

$7.2/40$

$7.0/20$

$5.5/13$

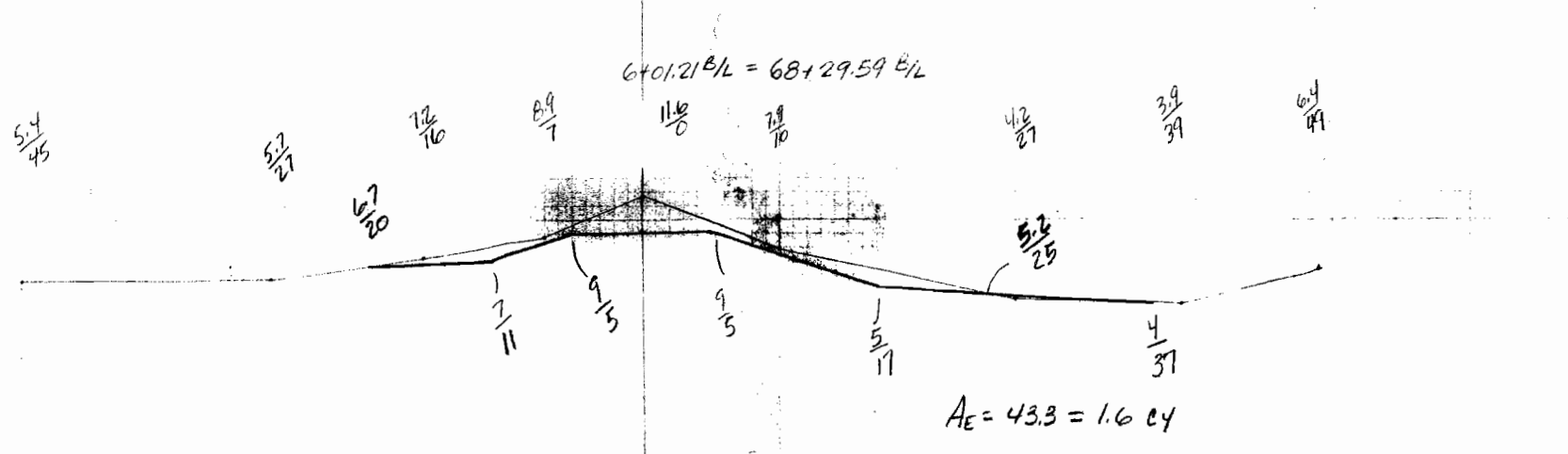
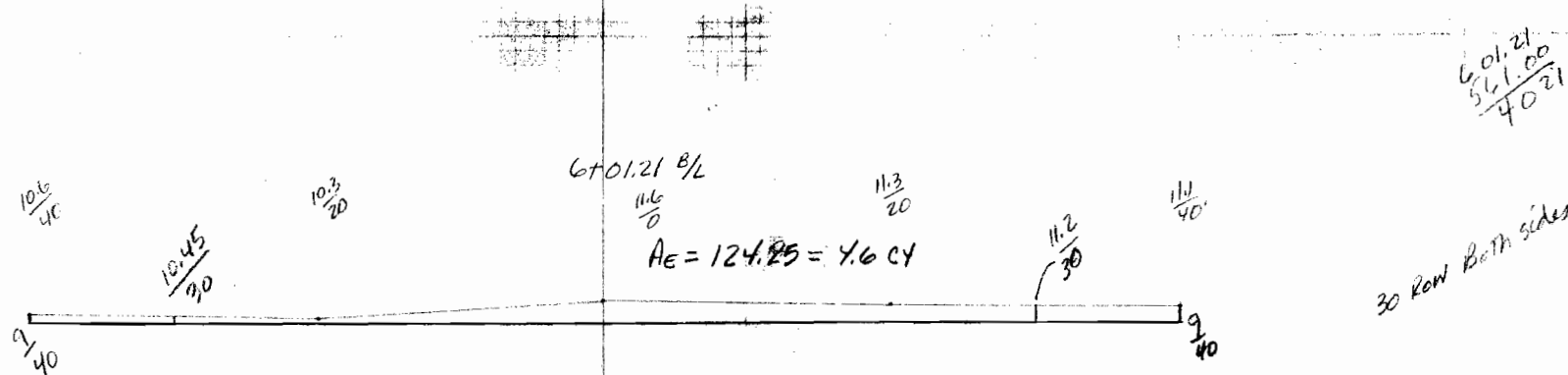
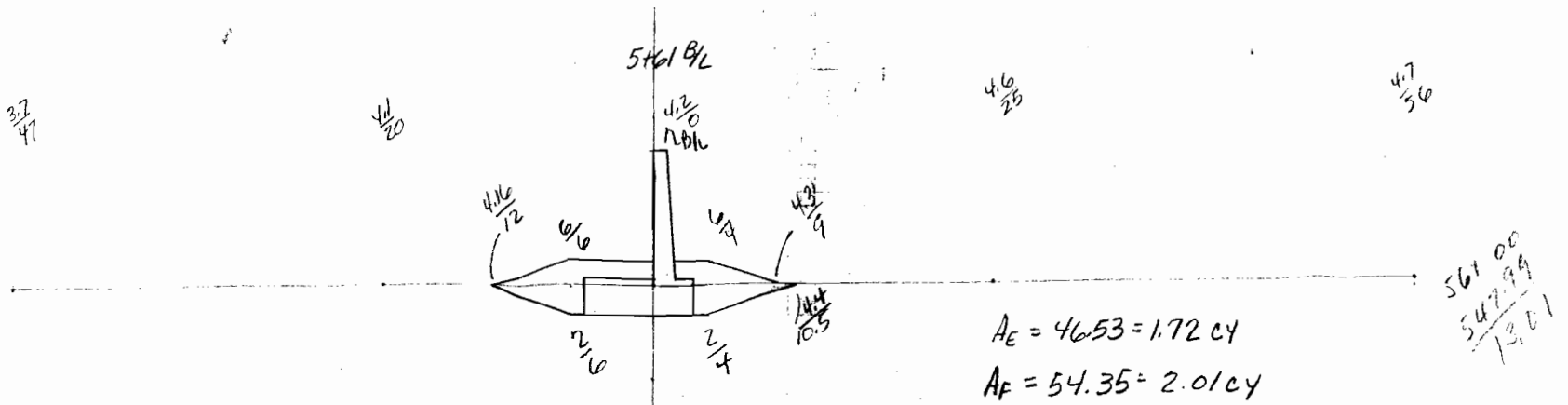
$2/11$

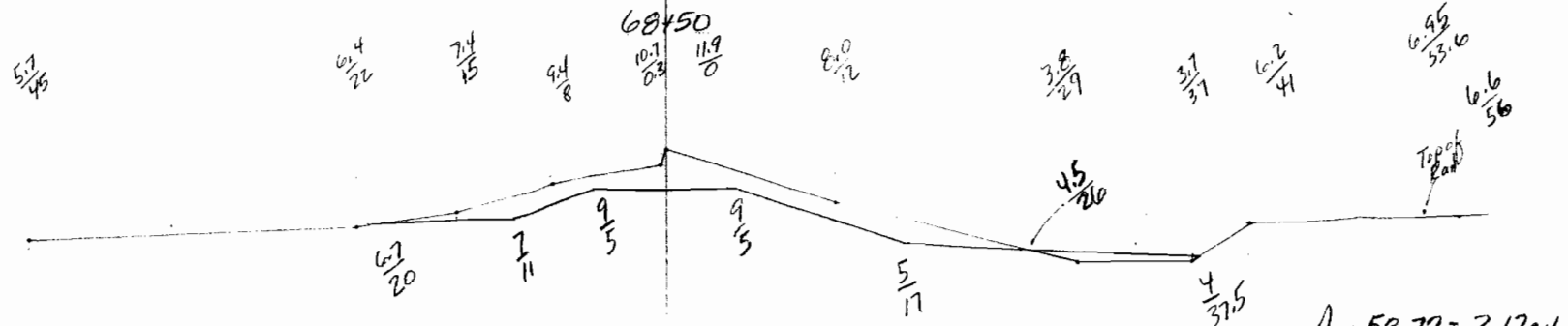
$6.5/9$

$7.0/20$

$6.9/40$

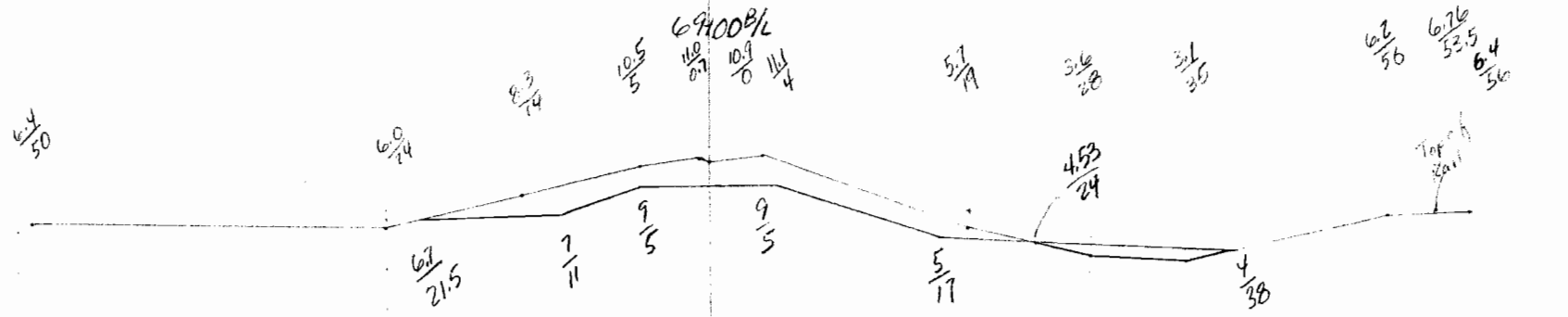
x



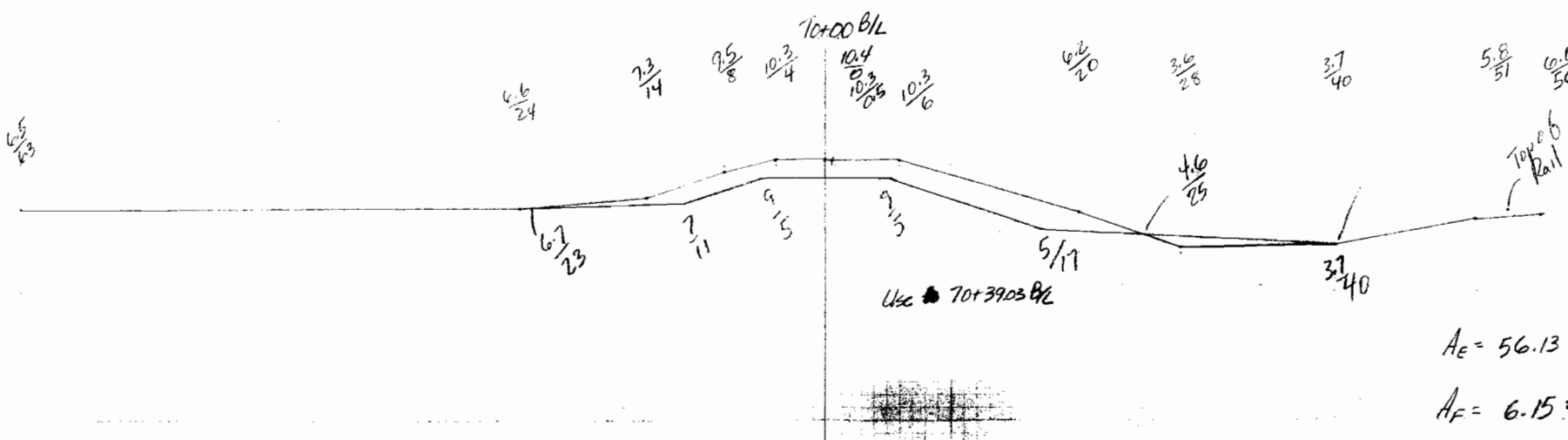
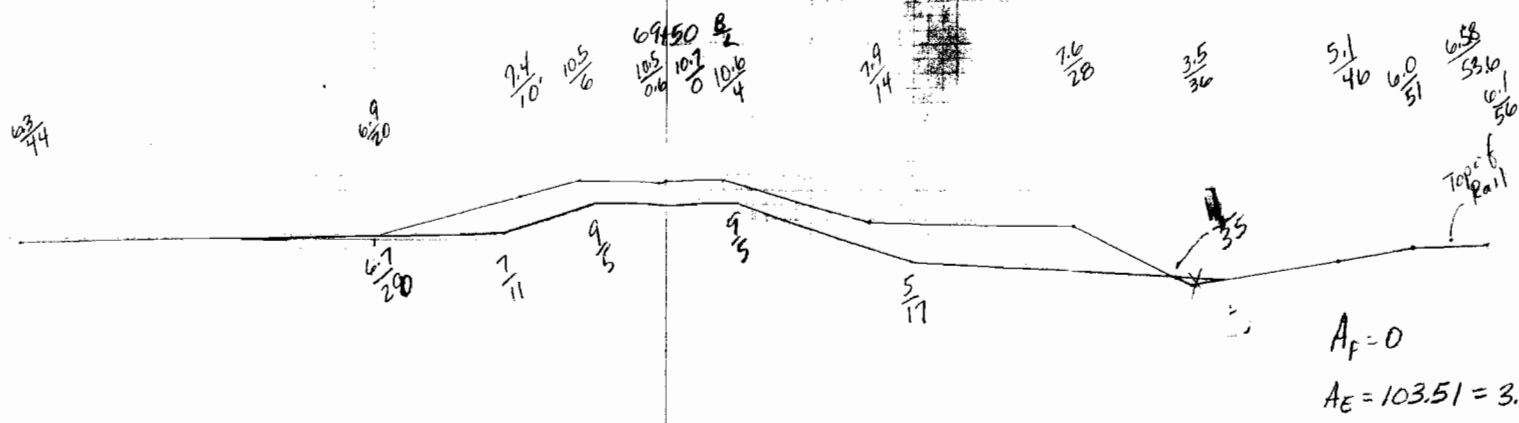


$A_E = 58.72 = 2.17 \text{ cy}$   
 $A_F = 9.0 = 0.32 \text{ cy}$

$68.50.00$   
 $68.29.57$   
 $20.43$



$A_E = 63.04 = 2.33 \text{ cy}$   
 $A_F = 9.35 = 0.35 \text{ cy}$





PROJECT	Florida Avenue Complex	Page		DATE	9 Sept 78
SUBJECT	Bypass Channel Excavation	CHECKED BY		DATE	

Bypass Channel Excavation

End Area $A_n$ (CY/1)	Average End Area $\frac{1}{2}(A_n + A_{n+1})$ (CY/1)	Length $L_n$ (ft.)	Volume $\frac{1}{2}(A_n + A_{n+1})L_n$ (CY)
18.9	19.5	11.0	214.5
20.1	20.0	14.0	280.0
19.9	19.6	20.	392.0
19.2	18.7	20.	374.0
18.2	18.9	20.	378.0
19.6	18.2	20.	364.0
16.8	17.8	20	356.0
18.8	17.9	23.	411.7
17.0			

2770.2 CY  
(Use 2800 CY)

PROJECT	Florida Avenue Complex	Page ___ of ___	COMPUTED BY	DATE
SUBJECT	Bypass Channel Fill		RJR	9 Sep 76
			CHECKED BY	DATE

Bypass Channel Fill-

End Area $A_n$ (cy/l)	Average End Area $\frac{1}{2}(A_n + A_{n+1})$ (cy/l)	Length $L_n$ (ft)	Volume $\frac{1}{2}(A_n + A_{n+1})L_n$ (cy)
18.9	19.0	11	209.0
19.9	19.9	14	278.6
19.9	20.7	20	414.0
21.5	22.8	20	456.0
24.1	22.2	20	444.0
20.2	18.5	20	370.0
16.8	17.8	20	356.0
18.8	17.9	23	411.7
17.0			

2934.3 cy  
(Use 3000 cy)

*Structure Excavation*

<i>End Area An (cy/ft)</i>	<i>Average End Area <math>\frac{1}{2}(A_n + A_{n+1})</math> (cy/ft)</i>	<i>Length Ln (ft)</i>	<i>Volume <math>\frac{1}{2}(A_n + A_{n+1})L_n</math> (cy)</i>
<i>9.4</i>			
<i>14.8</i>	<i>12.1</i>	<i>11</i>	<i>133.1</i>
<i>20.1</i>	<i>17.5</i>	<i>14</i>	<i>245.0</i>
<i>30.6</i>	<i>25.4</i>	<i>20</i>	<i>508.0</i>
<i>31.7</i>	<i>31.2</i>	<i>20</i>	<i>624.0</i>
<i>28.4</i>	<i>30.1</i>	<i>20</i>	<i>602.0</i>
<i>22.9</i>	<i>25.7</i>	<i>20</i>	<i>514.0</i>
<i>14.0</i>	<i>18.5</i>	<i>20</i>	<i>370.0</i>
<i>7.1</i>	<i>10.6</i>	<i>23</i>	<i>243.8</i>

*3239.9 cy  
(Use 3300cy)*

PROJECT <i>Florida Avenue Complex</i>	Page <i>—</i> of <i>—</i>	COMPUTED BY <i>RJR</i>	DATE <i>9 Sep 76</i>
SUBJECT <i>Structure Fill</i>		CHECKED BY	DATE

*Structure Fill-*

<i>End Area An (cy/l)</i>	<i>Average End Area 1/2 (An + An+1) (cy/l)</i>	<i>Length Ln (ft)</i>	<i>Volume 1/2 (An + An+1) Ln (cy)</i>
<i>9.4</i>	<i>7.5</i>	<i>11</i>	<i>82.5</i>
<i>5.6</i>	<i>12.0</i>	<i>14</i>	<i>168.0</i>
<i>18.4</i>	<i>21.3</i>	<i>20</i>	<i>426.0</i>
<i>24.1</i>	<i>23.5</i>	<i>20</i>	<i>470.0</i>
<i>22.8</i>	<i>22.6</i>	<i>20</i>	<i>452.0</i>
<i>22.4</i>	<i>19.1</i>	<i>20</i>	<i>382.0</i>
<i>15.7</i>	<i>12.6</i>	<i>20</i>	<i>252.0</i>
<i>9.4</i>	<i>8.3</i>	<i>23</i>	<i>190.9</i>
<i>7.1</i>			

*2673.9 cy  
(Use 2700 cy)*

PROJECT	Florida Avenue Complex	Page — of —	COMPUTED BY	DATE
SUBJECT	Cofferdam Excavation		RJgr	9 Sep 76
			CHECKED BY	DATE

Cofferdam Excavation

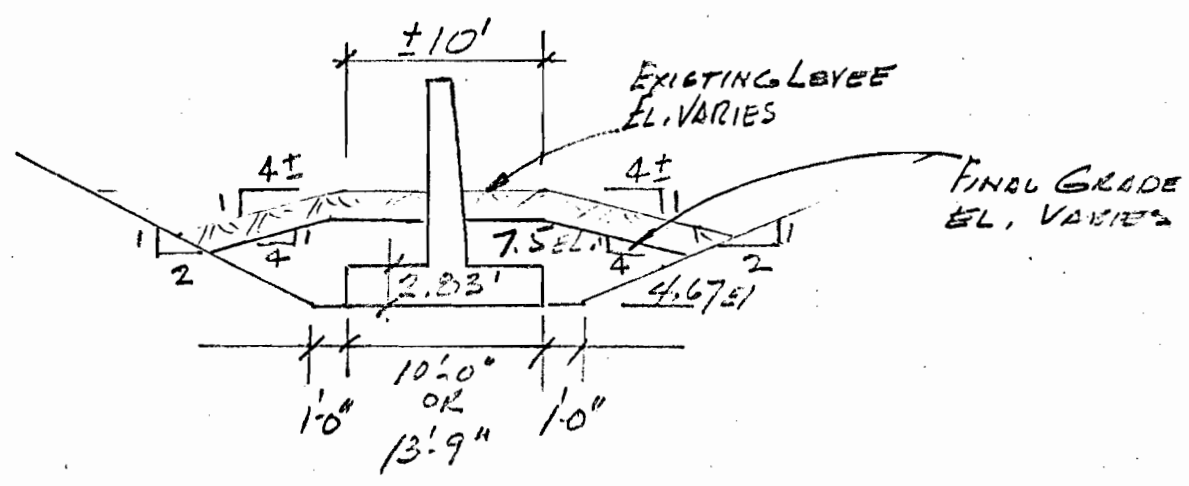
End Area $A_n$ (cy/ft)	Average End Area $\frac{1}{2}(A_n + A_{n+1})$ (cy/ft)	Length $L_n$ (ft)	Volume $\frac{1}{2}(A_n + A_{n+1})L_n$ (cy)
23.5	23.7	11	260.7
23.8	23.4	14	327.6
22.9	22.7	20	454.0
22.5	21.8	20	436.0
20.7	20.0	20	400.0
19.3	19.1	20	382.0
18.9	18.5	20	370.0
18.1	18.5	23	425.5
18.8			

3055.8cy  
(use 3100cy)

*Cofferdam Fill*

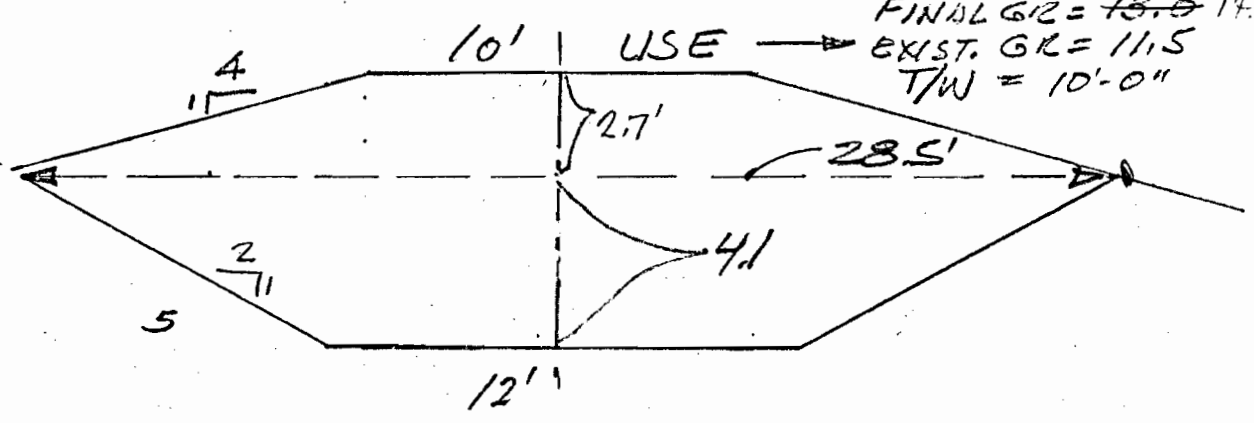
End Area $A_n$ (cy/l)	Average End Area $\frac{1}{2}(A_n + A_{n+1})$ (cy/l)	Length $L_n$ (ft)	Volume $\frac{1}{2}(A_n + A_{n+1})L_n$ (cy)
28.7	28.4	11	312.4
28.0	28.3	14	396.2
28.5	28.4	20	568.0
28.2	28.1	20	562.0
27.9	28.2	20	564.0
28.4	29.1	20	582.0
29.8	30.1	20	602.0
30.4	31.4	23	722.2
32.3			

*4308.8cy*  
*(Use 4350cy)*



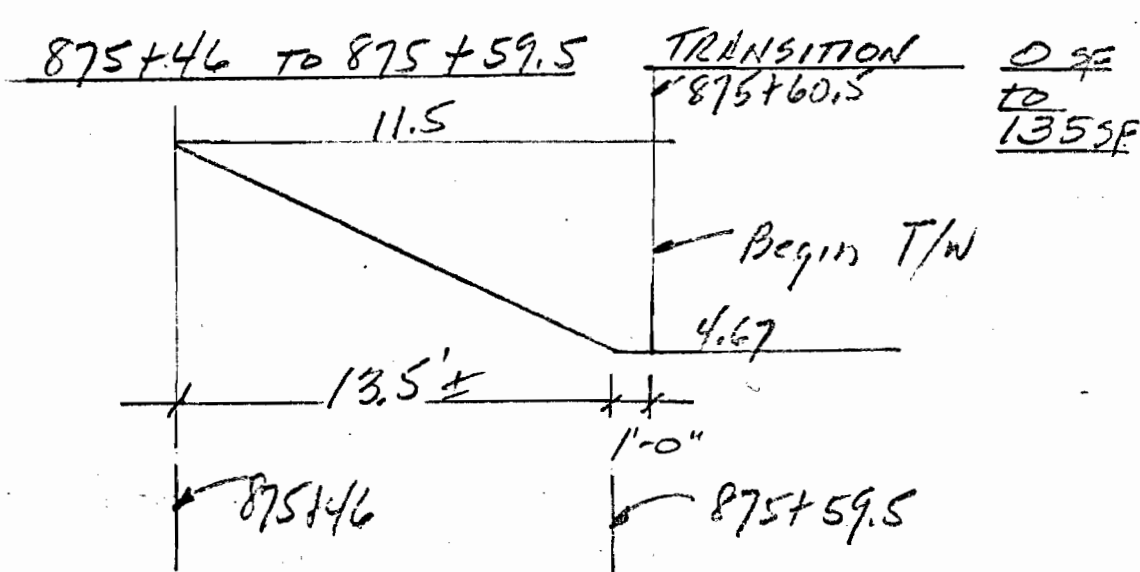
875+59.5 →  
 -875+60.52 TO 875+70

DIST = 9.5' + 1' (see below)  
 FINAL GR = 15.0 14.0  
 EXIST. GR = 11.5  
 T/W = 10'-0"



END AREA

$\frac{1}{2} (10 + 28.5) 2.7 = 51.9$	}	135 SF
$\frac{1}{2} (12 + 28.5) 4.1 = 83.0$		



837-7330

Telecon: (with Mr. James G. Bigham)

Date 24 Aug 1976

1. Mr. Sidhur Mehta of Pepper & Associates (AE in charge of West LINC Pumping Station Design) requested a preliminary estimate of the floodwall from W/L sta 0+10 to W/L sta 4+63.47, excluding excavation and fill and manholes 4 and 5.

2. The following estimate compiles with that request.

Robert J. Gubb



~~REASONABLE CONTRACT~~ ESTIMATE

SHEET OF

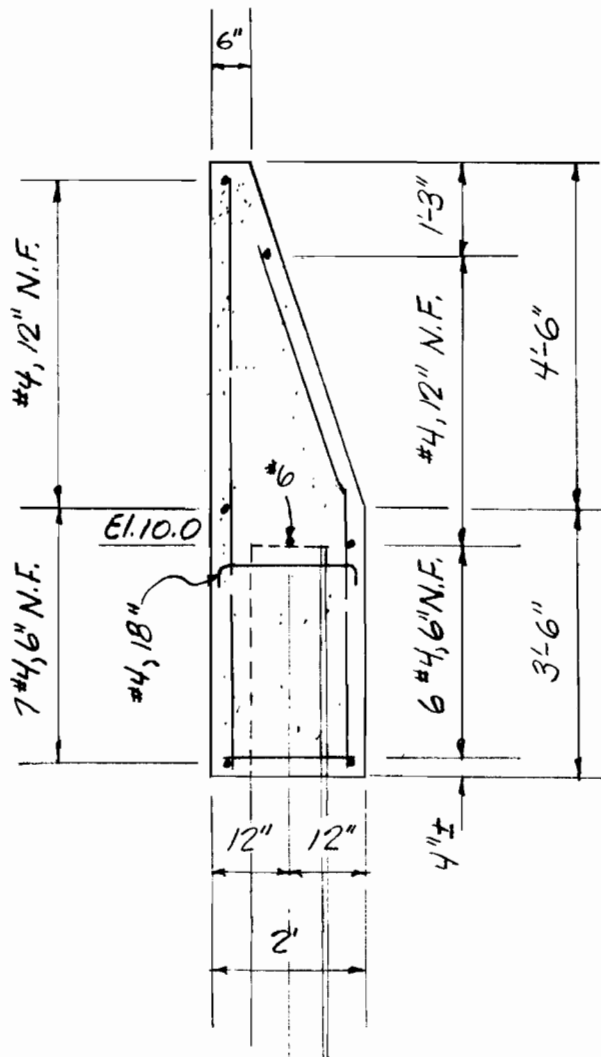
PROJECT

Florida Avenue Complex - West: IHNC (Pepper & Assoc. <sup>-0+10 to</sup> 463.46)

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
	Concrete				
	Wall	152.3	CY		
	Columns	8.5	CY		
	Base Slab	237.4	CY		
	Stabilization Slab	31.0	CY		
	Sill	7.5	CY		
	Cement	2187.0	cwt		
	Reinforcement	78100.0	lb	<del>2.25</del>	
	Steel Sheet Piling, PMA-22	6900	SF		
	Steel Sheet Piling, PZ-27	900	SF		
	Concrete Prestressed Piles, 12" x 12"	7275	LF		
	Waterstops, L-Type	25	LF		
	Waterstops, 3-Bulb Type	119	LF		
	Construction Joint Material	325	SF	2.25	
	Structural Steel				
	Gates	19800	lb		
	Embedded Steel	1600	lb		





Length = 50'-0"  
 2 transitions from  
 I-Wall to T-Wall  
 1-Joint

Concrete-

$$\begin{aligned} \text{Volume} &= (13.75 \text{ sf})(50) \\ &= 685 \text{ cf} = 25.4 \text{ cy} \\ &\text{(Use 26 cy)} \end{aligned}$$

Cement-

$$\begin{aligned} \text{Volume} \times 5.17 \frac{\text{cwt}}{\text{cy}} &= \\ 25.4 \times 5.17 &= 131.16 \text{ cwt} \\ &\text{(Use 132 cwt)} \end{aligned}$$

Reinforcement-

$$\begin{aligned} 21.16(0.662)50 + 50(1.502) &= 782.06 \text{ lb} \\ &\text{(Use 800 lb)} \end{aligned}$$

PROJECT	Page <u>  </u> of <u>  </u>	COMPUTED BY	DATE
SUBJECT	I-Wall (-0+10 to 0+40, W/L)		CHECKED BY
			DATE

Steel Sheet Piling -

$40' \times 20' = 800 \text{ sf}$  (Use 900 sf)

Waterstops, L-Type -

El. 1.5 to El. 6 (sta 0+00 W/L) = 9'

El. 1.5 to El. 0.5 (sta. 0+40 W/L) = 14.5'

---

23.5'

(Use 25 ft)

Waterstops, 3-Bulb Type

El. 14.5 to El. 7.25 (sta. 0+40) = 7.25'

(Use 9 ft)

Construction Joint Material -

$13.75(3) = 41.25 \text{ sf}$ , (Use 45 sf)

~~CONFIDENTIAL~~ ESTIMATE Pepper & Assoc.

SHEET OF

PROJECT Florida Avenue Complex - Swing Gate, Harbor Rd, West

INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
	Concrete				
	1 <sup>st</sup> pour - Base Slab	28.0	CY		
	2 <sup>nd</sup> pour - Base Slab	1.0	CY		
	Step - Base Slab	1.0	CY		
	Stabilization Slab	4.0	CY		
	Columns	4.0	CY		
	Walls	0.5	CY		
	Cement	200.0	CWT		
	Reinforcement	4800	lb		
	Structural Steel	—			
	Gates	10800	lb		
	Embedded steel	600	lb		
	Concrete Piling	1019	CY		
	Steel Sheet Piling, HMA-22	1050	CY		

PROJECT *Florida Avenue Complex - Swing Gate, Railroad, West IHNC* INVITATION NO.

ITEM NO.	DESCRIPTION	ESTIMATED QUANTITY	UNIT	UNIT PRICE	ESTIMATED AMOUNT
	<u>Concrete</u>				
	Stabilization Slab	5	CY		
	Base Slab	37	CY		
	Wall	5	CY		
	Columns	<del>8</del> 4.5	CY		
	Sill	7.5	CY		
	<u>Structural Steel</u>	—			
	Gate	9000	lb		
	Embedded Steel	1000	lb		
	Reinforcement	10800	lb		
	Conc. Prestressed Piles	1250	sf		
	Steel Sheet Piles, PMA-22	1350	sf		
	Cement	305	CWT		
	<u>Support Pedestal</u>				
	Concrete				
	Base Slab	0.4	CY		
	Wall	0.8	CY		
	Reinforcement				
	Concrete Pile	56	sf		
	Jack				



Concrete ~

$$\text{Base Slab} = 2.5(8)(216.17) = 4334.2 \text{ cf} = 160.52 \text{ cy}$$

(Use 170 cy)

Walls:

w/L sta 0+40 to sta 0+98.09:

$$\left[ 1(11) + \frac{11^2}{24}(1/2) \right] 58.09 = 785.4 \text{ cf} = 29.1 \text{ cy}$$

w/L sta 0+98.09 to w/L sta 3+79.71:

$$\left[ 1(12) + \frac{12^2}{24}(1/2) \right] (158.62) = 2379.3 \text{ cf} = 88.12 \text{ cy}$$

$$\text{Total} = 117.22 \text{ cy}$$

(Use 120 cy)

$$\text{Stabilization Slab} = 4/12(8)(216.17) = 576.45 \text{ cf}$$

$$= 21.35 \text{ cy}$$

(Use 22 cy)

$$\text{Cement} = 299.09 \times 5.17 = 1546.29 \text{ cwt.}$$

(Use 1550 cwt)



PROJECT	Florida Avenue - Prelim	Page <u>  </u> of <u>  </u>	CHECKED BY	DATE
SUBJECT	Steel Sheet Piling (0+40 to 3+79.72, W/L)		CHECKED BY	DATE

W/L sta. 0+40 to W/L sta 0+98.09;

Top sheet piling = El +1.25

Bottom sheet piling = El. -20.

$$A = 58.09 \times 21.25 = 1234.41 \text{ sf}$$

W/L sta. 0+98.09 to W/L sta 3+79.07;

Top sheet piling = El. +0.25

Bottom sheet piling = El. -20

$$A = 20.25 \times 157.98 = 3199.09 \text{ sf}$$

Total = 4433.5 sf

(Use 4500 sf)

PROJECT	Page <u>  </u> of <u>  </u>	CHECKED BY	DATE
SUBJECT	T-Wall Reinf. (0+40 to 3+79.72, WL)	CHECKED BY	DATE

Reinforcement:

$$\#5 - 22 \times 56.83' \times 1.043 = 1304$$

$$24 \times 163.3' \times 1.043 = 4088$$

$$\#6 - 114 \times 13'5' \times 1.502 = 2312 \text{ vert}$$

$$327 \times 14'5' \times 1.502 = 7122 \text{ vert}$$

$$106 \times 220.13' \times 1.502 = 35047$$

$$\#8 - 441 \times 10' \times 2.67 = 11775 \text{ trans bar}$$

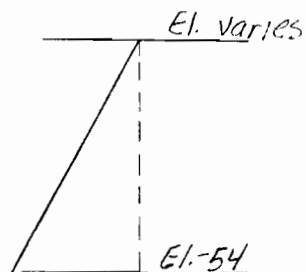
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$$\text{Total} = 61648 \text{ lb}$$

(Use 61700 lb)

Pile, Concrete Prestressed - Preliminary

0+40 to 3+79.72, W/L

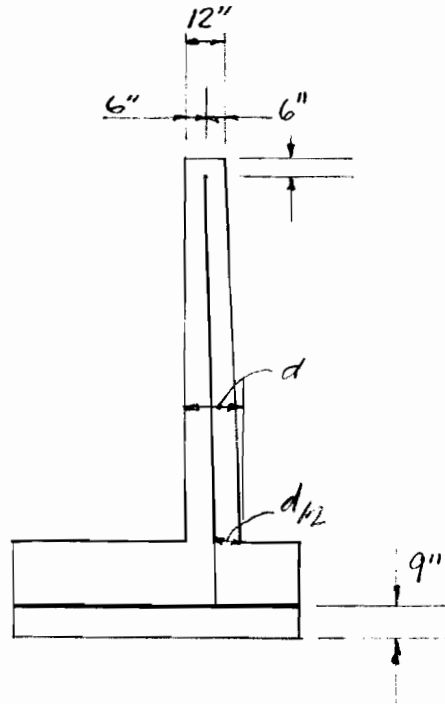


$$\text{Length} = \Delta \text{elev} \sqrt{1 + 1/n^2}$$

Monolith	AElev	No Piles	Batter	Pile Length	Length
3	54.5	15	2:1	61'	915'
		1	4:1	56'	56'
6	53.5	32	2:1	60'	1920'
		8	0	53.5	428'
7	53.5	14	2:1	60'	840'
		4	0	53.5	214'
8	53.5	16	2:1	60'	960'
		3	4:1	55.5'	167'

Total = 5500'

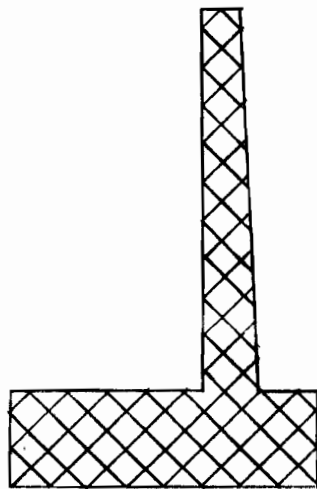
(Use 5550')



Typical, 3-Bulb Type  
Waterstop Detail

$$\text{Length} = 5(12 + 2.5 - 0.75 + 8) = 108.75 \text{ lf}$$

(Use 110 lf)

Construction Joint Material

Typical Area

$$A = 5 \left[ \left( \frac{1}{2} \right) (12) \left( 2 + \frac{12}{24} \right) + 20 \right] = 175 \text{ sf}$$

(Use 180 sf)