LANL Distribution Center Traffic Impact and Access Analysis

November 15, 2005



LSC TRANSPORTATION CONSULTANTS, INC.



516 North Tejon Street Colorado Springs, CO 80903 (719) 633-2868 FAX (719) 633-5430 E-mail: lsc@lsccs.com

Web Site: http://www.lsccs.com

November 15, 2005

Mr. Kirt Anderson, ASLA, AICP Urban Planner Los Alamos National Laboratory MS K773; TA 63, Building 111, Room 145 Los Alamos, NM 87545

RE:

LANL Distribution Center

Los Alamos, New Mexico

LSC #056010

Dear Mr. Anderson:

We are pleased to present our traffic impact and access analysis regarding the LANL Distribution Center. We trust that this report will assist you in planning the LANL Distribution Center. Please contact me if you have any questions or need further assistance.

Sincerely,

LSC TRANSPORTATION CONSULTANTS, INC.

Albert T. Stoddard III, Ph.D., P.E.

Vice President

ATS:CSM:bjwb:rf

LANL Distribution Center Traffic Impact and Access Analysis

November 15, 2005

Prepared for:

Mr. Kirt Anderson, ASLA, AICP Los Alamos National Laboratory MS K773; TA 63, Building 111, Room 145 Los Alamos, NM 87545 (505) 665-2335

Prepared by:

LSC Transportation Consultants, Inc. 516 North Tejon Street Colorado Springs, CO 80903 (719) 633-2868

LSC #056010

November 15, 2005

TABLE OF CONTENTS

Sect	ction Title	Page
A	INTRODUCTION	
В	SITE DEVELOPMENT AND LAND USE	4
С	ROADWAY AND TRAFFIC CONDITIONS Area Roadways	
D	TRIP GENERATION	7
E	TRIP DISTRIBUTION AND ASSIGNMENT	9
F	EXISTING PLUS SITE-GENERATED TRAFFIC, REGEOMETRY, AND TRAFFIC CONTROL	
G	2025 TOTAL TRAFFIC, RECOMMENDED LANE GAND TRAFFIC CONTROL	
Н	PROJECTED LEVELS OF SERVICE	
I	CONCLUSIONS AND RECOMMENDATIONS Trip Generation	

APPENDIX: Level of Service Reports

LIST OF TABULATIONS

<u>Table</u>	e Title	Page
1 2	Trip Generation Estimate	8
	LIST OF ILLUSTRATIONS	
<u>Figur</u>	re <u>Title</u>	<u>Page</u>
1	Vicinity Map	2
2	Site Plan	
3	Existing Traffic	6
4	Directional Distribution	10
5	Site-Generated Traffic Volumes - LANL Distribution Center (south)	
	and Shooting Ranch (north)	11
6	Existing Plus Site-Generated Traffic with Four-Lane East Jemez	
	Cross Section, Lane Geometry and Traffic Control	13
7	Projected 2025 Total Traffic with Four-Lane East Jemez	
	Cross Section, Lane Geometry and Traffic Control	15

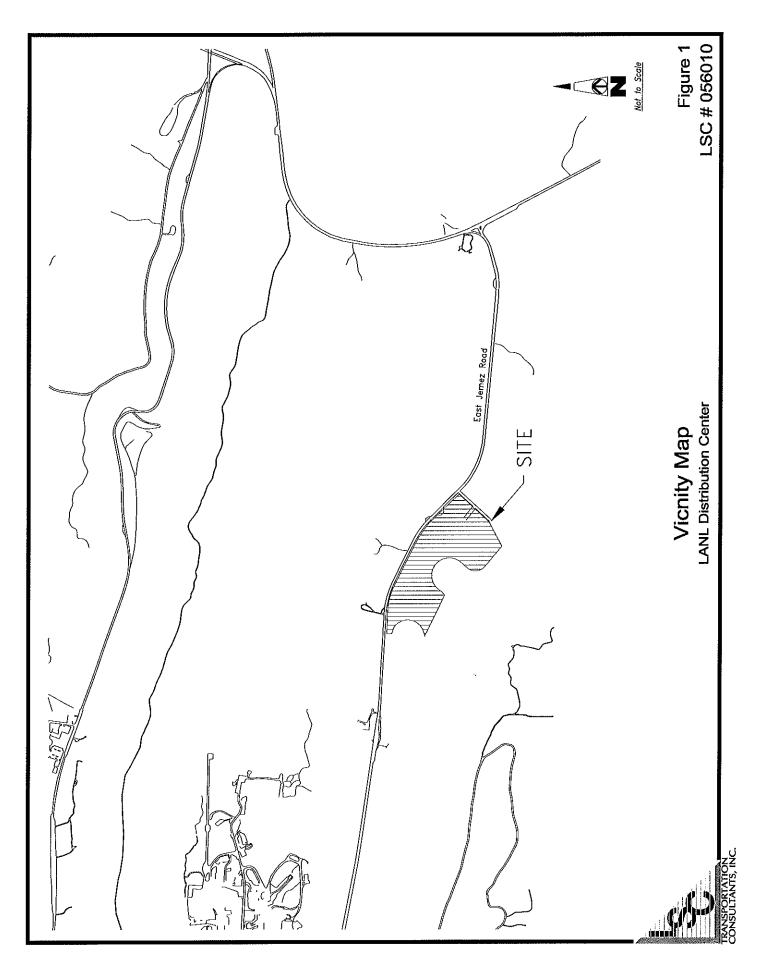
Introduction

In response to your request, LSC Transportation Consultants, Inc. (LSC) has prepared this traffic impact and access analysis report for the proposed distribution center to be relocated from the north side of East Jemez Road near State Highway 4 to the south side of East Jemez Road approximately $3/4^{th}$ mile west of State Highway 4. The site location and area roadways are shown in Figure 1. Access to the site is proposed from East Jemez Road as shown in Figure 2.

REPORT CONTENTS

The analysis covered by the report includes the following:

- A review of the existing and planned roadway system including the roadway alignments, existing and proposed access points, traffic controls, lane configurations, intersection spacing, intersection alignments, intersection sight distances, and any improvement plans for the roadways in the vicinity of the site.
- A determination of the existing average weekday and peak-hour traffic volumes on the surrounding roadway system based on counts performed in 2004.
- A projection of the vehicle-trips to be generated by the distribution center
 and the existing shooting range on the north side of East Jemez Road. The
 shooting range access is being considered for relocation to align with the
 proposed distribution center access. These projections are based on
 information provided by LANL.
- The resultant site-generated traffic volumes on East Jemez Road were then
 projected. The vehicle-trips generated by the distribution center and shooting
 range were distributed geographically and assigned to East Jemez Road in
 order to complete a level of service analysis at the site access intersection
 with East Jemez Road.
- A determination of the projected short-term and long-term traffic impacts of the proposed distribution center and shooting range on East Jemez Road, along with recommendations for any roadway improvements to mitigate these traffic impacts.



SECTION B

Site Development and Land Use

The site is currently vacant. The site is planned to be developed as a distribution center for the National Laboratory. The site plan is shown in Figure 2.

Access to the site is proposed from East Jemez Road as shown in Figure 2.

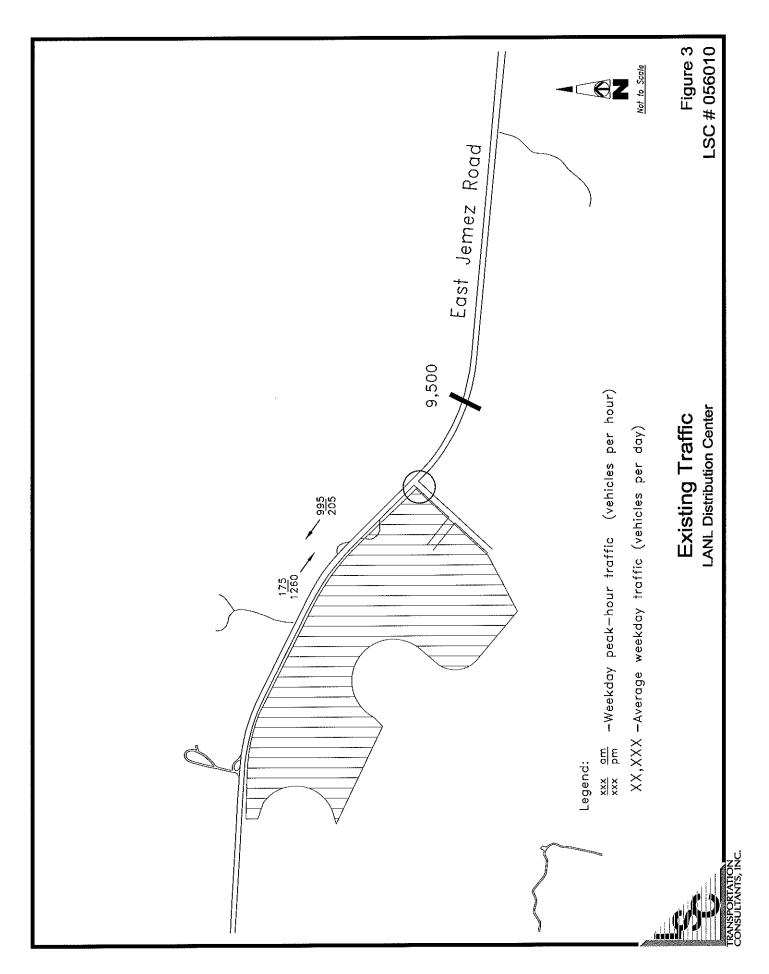
Roadway and Traffic Conditions

AREA ROADWAYS

The roadways in the vicinity of the site are identified below, followed by a brief description. The roadway system is shown in Figure 1.

• East Jemez Road is an existing two-lane east/west roadway in the vicinity of the site which connects the National Laboratory to State Highway 4 via a traffic signal controlled intersection. There are currently plans to widen East Jemez Road to a four-lane section and convert the traffic signal at the East Jemez Road/State Highway 4 intersection to a grade-separated interchange.

The existing traffic volumes counted in the fall of 2004 are shown in Figure 3.



SECTION D

Trip Generation

Estimates of the amount of vehicular traffic expected to be generated by the site have been made based on information provided by LANL. Table 1 shows the results of the trip generation estimates.

As shown in Table 1, the distribution center site is expected to generate nearly 540 vehicle-trips on the average weekday, with about 270 vehicles entering and 270 vehicles exiting in a 24-hour period. These vehicle-trips would be moved from the existing access (to the east) to the proposed distribution center access. During the morning peak hour about 140 vehicles would enter and 25 vehicles would exit the site. During the afternoon peak hour about 25 vehicles would enter and 140 vehicles would exit the site.

As shown in Table 1, the shooting range site is expected to generate about 100 vehicle-trips on the average weekday, with about 50 vehicles entering and 50 vehicles exiting in a 24-hour period. These vehicle-trips would be moved from the existing access (to the west) to the proposed distribution center access. During the morning peak hour, about 4 vehicles would enter and 4 vehicles would exit the site. During the afternoon peak hour, about 20 vehicles would enter and 10 vehicles would exit the site.

and the state of t												
	os A	Table 1 Trip Generation Estimate Los Alamos National Laboratory - Distribution Center	Table 1 Trip Generation Estimate ational Laboratory - Distri	Table 1 eration E aboratory	stima 7 - Dis	te tributi	on Ce	nter				
			Ţ	Trip Generation Rates	ation R	ates		F	otal Trip	Total Trips Generated (1)	ated (1)	
Land Land Use Use	g	Trip Generation	Average Weekday	Morning Peak Hour	ing Jour	Afternoon Peak Hour	noc	Average Weekdav	Morning Peak Hour	ning Hour	Afternoon Peak Hour	Afternoon
Code Description		Units	Traffic	ㅁ	Out	드	Out	Traffic	드	Out	ln	Out
Proposed Distribution Center												
SUP-03	8	Employees	·	1	•	ı	1	285	82	10	10	85
SUP-05	4	Employees			ı		,	130	40	4	4.	40
Mail Delivery (twice per day)	14	Trucks	•	ı		•	ı	28	4	7	7	4
Box Trucks (twice per day)	10	Trucks	•	r	1	ı	1	20	ო	7	2	ო
Flatbed and Tractor Trailer Trucks	က	Trucks	•	•	1		1	30	ო	7	2	ო
Trucks arriving at inspection station	r	1	•	ı	•	ı		45	S	5	ß	5
				ă	stributior	Distribution Center total =	total =	538	140	25	25	140
Proposed Shooting Range												
Shooting Range								100	4	4	50	10
Notes: (1) Source: Information provided by LANL	LAN									THE PARTY OF THE P		
Source: LSC Transportation Consultants, Inc.	ij									Date:	Date: 11/15/05	

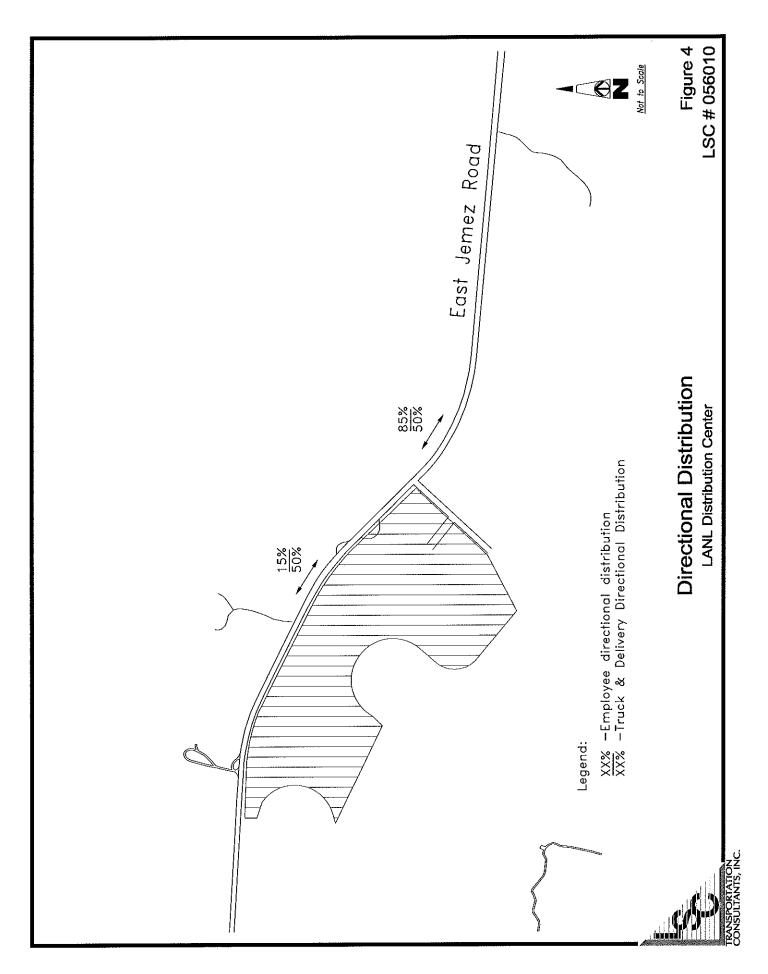
SECTION E

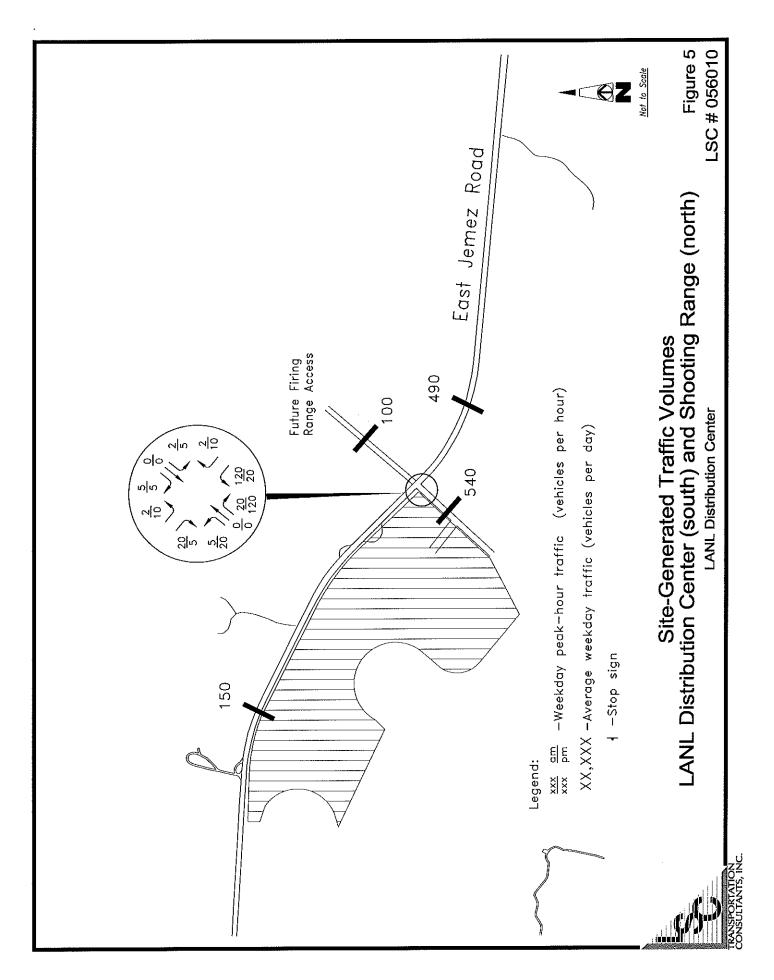
Trip Distribution and Assignment

The directional distribution of the site-generated traffic volumes on the adjacent roadways is an important factor in determining the traffic impacts of the site. The specific distribution estimates for the site-generated traffic volumes are shown in Figure 4.

The distribution estimates are based on the following factors: the location of the site with respect to the nearby residential, commercial, and activity centers and the balance of the Los Alamos area; the land use proposed for the site; the proposed access points for the site; and the roadway system serving the site.

The site-generated traffic volumes on East Jemez Road were determined by applying the distribution percentages (from Figure 4) to the trip generation estimates (from Table 1). Figure 5 shows the morning and afternoon peak-hour and average daily site-generated traffic volumes.



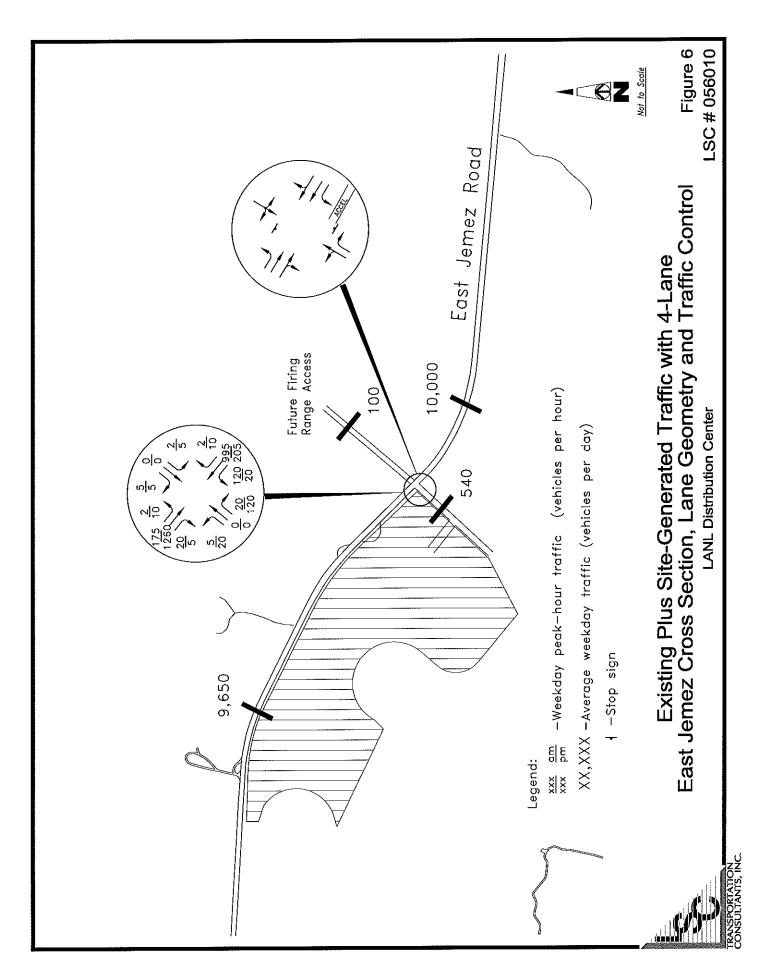


SECTION F

Existing Plus Site-Generated Traffic, Recommended Lane Geometry, and Traffic Control

Figure 6 shows the existing plus site-generated traffic volumes and the recommended lane geometry and traffic controls. The existing plus site-generated traffic volumes are the sum of the site-generated traffic volumes (from Figure 5) plus the existing traffic volumes (from Figure 3).

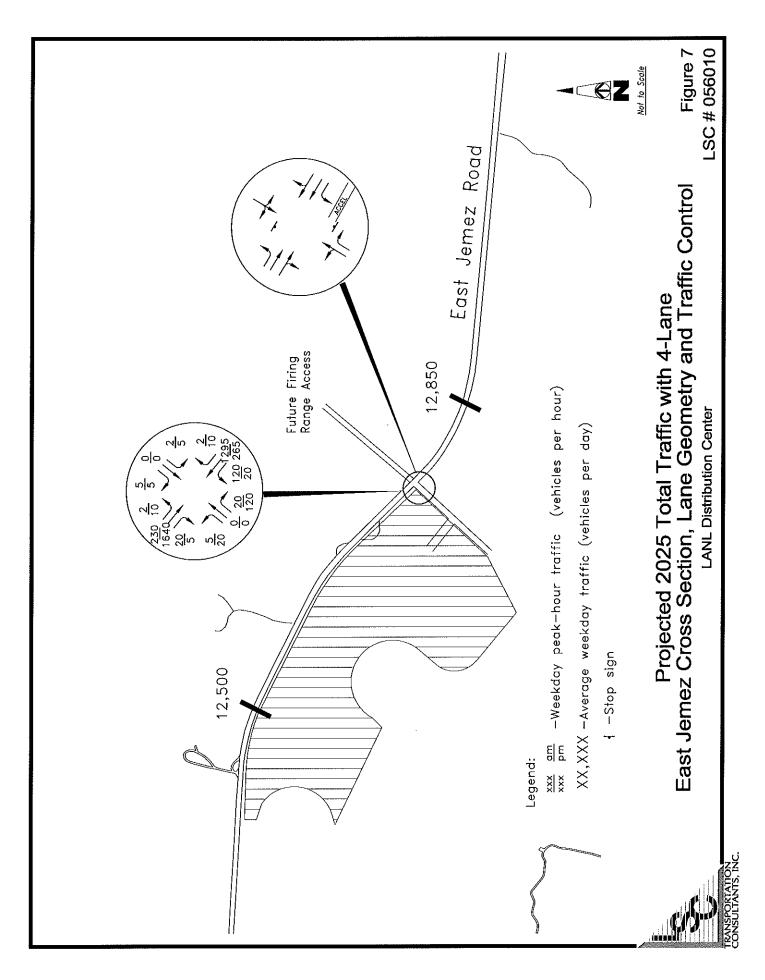
Figure 6 recommends a four-lane section for East Jemez Road. A temporary traffic signal may be needed if the distribution center is constructed and open to traffic prior to East Jemez Road being widened or the East Jemez Road/State Highway 4 interchange being built. The major purpose for this signal would be based on a system warrant. It is expected that a two-lane East Jemez Road would frequently block the site access due to capacity problems at the East Jemez Road/State Highway 4 intersection. The signal would meter the eastbound traffic, allowing the site traffic to enter and exit, and would potentially improve operations at State Highway 4. This temporary signal would be removed once East Jemez Road is widened to four lanes or the interchange is built.



SECTION G

2025 Total Traffic, Recommended Lane Geometry, and Traffic Control

Figure 7 shows the projected total traffic volumes and the recommended lane geometry and traffic controls. The 2025 total traffic volumes are the sum of the site-generated traffic volumes (from Figure 5) plus the projected 2025 background traffic volumes.



Projected Levels of Service

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection. Level of service is indicated on a scale from "A" to "F." LOS A is indicative of very little congestion or delay. LOS F is indicative of a high level of congestion or delay.

The East Jemez Road/site access intersection has been analyzed to determine the projected levels of service based on the unsignalized method of analysis procedures outlined in the *Highway Capacity Manual, 2000 Edition* by the Transportation Research Board. Table 2 shows the level of service analysis results. The level of service reports are attached.

TWO-LANE EAST JEMEZ ROAD

The unsignalized East Jemez Road/site access intersection is projected to operate at Level of Service F for the north and south approaches during the afternoon peak hour with the addition of the distribution center and shooting range site-generated traffic and the recommended lane geometry and traffic controls shown. See Section F regarding the potential for a temporary traffic signal based on a system warrant.

FOUR-LANE EAST JEMEZ ROAD

The unsignalized East Jemez Road/site access intersection is projected to operate at an acceptable level of service during the short-term peak hours with the addition of the distribution center and shooting range site-generated traffic and the recommended lane geometry and traffic controls shown. The northbound and southbound left-turn movements can be expected to experience significant delays as the through traffic on East Jemez Road increases over time.

	Tal Level o LANL - Distr	Table 2 Level of Service LANL - Distribution Center			
		Existing + Si Tra	Existing + Site-Generated Traffic	2025 Tot	2025 Total Traffic
Intersection	Traffic Control	MA	М	AM	PM
East Jemez Road & Distribution Center / Shooting Range 2-lane East Jemez Road	TWSC ⁽¹⁾	D SB	F ⁽³⁾ NB/SB	1	1
East Jemez Road & Distribution Center / Shooting Range 4-lane East Jemez Road	TWSC (1)	D SB	D ⁽⁴⁾ SB NB LT	F ⁽⁵⁾ SB	F ^{(4) (5)} SB NB LT

⁽¹⁾ TWSC = two-way Stop-sign control

LSC Transportation Consultants, Inc.

⁽²⁾ Worst case turning movement level of service; SB = southbound, NB = northbound

⁽³⁾ A temporary traffic signal system warrant may be met. If installed, the temporary traffic signal should be removed once East Jemez Road is widened to four lanes or an interchange is built at State Highway 4.

⁽⁴⁾ Synchro analysis shows the NB RT movement operating at these levels of service as well. An acceleration lane will improve the level of service to LOS A or B.

⁽⁵⁾ Deliveries/pickups and employee left turns could be discouraged during the peak hours to decrease the volume from it's projected 20-30 vehicles per hour.

Conclusions and Recommendations

TRIP GENERATION

- 1. As shown in Table 1, the distribution center site is expected to generate nearly 540 vehicle-trips on the average weekday, with about 270 vehicles entering and 270 vehicles exiting in a 24-hour period. These vehicle-trips would be moved from the existing access (to the east) to the proposed distribution center access. During the morning peak hour about 140 vehicles would enter and 25 vehicles would exit the site. During the afternoon peak hour about 25 vehicles would enter and 140 vehicles would exit the site.
- 2. As shown in Table 1, the shooting range site is expected to generate about 100 vehicle-trips on the average weekday, with about 50 vehicles entering and 50 vehicles exiting in a 24-hour period. These vehicle-trips would be moved from the existing access (to the west) to the proposed distribution center access. During the morning peak hour about 4 vehicles would enter and 4 vehicles would exit the site. During the afternoon peak hour about 20 vehicles would enter and 10 vehicles would exit the site.

PROJECTED LEVELS OF SERVICE

- 3. Two-lane East Jemez Road The unsignalized East Jemez Road/site access intersection is projected to operate at level of service F for the north and south approaches during the afternoon peak hour with the addition of the distribution center and shooting range site-generated traffic and the recommended lane geometry and traffic controls shown.
- 4. Four-lane East Jemez Road The unsignalized East Jemez Road/site access intersection is projected to operate at an acceptable level of service during the short-term peak hours with the addition of the distribution center and shooting range site-generated traffic and the recommended lane geometry

and traffic controls shown. The northbound and southbound left-turn movements can be expected to experience significant delays as the through traffic on East Jemez Road increases over time.

RECOMMENDED TURN LANES ON EAST JEMEZ ROAD

5. Eastbound and westbound deceleration left-turn lanes should be provided on East Jemez Road at the proposed site access intersection. The northbound approach to East Jemez Road should have a shared left-through lane and a dedicated right-turn lane. The northbound right-turn lane should have an eastbound acceleration lane on East Jemez Road. The southbound approach to East Jemez Road should operate at an acceptable level of service with a left-through-right shared lane.

RECOMMENDED TRAFFIC CONTROL

- 6. It is unlikely that a traffic signal will be warranted in the future at the site access intersection on East Jemez Road based on the traffic volumes without additional development as most of the trips entering East Jemez Road are right-turns. With one exception, the existing plus site-generated traffic should operate at acceptable levels of service with the recommended auxiliary lanes and two-way stop control on the north and south approaches to East Jemez Road. The northbound and southbound left-turn movements are expected to experience significant delays during some of the peak hours in the short term with the through lanes on East Jemez Road, and in the long term with the four through lanes on East Jemez Road. These traffic volumes are projected to be relatively low.
- 7. If east-west through traffic increases over time it is likely that the northbound left-turn movement will eventually operate at level of service E or F. This movement is projected to be only about 20 vehicles per hour during the afternoon peak hour with fewer projected for the morning peak hour. Converting this intersection to a three-quarter movement access is not acceptable as it would eliminate the ability for vehicles exiting the distribution center from heading west on East Jemez Road into a large portion of the Laboratory.

- 8. If this intersection is to remain as a full-movement intersection, it is recommended that truck deliveries and pick-ups not be scheduled during the afternoon peak hour. Employees should be encouraged to turn right onto East Jemez Road and avoid attempting to turn left onto East Jemez Road.
- 9. Signalizing the site access intersection on East Jemez Road is not recommended based on traffic volume warrants for two reasons. First, the projected volumes are well below what would be necessary to warrant a traffic signal. Second, intermittently stopping an eastbound through volume of over 1,000 vehicles during the afternoon peak hour on East Jemez Road to facilitate approximately 20 northbound left-turn movements could lead to a higher incidence of rear-end collisions due to the downhill grade and higher speed of East Jemez Road.
- 10. The one condition that may meet the requirements for a temporary traffic signal for the short-term is if the distribution center is generating significant traffic volumes before East Jemez Road is widened to four lanes and the East Jemez Road/State Highway 4 interchange is not built. This scenario would likely result in the site access being frequently blocked due to capacity problems at State Highway 4. A temporary signal would meter the eastbound traffic, allowing the site traffic to enter and exit, and would potentially improve the operations at State Highway 4. This temporary signal would be removed once East Jemez Road is widened to four lanes or the interchange is built.

RELOCATED FIRING RANGE ACCESS

11. The relocation of the existing firing range access, currently located to the west of the proposed distribution center, to align with the proposed distribution center access was assumed in the level of service analysis in this study. Consolidating the access points will improve the access control along East Jemez Road and improve the flow of through traffic. With this realignment, the short-term traffic signal installed at the distribution center access would also aid the firing range traffic prior to the widening of East

ite.			

Appendix



	≯	-	•	✓	₩-	•	4	†	<i>></i>	>	ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	ŞBL	SBT	SBR
Lane Configurations Sign Control Grade	*1	Free 0%		ሻ	₽ Free 0%			4 Stop 0%	۲		4 } Stop 0%	-
Volume (veh/h) Peak Hour Factor Hourly flow rate (vph) Pedestrians Lane Width (ft)	2 0.60 3	175 0.90 194	20 0.75 27	120 0.90 133	995 0.95 1047	2 0.60 3	5 0.60 8	0 0.92 0	20 0.70 29	2 0.60 3	0 0.92 0	2 0.60 3
Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type Median storage veh) Upstream signal (ft)								None	6		None	
pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol	1051			221			1532	1532	208	1531	1543	1049
vCu, unblocked vol tC, single (s) tC, 2 stage (s)	1051 4.1			221 4.1		٠	1532 7.3	1532 6.5	208 6.4	1531 7.2	1543 6.5	1049 6.2
tF (s) p0 queue free % cM capacity (veh/h)	2.2 99 651			2.2 90 1330		94.4 94.43	3.7 89 79	4.0 100 104	3.5 96 789	3.5 96 83	4.0 100 103	3.3 99 273
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	SB1						
Volume Total Volume Left	ે:્ેલ 3	221 0	133 133	1051 0	37 8	3		MANNAN.	HINES.	N PARENCE		NAME:
Volume Right	0 651	27 1700	0 1330	3 1700	29 349	3 128		sajira sa r	* **	* ** *	in the line of	
Volume to Capacity Queue Length 95th (ft)	0.01	0.13 0	0.10 8	0.62 0	0.11 9	0.05 4			*, *		Talley a	A HAZAR
Control Delay (s) Lane LOS	10.6 B	0.0	8.0 A	0.0	20.2 C	34.8 D						9.
Approach LOS	0.2		0.9		20.2 C	34.8 D						
Intersection Summary				9)								
Average Delay Intersection Capacity Uti Analysis Period (min)	ilization	(1.4 69.2% 15	IC	CU Leve	l of Serv	vice		С			

8/18/2005 Synchro 6 Report Page 1

Movement FB FBT FBT FBT FBT FBT FFC FFC Stop
Sign Control Free Own Own Free Own Own Free Own Own Stop Own Own Own Own Own Stop Own Own Own Own Own Own Own Own Own Own Own Own Own Own Stop Own Own Own Own Own Stop Own Own Own Own Own Own Own Own Stop Own Own Own Own Own Own Own Own Stop Own Own Own Own Own Stop Own Own Own Own Own Own Own Own Stop Own Own Own Own Own Stop Own Own Own Own Own Stop Own Own Own Stop Own Own Own Own Own Own Own Stop Own Own Own Own Own Own Own Own Stop Own
Grade
Volume (veh/h) 10 1260 5 20 205 10 20 0 120 5 0 5 Peak Hour Factor 0.65 0.95 0.60 0.70 0.95 0.65 0.70 0.92 0.75 0.60 0.92 0.60 Hourly flow rate (vph) 15 1326 8 29 216 15 29 0 160 8 0 8 Pedestrians Lane Width (ft) Walking Speed (ft/s) Fercent Blockage 8 8 8 8 8 9 8 8 8 8 8 9 8 8 9 8 8 9 8 8 9 8 8 9 8 9 9 8 9
Peak Hour Factor 0.65 0.95 0.60 0.70 0.95 0.65 0.70 0.92 0.75 0.60 0.92 0.60 Hourly flow rate (vph) 15 1326 8 29 216 15 29 0 160 8 0 8 Pedestrians Lane Width (ft) Walking Speed (ff/s) Percent Blockage Right turn flare (veh) Median type 6 Median type 6 None None None None Median storage veh) Upstream signal (ft) PX, platoon unblocked VC, conflicting volume 231 1335 1643 1650 1330 1718 1646 223 VC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC1, single (s) 4.1 4.2 7.2 6.5 6.2 7.2 6.5 6.2 (C, 2 stage (s) 15 15 15 15 15 15 15 1
Hourly flow rate (vph)
Pedestrians Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Malking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type None None None Median storage veh) Upstream signal (ft) PX, platoon unblocked VC, conflicting volume 231
Lane Width (ft) Walking Speed (ft/s) Percent Blockage Right turn flare (veh) Median type Median storage veh) Upstream signal (ft) pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC4, stage 2 conf vol vC5, stage (s) tF (s)
Percent Blockage Right turn flare (veh) Median type
Right turn flare (veh) Median type None None None Median storage veh) Upstream signal (ft) pX, platoon unblocked VC, conflicting volume 231 1335 1643 1650 1330 1718 1646 223 VC1, stage 1 conf vol VC2, stage 2 conf vol VC4, stage 2 conf vol VC4, unblocked vol 231 1335 1643 1650 1330 1718 1646 223 1650 16
Median type Median storage veh) None None None Median storage veh) Upstream signal (ft) pX, platoon unblocked vC, conflicting volume vC2, stage 1 conf vol vC2, stage 2 conf vol vC2, stage (s) 1335 1643 1650 1330 1718 1646 223 tC, single (s) 4.1 4.2 7.2 6.5 6.2 7.2 6.5 6.2 tF (s) 2.2 2.3 3.6 4.0 3.3 3.5 4.0 3.3 p0 queue free % 99 94 59 100 14 10 100 99 cM capacity (veh/h) 1319 477 89 92 186 9 92 809 Direction, Lane # EB 1 EB 2 WB 1 WB 2 NB 1 SB 1 Volume Left 15 0 29 0 29 8 Volume Right 0 8 0 15 160 8 cSH 1319 1700 477 1700
Median storage veh) Upstream signal (ft) pX, platoon unblocked vC, conflicting volume 231 1335 1643 1650 1330 1718 1646 223 vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 231 1335 1643 1650 1330 1718 1646 223 tC, single (s) 4.1 4.2 7.2 6.5 6.2 7.2 6.5 6.2 tF (s) 2.2 2.3 3.6 4.0 3.3 3.5 4.0 3.3 p0 queue free % 99 94 59 100 14 10 100 99 cM capacity (veh/h) 1319 477 89 92 186 9 92 809 Direction, Lane # EB 1 EB 2 WB 1 WB 2 NB 1 SB 1 Volume Left 15 0 29 0 29 8 Volume Right 0 8 0 15 160 8 cSH 1319 1700 477 1700
Upstream signal (ft) pX, platoon unblocked vC, conflicting volume 231 1335 1643 1650 1330 1718 1646 223 vC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 231 1335 1643 1650 1330 1718 1646 223 tC, single (s) 4.1 4.2 7.2 6.5 6.2 7.2 6.5 6.2 tC, 2 stage (s) tF (s) 2.2 2.3 3.6 4.0 3.3 3.5 4.0 3.3 p0 queue free % 99 94 59 100 14 10 100 99 cM capacity (veh/h) 1319 477 69 92 186 9 92 809 Direction, Lane # EB 1 EB 2 WB 1 WB 2 NB 1 SB 1 Volume Total 15 1335 29 231 189 17 Volume Left 15 0 29 0 29 8 Volume Right 0 8 0 15 160 8 cSH 1319 1700 477 1700 219 18 Volume to Capacity 0.01 0.79 0.06 0.14 0.86 0.91
pX, platoon unblocked vC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vCu, unblocked vol 231 1335 1643 1650 1330 1718 1646 223 tC, single (s) 4.1 4.2 7.2 6.5 6.2 7.2 6.5 6.2 tC, 2 stage (s) tF (s) 2.2 2.3 3.6 4.0 3.3 3.5 4.0 3.3 pO queue free % 99 94 59 100 14 10 100 99 cM capacity (veh/h) 1319 477 69 92 186 9 92 809
VC, conflicting volume vC1, stage 1 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage 2 conf vol vC2, stage (s) tC, single (s) 4.1 4.2 7.2 6.5 6.2 7.2 6.5 6.2 tC, 2 stage (s) tF (s) 2.2 2.3 3.6 4.0 3.3 3.5 4.0 3.3 pO queue free % 99 94 59 100 14 10 100 99 cM capacity (veh/h) 1319 477 69 92 186 9 92 809 Direction, Lane #
VC1, stage 1 conf vol vC2, stage 2 conf vol vCu, unblocked vol 231 1335 1643 1650 1330 1718 1646 223 tC, single (s) 4.1 4.2 7.2 6.5 6.2 tC, 2 stage (s) tF (s) 2.2 2.3 3.6 4.0 3.3 3.5 4.0 3.3 p0 queue free % 99 94 59 100 14 10 100 99 cM capacity (veh/h) 1319 477 69 92 186 9 92 809 Diffection, Lane # EB 1 EB 2 WB 1 WB 2 NB 1 SB 1 Volume Total 15 1335 29 231 189 17 Volume Left 15 0 29 0 29 8 Volume Right 0 8 0 15 160 8 CSH 1319 1700 477 1700 219 18 Volume to Capacity 0.01 0.79 0.06 0.14 0.86 0.91
vCu, unblocked vol 231 1335 1643 1650 1330 1718 1646 223 tC, single (s) 4.1 4.2 7.2 6.5 6.2 7.2 6.5 6.2 tC, 2 stage (s) tF (s) 2.2 2.3 3.6 4.0 3.3 3.5 4.0 3.3 p0 queue free % 99 94 59 100 14 10 100 99 cM capacity (veh/h) 1319 477 69 92 186 9 92 809 Direction, Lane # EB 1 EB 2 WB 1 WB 2 NB 1 SB 1 Volume Total 15 1335 29 231 189 17 Volume Right 0 8 0 15 160 8 cSH 1319 1700 477 1700 219 18 Volume to Capacity 0.01 0.79 0.06 0.14 0.86 0.91
tC, single (s) 4.1 4.2 7.2 6.5 6.2 7.2 6.5 6.2 tC, 2 stage (s) tF (s) 2.2 2.3 3.6 4.0 3.3 3.5 4.0 3.3 p0 queue free % 99 94 59 100 14 10 100 99 cM capacity (veh/h) 1319 477 69 92 186 9 92 809 Direction, Lane # EB 1 EB 2 WB 1 WB 2 NB 1 SB 1 Volume Total 15 1335 29 231 189 17 Volume Left 15 0 29 0 29 8 Volume Right 0 8 0 15 160 8 cSH 1319 1700 477 1700 219 18 Volume to Capacity 0.01 0.79 0.06 0.14 0.86 0.91
tC, 2 stage (s) tF (s) 2.2 2.3 3.6 4.0 3.3 3.5 4.0 3.3 p0 queue free % 99 94 59 100 14 10 100 99 cM capacity (veh/h) 1319 477 69 92 186 9 92 809 Direction, Lane # EB 1 EB 2 WB 1 WB 2 NB 1 SB 1 Volume Total 15 1335 29 231 189 17 Volume Left 15 0 29 0 29 8 Volume Right 0 8 0 15 160 8 cSH 1319 1700 477 1700 219 18 Volume to Capacity 0.01 0.79 0.06 0.14 0.86 0.91
tF (s) 2.2 2.3 3.6 4.0 3.3 3.5 4.0 3.3 p0 queue free % 99 94 59 100 14 10 100 99 cM capacity (veh/h) 1319 477 69 92 186 9 92 809 Direction, Lane # EB 1 EB 2 WB 1 WB 2 NB 1 SB 1 Volume Total 15 1335 29 231 189 17 Volume Left 15 0 29 0 29 8 Volume Right 0 8 0 15 160 8 cSH 1319 1700 477 1700 219 18 Volume to Capacity 0.01 0.79 0.06 0.14 0.86 0.91
p0 queue free % 99 94 59 100 14 10 100 99 cM capacity (veh/h) 1319 477 69 92 186 9 92 809 Direction, Lane # EB 1 EB 2 WB 1 WB 2 NB 1 SB 1 Volume Total 15 1335 29 231 189 17 Volume Left 15 0 29 0 29 8 Volume Right 0 8 0 15 160 8 cSH 1319 1700 477 1700 219 18 Volume to Capacity 0.01 0.79 0.06 0.14 0.86 0.91
cM capacity (veh/h) 1319 477 69 92 186 9 2809 Direction, Lane # EB 1 EB 2 WB 1 WB 2 NB 1 SB 1 Volume Total 15 1335 29 231 189 17 Volume Left 15 0 29 0 29 8 Volume Right 0 8 0 15 160 8 cSH 1319 1700 477 1700 219 18 Volume to Capacity 0.01 0.79 0.06 0.14 0.86 0.91
Direction, Lane # EB 1 EB 2 WB 1 WB 2 NB 1 SB 1 Volume Total 15 1335 29 231 189 17 Volume Left 15 0 29 0 29 8 Volume Right 0 8 0 15 160 8 cSH 1319 1700 477 1700 219 18 Volume to Capacity 0.01 0.79 0.06 0.14 0.86 0.91
Volume Total 15 1335 29 231 189 17 Volume Left 15 0 29 0 29 8 Volume Right 0 8 0 15 160 8 cSH 1319 1700 477 1700 219 18 Volume to Capacity 0.01 0.79 0.06 0.14 0.86 0.91
Volume Left 15 0 29 0 29 8 Volume Right 0 8 0 15 160 8 cSH 1319 1700 477 1700 219 18 Volume to Capacity 0.01 0.79 0.06 0.14 0.86 0.91
Volume Right 0 8 0 15 160 8 cSH 1319 1700 477 1700 219 18 Volume to Capacity 0.01 0.79 0.06 0.14 0.86 0.91
cSH 1319 1700 477 1700 219 18 Volume to Capacity 0.01 0.79 0.06 0.14 0.86 0.91
Queue Length 95th (ft) 1 0 5 0 168 60
Control Delay (s) 7.8 0.0 13.0 0.0 85.4 468.0
Lane LOS A B F F Approach Delay (a) 0.4 1.4 95.4 468.0
Approach Delay (s) 0.1 1.4 85.4 468.0 Approach LOS F F
Intersection Summary
Average Delay 13.4
Intersection Capacity Utilization 87.4% ICU Level of Service E Analysis Period (min) 15
Analysis Ferror (IIIII)

8/18/2005 Synchro 6 Report Page 1

	≯	-	•	€	←	4	*	†	<i>></i>	/	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	Ŋ	↑ ₽		*	^ }			4	74		44+	
Sign Control	_	Free			Free			Stop			Stop	
Grade		0%			0%			0%		_	0%	_
Volume (veh/h)	2	175	20	120	995	2	5	0	20	2	0	2
Peak Hour Factor	0.60	0.90	0.75	0.90	0.95	0.60	0.60	0.92	0.70	0.60	0.92	0.60
Hourly flow rate (vph)	3	194	27	133	1047	3	8	0	29	3	0	3
Pedestrians												
Lane Width (ft) Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)									6			
Median type								None	_		None	
Median storage veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	1051			221			1008	1532	111	1434	1543	525
vC1, stage 1 conf vol												
vC2, stage 2 conf vol	+ 1 + 1, + 1 - 1											
vCu, unblocked vol	1051			221			1008	1532	111	1434	1543	525
tC, single (s)	4.2	* **		4.2			7.9	6.5	7.3	7.6	6.5	7.0
tC, 2 stage (s)	2.2	greates a	1000	2.2			3.7	4.0	3.5	3.6	4.0	3.4
tF (s) p0 queue free %	99			90			95	100	97	96	100	99
cM capacity (veh/h)	641	. See See S		1324	41344		156	103	867	82	102	489
					Tre a				00.	-		
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1	SB 1			20.00	
Volume Total	11/11/3	130	91	133	698	352	37	7				1777442
Volume Left	3	0	0 27	133 0	10	0 3	8 29	3				
Volume Right	641	1700	1700	1324	1700	1700	692	140				
Volume to Capacity	0.01	0.08	0.05	0.10	0.41	0.21	0.05	0.05				
Queue Length 95th (ft)	0.01	0.00	0.00	8	0.41	0.21	4	4				
Control Delay (s)	10.6	0.0	0.0	8.0	0.0	0.0	13.8	32.0				
Lane LOS	В	0.0	0.0	A	0.0	0.0	В	D				
Approach Delay (s)	0.2			0.9			13.8	32.0				
Approach LOS							В	D				
Intersection Summary		1016										
			1.3									
Average Delay Intersection Capacity Ut	ilization		44.2%	lí	III eve	el of Ser	vice		Α			
Analysis Period (min)		,	15		J L L V V	, 0, 00,	1.00					
Secular Secular												

Synchro 6 Report Page 1

8/18/2005

	۶		*	•	←	•	*	†	/	>	ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	A	∱ }		*1	∱ 1}			4	۳		↔	
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%		_	0%	_
Volume (veh/h)	10	1260	5	20	205	10	20	0	120	5	0	5
Peak Hour Factor	0.65	0.95	0.60	0.70	0.95	0.65	0.70	0.92	0.75	0.60	0.92	0.60
Hourly flow rate (vph)	15	1326	8	29	216	15	29	0	160	8	0	8
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s) Percent Blockage												
Right turn flare (veh)									6			
Median type								None	U		None	
Median storage veh)								140110			110110	
Upstream signal (ft)	signali i		100	4344, 343	*	٠	- 1 - 1 - 1				* .	
pX, platoon unblocked												
vC, conflicting volume	231	ded to	Contract Contract	1335	11-11-53	\$ 70 \$ 10 \$	1535	1650	667	1055	1646	116
vC1, stage 1 conf vol												
vC2, stage 2 conf vol	De Meller						4.76.6				V .	*1
vCu, unblocked vol	231			1335			1535	1650	667	1055	1646	116
tC, single (s)	4.2	e a San San	9 1 1 1 4 4	4.4	de titil		7.8	6.5	7.0	7.6	6.5	7.0
tC, 2 stage (s)												
tF (s)	2.2		i in the single	2.4	g diabete te te	i a si si si si	3.6	4.0	3.4	3.6	4.0	3.4
p0 queue free %	99			94	a salaha	* .	56	100	59	92	100	99
cM capacity (veh/h)	1312	\$24,534,23 http://		449	na Shail na t		65	91	394	99	91	905
Direction, Lane#	EB1	EB2	EB 3	WB 1	WB 2	WB 3	NB 1	SB 1	16			
Volume Total		884	450	29	144	87	189	17	June 1999 (SA)			
Volume Left	15	0	0	29	0	0	29	8				
Volume Right		0	8	_	0	15	160	8				* ** * * 1.0
cSH	1312	1700	1700	449	1700	1700	431	178				s t
Volume to Capacity			0.26	0.06	0.08	0.05	0.44	0.09				
Queue Length 95th (ft)		0	0	5 :-13.6	0	0 0.0	54	8 27.3				
Control Delay (s) Lane LOS	_	0.0	0.0	⊟13,6 B	0.0	0.0	31.9 D	27.3 D				
Approach Delay (s)	0.1			1.5			31.9	27.3				
Approach LOS	0.1			1.5			31.9 D	27.3 D				
				305-7305 (San San San San San San San San San San			U	ט				
Intersection Summary						6 6 6 8						
Average Delay	Hatting at a		3.8	1.4	OLL 1		a da e		г.			
Intersection Capacity L	Julization		55.8%	Į(JU Leve	el of Ser	vice		В			
Analysis Period (min)			15									

8/18/2005 Synchro 6 Report Page 1

	۶	-	*	€	4—	٨.	*	†	<i>/</i> *	1	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*5	ተ _ጉ		ሻ	ተ ኍ		•	र्भ	77		4	
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%		_	0%	_
Volume (veh/h)	2	230	20	120	1295	2	5	0	20	2	0	2
Peak Hour Factor	0.60	0.90	0.75	0.90	0.95	0.60	0.60	0.92	0.70	0.60	0.92	0.60
Hourly flow rate (vph)	3	256	27	133	1363	3	8	0	29	3	0	3
Pedestrians Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)									6			
Median type								None	·		None	
Median storage veh)												
Upstream signal (ft)	4-1-6	•	**	To some the		15 54 F.	100					
pX, platoon unblocked												
	1366	******	te de tes	282	11111	14.74.74.74	1227	1909	141	1780	1920	683
vC1, stage 1 conf vol												
vC2, stage 2 conf vol	4.41.5.145											
vCu, unblocked vol	1366	e e est		282		a taken	1227	1909	141	1780	1920	683
tC, single (s)	4.2	, tara ji bara	e ki Para e ki Mili	4.2	g research to the	A A Section	7.9	6.5	7.3	7.6	6.5	7.0
tC, 2 stage (s) tF (s)	2.2	service s	na Zala sa sa	2.2	edine el el cod	uula hahaluh	3.7	4.0	3.5	3.6	4.0	3.4
p0 queue free %	99		* * * * * * * * * *	89		******	92	100	97	92	100	99
cM capacity (veh/h)			444264	1256	dayar.	as Estada	105	60	827	44	59	385
									021		00	000
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1	SB 1	A 1.11		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	45.
Volume Total	3				909			7	g ne filline a fre		es de tribueção e	4 - 44 - 4, 5 2
Volume Left	3 - () () ()	0	0 27	133	0	0	8 29	3 3		*******	ing salah	
Volume Right	483	1700	1700	1256	1700	1700	465	79				
Volume to Capacity		0.10	0.07	0.11	0.53	0.27	0.08	0.08	100		4.35	er eg e gja
Queue Length 95th (ft)	1	0.10	0.01	9	0.00	0.2.7	6	7				
Control Delay (s)	12.5	_	0.0			0.0	_	54.4	5 - 5 - 5 - 5	1 1 1 1	1000	Section 5
Lane LOS	В			Α			С	F				
Approach Delay (s)	0.1	1 11 11	**	0.7		13 + 4,5 4	16.9	54.4				
Approach LOS							С	F				
Intersection Summary												
Average Delay			1.2			3						
Intersection Capacity Ut	ilization		52.5%	10	CU Leve	el of Ser	vice		Α			
Analysis Period (min)			15	•								

8/19/2005 Synchro 6 Report Page 1

	۶	-	•	•	←	•	*	†	<i>></i>	>	↓	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	14		*	_ተኍ			4	P.			
Sign Control		Free			Free			Stop			Stop	
Grade		0%	_		0%	4.0		0%	400	_	0%	-
Volume (veh/h)	10	1640	5	20	265	10	20	0	120	5	0	5
Peak Hour Factor	0.65	0.95	0.60	0.70	0.95	0.65	0.70	0.92	0.75	0.60	0.92	0.60
Hourly flow rate (vph)	15	1726	8	29	279	15	29	0	160	8	0	8
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage									6			
Right turn flare (veh)								None	O		None	
Median type								NOHE			NOTIC	
Median storage veh) Upstream signal (ft)	arren, s					5.151,5	eg sa distri				*	
pX, platoon unblocked												
vC, conflicting volume	294		eres eres in	1735			1966	2113	867	1318	2109	147
vC1, stage 1 conf vol	~ Z3 *1			1733			1000	2110	001	1010	2100	
vC2, stage 2 conf vol			1111			18, 4, 4	44.54.5	1.5	1	* * * .	* * * *	. 1 1.35
vCu, unblocked vol	294			1735			1966	2113	867	1318	2109	147
tC, single (s)	4.2	era a era		4.4	Section 8	sales de		6.5	7.0	7.6		7.0
tC, 2 stage (s)				1.1				0.0		,,,		, , , -
tF (s)	2.2	3,774,74		2.4	4,344,64	eg a bi	3.6	4.0	3.4	3.6	4.0	3.4
p0 queue free %	99			91			2	100	45	82	100	99
cM capacity (veh/h)		Johan e.	na diga	307	en septim	99.34.53	29	45	290	46	45	864
Direction, Lane #	EB 1	EB 2	EB 3	WB 1	WB 2	WB 3	NB 1	SB 1	6.5			
Volume Total	15	Control of the Contro	584	29	186	108	189	17				
Volume Left	15	0	0	29	0	0	29	8				
Volume Right		-			0	15	160	8	1 1 1			
cSH	1243	1700	1700	307	1700	1700	193	88				
Volume to Capacity			0.34		0.11	0.06	0.98	0.19	1.0	$\{\{a_i,b_i\}\}_{i=1}^n$	11-41-425	$\{\{1,1\},\{1\}\}$
Queue Length 95th (ft)	1	0	0	8	0	0	203	16				
Control Delay (s)		0.0	0.0	17.9	0.0	0.0	80.6	55.3	100	5 to 1 co		
Lane LOS	Α			С			F	F				
Approach Delay (s)	0.1	er er er er	THE SALE.	1.6	*****	t testinen	80.6	55.3				
Approach LOS							F	F				
Intersection Summary												
Average Delay			7.4									<u></u>
Intersection Capacity Uti	lization		66.3%	1	CU Lev	el of Se	rvice		С			
Analysis Period (min)			15									
koyibidhayabbataba Afala a selim					•							

8/19/2005 Synchro 6 Report Page 1

LSC, Inc.

ત્ર∙	Fast Jemez	Road	& Site	Access	Performance	by move	ment
Ų.	Lasi acilica	Noau	O OILC	7.00033	CHOHHANCC	Dy IIIO V	THOTIL

Movement	EBL	EBT	EBR \	WBL '	WBT \	WBR -	NBL	NBR	SBL	SBR	All
Delay / Veh (s)	9.6	0.4	0.2	2.1	0.7	0.3	20.0	7.0	39.7	9.2	0.9

Total Network Performance

Delay / Veh (s) 3.0

Baseline 8/19/2005

3: East Jemez Road & Sit	Access Perforn	nance by movement
--------------------------	----------------	-------------------

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBR	SBL	SBR	All	
Delay / Veh (s)	6.3	1.9	1.1	13.9	0.2	0.0	96.2	8.2	64.6	8.4	3.2	

Total Network Performance

Delay / Veh (s)

5.9