

US ARMY CORPS OF ENGINEERS NEW ORLEANS DISTRICT

WEST BANK OF THE MISSISSIPPI RIVER IN THE VICINITY OF NEW ORLEANS, LOUISIANA EAST OF HARVEY CANAL HURRICANE PROTECTION

SOIL REPORT HERO CANAL

SEPTEMBER 1996



WEST BANK OF THE MISSISSIPPI RIVER IN THE VICINITY OF NEW ORLEANS, LOUISIANA EAST OF HARVEY CANAL HURRICANE PROTECTION

SOIL REPORT - HERO CANAL

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GENERAL

- 1. <u>General</u>. This report presents additional foundation investigations for preparation of plans and specifications for the Hero Canal hurricane protection levee subsequent to "West Bank of the Mississippi River in the Vicinity of New Orleans, LA (East of the Harvey Canal)" Feasibility Report dated August 94.
- 2. Project Description. The Hero Canal levee is shown on plates 2 thru 10. The protection begins at Hwy 23 near the community of Oakville. The existing Plaquemines Parish levee between Hwy 23 and Station 29+01.00 will be enlarged. A new levee will tie the existing levee to the landfill. The levee will continue on the opposite side of the landfill and extend to the south bank of Hero Canal. The levee will run along the south bank of the canal, around the head of the canal, along the north bank of the canal and to Station 318+07.16 along the Intracoastal Waterway.

GEOLOGY

3. Geology. The study area is on the west bank of the Mississippi River approximately 2.5 miles downstream from the U.S. Naval Air Station near Mississippi River Mile 70 at Oakville in Plaquemines Parish, Louisiana. The levee area is perpendicular to the river, adjacent to Hero Canal and the Intracoastal Waterway. This is an area of low relief ranging in elevation from 0.0 feet up to a maximum of 5 feet on the crest of the natural levee adjacent to the river.

Natural levee deposits overlie the entire study area and consist of interbedded very soft to stiff clays with occasional lenses of silt, silty sand and sand. Natural levee deposits average 8 feet thick and range in elevation from +1 to -9 feet. Swamp deposits underlie natural levee deposits from distance 0 to 3350 and consist of interbedded very soft to soft clays and silt with occasional lenses of silty sand and sand. Interdistributary deposits average 23 feet thick and range in elevation from -8 to -36 feet. A lens of swamp deposits occurs in interdistributary deposits in boring EHC-4 and consists of organic clay. This lens is 2.5 feet thick and ranges in elevation from -18 to -20.5 feet. Prodelta deposits underlie interdistributary deposits and

consist of interbedded very soft to medium clays. Where borings penetrate the entire thickness of prodelta deposits, these deposits average 34 feet thick and range in elevation from -30 to -76 feet. Nearshore gulf deposits underlie prodelta deposits from distance 0 to 9700 and consist of interbedded clays, silt and silty sand with occasional lenses of clayey sand. The surface of nearshore gulf deposits averages -67 feet in elevation and these deposits extend to an unknown depth. The surface of the Pleistocene is not penetrated by any of the borings; however, estimates of the Pleistocene surface in the area are -85 to -95 feet in elevation.

FOUNDATIONS

- 4. <u>General</u>. The following paragraphs address design assumptions and parameters for the Hero Canal levee. The project consists of three design reaches for approximately 33,200 feet of levee.
- 5. <u>Foundation Conditions</u>. The foundation soils are predominantly fat clays (CH) varying in consistency from very soft to medium. There are occasional layers of silt (ML), silty sand (SM), and lean clays (CL). Layers of organic clays, which typically display high moisture contents, exist in the area near the Intracoastal Waterway form the original ground surface down to approximately elevation -20.

6. Field Exploration and Laboratory Investigation.

- a. <u>Soil Borings</u>. Four undisturbed borings and four general type borings were taken along parts of the proposed alignment in Jan 1995. Location of these borings can be found on plates 2 thru 10 and the boring logs are presented as plates thru 17 thru 21.
- b. <u>Laboratory Tests</u>. Visual classifications were made on all boring samples and water content determinations were performed on all cohesive samples. Unconfined compression shear tests (UCT's) were made on selected samples of cohesive soils.

Unconsolidated-undrained (Q) triaxial compression shear tests and consolidation (C) tests were performed on selected undisturbed samples and included Atterberg limits.

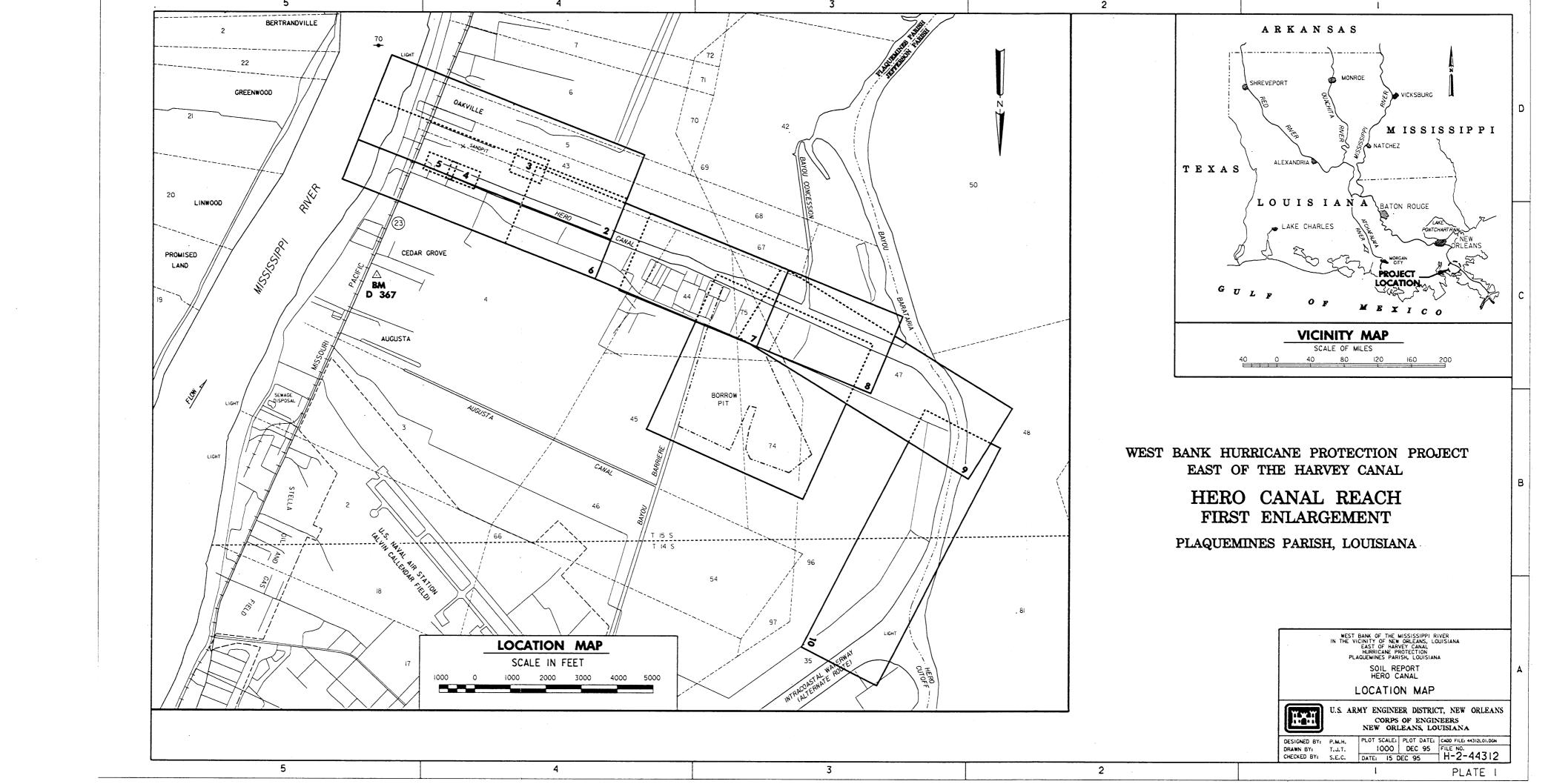
c. <u>Design Parameters</u>. The shear strength and soil density results were determined from undisturbed borings EHCH-1U, EHCH-2U, EHCH-3U and EHCH-4U. These were used to determine the design parameters for all three reaches.

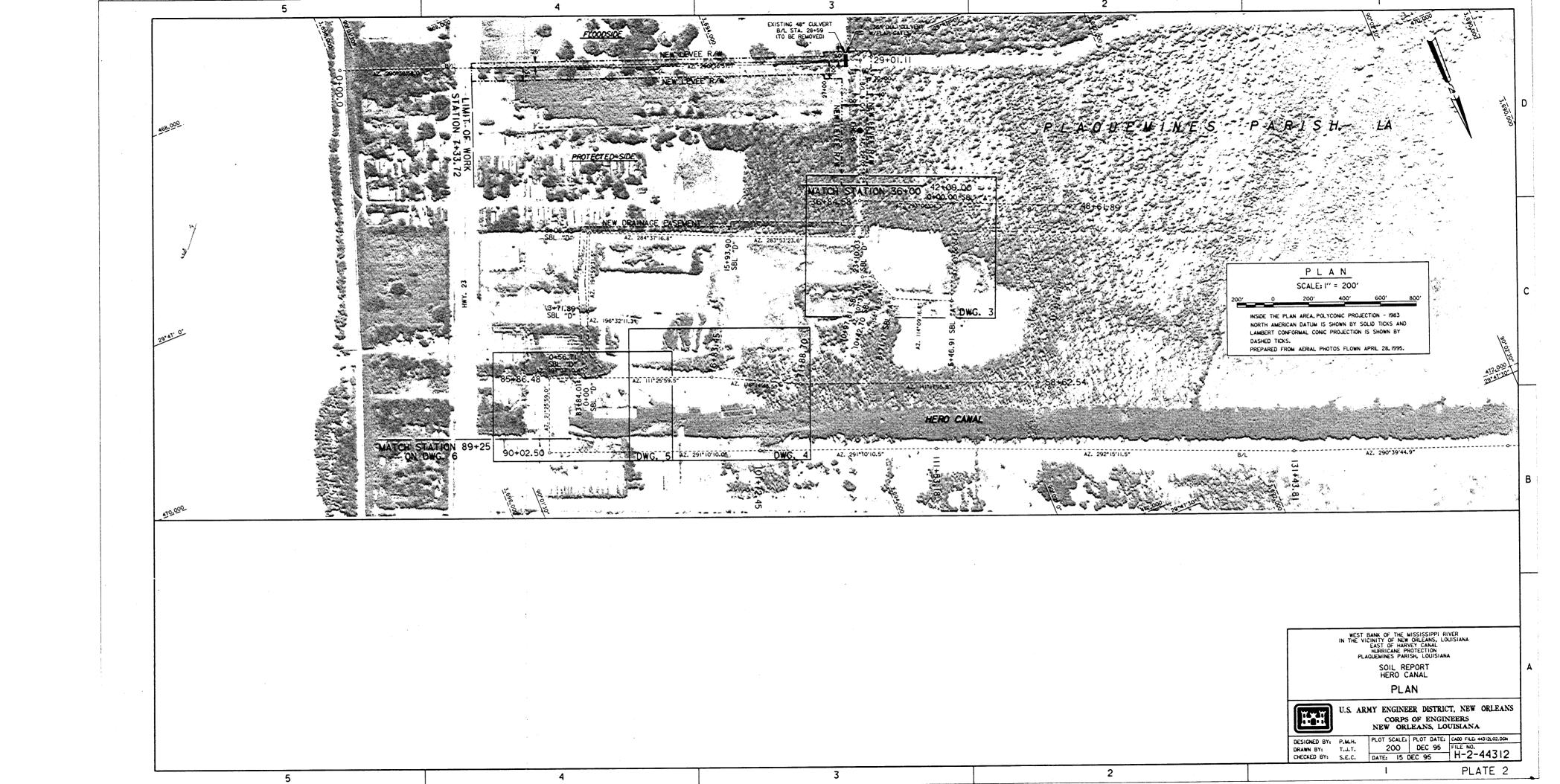
7. <u>Design Reaches</u>. The job was divided into three design reaches based on hydraulic considerations. The reaches could be divided as follows:

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Reach I - Oakville Levee - Sta. 0+00 to 140+00
Reach II - Hero Canal Levee - Sta. 140+00 to 237+00
Reach III - Algiers Levee - Sta 237+00 to 332+38.79
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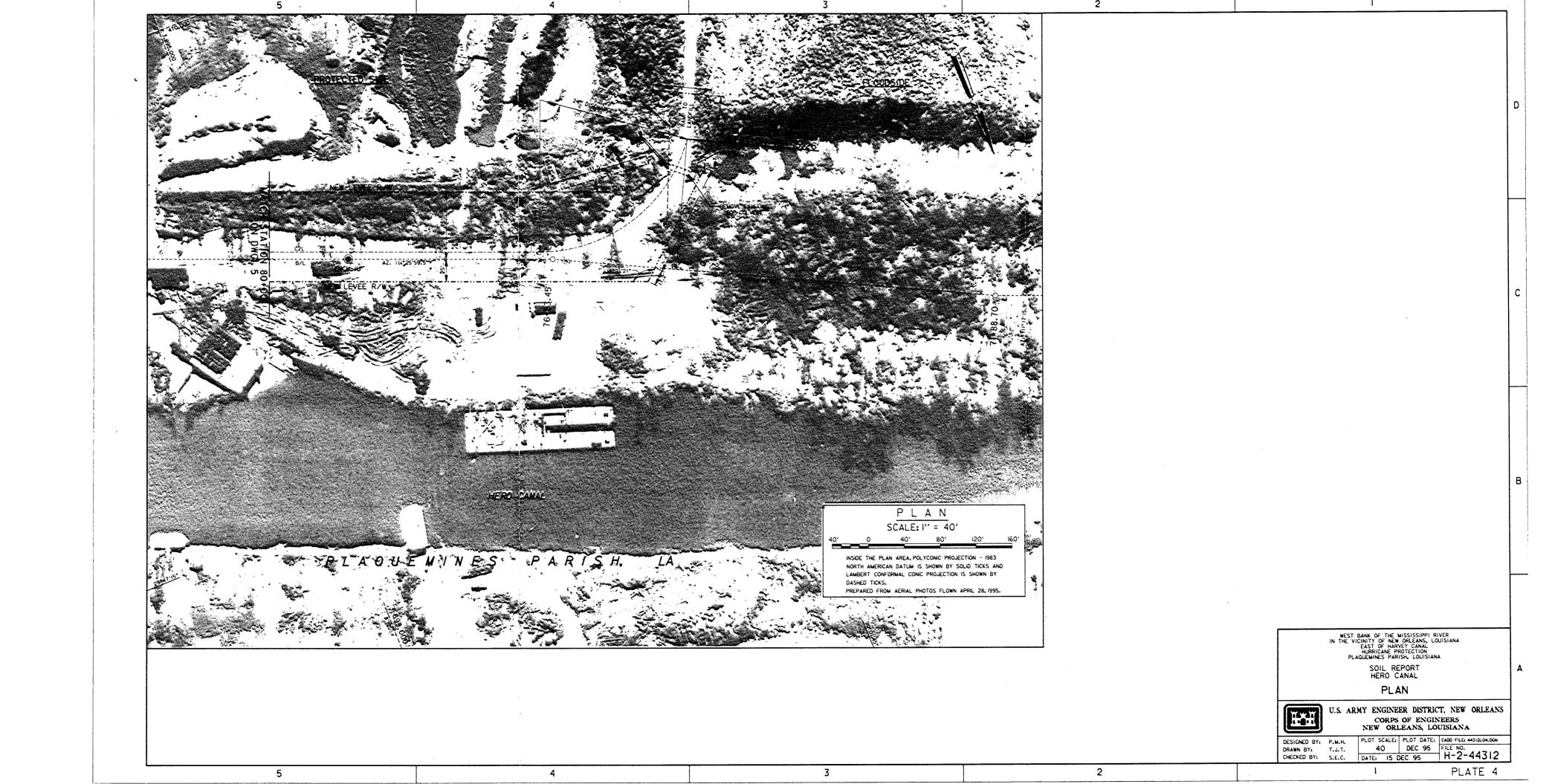
The still water level (SWL) used for Reaches II and III was 7.5 and 7.0 for Reach I. The low water level used for the canals in all three reaches was -2.0.

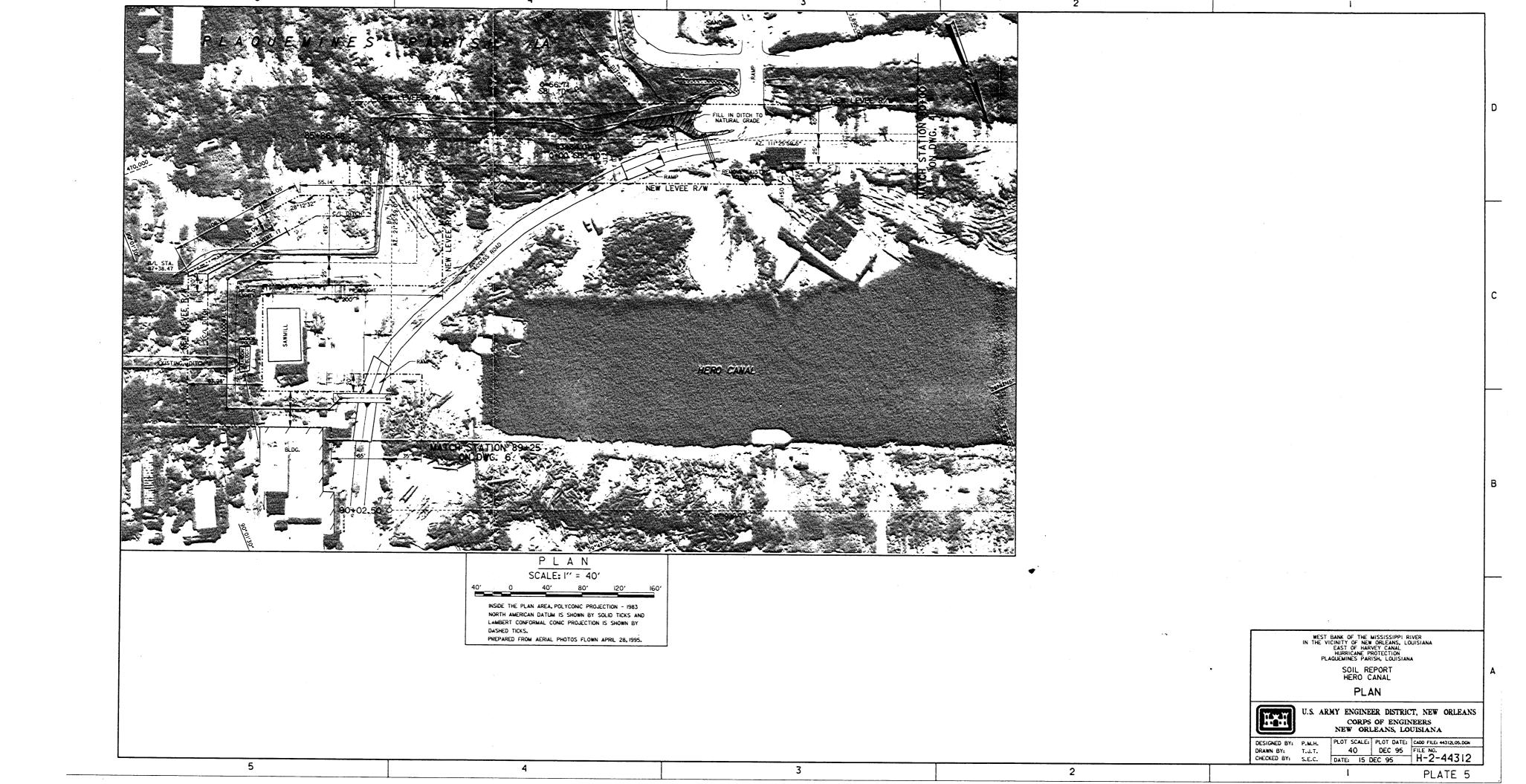
- 8. Stability of Levees. Existing conditions along the proposed alignment were determined from surveys taken in Jan 1995. The slopes and berm distances for the proposed levee were designed for the (Q) construction case. A "Factor of Safety" of 1.3 is required for the levee stability. Typical levee and floodwall sections are presented on Plates 11 thru 14.
- 9. <u>Borrow Material</u> Borrow material will be hauled from a nearby pit where the limits have been preliminarily established.
- 10. <u>Settlement</u>. Based on historical data, shrinkage and settlement of levee fill should be in the range of 10 to 20 percent over the 3 or 4 years after the enlargement. The enlargement should fall within this range of settlement since the centerline of the levee will straddle the existing levee base with approximately three feet of fill being added to the existing levee. A future enlargement will be required to restore the levee to design grade.

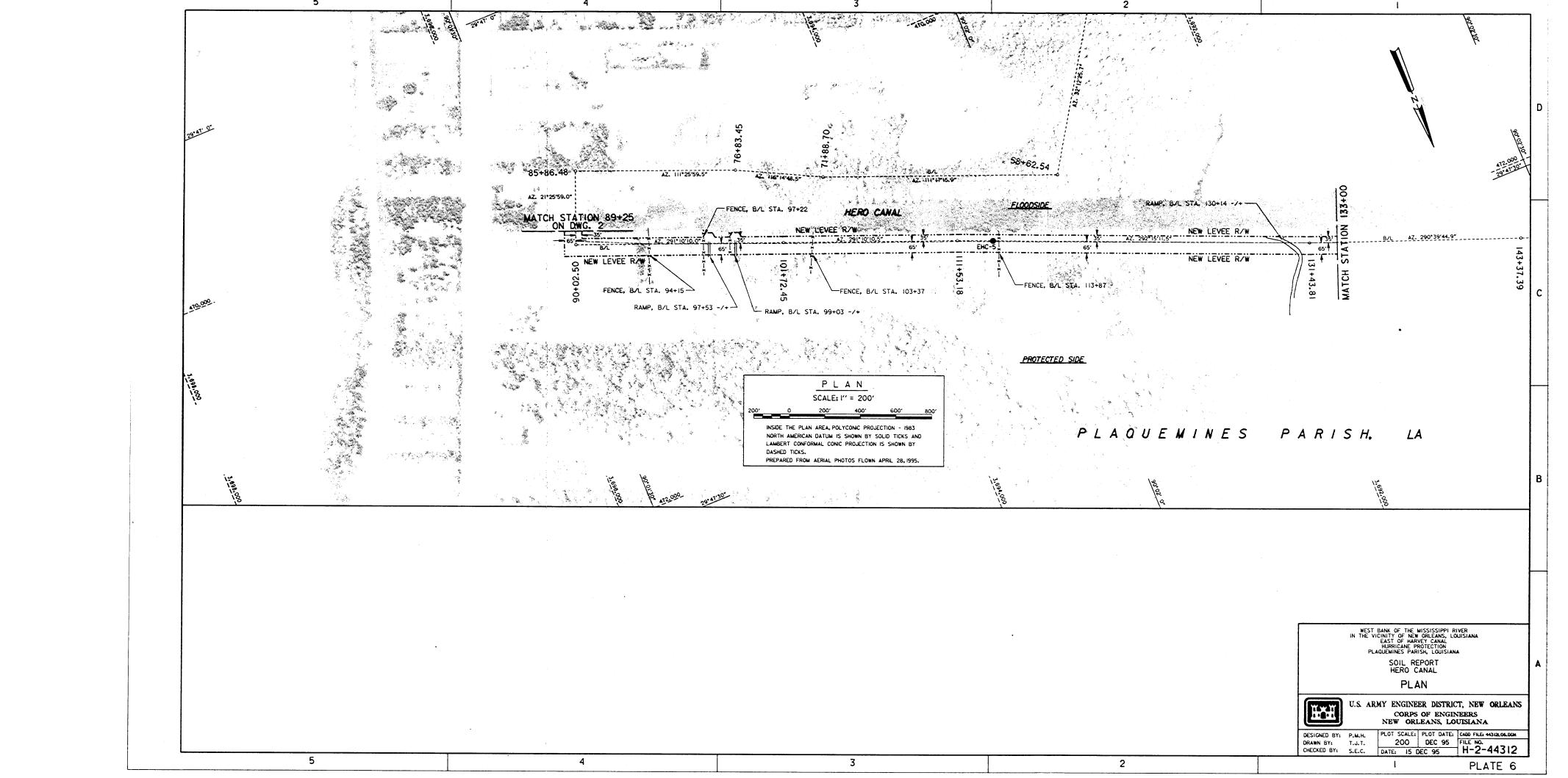


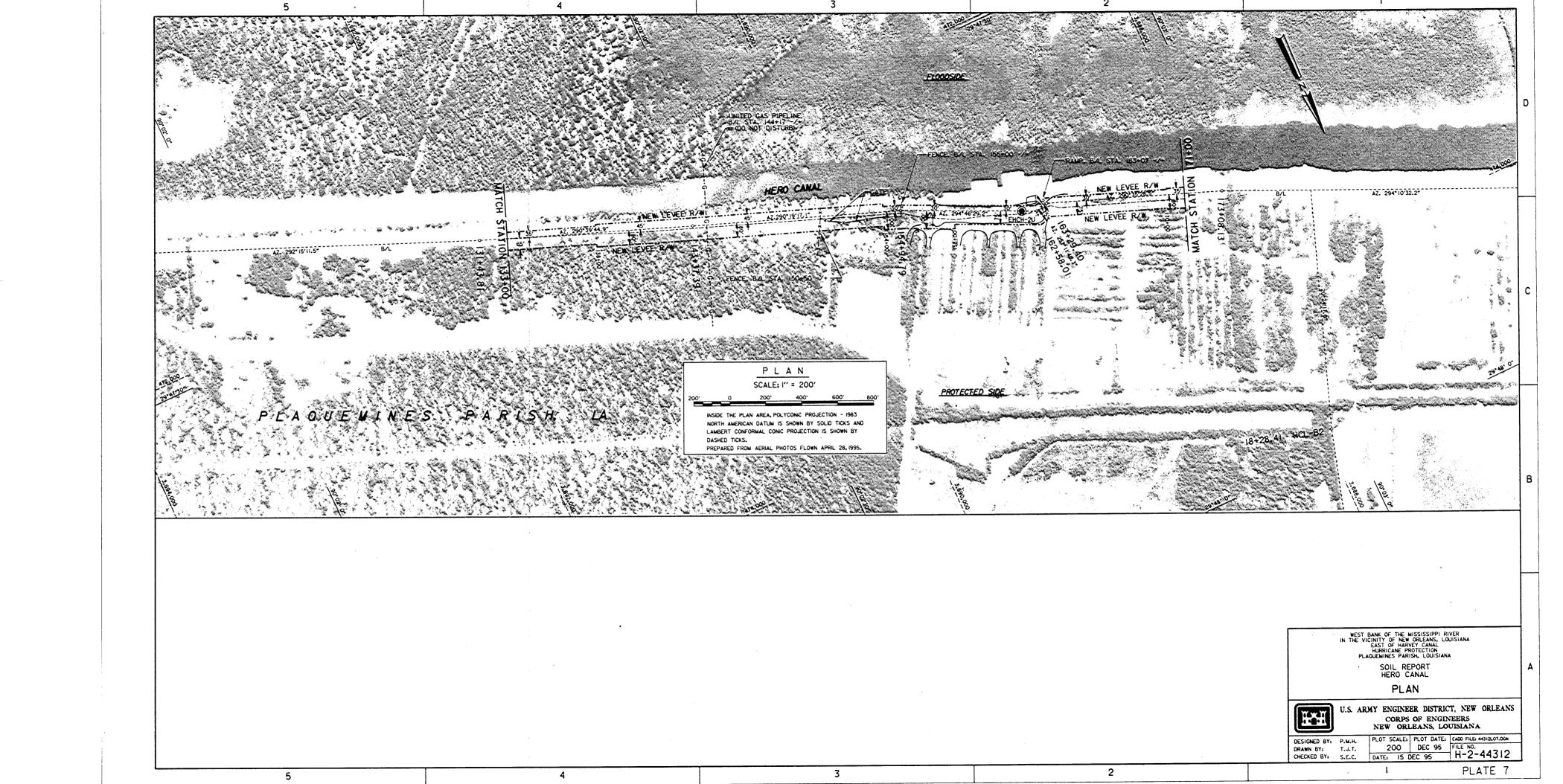


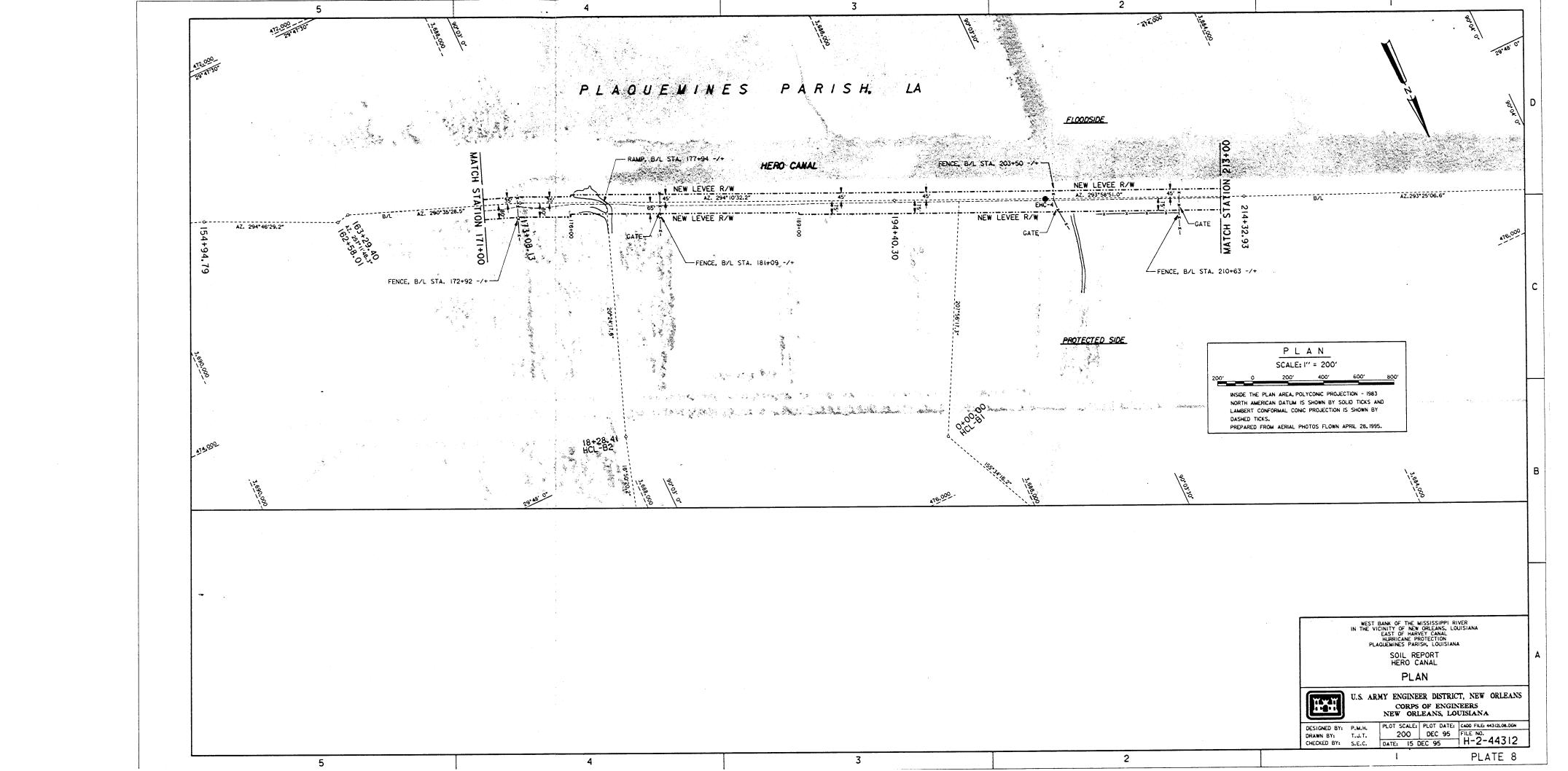


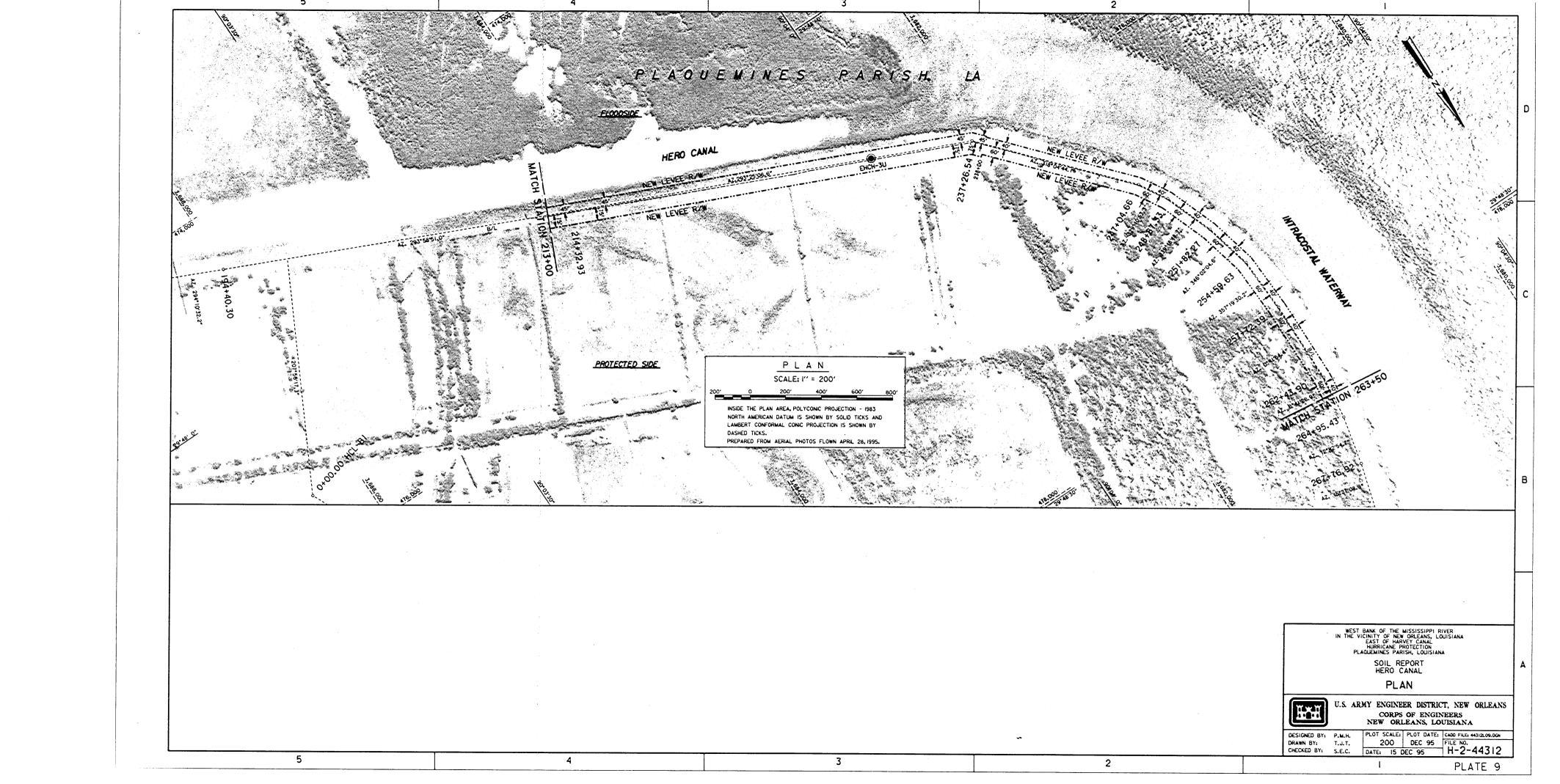


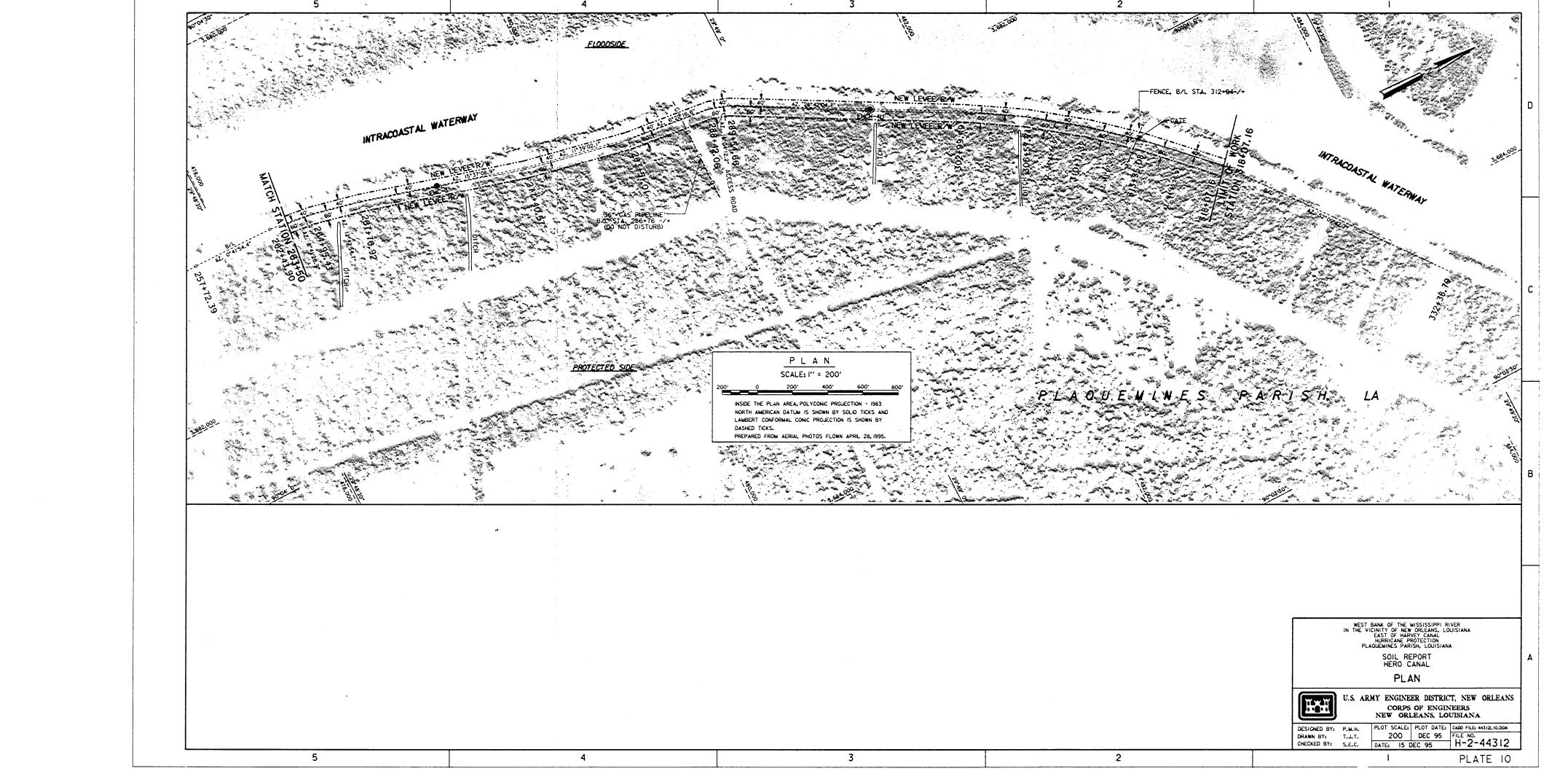


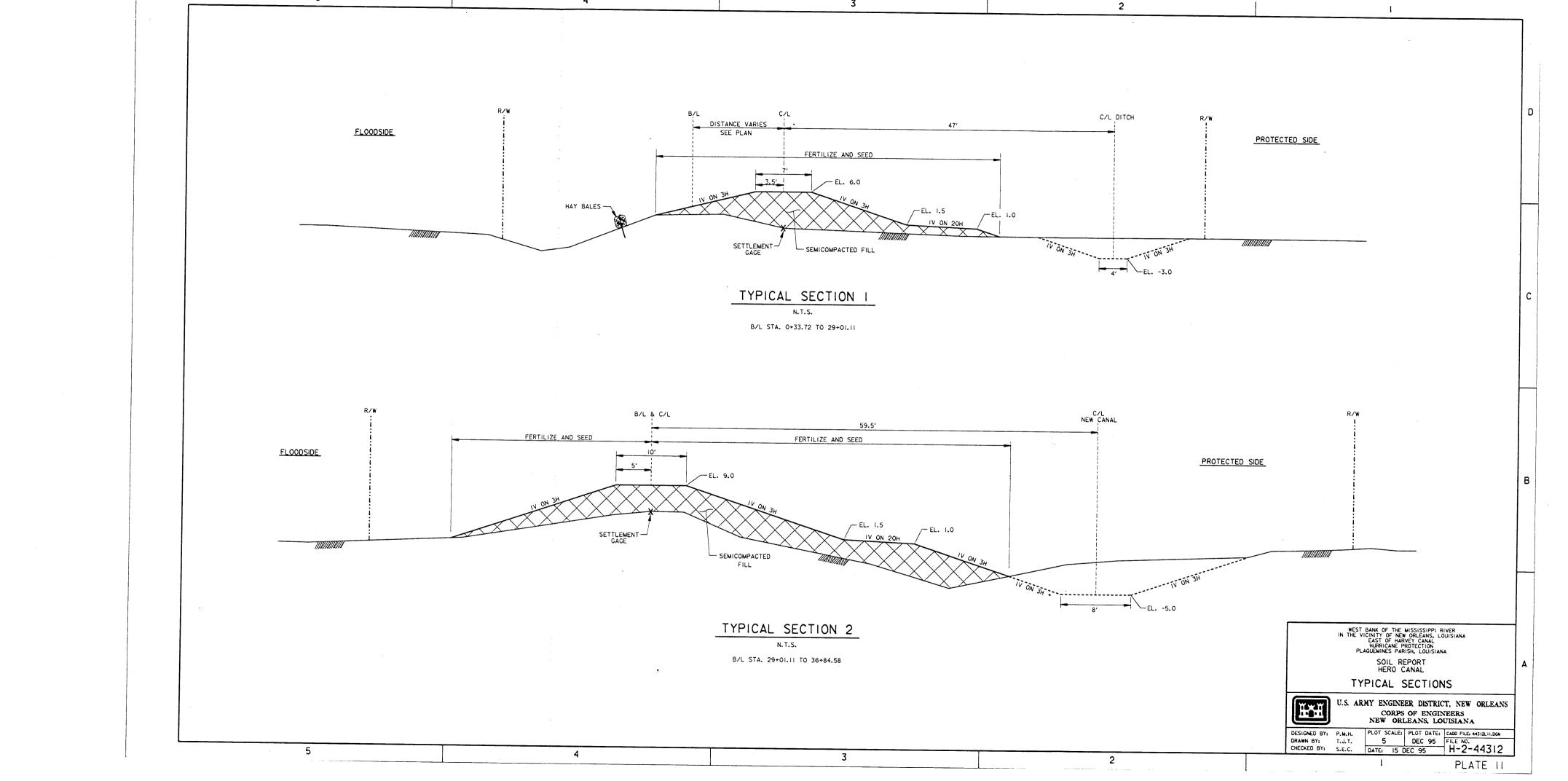


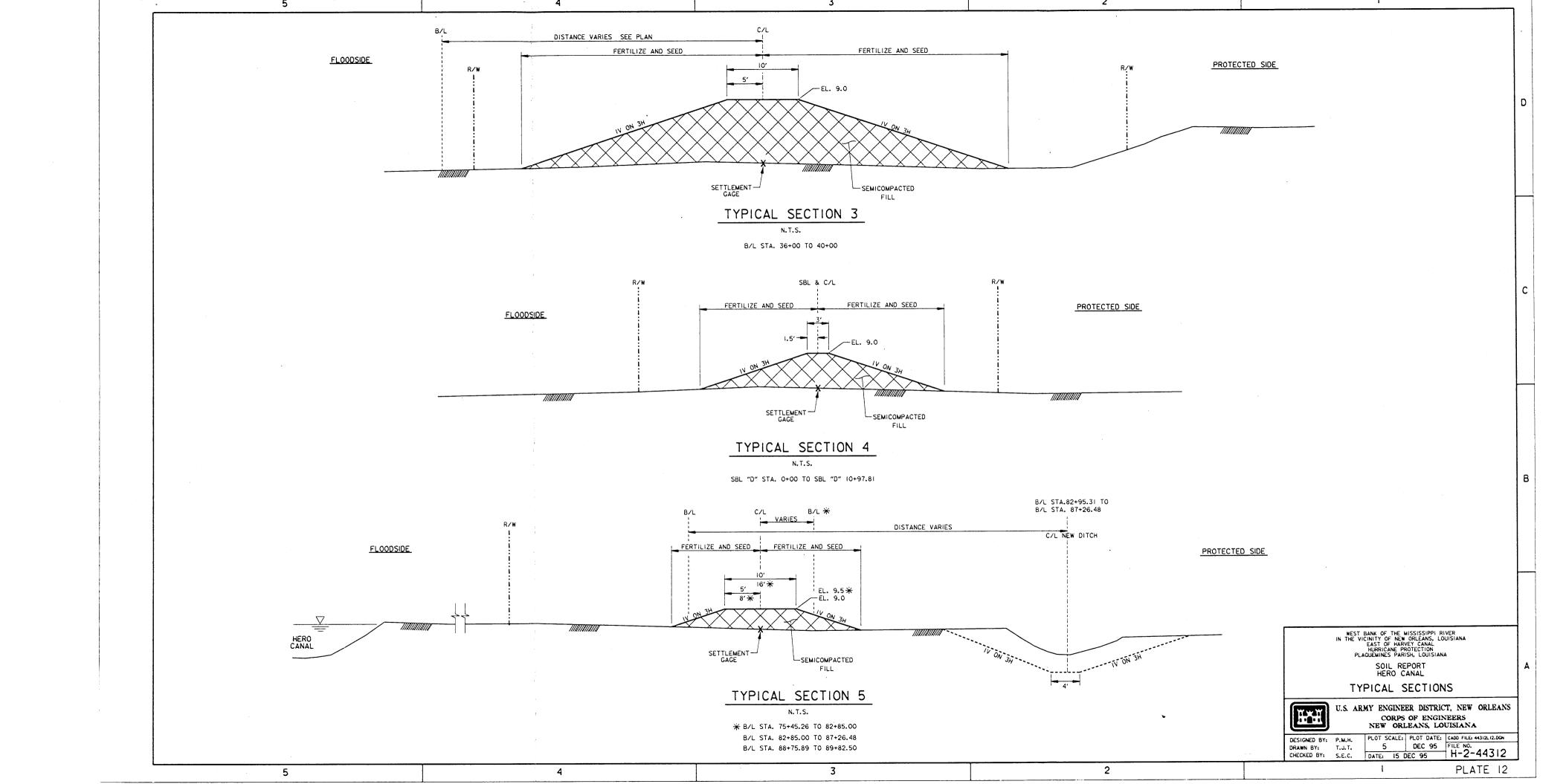


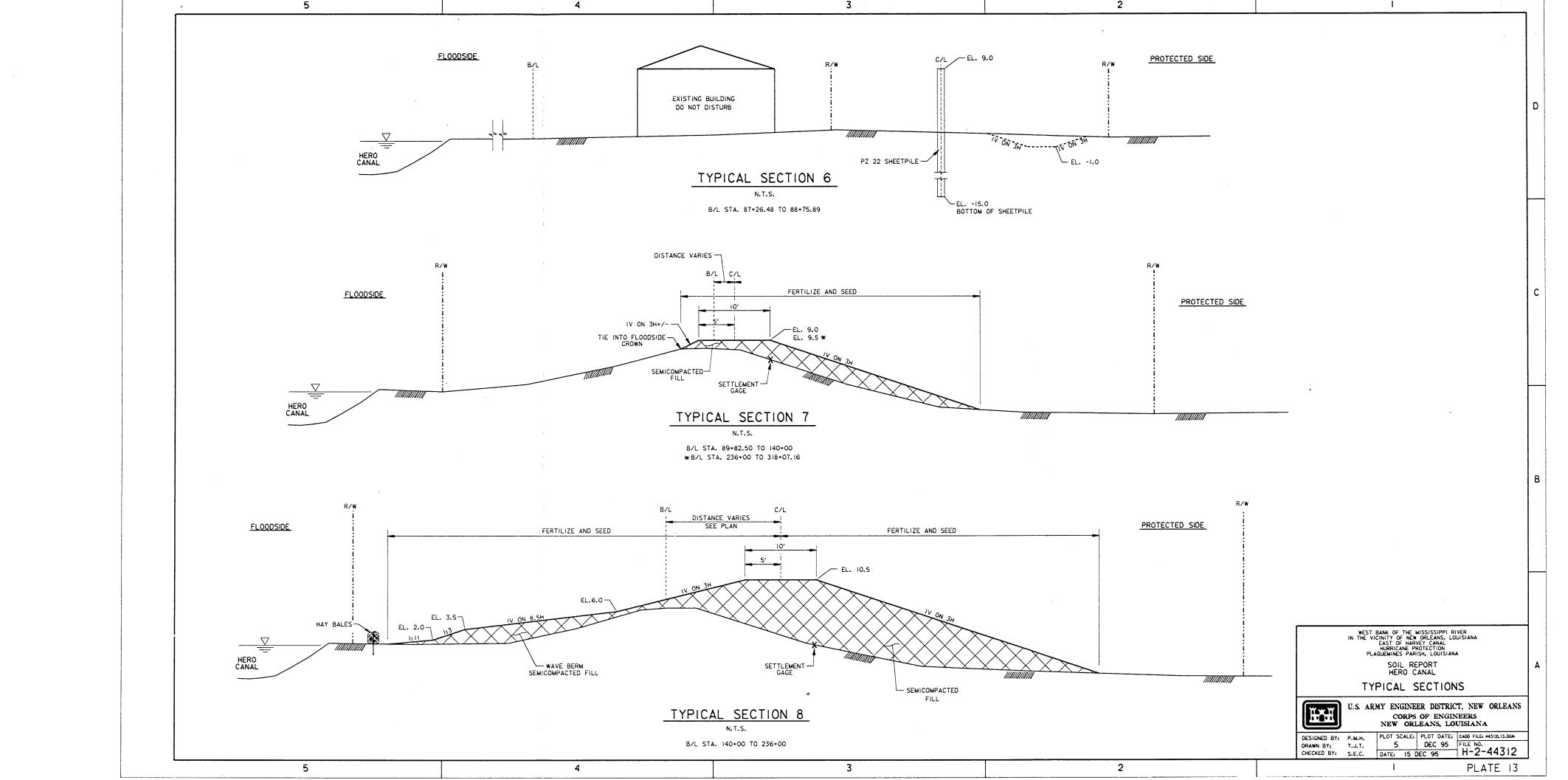


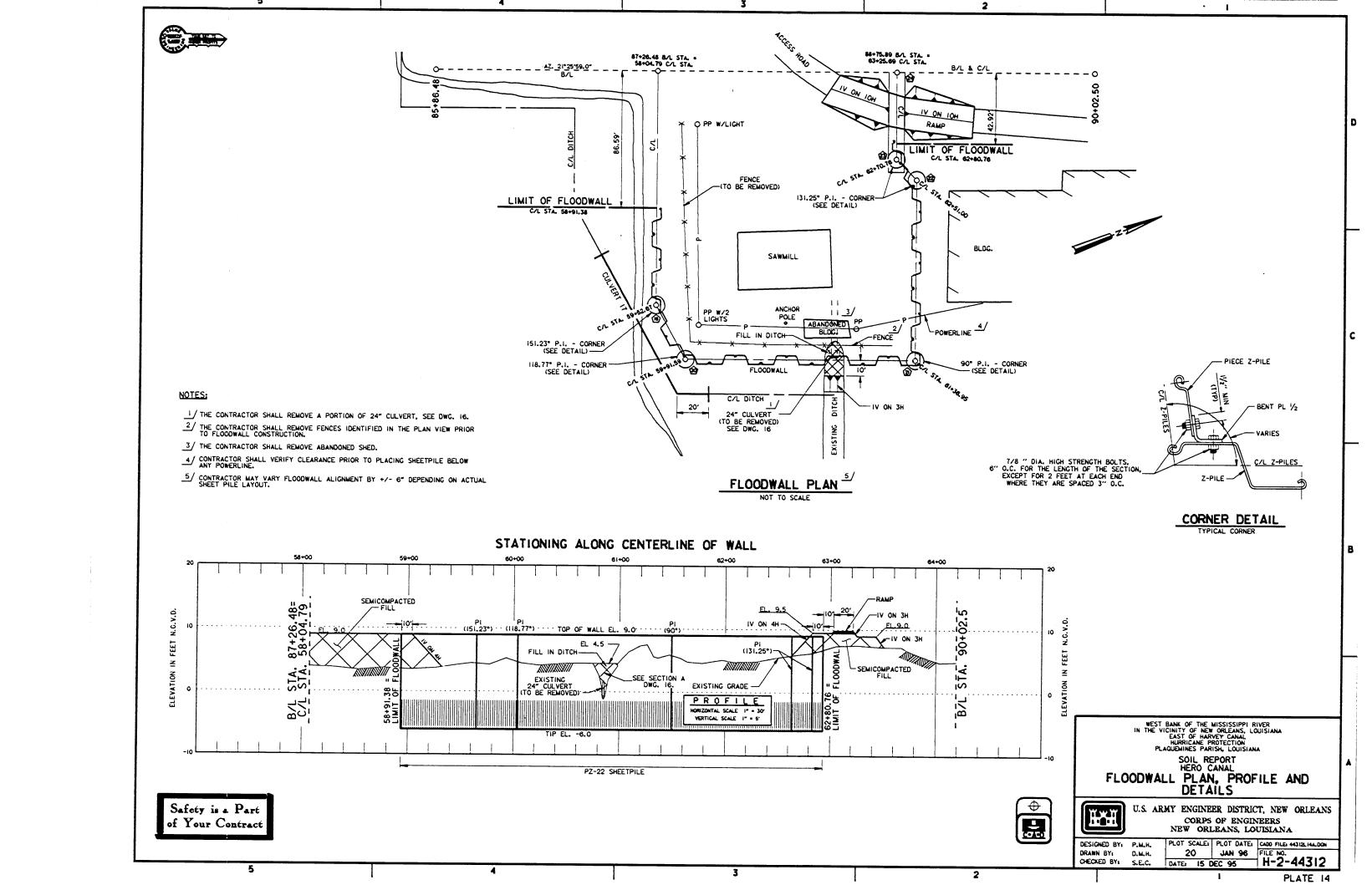


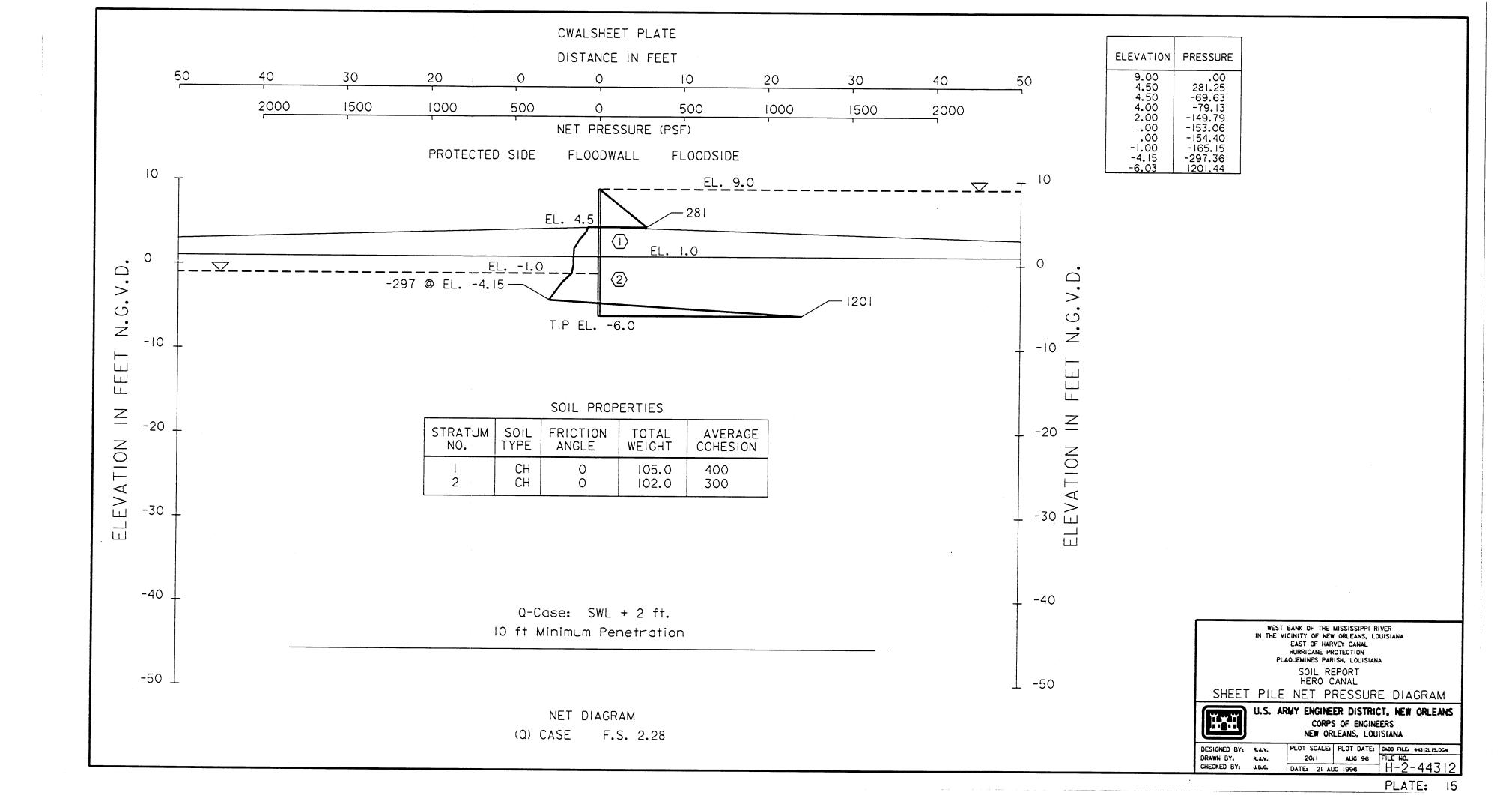


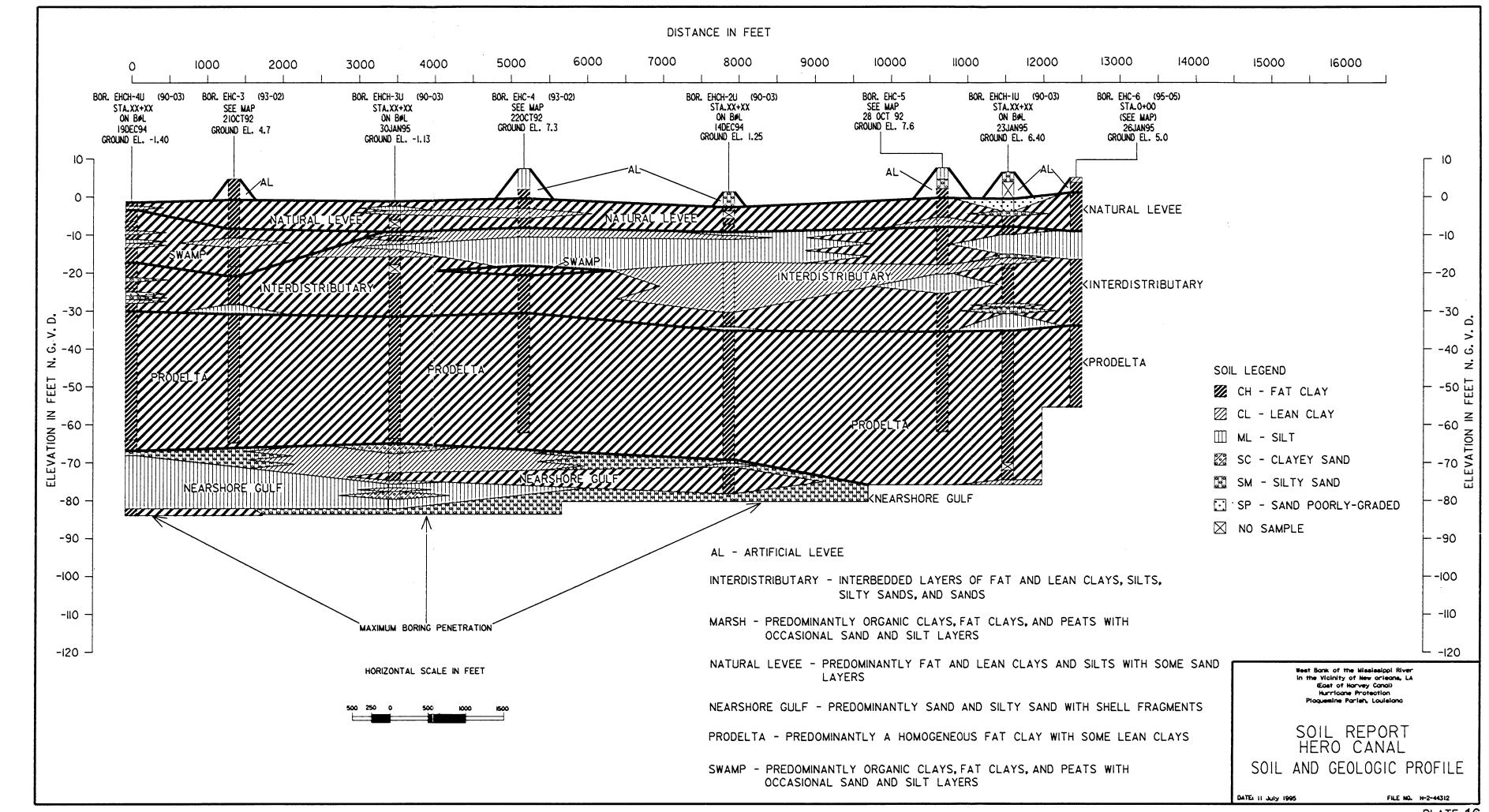


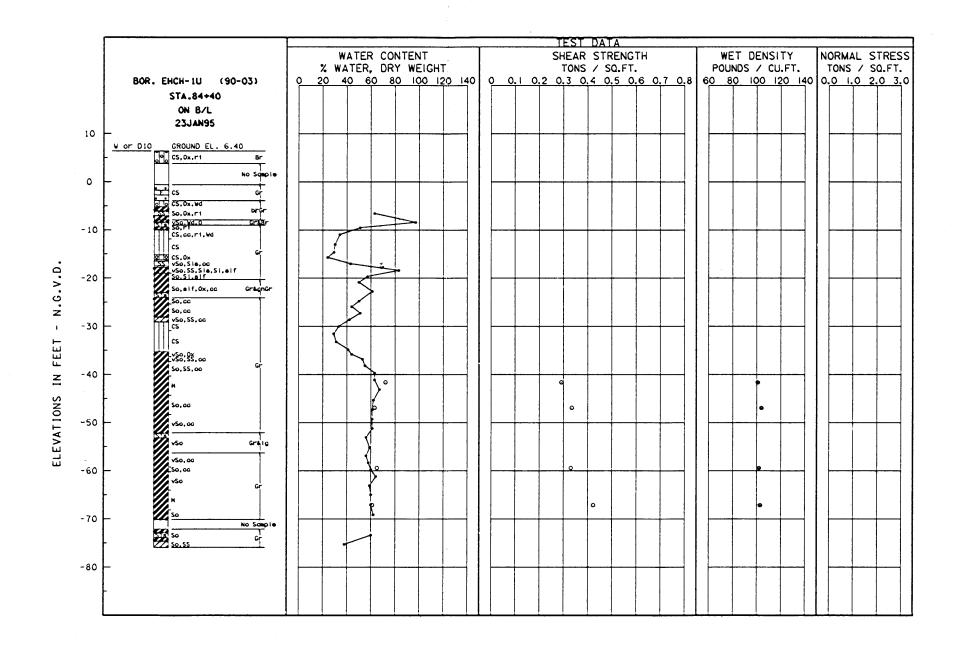










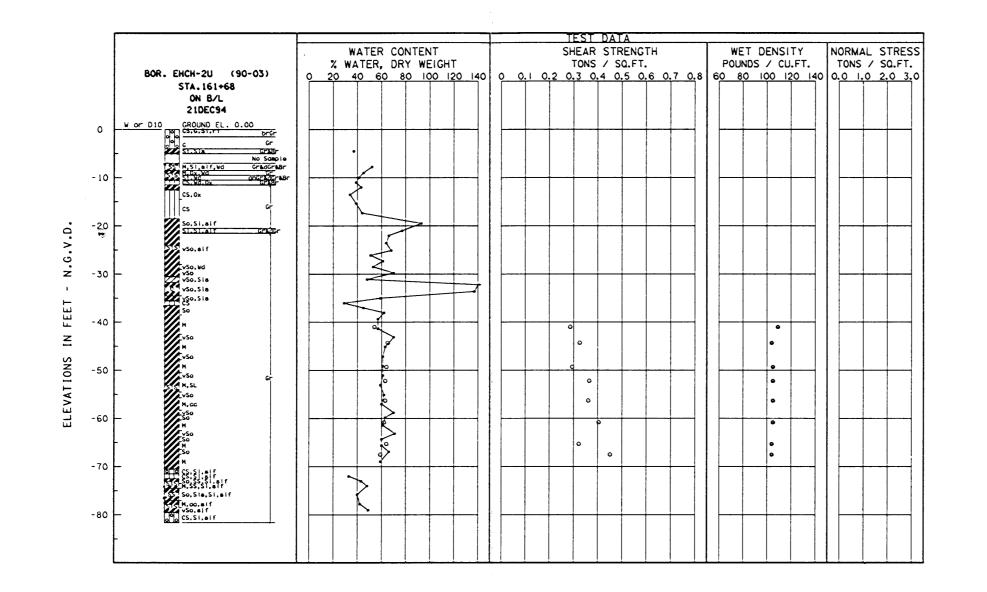


SOIL REPORT HERO CANAL UNDISTURBED BORING PLOT: EHCH-IU



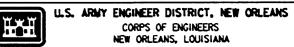
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS
NEW ORLEANS, LOUISIANA

DESIGNED BY: R.LV. PLOT SCALE: PLOT DATE: CADO FILE: 44312L17.DON
DRAWN BY: R.LV.
CHECKED BY: J.B.G. DATE: 21 AUG 1996 H-2-44312



SOIL REPORT HERO CANAL

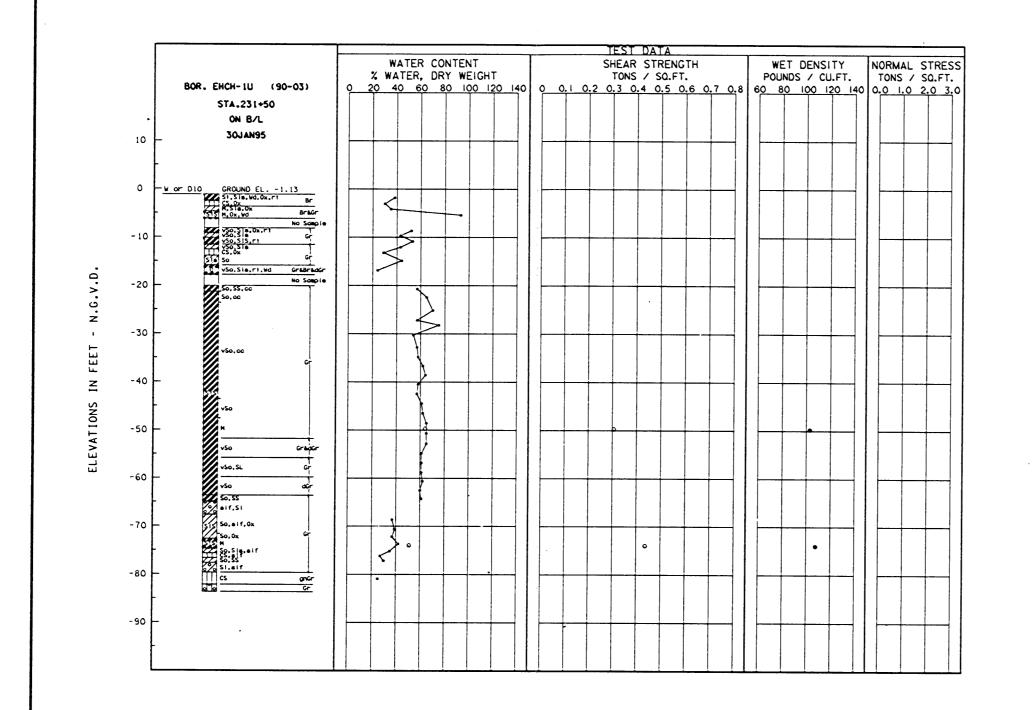
UNDISTURBED BORING PLOT: EHCH-2U



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CHECKED BY: J.B.G.

DATE: 21 AUG 1996

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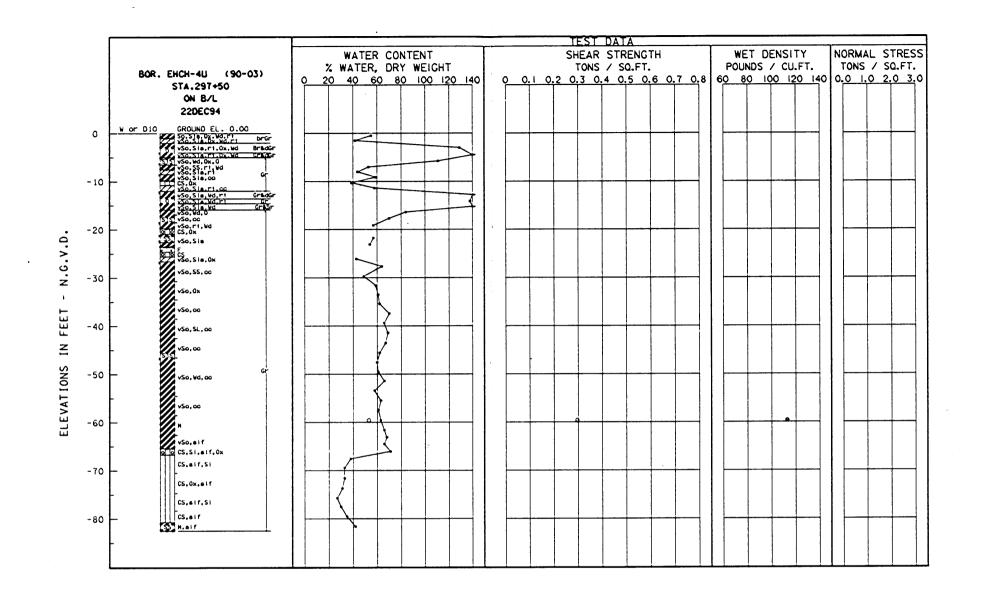


UNDISTURBED BORING PLOT: EHCH-3U



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SOIL REPORT HERO CANAL

UNDISTURBED BORING PLOT: EHCH-4U



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CHECKED BY: J.B.G.

PLOT SCALE: PLOT DATE: CADD FILE 443/2L20.DGN
FILE NO.
H-2-443/2

BOR. EHC-3 STA. 272+21 10 __ 21 OCT 92 2 CROUNI
34 SI, 0x, r1
52 M, r1, 0x
29 M, r1, 0x, e1f
29 M, 0x
31 SIS, So, 0x, r1
6 0 So, SIS, r1, 0x
SIS SSO, r1, 0x
SIS SSO, r1, 0x CROUND EL. 4.7 dGr&BK -10 50,F1,0x,0,SL 323 GradGr // vSo.r1.0x H.SIS.F1.0x -20 118 GrædGr SIS So, 0x 337 Tcs.ox -30 -206 -40 H 321 318 398 -50 ├ 53 408 63 63 -60 | 53 -70 |

-80 <u></u>

BOR. EHC-4 STA. 204+33

22 OCT 92

GROUND EL. 7.3

CS.Wd.rt.elf

CS.rt.0x

CS.Ox

50,0x 50,0x 50,0d,0x

0 So.SIS.Wd.r1.0x

W or 010

32

42

55

59

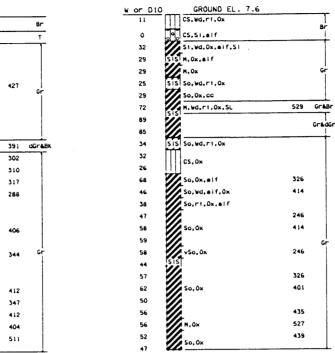
59

59

STA. 113+50

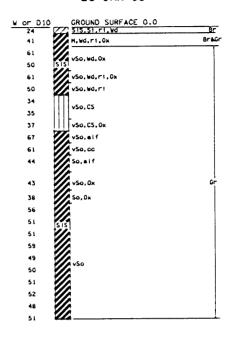


BOR. EHC-5



BOR. EHC-6 STA. 32+95

26 JAN 95



-20 -30 Z -40 -50 0 -60 -70 _____-80

___ 10

- 0

-10

NOTES

- BORROW BORINGS (BB) WERE TAKEN WITH A HAND AUGER AND ARE DISCONTINUOUS.
- GENERAL TYPE BORINGS WERE TAKEN WITH A 178" 1.D. TUBE SAMPLER AND/OR A 13/8" I.D. SPLIT SPOON SAMPLER.
- FOR SOIL BORING LEGEND SEE PLATE A
- FOR LOCATION OF BORINGS SEE DWGS.



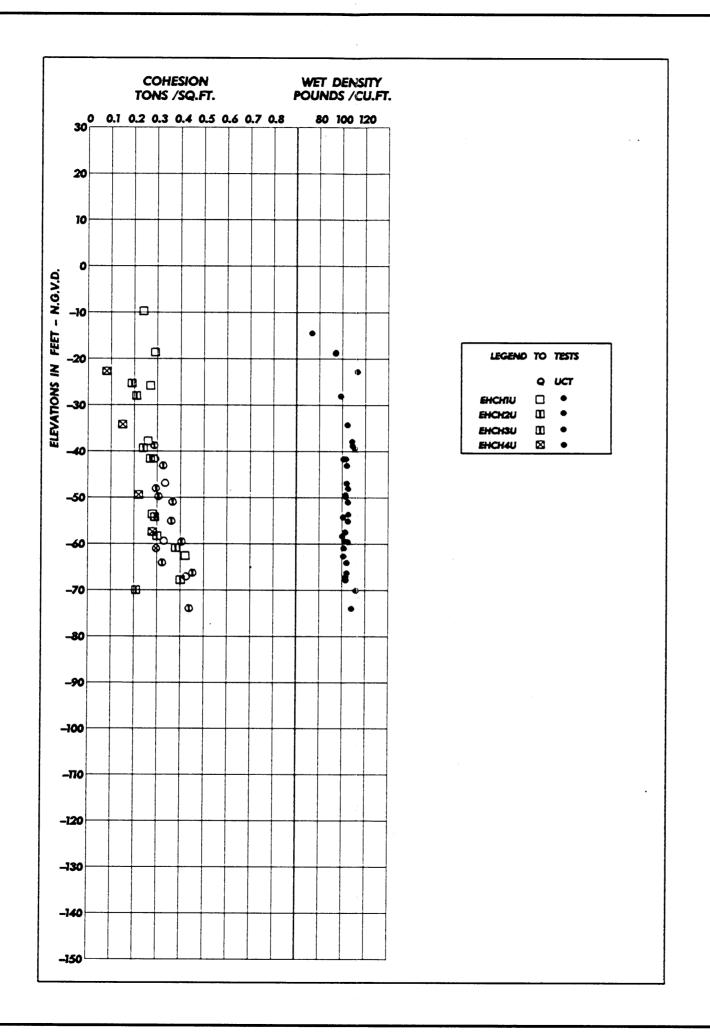
WEST BANK OF THE MISSISSIPPI RIVER IN THE VICINITY OF NEW ORLEANS, LOUISIANA EAST OF HARVEY CANAL HURRICANE PROTECTION PLAQUEMINES PARISH, LOUISIANA SOIL REPORT

HERO CANAL GENERAL BORING PLOTS



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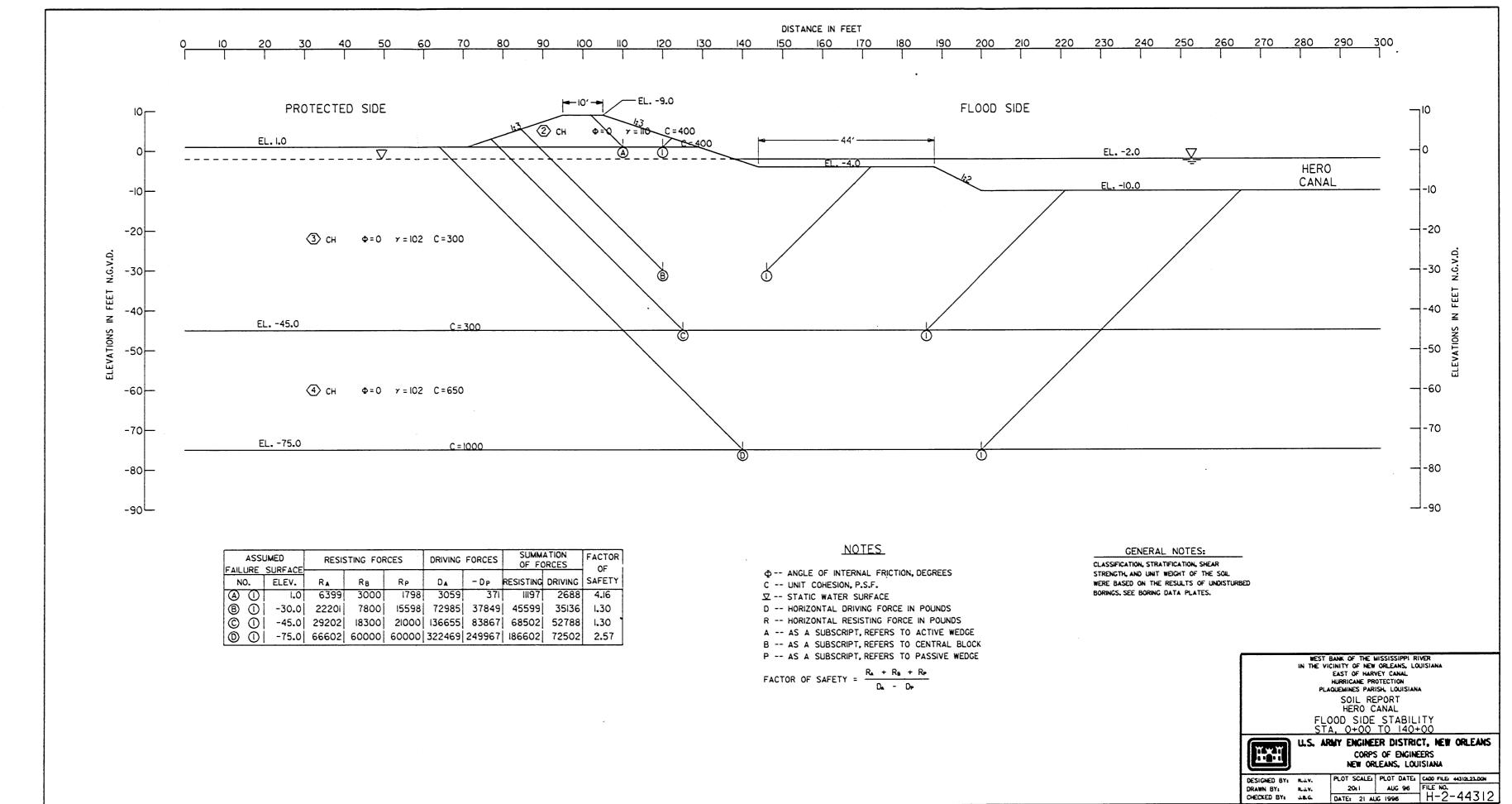
SOIL REPORT HERO CANAL SHEAR STRENGTH PLOTS

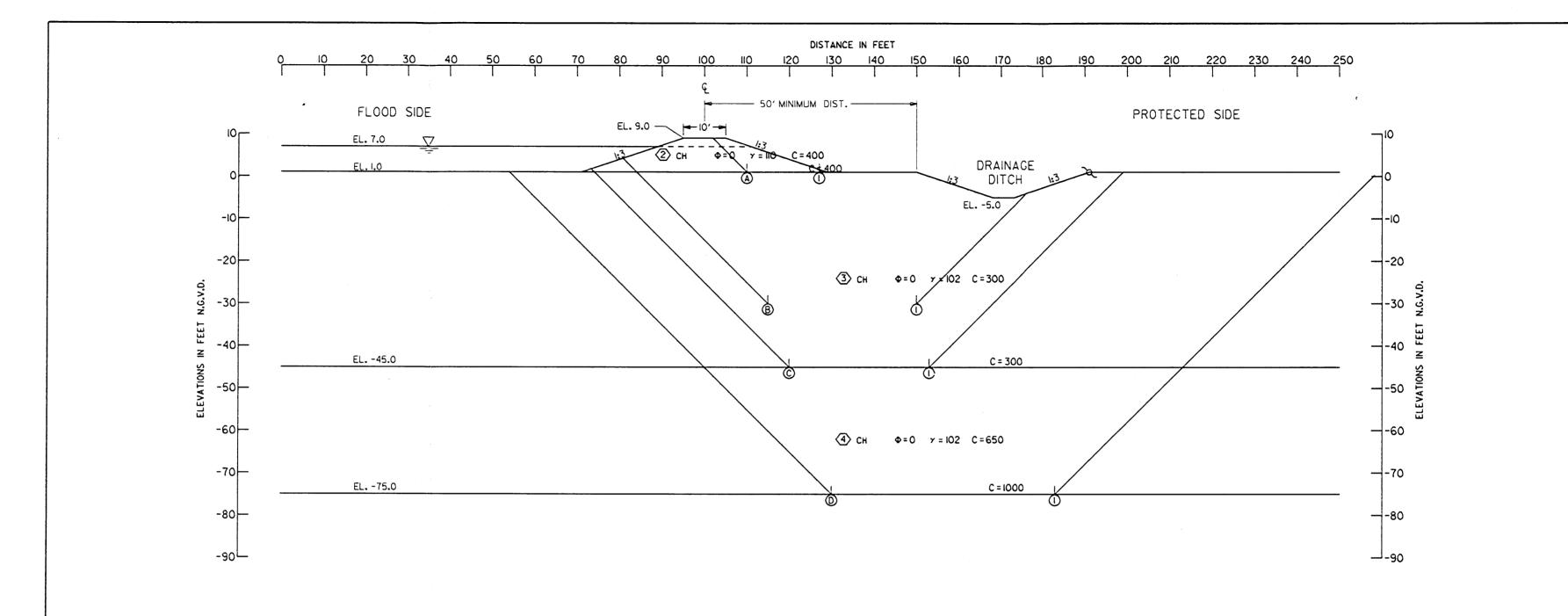
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NOTES

		JMED SURFACE	RESISTING FORCES		CES	DRIVING FORCES		SUMMATION OF FORCES		FACTOR OF
NO.		ELEV.	R _A	RB	Rp	DA	- Dp	RESISTING	DRIVING	
(A) (O I	1.0	6399	5100	400	3059	18	11899	3041	3.91
(B) (0	-30.0	21202	10500	15602	73957	37483	47304	36474	1.30
(C) (01	-45.0	28202	9900	27600	139670	93992	65702	45678	1.44
0	<u> </u>	-75.0	66602	53000	66143	335365	293129	185745	42236	4.40

Φ -- ANGLE OF INTERNAL FRICTION, DEGREES

C -- UNIT COHESION, P.S.F.

☑ -- STATIC WATER SURFACE

D -- HORIZONTAL DRIVING FORCE IN POUNDS

R -- HORIZONTAL RESISTING FORCE IN POUNDS A -- AS A SUBSCRIPT, REFERS TO ACTIVE WEDGE

B -- AS A SUBSCRIPT, REFERS TO CENTRAL BLOCK

P -- AS A SUBSCRIPT, REFERS TO PASSIVE WEDGE

FACTOR OF SAFETY = $\frac{R_A + R_B + R_P}{D_A - D_P}$

GENERAL NOTES:

CLASSIFICATION, STRATIFICATION, SHEAR STRENGTH, AND UNIT WEIGHT OF THE SOIL WERE BASED ON THE RESULTS OF UNDISTURBED BORINGS. SEE BORING DATA PLATES.

> WEST BANK OF THE MISSISSIPPI RIVER IN THE VICINITY OF NEW ORLEANS, LOUISIANA EAST OF HARVEY CANAL HURRICANE PROTECTION PLAQUEMINES PARISH, LOUISIANA SOIL REPORT HERO CANAL PROTECTED SIDE STABILITY STA. 0+00 TO 140+00



U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA

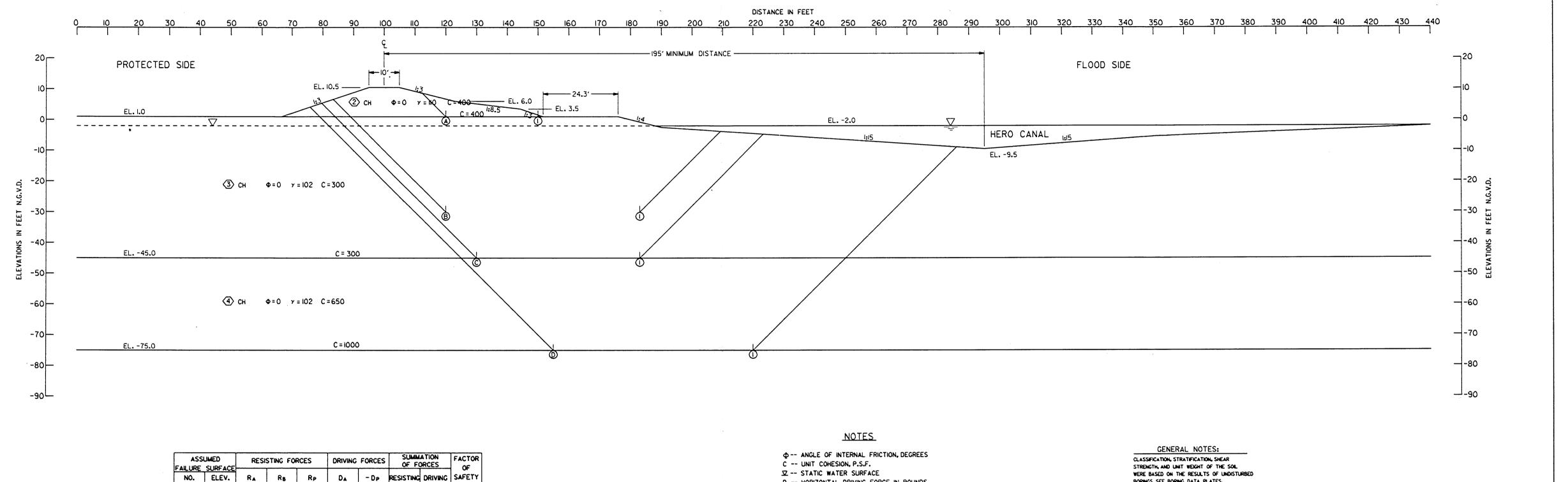
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DATE: 21 AUG 1996

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PLATE: 24



6|3| 9000 350 2423 | |4 | |548| 2409 6.43

(B) (1) -30.0 | 23101 | 18900 | 15731 | 79968 | 39352 | 57732 | 40616 | 1.42

© ① | -45.0 | 31101 | 15900 | 24169 | 146928 | 92333 | 71170 | 54595 | 1.30

(D) (1) -75.0 69102 65000 60657 341905 258553 194759 83352 2.34

D -- HORIZONTAL DRIVING FORCE IN POUNDS R -- HORIZONTAL RESISTING FORCE IN POUNDS A -- AS A SUBSCRIPT, REFERS TO ACTIVE WEDGE

B -- AS A SUBSCRIPT, REFERS TO CENTRAL BLOCK P -- AS A SUBSCRIPT, REFERS TO PASSIVE WEDGE

FACTOR OF SAFETY = $\frac{R_A + R_B + R_P}{D_A - D_P}$

BORINGS. SEE BORING DATA PLATES.

WEST BANK OF THE MISSISSIPPI RIVER IN THE VICINITY OF NEW ORLEANS, LOUISIANA EAST OF HARVEY CANAL HURRICANE PROTECTION PLAQUEMINES PARISH, LOUISIANA SOIL REPORT HERO CANAL FLOOD SIDE STABILITY STA, 140+00 TO 237+00



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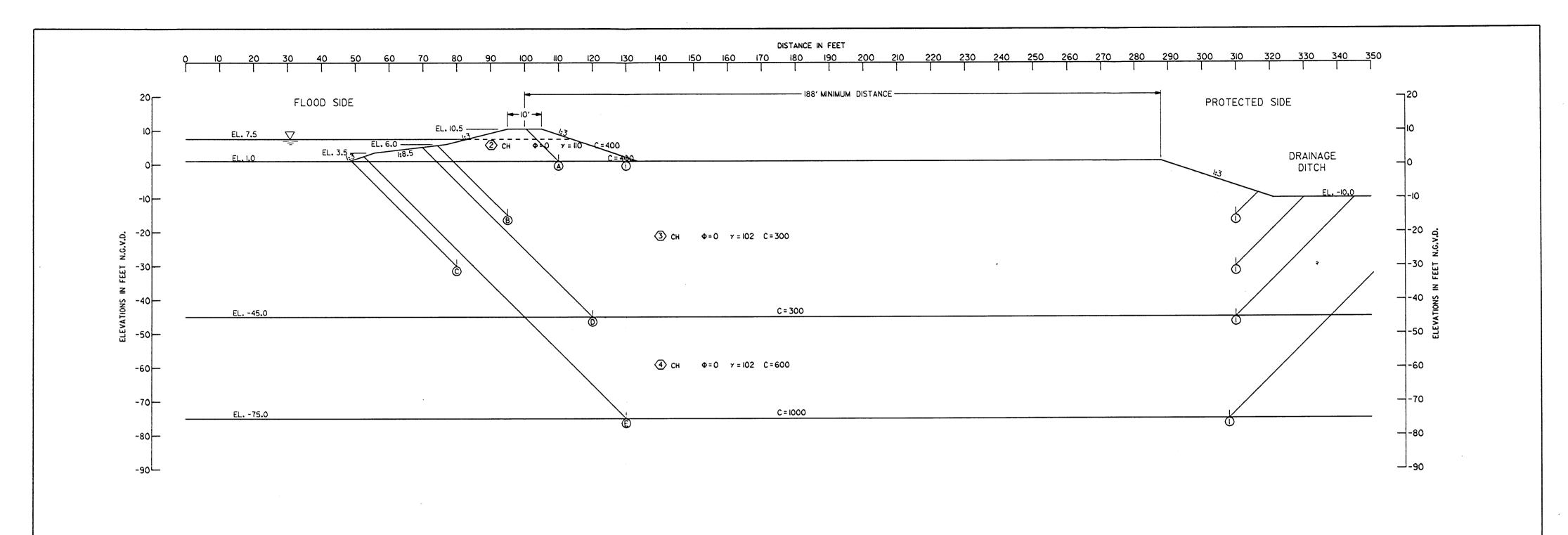
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DATE: 21 AUG 1996

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ASSUMED FAILURE SURFACE		RESISTING FORCES			DRIVING FORCES		SUMMATION OF FORCES		FACTOR OF	
NC		ELEV.	R _A	RB	Rp	DA	- DP	RESISTING	DRIVING	SAFETY
(A)	0	1.0	7600	6000	700	4503	56	14300	4447	3.22
⑱	①	-15.0	13349	64500	3900	28299	2872	81749	25427	3.22
©	①	-30.0	18752	69000	12000	67919	22456	99752	45463	2.19
0	1	-45.0	30928	57000	21000	148232	64529	108928	83703	I . 30
(E)	①	-75.0	64752	178000	55834	349016	216061	298586	132955	2.25

NOTES

Φ -- ANGLE OF INTERNAL FRICTION, DEGREES

C -- UNIT COHESION, P.S.F.

▼ -- STATIC WATER SURFACE

D -- HORIZONTAL DRIVING FORCE IN POUNDS

R -- HORIZONTAL RESISTING FORCE IN POUNDS

A -- AS A SUBSCRIPT, REFERS TO ACTIVE WEDGE B -- AS A SUBSCRIPT, REFERS TO CENTRAL BLOCK

P -- AS A SUBSCRIPT, REFERS TO PASSIVE WEDGE

FACTOR OF SAFETY = $\frac{R_A + R_B + R_F}{D_A - D_F}$

GENERAL NOTES:

CLASSIFICATION, STRATIFICATION, SHEAR STRENGTH, AND UNIT WEIGHT OF THE SOIL WERE BASED ON THE RESULTS OF UNDISTURBED BORINGS. SEE BORING DATA PLATES.

WEST BANK OF THE MISSISSIPPI RIVER IN THE VICINITY OF NEW ORLEANS, LOUISIANA EAST OF HARVEY CANAL HURRICANE PROTECTION PLAQUEMINES PARISH, LOUISIANA SOIL REPORT HERO CANAL PROTECTED SIDE STABILITY STA, 140+00 TO 237+00



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DATE: 21 AUG 1996

PLOT DATE: 44312226.00N

FILE 44312226.00N

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