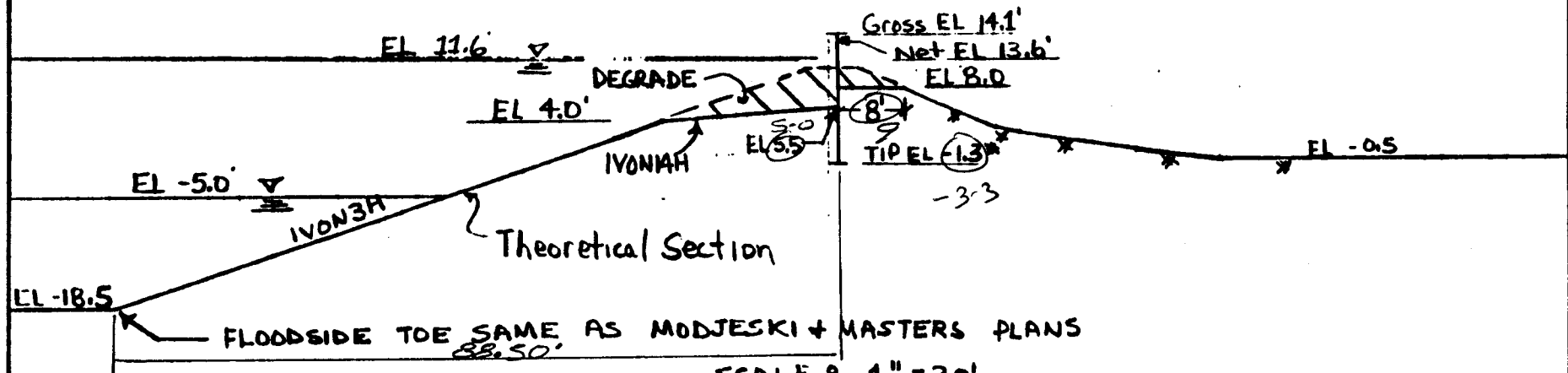


(ADD 6678)

REF SED 4/29/88

### STA 614+00 TO STA 625+19 (N. Veterans Hwy) JEFFERSON SIDE



FLOODSIDE TOE SAME AS MODJESKI + MASTERS PLANS

SCALE 8 1" = 20'

Elevation in Feet NGVD

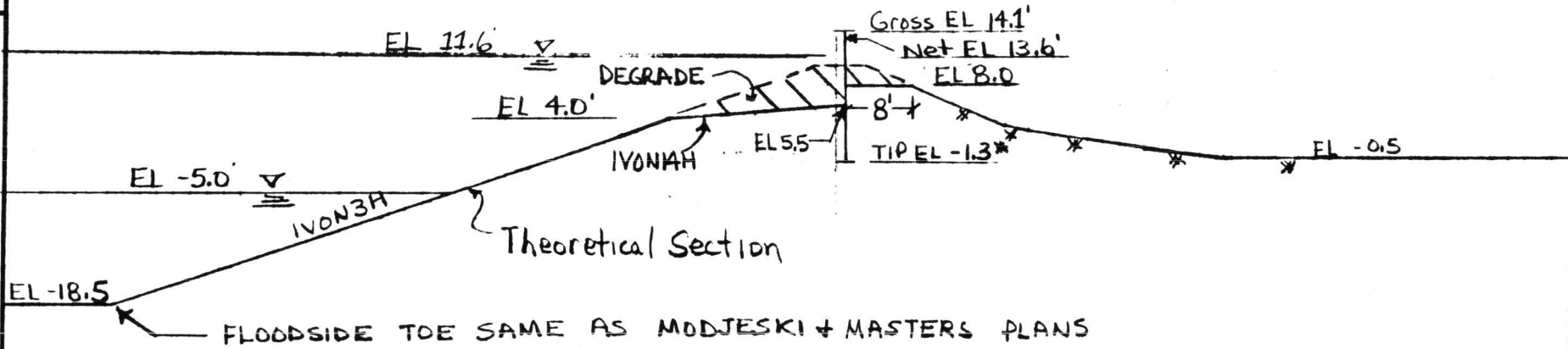
Q FILES	F.S.	SWL	CASE
Q625JA	1.0	11.6	S
* Q625JB	1.5	11.6	S
Q625JD	1.0	13.6	Q

NOTE: TRANSITION STA 614+00 TO 615+00

PROJECT 1744 St Offfall Canal	PAGE	OF	COMPUTED BY	DATE
SUBJECT STA 614+00 TO STA 625+19 JEFFERSON				4/24/87
	CHECKED BY			

PROJECT 17th St Outfall Canal	PAGE OF	COMPUTED BY	DATE
SUBJECT STA 614+00 TO STA 625+00		7/24/87	
		CHECKED BY	DATE

STA 614+00 TO STA 625+19 (N. Veterans Hwy)  
**JEFFERSON SIDE**



SCALE 8 1" = 20'  
 Elevation in Feet NGVD

**ADVANCE COPY**  
 SUBJECT TO CORRECTION

- Q625J11
- Q625J12 \*
- Q625J13
- Q625J10

Q FILES	FS.	SWL	CASE
Q625JA	1.0	11.6	S
Q625JB	1.5	11.6	S
Q625JD	1.0	13.6	Q

DRWZZE

REVISED 4/29/88

Encl 3

LI Q625JL

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH STR Q625JA-SWL=11.6-S CASE FS=1.0  
=1

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT  
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS  
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.05 INCHES AND OCCURS AT MEMBER COORDINATE  
13.60 FT.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE(FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
0.86	POINT LD	0.00 LBF
0.86	COUPLE	-34.21 LBF-FT

$\Delta_F \approx 3 \Delta_{P222}$

$\Delta_F \approx 3 \times .05$   
 $< 1.5''$

INPUTTED LOADS

DISTANCE FROM REFERENCE(FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
11.60	CONTN LD	0.00 LBF/SQ FT
10.60	CONTN LD	62.50 LBF/SQ FT
9.60	CONTN LD	125.00 LBF/SQ FT
8.60	CONTN LD	187.50 LBF/SQ FT
8.00	CONTN LD	225.00 LBF/SQ FT
8.00	CONTN LD	225.00 LBF/SQ FT
7.00	CONTN LD	13.59 LBF/SQ FT
6.94	CONTN LD	0.00 LBF/SQ FT
6.00	CONTN LD	-197.83 LBF/SQ FT
5.50	CONTN LD	-303.54 LBF/SQ FT
5.50	CONTN LD	-303.54 LBF/SQ FT
4.50	CONTN LD	-490.90 LBF/SQ FT
4.08	CONTN LD	-569.33 LBF/SQ FT
2.70	CONTN LD	0.00 LBF/SQ FT
0.86	CONTN LD	763.05 LBF/SQ FT
0.86	CONTN LD	0.00 LBF/SQ FT

Z-22 PROPERTIES ARE AS FOLLOWS.

58 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL  
 59 CROSS SECTIONAL AREA= 1.84 SQ IN.  
 60 ELASTIC MODULUS= 29000000. LBF/SQ IN.  
 61 DEFLECTION REFERENCE IS AT -1.300

62  
 63  
 64 THE MAXIMUM BENDING MOMENT IS 1783.07 LBF-FT AND OCCURS AT 4.73  
 65 WHICH HAS THE SHEAR FORCE OF 17.90 LBF.  
 66  
 67

DEFLECTION  
 FROM TANG.  
 THRU DEFLE

70	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	REFERENCE
71	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	(INCHES )
72	13.600	0.0	0.0	0.0	0.0491
73	13.599	0.0	0.0	0.0	0.0491
74	13.000	0.0	0.0	0.0	0.0457
75	12.000	0.0	0.0	0.0	0.0399
76	11.000	11.2	6.1	2.2	0.0342
77	10.000	80.0	43.5	42.7	0.0284
78	9.000	211.2	114.8	183.1	0.0227
79	8.000	405.0	220.1	486.0	0.0171
80	7.000	524.3	284.9	968.3	0.0119
81	6.936	524.7	285.2	1002.0	0.0116
82	6.000	432.2	234.9	1464.1	0.0074
83	5.000	131.6	71.5	1762.6	0.0039
84	4.731	17.9	9.7	1783.1	0.0032
85	4.000	-357.3	-194.2	1664.5	0.0016
86	3.000	-686.3	-373.0	1108.3	0.0005
87	2.705	-704.5	-382.9	902.1	0.0003
88	2.703	-704.5	-382.9	900.7	0.0003
89	2.000	-602.1	-327.2	429.6	0.0001
90	1.000	-104.7	-56.9	41.8	0.0000
91	0.858	-0.8	-0.4	34.2	0.0000
92	0.856	0.0	0.0	0.0	0.0000
93	0.000	0.0	0.0	0.0	0.0000
94	-1.000	0.0	0.0	0.0	0.0000
95	-1.299	0.0	0.0	0.0	0.0000
96	-1.300	0.0	0.0	0.0	0.0000

100 \*RUN COMPLETED\*

101  
 EOT..  
 LI DRW22E  
 1 100 1 13.6 -1.3 1 -1.3 0 -1  
 2 200 PZ-22  
 3 300 29000000 1.84 84.38

EOT..  
 LI Q625J2  
 1  
 2 BEAMS (SHEAR, MOMENT, DEFLECTION)  
 3

4  
 5 17TH STR -Q625JB-SWL=11.6-S CASE-FS=1.5  
 6 =1.  
 7

8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT  
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS  
10 AS COUNTERCLOCKWISE.

11

13 THE MAXIMUM DEFLECTION IS 0.09 INCHES AND OCCURS AT MEMBER COORDINATE  
14 13.60 FT.

15

16

17

18 Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19

20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21

22

23 CALCULATED EXTERNAL LOADS

24

25	DISTANCE FROM	TYPE OF	MAGNITUDE OF
26	REFERENCE(FT)	LOAD	LOAD
28	-1.33	POINT LD	16.97 LBF
29	-1.33	COUPLE	-6.10 LBF-FT

30

31

32 INPUTTED LOADS

33

34	DISTANCE FROM	TYPE OF	MAGNITUDE OF
35	REFERENCE(FT)	LOAD	LOAD
37	11.60	CONTN LD	0.00 LBF/SQ FT
38	10.60	CONTN LD	62.50 LBF/SQ FT
39	9.60	CONTN LD	125.00 LBF/SQ FT
40	8.60	CONTN LD	187.50 LBF/SQ FT
41	8.00	CONTN LD	225.00 LBF/SQ FT
42	8.00	CONTN LD	225.00 LBF/SQ FT
43	7.00	CONTN LD	77.70 LBF/SQ FT
44	6.47	CONTN LD	0.00 LBF/SQ FT
45	6.00	CONTN LD	-69.60 LBF/SQ FT
46	5.50	CONTN LD	-143.26 LBF/SQ FT
47	5.50	CONTN LD	-143.26 LBF/SQ FT
48	4.50	CONTN LD	-259.35 LBF/SQ FT
49	3.50	CONTN LD	-375.45 LBF/SQ FT
50	3.50	CONTN LD	-375.45 LBF/SQ FT
51	2.59	CONTN LD	-462.70 LBF/SQ FT
52	0.98	CONTN LD	0.00 LBF/SQ FT
53	-1.33	CONTN LD	662.78 LBF/SQ FT
54	-1.33	CONTN LD	0.00 LBF/SQ FT

55

56

57 Z-22 PROPERTIES ARE AS FOLLOWS.

58

59

60 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL

61 CROSS SECTIONAL AREA= 1.84 SQ IN.

62 ELASTIC MODULUS= 29000000. LBF/SQ IN.

63 DEFLECTION REFERENCE IS AT -1.300

64

65

66 THE MAXIMUM BENDING MOMENT IS 2391.02 LBF-FT AND OCCURS AT 3.57  
 67 WHICH HAS THE SHEAR FORCE OF 13.17 LBF.  
 68  
 69

DEFLECTION

FROM TANG.  
 THRU DEFLE

72	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	REFERENCE
73	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	(INCHES )
74	13.600	0.0	0.0	0.0	0.0927
75	13.599	0.0	0.0	0.0	0.0927
76	13.000	0.0	0.0	0.0	0.0869
77	12.000	0.0	0.0	0.0	0.0773
78	11.000	11.2	6.1	2.2	0.0677
79	10.000	80.0	43.5	42.7	0.0581
80	9.000	211.2	114.8	183.1	0.0485
81	8.000	405.0	220.1	486.0	0.0391
82	7.000	556.3	302.4	978.9	0.0301
83	6.473	576.8	313.5	1279.6	0.0255
84	6.000	560.4	304.6	1549.6	0.0217
85	5.000	421.0	228.8	2051.3	0.0144
86	4.000	161.7	87.9	2352.3	0.0086
87	3.566	13.2	7.2	2391.0	0.0066
88	3.000	-211.3	-114.8	2336.4	0.0044
89	2.000	-615.6	-334.6	1910.6	0.0018
90	1.000	-764.6	-415.5	1196.6	0.0005
91	0.982	-764.6	-415.6	1183.2	0.0005
92	0.980	-764.6	-415.6	1181.7	0.0005
93	0.000	-626.3	-340.4	477.2	0.0001
94	-1.000	-200.7	-109.1	39.8	0.0000
95	-1.299	-17.6	-9.6	6.6	0.0000
96	-1.300	-17.0	-9.2	6.5	0.0000

100 \*RUN COMPLETED\*

101

EOT..

LI DRW22E

1 100 1 13.6 -1.3 1 -1.3 0 -1

2 200 PZ-22

3 300 29000000 1.84 84.38

EOT..

LI Q625J3

1

2

BEAMS (SHEAR, MOMENT, DEFLECTION)

3

4

5 17TH STR -Q625JD-SWL=13.6-Q CASE-FS=1.0

6 =1.

7

8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT

9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS

10 AS COUNTERCLOCKWISE.

11

12

13 THE MAXIMUM DEFLECTION IS 0.05 INCHES AND OCCURS AT MEMBER COORDINATE

14 13.60 FT.

15

16  
 17  
 18 Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.  
 19  
 20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21  
 22  
 23 CALCULATED EXTERNAL LOADS

24  
 25 DISTANCE FROM TYPE OF MAGNITUDE OF  
 26 REFERENCE(FT) LOAD LOAD  
 27  
 28 3.24 POINT LD 0.00 LBF  
 29 3.24 COUPLE -47.16 LBF-FT  
 30  
 31

32 INPUTTED LOADS

33  
 34 DISTANCE FROM TYPE OF MAGNITUDE OF  
 35 REFERENCE(FT) LOAD LOAD  
 36  
 37 13.60 CONTN LD 0.00 LBF/SQ FT  
 38 12.60 CONTN LD 62.50 LBF/SQ FT  
 39 11.60 CONTN LD 125.00 LBF/SQ FT  
 40 10.60 CONTN LD 187.50 LBF/SQ FT  
 41 9.60 CONTN LD 250.00 LBF/SQ FT  
 42 8.60 CONTN LD 312.50 LBF/SQ FT  
 43 8.00 CONTN LD 350.00 LBF/SQ FT  
 44 8.00 CONTN LD 0.00 LBF/SQ FT  
 45 8.00 CONTN LD -650.00 LBF/SQ FT  
 46 7.00 CONTN LD -707.50 LBF/SQ FT  
 47  
 48 6.00 CONTN LD -765.00 LBF/SQ FT  
 49 5.50 CONTN LD -793.75 LBF/SQ FT  
 50 5.50 CONTN LD -793.75 LBF/SQ FT  
 51 5.35 CONTN LD -802.30 LBF/SQ FT  
 52 4.67 CONTN LD 0.00 LBF/SQ FT  
 53 3.24 CONTN LD 1697.36 LBF/SQ FT  
 54 3.24 CONTN LD 0.00 LBF/SQ FT  
 55

56 Z-22 PROPERTIES ARE AS FOLLOWS.

57  
 58  
 59 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL  
 60 CROSS SECTIONAL AREA= 1.84 SQ IN.  
 61 ELASTIC MODULUS= 29000000. LBF/SQ IN.  
 62 DEFLECTION REFERENCE IS AT -1.300  
 63

64  
 65 THE MAXIMUM BENDING MOMENT IS 2538.13 LBF-FT AND OCCURS AT 6.59  
 66 WHICH HAS THE SHEAR FORCE OF 6.95 LBF.  
 67

68 DEFLECTION  
 FROM TANG.

69  
 70 THRU DEFLE  
 71 DISTANCE SHEAR FOR SHEAR STR BENDING MOM REFERENCE  
 72 (FEET) (LBF) (LBF/SQIN) (LBF-FT) (INCHES )

73	13.600	0.0	0.0	0.0	0.0492
74	13.599	0.0	0.0	0.0	0.0492
75	13.000	11.2	6.1	2.2	0.0448
76	12.000	80.0	43.5	42.7	0.0374
77	11.000	211.2	114.8	183.1	0.0300
78	10.000	405.0	220.1	486.0	0.0228
79	9.000	661.2	359.4	1013.9	0.0160
80	8.000	980.0	532.6	1829.3	0.0099
81	7.000	301.2	163.7	2474.7	0.0051
82	6.591	7.0	3.8	2538.1	0.0036
83	6.000	-435.0	-236.4	2412.7	0.0020
84	5.000	-1152.0	-626.1	1594.6	0.0005
85	4.676	-1214.7	-660.2	1207.5	0.0002
86	4.674	-1214.7	-660.2	1205.1	0.0002
87	4.000	-944.8	-513.5	447.3	0.0000
88	3.244	-1.7	-0.9	47.2	0.0000
89	3.242	0.0	0.0	0.0	0.0000
90	3.000	0.0	0.0	0.0	0.0000
91	2.000	0.0	0.0	0.0	0.0000
92	1.000	0.0	0.0	0.0	0.0000
93	0.000	0.0	0.0	0.0	0.0000
94	-1.000	0.0	0.0	0.0	0.0000
95	-1.299	0.0	0.0	0.0	0.0000
96	-1.300	0.0	0.0	0.0	0.0000

97  
98  
99

100 \*RUN COMPLETED\*

101

[ ..

LI DRW22E

1 100 1 13.6 -1.3 1 -1.3 0 -1

2 200 PZ-22

3 300 29000000 1.84 84.38

EOT..

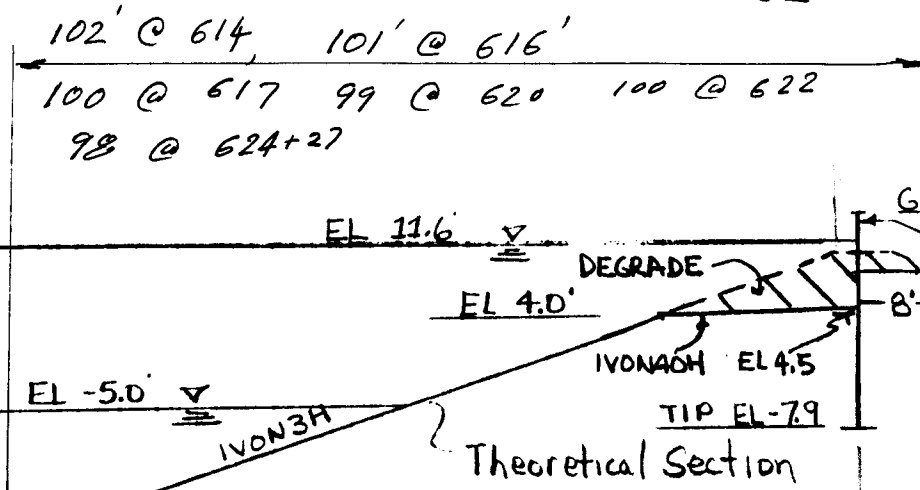


PROJECT 174. St Outfall Canal  
 SUBJECT: STA 614+00 TO STA 625+00

COMPUTED BY *R*  
 CHECKED BY  
 DATE 4/24/87  
 DATE

To be run  
 out put

STA 614+00 TO STA 625+19 (N. Veterans Hwy)  
 JEFFERSON SIDE



Sta	distance B/L to w/L
614	15.80
616	14.80
617	13.80
620	12.80
622	13.80
624+29	11.80

87.20  
 FLOODSIDE TOE SAME AS MODJESKI + MASTERS PLANS

SCALE 1" = 20'  
 Elevation in Feet NGVD

Q File : Q625IJ (F.S. = 1.5)

Enc 3

0625IJ

1 10001 17TH ST CANAL HLP STA 614+00 TO STA 625+19

2 10002 I-WALL TOP EL 14.1 BOTT EL. -7.9 F.S.=1.5

3 10003 3 0.85000000E+01 0.00000000E+01

4 10004 3 0.75000000E+01 0.68636101E+02

5 10005 3 0.65000000E+01 0.13727220E+03

6 10006 3 0.55000000E+01 0.20590830E+03

7 10007 3 0.45000000E+01 0.27454440E+03

8 10008 3 0.45000000E+01 0.27454440E+03

9 10009 3 0.35000000E+01 0.14009174E+03

10 10010 3 0.35000000E+01 0.14009174E+03

11 10011 3 0.25000000E+01 0.25637605E+02

12 10012 3 0.22760011E+01 0.00000000E+01

13 10013 3 0.15000000E+01 -0.88816534E+02

14 10014 3 0.50000000E+00 -0.20327067E+03

15 10015 3 0.00000000E+01 -0.26049774E+03

16 10016 3 0.00000000E+01 -0.26049774E+03

17 10017 3 -0.10000000E+01 -0.30142791E+03

18 10018 3 -0.20000000E+01 -0.34235808E+03

19 10019 3 -0.30000000E+01 -0.39744224E+03

20 10020 3 -0.40000000E+01 -0.45372411E+03

21 10021 3 -0.43896577E+01 -0.47565477E+03

22 10022 3 -0.54602769E+01 0.00000000E+01

23 10023 3 -0.78685239E+01 0.10699361E+04

24 10024 4 -0.78685239E+01 0.00000000E+01

25 10025 0 -0.78685239E+01 0.00000000E+01

26 10026 -0.78685239E+01 0.14901161E-07 0.27220145E+01

EOT..

## BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH ST CANAL HLP STA 614+00 TO STA 625+19

WAL

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT  
 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS  
 AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.37 INCHES AND OCCURS AT MEMBER COORDINATE  
 13.60 FT.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

## CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
-7.87	POINT LD	0.00 LBF
-7.87	COUPLE	-2.72 LBF-FT

## INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
8.50	CONTN LD	0.00 LBF/SQ FT
7.50	CONTN LD	68.64 LBF/SQ FT
6.50	CONTN LD	137.27 LBF/SQ FT
5.50	CONTN LD	205.91 LBF/SQ FT
4.50	CONTN LD	274.54 LBF/SQ FT
4.50	CONTN LD	274.54 LBF/SQ FT
3.50	CONTN LD	140.09 LBF/SQ FT
3.50	CONTN LD	140.09 LBF/SQ FT
2.50	CONTN LD	25.64 LBF/SQ FT
2.28	CONTN LD	0.00 LBF/SQ FT
1.50	CONTN LD	-88.82 LBF/SQ FT
0.50	CONTN LD	-203.27 LBF/SQ FT
0.00	CONTN LD	-260.50 LBF/SQ FT
0.00	CONTN LD	-260.50 LBF/SQ FT
-1.00	CONTN LD	-301.43 LBF/SQ FT
-2.00	CONTN LD	-342.36 LBF/SQ FT
-3.00	CONTN LD	-397.44 LBF/SQ FT
-4.00	CONTN LD	-453.72 LBF/SQ FT
-4.39	CONTN LD	-475.65 LBF/SQ FT
-5.46	CONTN LD	0.00 LBF/SQ FT
-7.87	CONTN LD	1069.94 LBF/SQ FT

59  
60  
61  
62  
63  
64  
65  
66  
67  
68  
69

Z-22 PROPERTIES ARE AS FOLLOWS.

MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL  
 CROSS SECTIONAL AREA= 6.47 SQ IN.  
 ELASTIC MODULUS= 29000000. LBF/SQ IN.  
 DEFLECTION REFERENCE IS AT -7.900

70 THE MAXIMUM BENDING MOMENT IS 4604.30 LBF-FT AND OCCURS AT -1.82  
 71 WHICH HAS THE SHEAR FORCE OF 2.99 LBF.

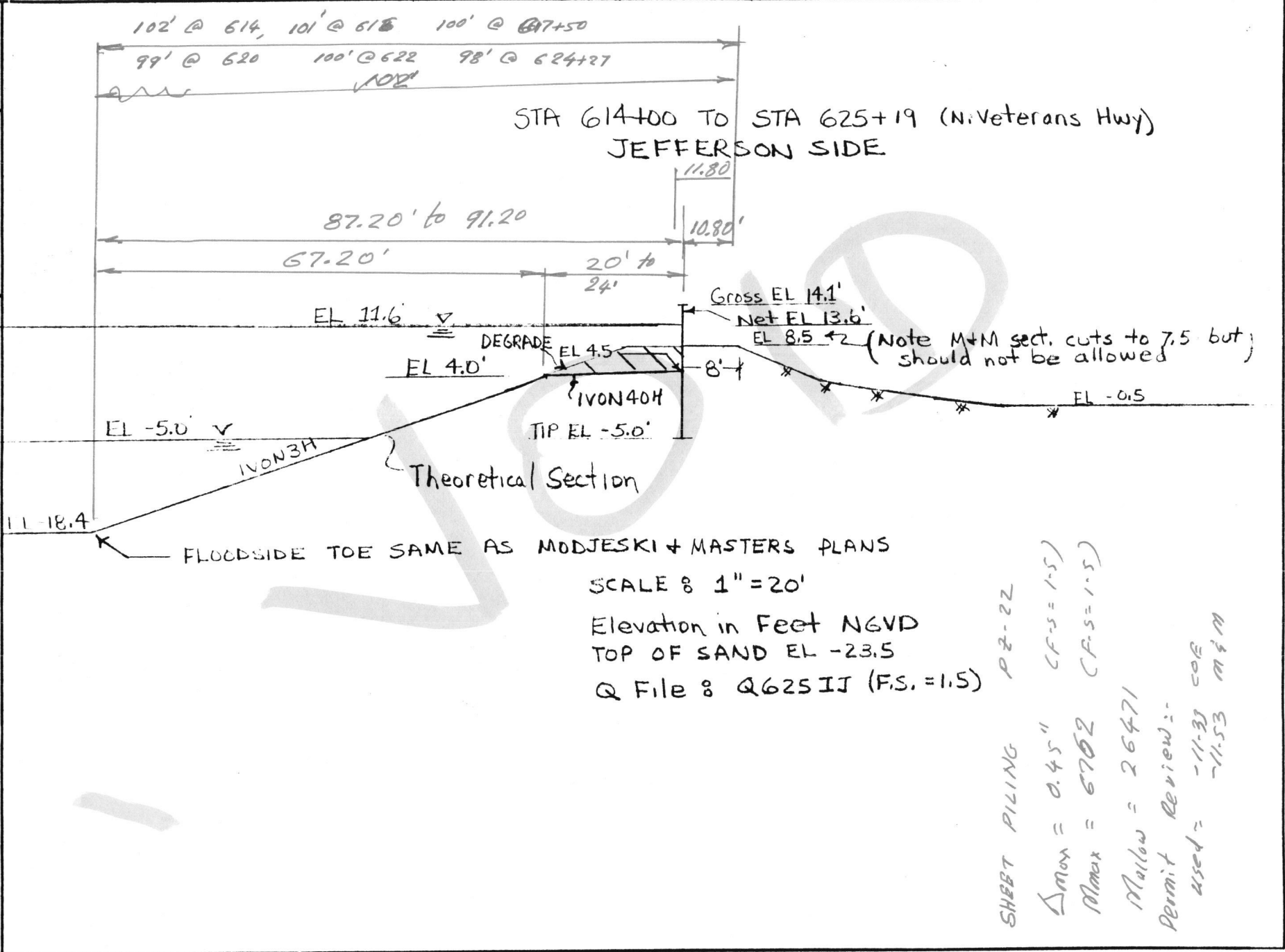
76	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	DEFLECTION
77	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	FROM TANG.
					THRU DEFLE
					REFERENCE
					(INCHES )
78	13.600	0.0	0.0	0.0	0.3692
79	13.599	0.0	0.0	0.0	0.3692
80	13.000	0.0	0.0	0.0	0.3543
81	12.000	0.0	0.0	0.0	0.3294
82	11.000	0.0	0.0	0.0	0.3045
83	10.000	0.0	0.0	0.0	0.2796
84	9.000	0.0	0.0	0.0	0.2547
85	8.000	8.6	1.3	1.4	0.2299
86	7.000	77.2	11.9	38.6	0.2050
87	6.000	214.5	33.2	178.7	0.1801
	5.000	420.4	65.0	490.5	0.1555
89	4.000	669.6	103.5	1038.2	0.1311
90	3.000	812.1	125.5	1789.4	0.1076
91	2.276	842.1	130.2	2391.9	0.0913
92	2.000	837.8	129.5	2623.9	0.0853
93	1.000	749.0	115.8	3426.8	0.0649
94	0.000	545.7	84.4	4083.7	0.0468
95	-1.000	264.7	40.9	4492.3	0.0316
96	-1.822	3.0	0.5	4604.3	0.0215
97	-2.000	-57.2	-8.8	4599.5	0.0196
98	-3.000	-427.1	-66.0	4362.0	0.0108
99	-4.000	-852.6	-131.8	3726.8	0.0050
100	-5.000	-1241.3	-191.9	2656.9	0.0018
101	-5.459	-1288.3	-199.2	2072.4	0.0009
102	-5.461	-1288.3	-199.2	2069.9	0.0009
103	-6.000	-1223.6	-189.1	1387.4	0.0004
104	-7.000	-761.7	-117.7	357.8	0.0000
105	-7.868	-1.1	-0.2	2.7	0.0000
106	-7.870	0.0	0.0	0.0	0.0000
107	-7.899	0.0	0.0	0.0	0.0000
108	-7.900	0.0	0.0	0.0	0.0000

111 \*RUN COMPLETED\*

LIST Q663IJ

1	10003	3	0.14600000E+02	0.00000000E+01
2	10004	3	0.13600000E+02	0.62500000E+02
3	10005	3	0.12600000E+02	0.12500000E+03
4	10006	3	0.12000000E+02	0.16250000E+03
5	10007	3	0.12000000E+02	0.16250000E+03
6	10008	3	0.11100674E+02	0.00000000E+01
7	10009	3	0.11000000E+02	-0.18190847E+02
8	10010	3	0.1	

PROJECT 1744 St. Catharine Canal	COMPUTED BY	DATE
SUBJECT STA 614+00 TO STA 625+00	CHECKED BY	DATE
PAGE OF		



SHEET PILING P2-22

$\Delta_{max} = 0.45"$  (F.S. = 1.5)

$M_{max} = 6702$  (F.S. = 1.5)

$M_{allow} = 26471$

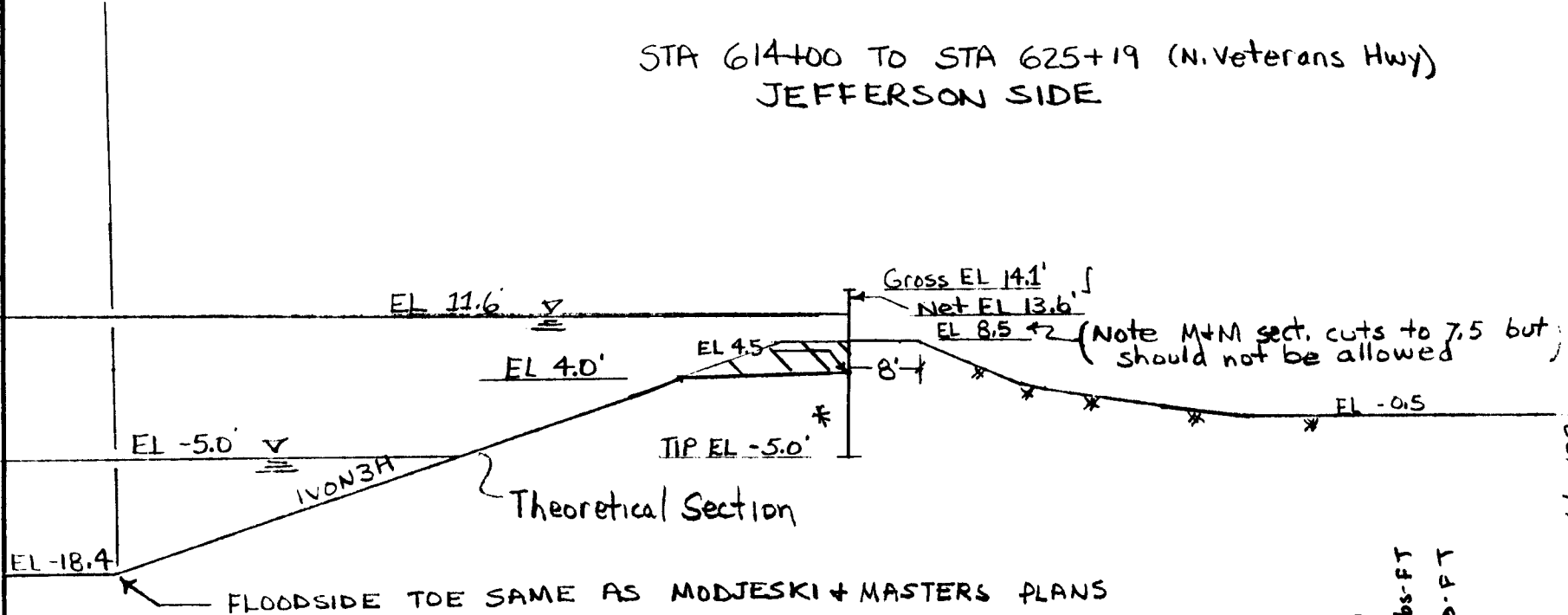
Permit Review:-

used:- -11.33 COE

-11.53 M+M

PROJECT 174, St. Outfall Canal	PAGE OF	COMPUTED BY	DATE
SUBJECT STA 614+00 TO STA 625+00		CHECKED BY	

STA 614+00 TO STA 625+19 (N. Veterans Hwy)  
JEFFERSON SIDE



FLOODSIDE TOE SAME AS MODJESKI + MASTERS PLANS

SCALE 8 1" = 20'

Elevation in Feet NGVD  
TOP OF SAND EL -23.5

Q File : Q625 II (F.S. = 1.5)

SHEET PILING PZ-22

Max. DEFLECTION = 0.45"

Max. Moment (F.S. 1.5) = 6762 lbs.-FT

Max. Allow M = 26471 lbs.-FT

\* PERMIT REVIEW  
USED ~ -11.33 COF  
-11.53 MEM

MWD 4/17/87

JOBCTRL ER 364 : LIST/DISPLAY AREA UNDEFINED.

LIST Q625IJ

1 10001 17TH ST CANAL HLP STA 614+00 STA 625+19  
10002 I-WALL TOP EL. 14.1 BOTT EL. -5.00 F.S.=1.5  
3 10003 3 0.13600000E+02 0.00000000E+01  
4 10004 3 0.12600000E+02 0.62500000E+02  
5 10005 3 0.11600000E+02 0.12500000E+03  
6 10006 3 0.10600000E+02 0.18750000E+03  
7 10007 3 0.96000000E+01 0.25000000E+03  
8 10008 3 0.86000000E+01 0.31250000E+03  
9 10009 3 0.85000000E+01 0.31875000E+03  
10 10010 3 0.85000000E+01 0.31875000E+03  
11 10011 3 0.75000000E+01 0.17144798E+03  
12 10012 3 0.65000000E+01 0.24145954E+02  
13 10013 3 0.63360786E+01 0.00000000E+01  
14 10014 3 0.55000000E+01 -0.12315607E+03  
15 10015 3 0.45000000E+01 -0.27045889E+03  
16 10016 3 0.45000000E+01 -0.27045889E+03  
17 10017 3 0.35000000E+01 -0.38548225E+03  
18 10018 3 0.35000000E+01 -0.38548225E+03  
19 10019 3 0.25000000E+01 -0.48858791E+03  
20 10020 3 0.15000000E+01 -0.58878795E+03  
21 10021 3 0.50000000E+00 -0.53353485E+03  
22 10022 3 0.33194598E+00 -0.53769273E+03  
23 10023 3 -0.16384381E+01 0.00000000E+01  
  
24 10024 3 -0.49257253E+01 0.89785882E+03  
25 10025 4 -0.49257253E+01 0.00000000E+01  
26 10026 0 -0.49257253E+01 0.00000000E+01  
7 10027 -0.49257253E+01 -0.12675476E+02 0.29835532E+02

EOT..

LIST M8D22

1 100 1 14.1 -5.0 1 -5.0 0 -1  
2 200 PZ-22  
3 300 29000000 6.4691 84.3818  
EOT..



## BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH ST CANAL HLP STA 614+00 STA 625+19  
WAL

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT  
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS  
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.45 INCHES AND OCCURS AT MEMBER COORDINATE  
14.10 FT.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

## CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
-4.93	POINT LD	12.68 LBF
-4.93	COUPLE	-29.84 LBF-FT

## INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
13.60	CONTN LD	0.00 LBF/SQ FT
12.60	CONTN LD	62.50 LBF/SQ FT
11.60	CONTN LD	125.00 LBF/SQ FT
10.60	CONTN LD	187.50 LBF/SQ FT
9.60	CONTN LD	250.00 LBF/SQ FT
8.60	CONTN LD	312.50 LBF/SQ FT
8.50	CONTN LD	318.75 LBF/SQ FT
8.50	CONTN LD	318.75 LBF/SQ FT
7.50	CONTN LD	171.45 LBF/SQ FT
6.50	CONTN LD	24.15 LBF/SQ FT
6.34	CONTN LD	0.00 LBF/SQ FT
5.50	CONTN LD	-123.16 LBF/SQ FT
4.50	CONTN LD	-270.46 LBF/SQ FT
4.50	CONTN LD	-270.46 LBF/SQ FT
3.50	CONTN LD	-385.48 LBF/SQ FT
3.50	CONTN LD	-385.48 LBF/SQ FT
2.50	CONTN LD	-480.51 LBF/SQ FT
1.50	CONTN LD	-500.79 LBF/SQ FT
0.50	CONTN LD	-533.53 LBF/SQ FT
0.33	CONTN LD	-537.69 LBF/SQ FT
-1.64	CONTN LD	0.00 LBF/SQ FT

58 -4.93 CONTN 897.06 LBF/SQ FT  
 59 -4.93 CONTN LD 0.00 LBF/SQ FT

60  
61  
62 Z-22

PROPERTIES ARE AS FOLLOWS.

63  
64  
65 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL  
 66 CROSS SECTIONAL AREA= 6.47 SQ IN.  
 67 ELASTIC MODULUS= 29000000. LBF/SQ IN.  
 68 DEFLECTION REFERENCE IS AT -5.000  
 69

70  
71 THE MAXIMUM BENDING MOMENT IS 6762.43 LBF-FT AND OCCURS AT 2.20  
 72 WHICH HAS THE SHEAR FORCE OF 3.96 LBF.  
 73  
 74

75	DEFLECTION				
76	FROM TANG.				
77	THRU DEFLE				
78	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	REFERENCE
79	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	(INCHES )
79	14.100	0.0	0.0	0.0	0.4492
80	14.099	0.0	0.0	0.0	0.4492
81	14.000	0.0	0.0	0.0	0.4453
82	13.000	11.2	1.7	2.2	0.4060
83	12.000	80.0	12.4	42.7	0.3666
84	11.000	211.2	32.7	183.1	0.3273
85	10.000	405.0	62.6	486.0	0.2882
86	9.000	661.2	102.2	1013.9	0.2494
87	8.000	953.8	147.4	1825.0	0.2114
3	7.000	1125.2	173.9	2876.7	0.1746
89	6.336	1157.7	179.0	3638.2	0.1514
90	6.000	1149.4	177.7	4026.3	0.1400
91	5.000	1026.2	158.6	5126.4	0.1081
92	4.000	759.8	117.4	6030.3	0.0799
93	3.000	376.8	58.2	6687.4	0.0559
94	2.202	4.0	0.6	6762.4	0.0400
95	2.000	-95.4	-14.7	6753.2	0.0365
96	1.000	-603.7	-93.3	6405.9	0.0218
97	0.000	-1120.8	-173.3	5539.3	0.0116
98	-1.000	-1431.5	-221.3	4240.4	0.0052
99	-1.637	-1487.1	-229.9	3304.3	0.0028
100	-1.639	-1487.1	-229.9	3301.3	0.0028
101	-2.000	-1469.3	-227.1	2767.3	0.0019
102	-3.000	-1234.2	-190.8	1392.8	0.0004
103	-4.000	-726.2	-112.3	389.9	0.0000
104	-4.925	-13.6	-2.1	29.9	0.0000
105	-4.927	0.0	0.0	0.0	0.0000
106	-4.999	0.0	0.0	0.0	0.0000
107	-5.000	0.0	0.0	0.0	0.0000
108					
109					
110					

111 \*RUN COMPLETED\*

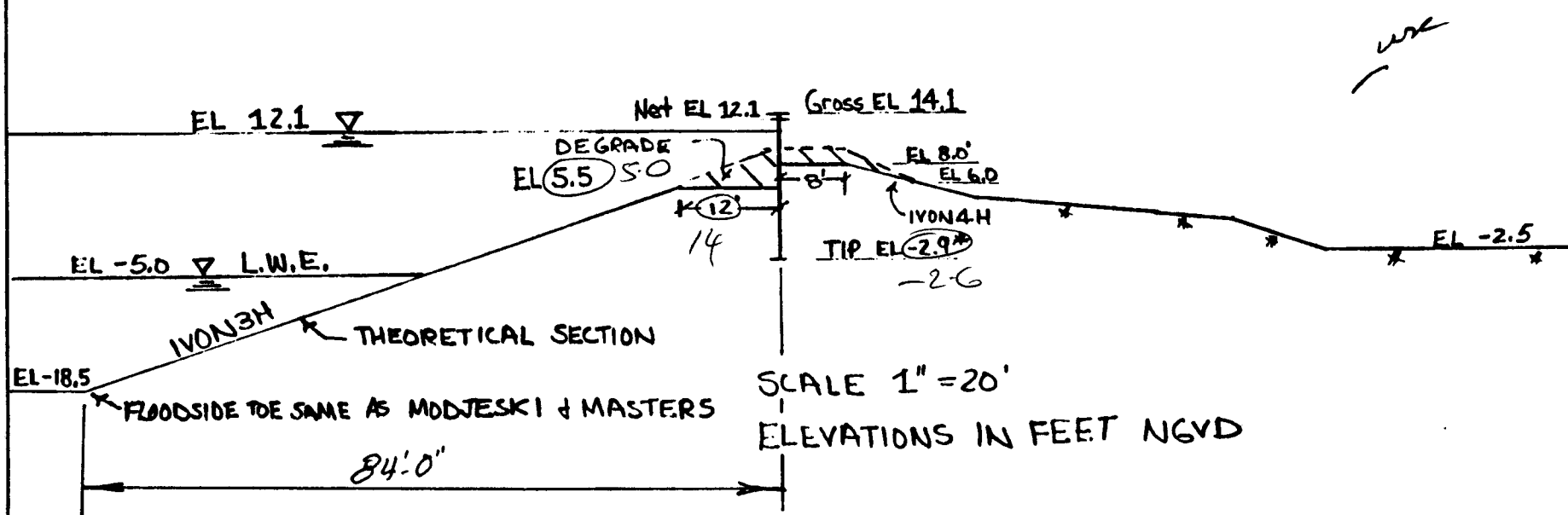
2  
EOT..

LMV Form 107e

PREVIOUS EDITIONS MAY BE USED

FOR USE WITH 10 x 10 GRID

# STA 625+19 TO STA 635+00 JEFFERSON SIDE



Q FILES	FS.	SWL	CASE
Q635JA	1.0	12.1	S
*Q635JB	1.5	12.1	S
Q635JD	1.0	14.1	Q

NOTE: TRANSITION FROM STA 635+00 TO STA 636+00

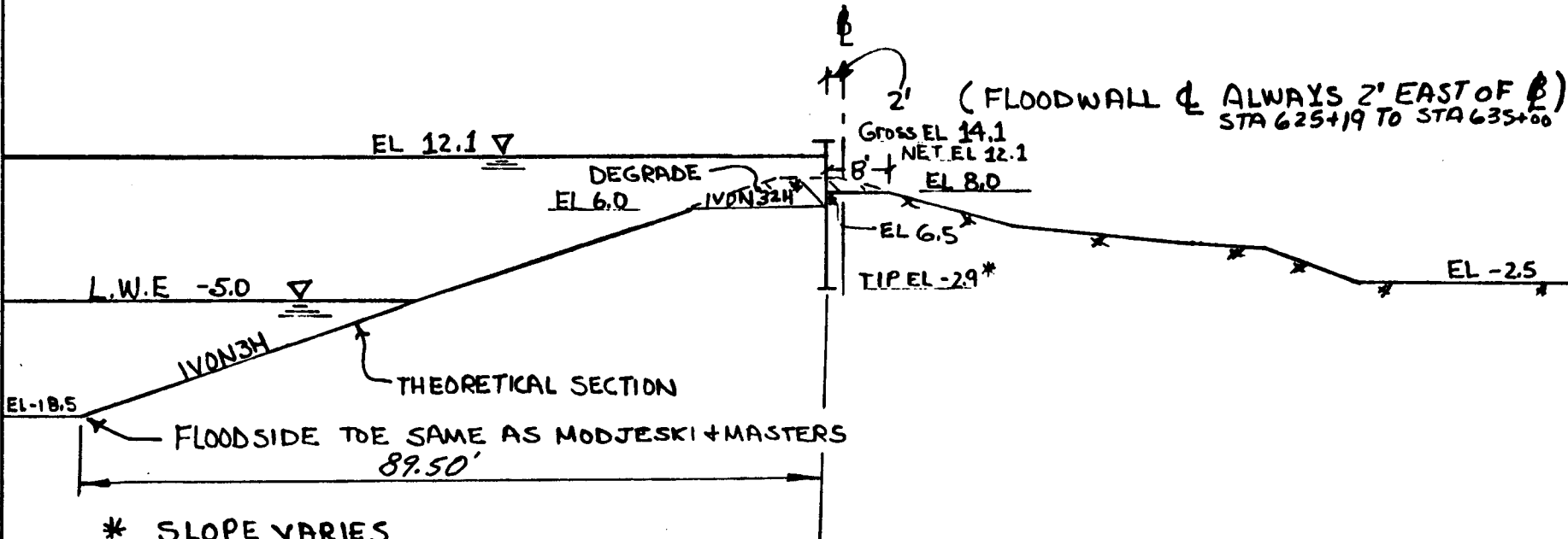
Encl 1/2

REVISED

PROJECT	17th St Offfall Canal	PAGE	OF	COMPUTED BY	FTV	DATE	4/24/87
STATION	STA 625+19 to STA 635+00 Jefferson Side	CHECKED BY	REVISED	DATE	5/3/88		

COMPUTATION SHEET

STA 625+19 to STA 635+00  
JEFFERSON SIDE



\* SLOPE VARIES

STA 627+28	1V:0.25H
STA 628+00	1V:0.26H
STA 630+00	1V:0.26H
STA 632+00	1V:0.26H
STA 634+00	1V:0.32H

Q FILES	F.S.	SWL	CASE
Q635JA	1.0	12.1	S
*Q635JB	1.5	12.1	S
Q635JD	1.0	14.1	Q

NOTE: TRANSITION FROM STA 635+00 TO STA 636+00

ELEVATIONS IN FEET NGVD  
SCALE 1" = 20'

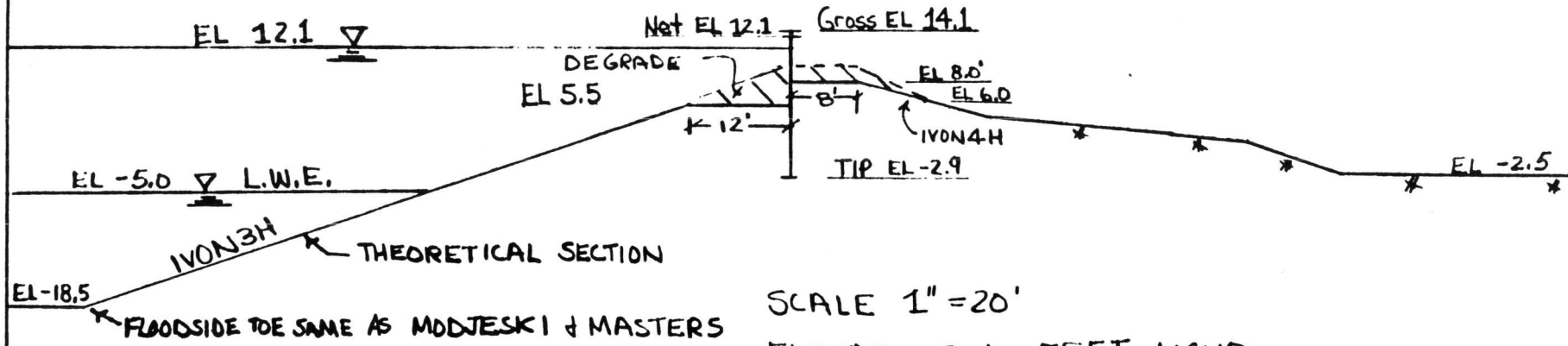
ENC 1/3

ALTERNATIVE

PROJECT: 174th St Offfall Canal	DATE: 4/24/87
SUBJECT: Sta 625+19 to Sta 635+00 Jefferson Side	DATE: 5/3/88
DRAWN BY: [blank]	COMPUTED BY: FJV
CHECKED BY: [blank]	REVISOR: [blank]

PROJECT 174th St Outfall Canal  
 SUBJECT STA 625+19 to STA 635+00 Jefferson Side  
 PAGE OF  
 COMPUTED BY EJV  
 CHECKED BY REVISD  
 DATE 4/24/87  
 DATE 5/3/88

STA 625+19 TO STA 635+00  
**JEFFERSON SIDE**



SCALE 1" = 20'  
 ELEVATIONS IN FEET NGVD

Q FILES	FS.	SWL	CASE
Q635I0			
Q635I1	1.0	12.1	S
2 Q635JB	1.5	12.1	S
3 Q635JD	1.0	14.1	Q

DRW 22F

**ADVANCE CO.**  
 SUBJECT TO CORRECTION

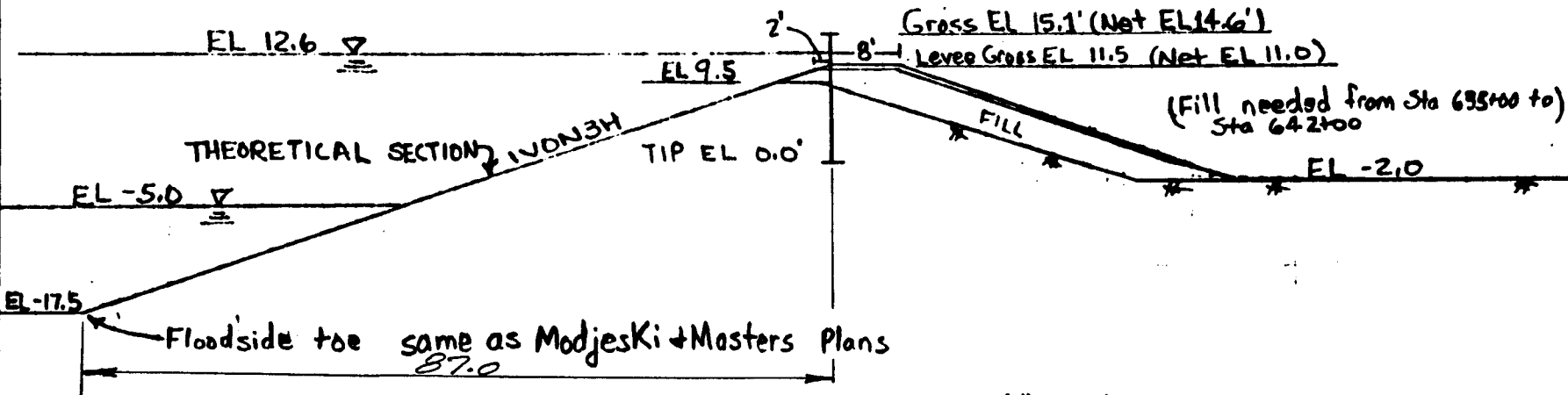
ENV Form 107a  
 OCT 88

PREVIOUS EDITIONS MAY BE USED

FOR USE WITH 10 x 10 GRID

PROJECT	17th St Outfall Canal	PAGE	OF	COMPUTED BY	DATE
SUBJECT	Jefferson side STA 635+00 to STA 643+00			ELV	2/24/87

STA 635+00 TO STA 643+00  
JEFFERSON SIDE



SCALE : 1" = 20'  
Elevations in Feet N.G.V.D

NOTE : SECTION PROVIDED ONLY TO SHOW CHANGE OF STATION LIMITS

FORM 1070

PREVIOUS EDITIONS MAY BE USED

(FOR USE WITH 10 x 10 GRID)

Project of 17th St Outfall Canal  
 Station Jefferson Side Sta 635+00 to Sta 643+00

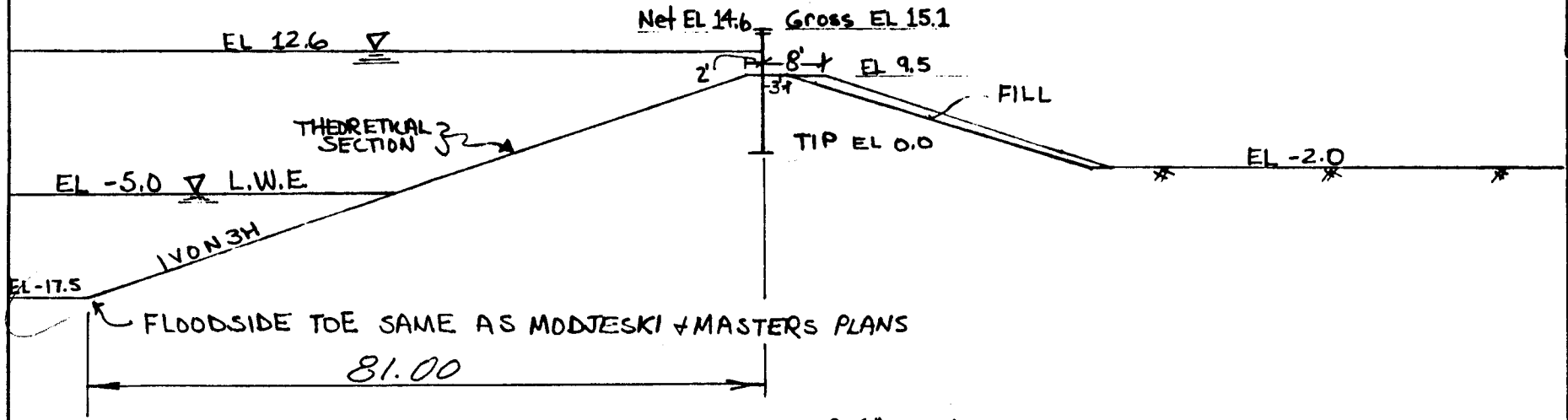
PAGE 01

COMPUTED BY  
 EJP

DATE  
 5/4/88

CHECKED BY  
 REVISED

STA 635+00 TO STA 643+00<sup>5</sup>  
 JEFFERSON SIDE



FLOODSIDE TOE SAME AS MODJESKI + MASTERS PLANS

SCALE : 1" = 20'

ELEVATIONS IN FEET NGVD

Q-FILES SAME AS STA 635+00 TO 643+00 Orleans Side

FOR A Q = 90 ±

ENC 15

ALTERNATIVE

1 MAY FORM 1078

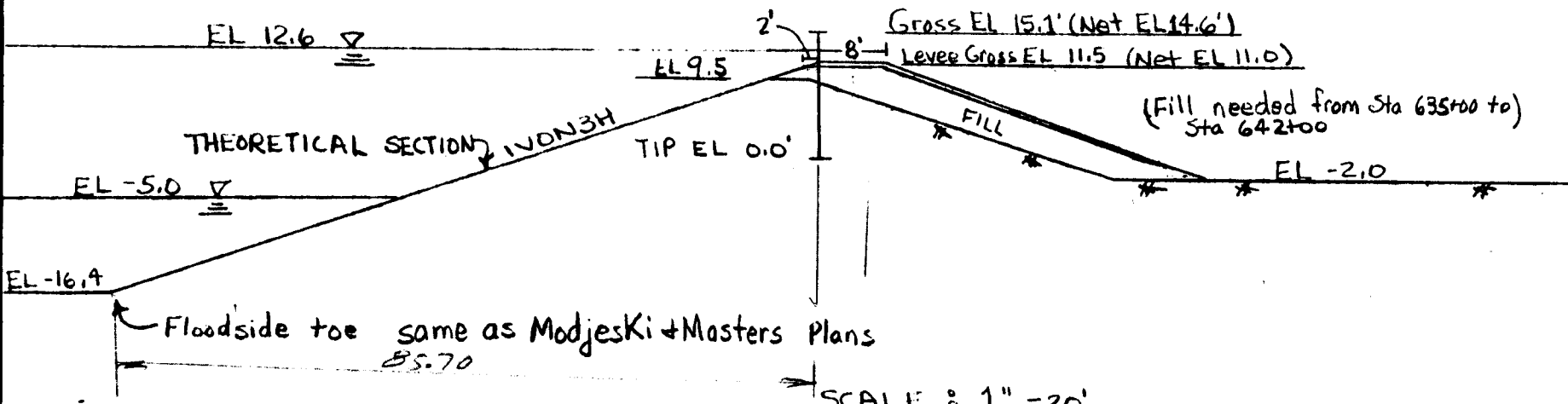
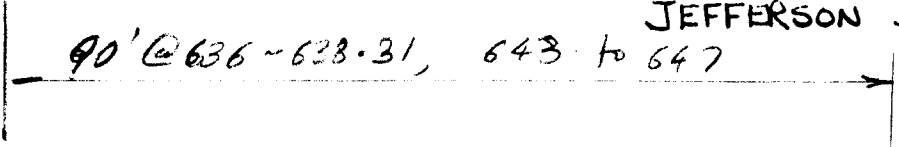
PREVIOUS EDITIONS MAY BE USED

CONSULT WITH CIVIL ENGINEER

PROJECT 17th St Outfall Canal  
 SUBJECT Jefferson side STA 635+00 to STA 647+00  
 PAGE OF  
 COMPUTED BY FJV  
 CHECKED BY  
 DATE 2/24/87  
 DATE

STA 635+00 TO STA 647+00 (M+M)

JEFFERSON SIDE



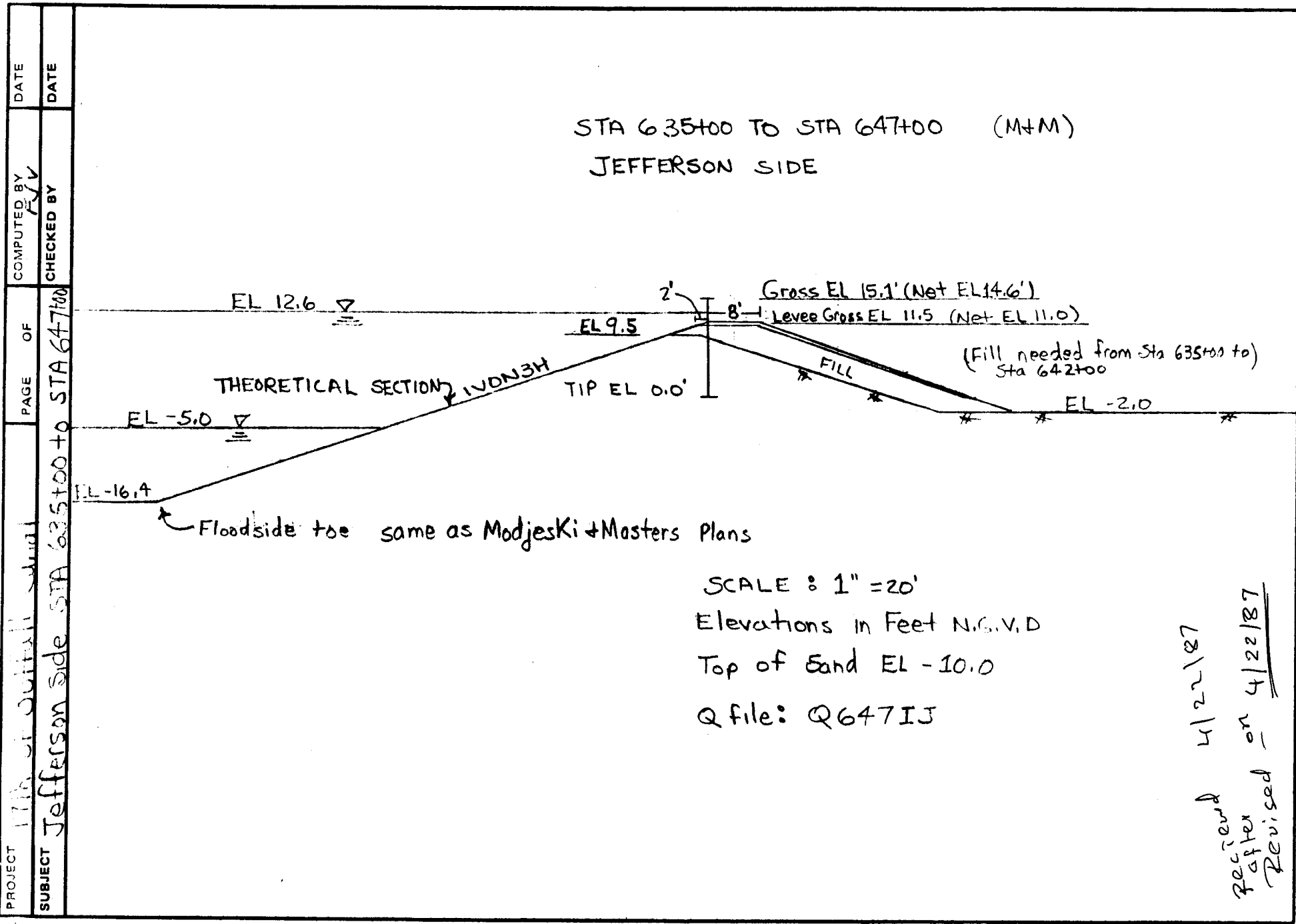
Floodside toe same as Modjeski + Masters Plans

SCALE : 1" = 20'  
 Elevations in Feet N.G.V.D

sta	dist 4/L to 4/L
636	5.30
638+31	5.30
643	5.30
647	5.30

Q file: Q647IJ

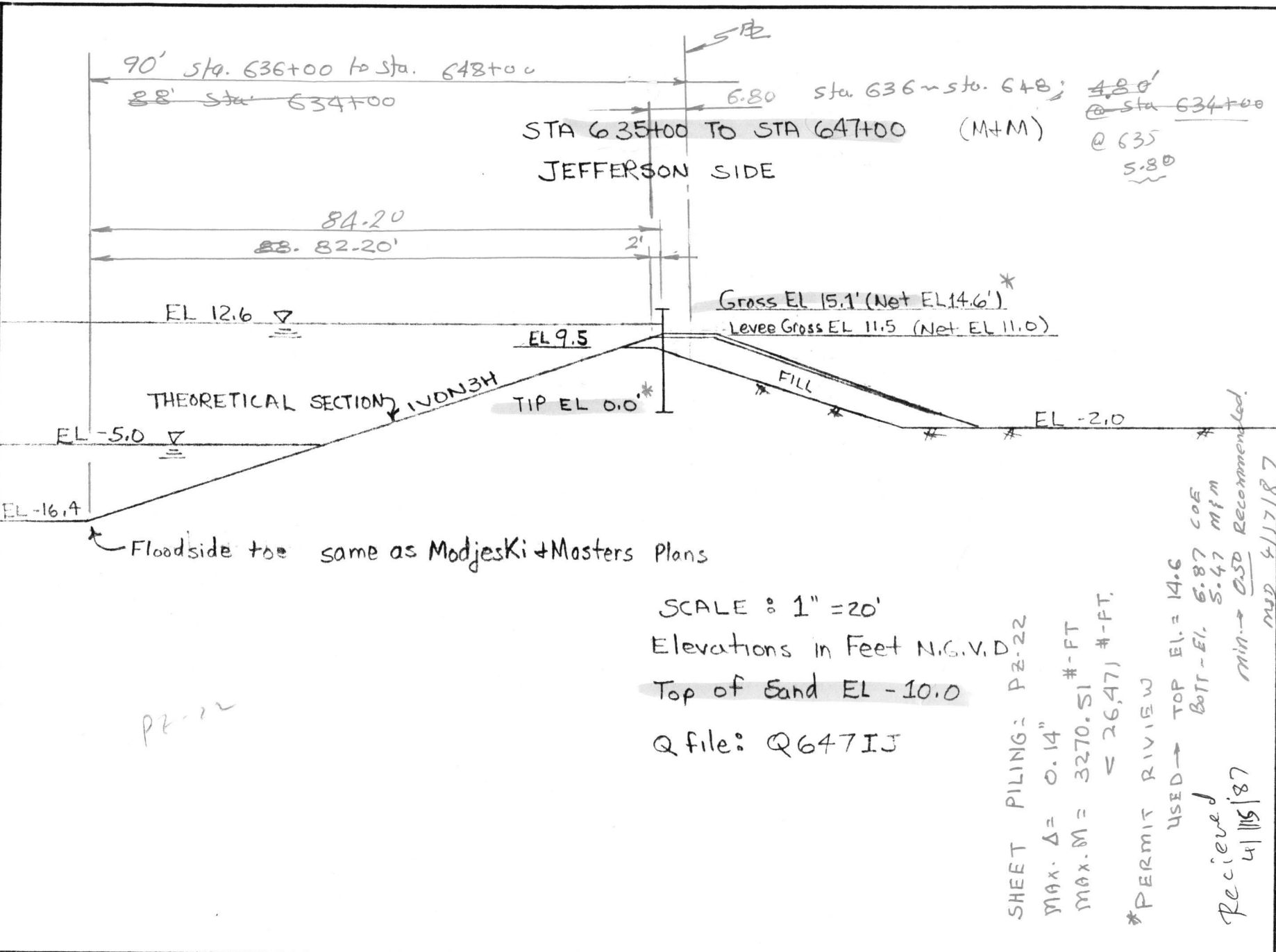




PROJECT	17th St. Outlet	DATE	
SUBJECT	Jefferson side STA 635+00 to STA 647+00	COMPUTED BY	EJV
		CHECKED BY	
		DATE	

Reviewed 4/22/87  
 after Revised on 4/22/87

PROJECT 17th St Outfall Sewer	COMPUTED BY P.V.V.	DATE
SUBJECT Jefferson side STA 635+00 to STA 647+00	CHECKED BY	DATE
PAGE OF STA 647+00		



PZ-22

SCALE : 1" = 20'  
 Elevations in Feet N.G.V.D.  
 Top of Sand EL -10.0  
 Q file: Q647IJ

SHEET PILING: PZ-22  
 MAX. Δ = 0.14"  
 MAX. M = 3270.51 #-FT  
 < 26,471 #-FT.

\*PERMIT REVIEW

USED → TOP EL. = 14.6  
 BOT. EL. 6.87 COE  
 5.47 MFM

Received  
 4/11/87

min → ASD Recommended.  
 MJD 4/17/87

## LIST

1	10001	17TH ST CANAL HLF STA 635+00 TO SAT 647+00		
2	10002	I-WALL TOP EL 15.1 BOTT EL 0.0 F.S.=1.5		
3	10003		3	0.14600000E+02 0.00000000E+01
4	10004		3	0.13600000E+02 0.62500000E+02
	10005		3	0.12600000E+02 0.12500000E+03
6	10006		3	0.11600000E+02 0.18750000E+03
7	10007		3	0.11000000E+02 0.22500000E+03
8	10008		3	0.11000000E+02 0.22500000E+03
9	10009		3	0.10000000E+02 0.12285702E+03
10	10010		3	0.90000000E+01 0.20714046E+02
11	10011		3	0.80007707E+01 0.00000000E+01
12	10012		3	0.80000000E+01 -0.83261027E+02
13	10013		3	0.70000000E+01 -0.19055764E+03
14	10014		3	0.60000000E+01 -0.29783424E+03
15	10015		3	0.55000000E+01 -0.35150255E+03
16	10016		3	0.55000000E+01 -0.35150255E+03
17	10017		3	0.45000000E+01 -0.42883763E+03
18	10018		3	0.35000000E+01 -0.47320009E+03
19	10019		3	0.34833075E+01 -0.47394956E+03
20	10020		3	0.23660300E+01 0.00000000E+01
21	10021		3	0.15394159E+00 0.93836901E+03
22	10022		4	0.15394159E+00 0.00000000E+01
23	10023		0	0.15394159E+00 0.00000000E+01
24	10024			0.15394159E+00 0.22351742E-07 0.94720791E+01

EOT..

LIST MSD22

1 100 1 15.1 0 1 0 0 -1

2 200 PZ-22

3 300 29000000 6.4691 04.3018

EOT..

DRPS

ARE YOU USING A PRINTER TERMINAL OR CRT?

ENTER P OR C

C

CORPS SYSTEM COMMANDS:

BRIEF - LIST EXPLANATION OF A PROGRAM.

EXECUTE - RUN A CORPS PROGRAM

LIST - LIST THE AVAILABLE CORPS PROGRAMS.

STOP - EXIT FROM CORPS SYSTEM MACRO.

HELP - HELP AND EXPLANATION OF CORPS  
SYSTEM AND THE RUNNING OF ITS MACRO.

NOTE: COMMANDS MAY BE ABBREVIATED TO THE  
FIRST LETTER OF THE COMMAND.

ENTER COMMAND(BRIEF,EXECUTE,LIST,HELP,STOP):

E

WHICH CORPS PROGRAM DO YOU WANT TO RUN?

X0015

BEAMS (SHEAR, MOMENT, DEFLECTION)

DO YOU WANT OUTPUT SAVED IN A FILE (YES/NO)?

Y

ENTER EITHER A NEW OR EXISTING OUTPUT FILE NAME UP TO 6 CHARACTERS.

Q47IJO

IS THE LOADING ON THE MEMBER TO BE READ FROM A FILE CREATED BY THE  
"CANTILEVER RETAINING WALL STABILITY" PROGRAM (YES/NO)?

Y

ENTER THE NAME OF THE FILE IN WHICH THE MEMBER LOADING IS STORED.

Q647IJ

DO YOU WANT TO RUN AN EXISTING DATA FILE (YES/NO)?

Y

ENTER THE DATA FILE NAME.

MSD22

STOP 7774

WOULD YOU LIKE TO LIST A FILE?

N

ENTER COMMAND(BRIEF,EXECUTE,LIST,HELP,STOP):

S

STOP

LSIST Q647IJO

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH ST CANAL HLP STA 635+00 TO SAT 647+00

WAL

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.14 INCHES AND OCCURS AT MEMBER COORDINATE 15.10 FT.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE(FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
0.15	POINT LD	0.00 LBF
0.15	COUPLE	-9.47 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE(FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
14.60	CONTN LD	0.00 LBF/SQ FT
13.60	CONTN LD	62.50 LBF/SQ FT
12.60	CONTN LD	125.00 LBF/SQ FT
11.60	CONTN LD	187.50 LBF/SQ FT
11.00	CONTN LD	225.00 LBF/SQ FT
11.00	CONTN LD	225.00 LBF/SQ FT
10.00	CONTN LD	122.86 LBF/SQ FT
9.00	CONTN LD	20.71 LBF/SQ FT
8.00	CONTN LD	0.00 LBF/SQ FT
8.00	CONTN LD	-83.26 LBF/SQ FT
7.00	CONTN LD	-190.56 LBF/SQ FT
6.00	CONTN LD	-297.85 LBF/SQ FT
5.50	CONTN LD	-351.50 LBF/SQ FT
5.50	CONTN LD	-351.50 LBF/SQ FT
4.50	CONTN LD	-428.84 LBF/SQ FT
3.50	CONTN LD	-473.21 LBF/SQ FT
3.48	CONTN LD	-473.95 LBF/SQ FT
2.37	CONTN LD	0.00 LBF/SQ FT
0.15	CONTN LD	938.37 LBF/SQ FT
0.15	CONTN LD	0.00 LBF/SQ FT

58  
59 Z-22

PROPERTIES ARE AS FOLLOWS.

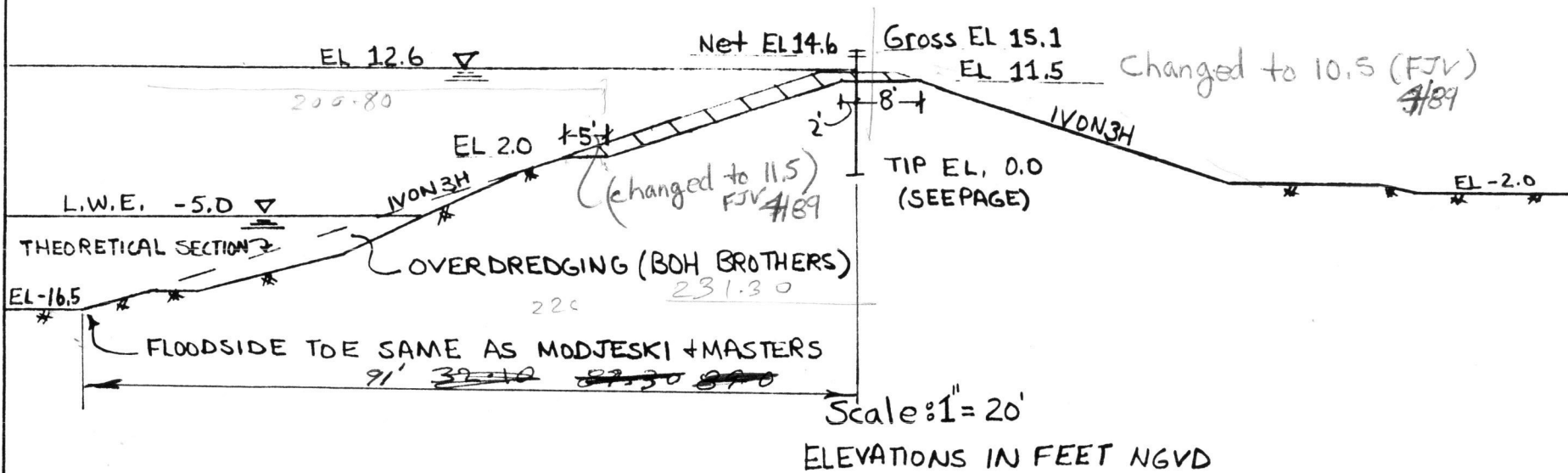
60  
61  
62 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL  
63 CROSS SECTIONAL AREA= 6.47 SQ IN.  
64 ELASTIC MODULUS= 29000000. LBF/SQ IN.  
65 DEFLECTION REFERENCE IS AT 0.000

66  
67  
68 THE MAXIMUM BENDING MOMENT IS 3270.51 LBF-FT AND OCCURS AT 5.31  
69 WHICH HAS THE SHEAR FORCE OF 9.52 LBF.

70					DEFLECTION
71					FROM TANG.
72					THRU DEFLE
73					REFERENCE
74	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	REFERENCE
75	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	(INCHES )
76	15.100	0.0	0.0	0.0	0.1388
77	15.099	0.0	0.0	0.0	0.1388
78	15.000	0.0	0.0	0.0	0.1373
79	14.000	11.2	1.7	2.2	0.1222
80	13.000	80.0	12.4	42.7	0.1070
81	12.000	211.2	32.7	183.1	0.0920
82	11.000	405.0	62.6	486.0	0.0771
83	10.000	578.9	89.5	986.5	0.0625
84	9.000	650.7	100.6	1609.8	0.0487
85	8.001	652.8	100.9	1739.7	0.0461
86	8.000	619.4	95.8	2253.6	0.0360
87	7.000	482.5	74.6	2813.5	0.0249
88	6.000	238.3	36.8	3182.8	0.0157
89	5.315	9.5	1.5	3270.5	0.0107
90	5.000	-109.4	-16.9	3255.0	0.0088
91	4.000	-534.1	-82.6	2938.3	0.0041
92	3.000	-952.6	-147.3	2180.0	0.0015
93	2.367	-1037.9	-160.4	1541.1	0.0006
94	2.365	-1037.9	-160.4	1539.0	0.0006
95	2.000	-1009.5	-156.0	1163.6	0.0003
96	1.000	-642.1	-99.3	302.5	0.0000
97	0.155	-0.9	-0.1	9.5	0.0000
98	0.153	0.0	0.0	0.0	0.0000
99	0.001	0.0	0.0	0.0	0.0000
100	0.000	0.0	0.0	0.0	0.0000

101  
102  
103  
104 \*RUN COMPLETED\*  
105  
EOT..

# STA 663+00 TO STA 670+00 JEFFERSON SIDE



Q FILES SAME AS 663+00 TO 670+00 Orleans side

USE Q6700B 1.5 S-CASE  
(20NOV87 P C)

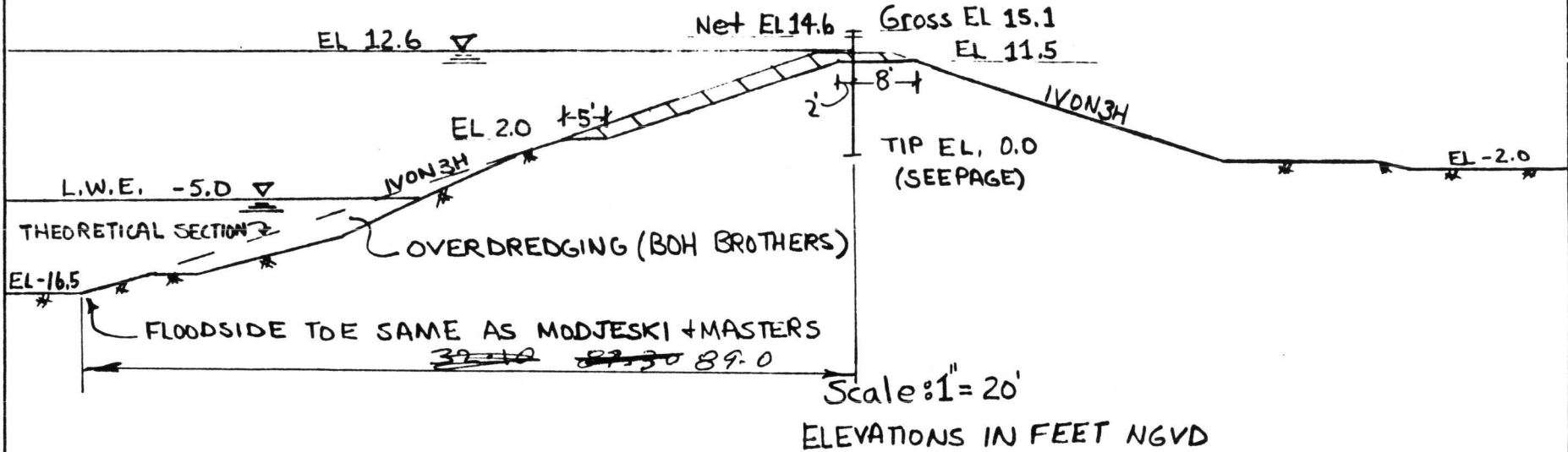
SPOKE WITH FRANK V. TWICE ON 17MAY88 TO VERIFY Q6700B TO  
BE USED FOR BOTH ~~JEFF & ORLEANS PARISH.~~

17th St Outfall Canal	PAGE	OR	COMPUTED BY	DATE
Jefferson Side Sta 663+00 TO STA 670+00			FJV	5/4/88
	CHECKED BY			

ENCL 18

STA 663+00 TO STA 670+00  
JEFFERSON SIDE

**SEE  
ORLEANS SIDE**



Q FILES SAME AS 663+00 TO 670+00 Orleans Side

USE Q6700B 1.5 S-CASE  
(20 NOV 87 P.C.)

SPOKE WITH FRANK V. TWICE ON 17 MAY 88 TO VERIFY Q6700B TO BE USED FOR BOTH JEFF & ORLEANS PARISH.

ENC 1B

PROJECT	17th St Outfall Canal	PAGE	OF	COMPUTED BY	DATE
STATION	Jefferson Side Sta 663+00 TO STA 670+00			ESTV	5/4/88
		CHECKED BY			

07e

PREVIOUS EDITIONS MAY BE USED

FOR USE WITH 10 x 10 GPIC

COMPUTATION SHEET





ORLEANS PARISH  
17<sup>TH</sup> STREET CANAL

STATIONS ORLEANS SIDE ONLY	SHEET PILE TYPE/TIP EL./LENGTH		FLOOD SIDE TOP/LEVEE		FLOOD SIDE SET BACK		PROTECTED SIDE TOP/LEVEE	
	CORPS/ENG	M/M	C/E	M/M	C/E	M/M	C/E	M/M
545+80 TO 552+70 8405.87	P222/0.0/14.0		6.0		7'		8.5	
553+70 TO 568+00 +045540A,B,D,F 23+35.87	P222/-13.2/27.3		2.1		8.5'		5.4	
X 568+00 TO 589+00 23+35.87 w/L 44+49.9 w/L	P222/-12.8/26.9		2.5		11'		5.5	
X 589+00 TO 614+00 44+49.9 w/L 69+29.75	P222/-7.46/21.56		3.5		15.5		6.5	
✓ 614+00 TO 624+27 69+29.75 to 79+61.79	P222/-5.3/19.4		4.5		16.5		7.0	
✓ 625+00 TO 634+00 30+34.79 w/L 81+34.79 w/L	P222/-4.6/19.2		5.0		13.0		7.5	
<del>634+00 TO 635+00</del>								
<del>635+00 TO 636+00</del>								
<del>636+00 TO 638+31</del>								
✓ 638+31 TO 643+00 81+34.79 98+31.72	P222/0.0/15.1		9.5		2'		9.5	
<del>643+00 TO 647+00</del>								
✓ 647+00 TO 663+00 98+31.72 w/L 118+14.89 w/L	P222/0.0/15.1		12.0		10.5		12.0	
✓ 663+00 TO 669+81 118+14.89 w/L	P222/0.0/15.1		11.5		2'		11.5	

✓ FRODINGHAM IS ACCEPTABLE  
X FROD. NOT ACCEPTABLE

PROJECT	PAGE	COMPUTED BY	DATE
SUBJECT	OF	CHECKED BY	DATE

COMPUTATION SHEET

LMV FORM 1070

PREVIOUS EDITIONS MAY BE USED

(FOR USE WITH 8 X 8 GRID)