

(A0006669)

PZ-22, SECTION MODULOUS/RT
18.1 in²/ft.
SHEAR AREA = 1.84 in²/ft.

ORLEANS SIDE - T-WALL ANALYSIS

STATION To STATION LIMITS	Sheet Pile TYPE	FACTOR OF SAFETY	STICK UP	MAX. Bending Stress fb (K.S.I)	MAX-SHEAR Stress fv (K.S.I)	Deflection (inches)	SWL (NGVD (FT))	LOAD CASE
PLATE #100 <u>met 12A</u> Sta. 545+80 To Sta. 552+70 B/L	ARBED BZ 12.1L	1.5	5 5.5	0.2	0.2	0.00	-5.0 8.5	Q
PLATE #101, <u>Q67</u> STA. 554+00 To STA. 568+00 B/L	PZ-22	1	8.6	6.2	1.1	0.59	13.6	Q
PLATE #102 <u>Q67-A</u> STA. 568+00 To STA. 589+00 B/L	PZ 22	1	8.6	6.2	1.1	0.59	13.6	Q
PLATE #103 <u>Q68</u> STA. 589+00 To STA. 614+00 B/L	PZ-22	1	8.6	6.2	1.1	0.59	13.6	Q
PLATE #104 <u>Q69</u> STA. 614+00 To STA. 625+00 B/L	PZ-22	1	7.6	3.8	0.9	0.23	13.6	Q
PLATE #105 <u>Q70</u> STA. 625+00 To STA. 635+00 B/L	PZ-22	1	7.1	3.0	0.9	0.14	14.1	Q
PLATE #106 <u>Q71</u> STA. 635+00 To STA. 642+00 B/L	PZ-22	1	5.1	0.8	0.5	0.01	14.1	Q
PLATE #107 <u>Q72</u> STA. 642+00 To STA. 663+00 B/L	PZ-22	1	3.1	0.1	0.2	0	14.6	Q
PLATE #108 <u>Q73</u> STA. 663+00 To STA. 670+63 B/L	PZ-22	1	4.1	0.4	0.3	0	14.6	Q

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH ST CANAL G.D.M., Q-FILE : MET17A, Q-CASE, FS=1.5
A 5

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.00 INCHES AND OCCURS AT MEMBER COORDINATE
14.00 FT.

RBED BZ 12 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
3.60	POINT LD	0.00 LBF
3.60	COUPLE	-9.85 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
8.50	CONTN LD	0.00 LBF/SQ FT
7.50	CONTN LD	62.50 LBF/SQ FT
6.50	CONTN LD	125.00 LBF/SQ FT
6.00	CONTN LD	156.25 LBF/SQ FT
6.00	CONTN LD	156.25 LBF/SQ FT
5.50	CONTN LD	55.20 LBF/SQ FT
5.50	CONTN LD	0.00 LBF/SQ FT
5.50	CONTN LD	-545.17 LBF/SQ FT
4.63	CONTN LD	-586.35 LBF/SQ FT
4.26	CONTN LD	0.00 LBF/SQ FT
3.60	CONTN LD	1055.65 LBF/SQ FT
3.60	CONTN LD	0.00 LBF/SQ FT

ARBED BZ 12.1L
 $S_{xx}/ft = 22.5 in^3/ft$

$$f_b = \frac{331.50 (12)}{(1000)(22.5)}$$

$$= 0.2 ksi$$

$$f_v = \frac{.351 kips/ft}{3.86 in^2 (12/22.69')} = 0.2 ksi$$

RBED BZ 12 PROPERTIES ARE AS FOLLOWS.

MOMENT OF INERTIA= 115.60 IN. TO THE 4TH PER FOOT OF WALL
CROSS SECTIONAL AREA= 6.73 SQ IN.
ELASTIC MODULUS= 29000000. LBF/SQ IN.
DEFLECTION REFERENCE IS AT 3.600

THE MAXIMUM BENDING MOMENT IS 331.50 LBF-FT AND OCCURS AT 5.05
WHICH HAS THE SHEAR FORCE OF 0.58 LBF.

62					DEFLECTION
63					FROM TANG.
64					THRU DEFLE
65					REFERENCE
66	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	
67	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	(INCHES
68	14.000	0.0	0.0	0.0	0.0027
69	13.999	0.0	0.0	0.0	0.0027
70	13.000	0.0	0.0	0.0	0.0024
71	12.000	0.0	0.0	0.0	0.0021
72	11.000	0.0	0.0	0.0	0.0018
73	10.000	0.0	0.0	0.0	0.0015
74	9.000	0.0	0.0	0.0	0.0012
75	8.000	7.8	1.2	1.3	0.0008
76	7.000	70.3	10.4	35.2	0.0005
77	6.000	195.3	29.0	162.8	0.0002
78	5.500	248.2	36.9	275.7	0.0001
79	5.054	0.6	0.1	331.5	0.0001
80	5.000	-30.3	-4.5	330.7	0.0001
81	4.265	-350.4	-52.1	165.3	0.0000
82	4.263	-350.4	-52.1	164.6	0.0000
83	4.000	-294.9	-43.8	77.2	0.0000
84	3.601	-0.6	-0.1	9.9	0.0000
85	3.600	0.0	0.0	0.0	0.0000
86					
87					
88					
89	*RUN COMPLETED*				
90					

EOT..

LIST MET17A

1	10001	17TH ST CANAL G.D.M., Q-FILE : MET17A, Q-CASE, FS=1.5
2	0002	STA 545+80 - 552+70 B/L, ORLEANS SIDE
3	10003	3 0.85000000E+01 0.00000000E+01
4	10004	3 0.75000000E+01 0.62500000E+02
5	10005	3 0.65000000E+01 0.12500000E+03
6	10006	3 0.60000000E+01 0.15625000E+03
7	10007	3 0.60000000E+01 0.15625000E+03
8	10008	3 0.55000000E+01 0.55203567E+02
9	10009	3 0.55000000E+01 0.00000000E+01
10	10010	3 0.55000000E+01 -0.54516667E+03
11	10011	3 0.46330562E+01 -0.58634650E+03
12	10012	3 0.42643142E+01 0.00000000E+01
13	10013	3 0.36004389E+01 0.10556450E+04
14	10014	4 0.36004389E+01 0.00000000E+01
15	10015	0 0.36004389E+01 0.00000000E+01
16	10016	0.36004389E+01 0.63329935E-07 0.98546018E+01

EOT..

LIST ARBED

1	100	1	14.0	3.6	1	3.6	0	-1
2	200	ARBED	BZ	12.1L				
3	300	29000000	6.73	115.6				

EOT..

DISPLAY OUTPUT

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH ST. CANAL G.D.M., Q-FILE: MET17B, Q-CASE, FS=1.0
A 5

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.00 INCHES AND OCCURS AT MEMBER COORDINATE
14.00 FT.

RBED BZ 12 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE(FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
4.05	POINT LD	0.00 LBF
4.05	COUPLE	-4.31 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE(FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
8.50	CONTN LD	0.00 LBF/SQ FT
7.50	CONTN LD	62.50 LBF/SQ FT
6.50	CONTN LD	125.00 LBF/SQ FT
6.00	CONTN LD	156.25 LBF/SQ FT
6.00	CONTN LD	156.25 LBF/SQ FT
5.51	CONTN LD	0.00 LBF/SQ FT
5.50	CONTN LD	-3.56 LBF/SQ FT
5.50	CONTN LD	-878.50 LBF/SQ FT
4.99	CONTN LD	-902.68 LBF/SQ FT
4.62	CONTN LD	0.00 LBF/SQ FT
4.05	CONTN LD	1367.83 LBF/SQ FT
4.05	CONTN LD	0.00 LBF/SQ FT

FS=1.15 case Governs

RBED BZ 12 PROPERTIES ARE AS FOLLOWS.

MOMENT OF INERTIA= 115.60 IN. TO THE 4TH PER FOOT OF WALL
L JSS SECTIONAL AREA= 6.73 SQ IN.
ELASTIC MODULUS= 29000000. LBF/SQ IN.
DEFLECTION REFERENCE IS AT 3.600

THE MAXIMUM BENDING MOMENT IS 304.15 LBF-FT AND OCCURS AT 5.24
WHICH HAS THE SHEAR FORCE OF 6.19 LBF.

DISTANCE (FEET)	SHEAR FOR (LBF)	SHEAR STR (LBF/SQIN)	BENDING MOM (LBF-FT)	DEFLECTION FROM TANG. THRU DEFLE REFERENCE (INCHES)
14.000	0.0	0.0	0.0	0.0022
3.999	0.0	0.0	0.0	0.0022
13.000	0.0	0.0	0.0	0.0019
12.000	0.0	0.0	0.0	0.0017
11.000	0.0	0.0	0.0	0.0014
10.000	0.0	0.0	0.0	0.0012
9.000	0.0	0.0	0.0	0.0009
8.000	7.8	1.2	1.3	0.0006
7.000	70.3	10.4	35.2	0.0004
6.000	195.3	29.0	162.8	0.0002
5.511	233.5	34.7	270.7	0.0001
5.243	6.2	0.9	304.1	0.0000
5.000	-211.7	-31.5	279.2	0.0000
4.616	-389.5	-57.9	152.6	0.0000
4.614	-389.5	-57.9	151.8	0.0000
4.047	-1.4	-0.2	4.3	0.0000
4.045	0.0	0.0	0.0	0.0000
4.000	0.0	0.0	0.0	0.0000
3.601	0.0	0.0	0.0	0.0000
3.600	0.0	0.0	0.0	0.0000

RUN COMPLETED

DISPLAY MET17B

1 01 17TH ST. CANAL G.D.M., Q-FILE: MET17B, Q-CASE, FS=1.0
 10002 STA 545+00 - 552+70 B/L, ORLEANS SIDE

10003	3	0.85000000E+01	0.00000000E+01
10004	3	0.75000000E+01	0.62500000E+02
10005	3	0.65000000E+01	0.12500000E+03
10006	3	0.60000000E+01	0.15625000E+03
10007	3	0.60000000E+01	0.15625000E+03
10008	3	0.55111447E+01	0.00000000E+01
10009	3	0.55000000E+01	-0.35621190E+01
10010	3	0.55000000E+01	-0.87850000E+03
10011	3	0.49909775E+01	-0.90267857E+03
10012	3	0.46151706E+01	0.00000000E+01
10013	3	0.40457106E+01	0.13678287E+04
10014	4	0.40457106E+01	0.00000000E+01
10015	0	0.40457106E+01	0.00000000E+01
10016	0.40457106E+01	0.22351742E-07	0.43121623E+01

EOT..

DISPLAY ARBED

100 1 14.0 3.6 1 3.6 0 -1
 200 ARBED BZ 12.1L
 300 29000000 6.73 115.6
 EOT..

LIST Q67

1	10001	17TH ST.CANAL G.D.M. , Q-FILE :Q-67 ,Q-CASE , F.S.=1.0		
2	10002	STA.553+70 TO STA. 568+00 B/L ,ORLEANS SIDE		
3	10003		3	0.13600000E+02 0.00000000E+01
4	10004		3	0.12600000E+02 0.62500000E+02
5	10005		3	0.11600000E+02 0.12500000E+03
6	10006		3	0.10600000E+02 0.18750000E+03
7	10007		3	0.96000000E+01 0.25000000E+03
8	10008		3	0.86000000E+01 0.31250000E+03
9	10009		3	0.76000000E+01 0.37500000E+03
10	10010		3	0.66000000E+01 0.43750000E+03
11	10011		3	0.56000000E+01 0.50000000E+03
12	10012		3	0.55000000E+01 0.50625000E+03
13	10013		3	0.55000000E+01 0.00000000E+01
14	10014		3	0.55000000E+01 -0.49375000E+03
15	10015		3	0.45000000E+01 -0.55125000E+03
16	10016		3	0.35000000E+01 -0.60875000E+03
17	10017		3	0.35000000E+01 -0.60875000E+03
18	10018		3	0.25000000E+01 -0.64925000E+03
19	10019		3	0.15000000E+01 -0.68975000E+03
20	10020		3	0.15000000E+01 -0.68975000E+03
21	10021		3	0.50000000E+00 -0.73025000E+03
22	10022		3	0.18499187E+00 -0.74300783E+03
23	10023		3	-0.18262978E+01 0.00000000E+01
24	10024		3	-0.51495525E+01 0.12276721E+04
25	10025		4	-0.51495525E+01 0.00000000E+01
26	10026		0	-0.51495525E+01 0.00000000E+01
27	10027	-0.51495525E+01	0	0.44703484E-07 0.21142396E+01

EOT..

List PZ22

1	100	1	13.68	-5.1496	1	-5.1496	0	-1
2	200		PZ-22					
3	300		29000,000	6.4691		84.3818		

LIST Q670

1
2 BEAMS (SHEAR, MOMENT, DEFLECTION)

3
4
5 17TH ST.CANAL G.D.M. , Q-FILE :Q-67 ,Q-CASE , F.S.=1.0
6 A.5

7
8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
10 AS COUNTERCLOCKWISE.

11
12
13 THE MAXIMUM DEFLECTION IS 0.59 INCHES AND OCCURS AT MEMBER COORDINATE
14 13.60 FT. O.K.

15
16
17
18 Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19
20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21
22
23 CALCULATED EXTERNAL LOADS

24
25 DISTANCE FROM TYPE OF MAGNITUDE OF
26 REFERENCE(FT) LOAD LOAD
27
28 -5.15 POINT LD 0.00 LBF
29 -5.15 COUPLE -2.12 LBF-FT

30
31
32 INPUTTED LOADS

33
34 DISTANCE FROM TYPE OF MAGNITUDE OF
35 REFERENCE(FT) LOAD LOAD
36
37 13.60 CONTN LD 0.00 LBF/SQ FT
38 12.60 CONTN LD 62.50 LBF/SQ FT
39 11.60 CONTN LD 125.00 LBF/SQ FT
40 10.60 CONTN LD 187.50 LBF/SQ FT
41 9.60 CONTN LD 250.00 LBF/SQ FT
42 8.60 CONTN LD 312.50 LBF/SQ FT
43 7.60 CONTN LD 375.00 LBF/SQ FT
44 6.60 CONTN LD 437.50 LBF/SQ FT
45 5.60 CONTN LD 500.00 LBF/SQ FT
46 5.50 CONTN LD 506.25 LBF/SQ FT

47 5.50 CONTN LD 0.00 LBF/SQ FT
48 5.50 CONTN LD -493.75 LBF/SQ FT
49 4.50 CONTN LD -551.25 LBF/SQ FT
50 3.50 CONTN LD -608.75 LBF/SQ FT
51 3.50 CONTN LD -608.75 LBF/SQ FT
52 2.50 CONTN LD -649.25 LBF/SQ FT

53	1.50	CONTN LD	-689.75	LBF/SQ FT
54	1.50	CONTN LD	-689.75	LBF/SQ FT
55	0.50	CONTN LD	-730.25	LBF/SQ FT
56	0.18	CONTN LD	-743.01	LBF/SQ FT
57	-1.83	CONTN LD	0.00	LBF/SQ FT
58	-5.15	CONTN LD	1227.67	LBF/SQ FT
59	-5.15	CONTN LD	0.00	LBF/SQ FT

60
61
62 Z-22 PROPERTIES ARE AS FOLLOWS.

63
64
65 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
66 CROSS SECTIONAL AREA= 6.47 SQ IN.
67 ELASTIC MODULUS= 29000000. LBF/SQ IN.
68 DEFLECTION REFERENCE IS AT -5.150
69

70
71 THE MAXIMUM BENDING MOMENT IS 9286.48 LBF-FT AND OCCURS AT 2.02
72 WHICH HAS THE SHEAR FORCE OF 0.33 LBF.

77	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	DEFLECTION
78	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	FROM TANG.
79					THRU DEFLE
80					REFERENCE
81					(INCHES)
79	-13.600	0.0	0.0	0.0	0.5874
80	13.599	0.0	0.0	0.0	0.5874
81	-13.000	11.2	1.7	2.2	0.5561
82	-12.000	80.0	12.4	-42.7	0.5039
83	-11.000	211.2	32.7	-183.1	0.4516
84	-10.000	405.0	62.6	-486.0	0.3996
85	-9.000	661.2	102.2	-1013.9	0.3479
86	-8.000	980.0	151.5	-1829.3	0.2970
87	-7.000	1361.2	210.4	-2994.7	0.2474
88	-6.000	1805.0	279.0	-4572.7	0.1999
89	-5.500	2050.3	316.9	-5535.8	0.1773
90	-5.000	1796.2	277.7	-6498.1	0.1557
91	-4.000	1245.0	192.5	-8023.5	0.1161
92	-3.000	638.4	98.7	-8969.3	0.0821
93	2.017	0.3	0.1	9286.5	0.0547
94	2.000	-10.9	-1.7	9286.4	0.0543
95	1.000	-700.6	-108.3	8934.0	0.0330
96	0.000	-1423.9	-220.1	7872.1	0.0179
97	-1.000	-1913.8	-295.8	6172.5	0.0084
98	-1.825	-2039.9	-315.3	4523.6	0.0038
99	-1.827	-2039.9	-315.3	4519.6	0.0038
100	-2.000	-2034.4	-314.5	4167.6	0.0031
101	-3.000	-1785.5	-276.0	2226.9	0.0008
102	-4.000	-1167.2	-180.4	719.8	0.0001
103	-5.000	-179.5	-27.7	15.6	0.0000
104	-5.149	-1.2	-0.2	2.1	0.0000
105	-5.150	0.0	0.0	0.0	0.0000

107 $I^0 = 1000^*$

$I'' = 2500^1/165$

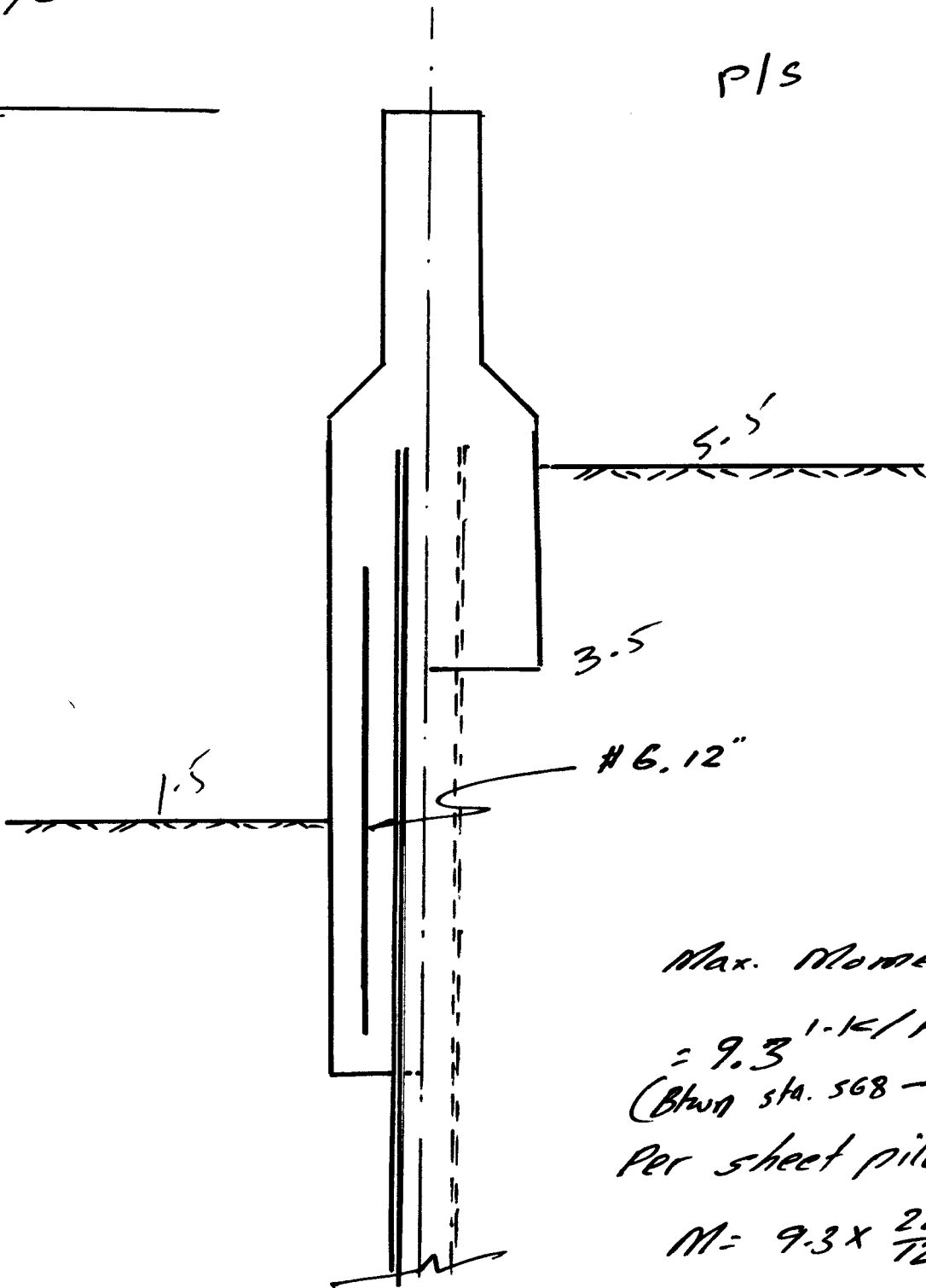
108
109 *RUN COMPLETED*
110
EOT..

$$f_s = \frac{9286.48 \times 12}{(8.1)} = 6.157 \text{ K.S.I. } < 18 \text{ O.K.}$$

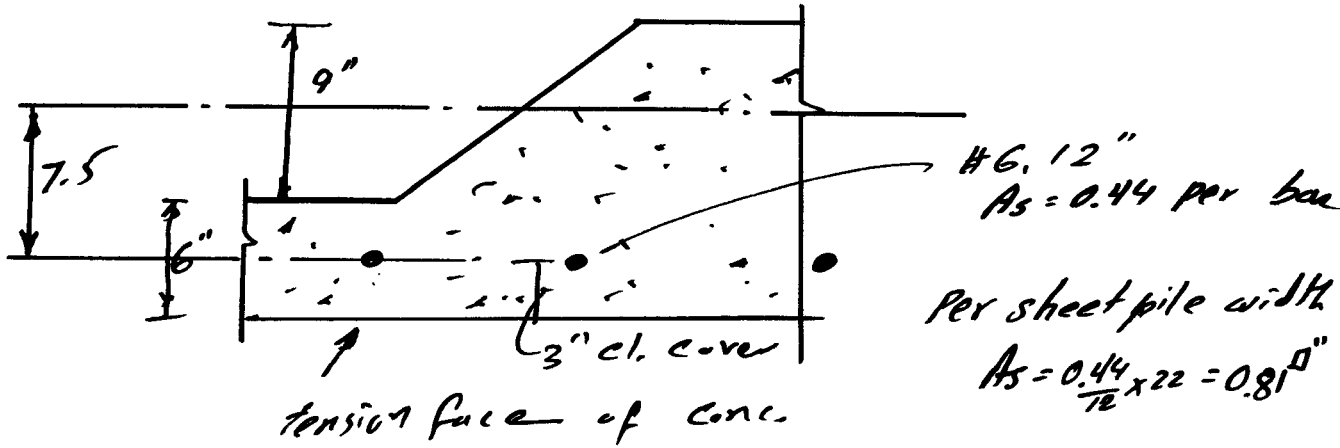
$$f_v = \frac{V}{A} = \frac{2050.3}{1.84} = 1.114 \text{ K.S.I.}$$

F/S

P/S



Max. Moment
= 9.3^{1-k/ft}
(Btw sta. 568 - 589 E/B)
Per sheet pile width
 $M = 9.3 \times \frac{22}{12}$
= 17.05^{1-k}



(Assume no tension carried by concrete.)

Composite property of section:

	A	d	Ad	Ad ²	I_0	$I_0 + Ad^2$
steel sheet pile	11.86	-	-	-	154.7	154.7
reinf. bars	<u>0.81</u>	7.5	<u>6.05</u>	45.38	-	45.38
	12.67	(0.48)	6.05			154.70
				- 6.05 x 0.45		200.08 in ⁴
						- 2.72
						<u>197.36 in⁴</u>

$$S_{\text{@ reinf bar}} = \frac{197.36}{(7.5 - 0.48)} = 28.11$$

$$f_b = \frac{\cancel{9.5 \times 12} \times 17.05 \times 12}{28.11}$$

$$= 7.28 < 20 \text{ allow. o.k.}$$

LIST Q67A

1 10001 17TH ST. CANAL G.D.M. ,Q-FILE :Q67A ,Q-CASE ,F.S.=1.0
2 10002 STA.568+00 B/L TO STA.589+00 B/L ,ORLEANS SIDE
3 10003 3 0.136000000E+02 0.000000000E+01
4 10004 3 0.126000000E+02 0.625000000E+02
5 10005 3 0.116000000E+02 0.125000000E+03
6 10006 3 0.106000000E+02 0.187500000E+03
7 10007 3 0.960000000E+01 0.250000000E+03
8 10008 3 0.860000000E+01 0.312500000E+03
9 10009 3 0.760000000E+01 0.375000000E+03
10 10010 3 0.660000000E+01 0.437500000E+03
11 10011 3 0.560000000E+01 0.500000000E+03
12 10012 3 0.550000000E+01 0.506250000E+03
13 10013 3 0.550000000E+01 0.000000000E+01
14 10014 3 0.550000000E+01 -0.493750000E+03
15 10015 3 0.450000000E+01 -0.551250000E+03
16 10016 3 0.350000000E+01 -0.608750000E+03
17 10017 3 0.350000000E+01 -0.608750000E+03
18 10018 3 0.250000000E+01 -0.649250000E+03
19 10019 3 0.150000000E+01 -0.689750000E+03
20 10020 3 0.150000000E+01 -0.689750000E+03
21 10021 3 0.500000000E+00 -0.730250000E+03
22 10022 3 0.25746630E+00 -0.74007261E+03
23 10023 3 -0.18374126E+01 0.000000000E+01

24 10024 3 -0.52142129E+01 0.11929460E+04
25 10025 4 -0.52142129E+01 0.000000000E+01
26 10026 0 -0.52142129E+01 0.000000000E+01
27 10027 -0.52142129E+01 0.000000000E+01 0.28462435E+01
EOT..

LIST PZ27

1 100 1 13.60 -5.2142 1 -5.2142 0 -1
2 200 PZ-22
3 300 29000000 6.4691 84.3818
EOT

LIST Q67A0

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH ST. CANAL G.D.M. ,Q-FILE :Q67A ,Q-CASE ,F.S.=1.0
A.5

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.59 INCHES AND OCCURS AT MEMBER COORDINATE
13.60 FT.

O.K.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
-5.21	POINT LD	0.00 LBF
-5.21	COUPLE	-2.85 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
13.60	CONTN LD	0.00 LBF/SQ FT
12.60	CONTN LD	62.50 LBF/SQ FT
11.60	CONTN LD	125.00 LBF/SQ FT
10.60	CONTN LD	187.50 LBF/SQ FT
9.60	CONTN LD	250.00 LBF/SQ FT
8.60	CONTN LD	312.50 LBF/SQ FT
7.60	CONTN LD	375.00 LBF/SQ FT
6.60	CONTN LD	437.50 LBF/SQ FT
5.60	CONTN LD	500.00 LBF/SQ FT
5.50	CONTN LD	506.25 LBF/SQ FT
5.50	CONTN LD	0.00 LBF/SQ FT
5.50	CONTN LD	-493.75 LBF/SQ FT
4.50	CONTN LD	-551.25 LBF/SQ FT
3.50	CONTN LD	-608.75 LBF/SQ FT
3.50	CONTN LD	-608.75 LBF/SQ FT
2.50	CONTN LD	-649.25 LBF/SQ FT
1.50	CONTN LD	-689.75 LBF/SQ FT
1.50	CONTN LD	-689.75 LBF/SQ FT
0.50	CONTN LD	-730.25 LBF/SQ FT
0.26	CONTN LD	-740.07 LBF/SQ FT
-1.84	CONTN LD	0.00 LBF/SQ FT
-5.21	CONTN LD	1192.95 LBF/SQ FT
-5.21	CONTN LD	0.00 LBF/SQ FT

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EOT..

PROPERTIES ARE AS FOLLOWS.

MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
 CROSS SECTIONAL AREA= 6.47 SQ IN.
 ELASTIC MODULUS= 29000000. LBF/SQ IN.
 DEFLECTION REFERENCE IS AT -5.214

THE MAXIMUM BENDING MOMENT IS 9286.48 LBF-FT AND OCCURS AT 2.02
 WHICH HAS THE SHEAR FORCE OF 0.33 LBF.

DISTANCE (FEET)	SHEAR FOR (LBF)	SHEAR STR (LBF/SQIN)	BENDING MOM (LBF-FT)	DEFLECTION FROM TANG. THRU DEFLE REFERENCE (INCHES)
13.600	0.0	0.0	0.0	0.5896
13.599	0.0	0.0	0.0	0.5896
13.000	11.2	1.7	2.2	0.5582
12.000	80.0	12.4	42.7	0.5058
11.000	211.2	32.7	183.1	0.4535
10.000	405.0	62.6	486.0	0.4013
9.000	661.2	102.2	1013.9	0.3495
8.000	980.0	151.5	1829.3	0.2984
7.000	1361.2	210.4	2994.7	0.2487
6.000	1805.0	279.0	4572.7	0.2011
5.500	2050.3	316.9	5535.8	0.1785
5.000	1796.2	277.7	6498.1	0.1568
4.000	1245.0	192.5	8023.5	0.1170
3.000	638.4	98.7	8969.3	0.0829
2.017	0.3	0.1	9286.5	0.0554
2.000	-10.9	-1.7	9286.4	0.0550
1.000	-700.6	-108.3	8934.0	0.0335
0.000	-1417.8	-219.2	7872.8	0.0183
-1.000	-1890.3	-292.2	6189.3	0.0086
-1.836	-2014.2	-311.4	4539.2	0.0039
-1.838	-2014.2	-311.4	4535.1	0.0039
-2.000	-2009.5	-310.6	4209.9	0.0032
-3.000	-1775.4	-274.4	2288.0	0.0008
-4.000	-1188.1	-183.7	776.8	0.0001
-5.000	-247.4	-38.2	29.6	0.0000
-5.213	-1.2	-0.2	2.8	0.0000
-5.214	0.0	0.0	0.0	0.0000

RUN COMPLETED

$$f_s = \frac{9286.48 \times 12}{18.1} = 6.156 < 18.0 \text{ k.s.i.} \quad \text{o.k.}$$

$$f_v = \frac{V}{A} = \frac{2050.3}{1.84} = 1.114 \text{ k.s.i.} \quad \text{o.k.}$$

LIST Q68

1	10001	17TH ST CANAL G.D.M. ,Q-FILE :Q68 ,Q-CASE ,F.S.1.0		
2	10002	STA.589+00 B/L TO STA. 614+25 B/L , ORLRANS SIDE		
3	10003		3	0.136000000E+02 0.000000000E+01
4	10004		3	0.126000000E+02 0.625000000E+02
5	10005		3	0.116000000E+02 0.125000000E+03
6	10006		3	0.106000000E+02 0.187500000E+03
7	10007		3	0.960000000E+01 0.250000000E+03
8	10008		3	0.860000000E+01 0.312500000E+03
9	10009		3	0.760000000E+01 0.375000000E+03
10	10010		3	0.660000000E+01 0.437500000E+03
11	10011		3	0.560000000E+01 0.500000000E+03
12	10012		3	0.550000000E+01 0.506250000E+03
13	10013		3	0.550000000E+01 0.000000000E+01
14	10014		3	0.550000000E+01 -0.493750000E+03
15	10015		3	0.450000000E+01 -0.551250000E+03
16	10016		3	0.350000000E+01 -0.608750000E+03
17	10017		3	0.350000000E+01 -0.608750000E+03
18	10018		3	0.250000000E+01 -0.649250000E+03
19	10019		3	0.150000000E+01 -0.689750000E+03
20	10020		3	0.150000000E+01 -0.689750000E+03
21	10021		3	0.500000000E+00 -0.730250000E+03
22	10022		3	0.39658023E+00 -0.73443850E+03
23	10023		3	-0.18628454E+01 0.000000000E+01
24	10024		3	-0.53409491E+01 0.11305764E+04
25	10025		4	-0.53409491E+01 0.000000000E+01
26	10026		0	-0.53409491E+01 0.000000000E+01
27	10027	-0.53409491E+01	0.000000000E+01	0.41232647E+01

EOT..

LIST PZ27

1	1	13.60 -5.3409 1 -5.3409 0 -1
2	200	PZ-22
3	300	29000000 6.4691 84.3818

EOT..

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH ST CANAL G.D.M. ,Q-FILE :Q68 ,Q-CASE ,F.S.1.0
A.5

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.59 INCHES AND OCCURS AT MEMBER COORDINATE
13.60 FT.

D.K.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
-5.34	POINT LD	0.00 LBF
-5.34	COUPLE	-4.13 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
13.60	CONTN LD	0.00 LBF/SQ FT
12.60	CONTN LD	62.50 LBF/SQ FT
11.60	CONTN LD	125.00 LBF/SQ FT
10.60	CONTN LD	187.50 LBF/SQ FT
9.60	CONTN LD	250.00 LBF/SQ FT
8.60	CONTN LD	312.50 LBF/SQ FT
7.60	CONTN LD	375.00 LBF/SQ FT
6.60	CONTN LD	437.50 LBF/SQ FT
5.60	CONTN LD	500.00 LBF/SQ FT
5.50	CONTN LD	506.25 LBF/SQ FT

5.50	CONTN LD	0.00 LBF/SQ FT
5.50	CONTN LD	-493.75 LBF/SQ FT
4.50	CONTN LD	-551.25 LBF/SQ FT
3.50	CONTN LD	-608.75 LBF/SQ FT
3.50	CONTN LD	-608.75 LBF/SQ FT
2.50	CONTN LD	-649.25 LBF/SQ FT
1.50	CONTN LD	-689.75 LBF/SQ FT
1.50	CONTN LD	-689.75 LBF/SQ FT
0.50	CONTN LD	-730.25 LBF/SQ FT
0.40	CONTN LD	-734.44 LBF/SQ FT
-1.86	CONTN LD	0.00 LBF/SQ FT
-5.34	CONTN LD	1130.58 LBF/SQ FT
-5.34	CONTN LD	0.00 LBF/SQ FT

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65 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
66 CROSS SECTIONAL AREA= 6.47 SQ IN.
67 ELASTIC MODULUS= 29000000. LBF/SQ IN.
68 DEFLECTION REFERENCE IS AT -5.341
69

70
71 THE MAXIMUM BENDING MOMENT IS 9286.48 LBF-FT AND OCCURS AT 2.02
72 WHICH HAS THE SHEAR FORCE OF 0.33 LBF.
73
74

DEFLECTION
FROM TANG.
THRU DEFLE
REFERENCE

77 DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	DEFLECTION
(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	(INCHES)
79 13.600	0.0	0.0	0.0	0.5941
80 13.599	0.0	0.0	0.0	0.5941
81 13.000	11.2	1.7	2.2	0.5625
82 12.000	80.0	12.4	42.7	0.5099
83 11.000	211.2	32.7	183.1	0.4573
84 10.000	405.0	62.6	486.0	0.4048
85 9.000	661.2	102.2	1013.9	0.3527
86 8.000	980.0	151.5	1829.3	0.3014
87 7.000	1361.2	210.4	2994.7	0.2514
88 6.000	1805.0	279.0	4572.7	0.2035
89 5.500	2050.3	316.9	5535.8	0.1807
90 5.000	1796.2	277.7	6498.1	0.1589
91 4.000	1245.0	192.5	8023.5	0.1189
92 3.000	638.4	98.7	8969.3	0.0845
93 2.017	0.3	0.1	9286.5	0.0567
94 2.000	-10.9	-1.7	9286.4	0.0563
95 1.000	-700.6	-108.3	8934.0	0.0346
96 0.000	-1402.1	-216.7	7875.4	0.0191
97 -1.000	-1845.1	-285.2	6224.7	0.0092
98 -1.862	-1966.1	-303.9	4565.0	0.0041
99 -1.864	-1966.1	-303.9	4561.1	0.0041
100 -2.000	-1963.1	-303.5	4293.5	0.0036
101 -3.000	-1756.0	-271.4	2406.9	0.0010
102 -4.000	-1223.8	-189.2	890.0	0.0001
103 -5.000	-366.6	-56.7	67.7	0.0000
104 -5.340	-1.2	-0.2	4.1	0.0000
105 -5.341	0.0	0.0	0.0	0.0000

106
107
108
109 *RUN COMPLETED*
110
EOT..

$$f_s = \frac{9286.48 \times 12}{18.1} = 6.156 \text{ K.S.I.} < 18 \text{ K.S.I.} \quad \text{O.K.}$$

$$f_v = \frac{V}{A} = \frac{2050.3}{1.84} = 1.114 \text{ K.S.I.}$$

Q-File - (Q69 - 17th ST. CANAL Floodwall) ORLEANS SIDE

STA. 614+25 To STA. 625+25

Q-File -

LIST0089 Q69, Q-Case, FS=110, SWL- EL13.6

1	10001	17TH STREET CANAL FLOODWALL				
2	10002	STA.614+25 TO STA.625+25				
3	10003		3	0.13600000E+02	0.00000000E+01	
4	10004		3	0.12600000E+02	0.62500000E+03	
5	10005		3	0.11600000E+02	0.12500000E+03	
6	10006		3	0.10600000E+02	0.18750000E+03	
7	10007		3	0.96000000E+01	0.25000000E+03	
8	10008		3	0.86000000E+01	0.31250000E+03	
9	10009		3	0.76000000E+01	0.37500000E+03	
10	10010		3	0.66000000E+01	0.43750000E+03	
11	10011		3	0.65000000E+01	0.44375000E+03	
12	10012		3	0.65000000E+01	0.00000000E+01	
13	10013		3	0.65000000E+01	-0.55625000E+03	
14	10014		3	0.55000000E+01	-0.61375000E+03	
15	10015		3	0.45000000E+01	-0.67125000E+03	
16	10016		3	0.35000000E+01	-0.72875000E+03	
17	10017		3	0.35000000E+01	-0.72875000E+03	
18	10018		3	0.25000000E+01	-0.76925000E+03	
19	10019		3	0.17814065E+01	-0.79835304E+03	
20	10020		3	0.10177405E+01	0.00000000E+01	
21	10021		3	-0.92323799E+00	0.20291411E+04	
22	10022		4	-0.92323799E+00	0.00000000E+01	
23	10023		0	-0.92323799E+00	0.00000000E+01	
24	10024			-0.92323799E+00	0.14901161E-07	0.21135420E+01

EOT..

Req'd tip El. -6.25 based on 2.5:1, penetration to head ratio
Use -6.50

LIST PZ22

1	100	1	13.6	-.9232	1	-.9232	0	-1
2	200	PZ-22						
3	300	290000000	6.4691	84.3818				

E ..

LIST 0690

1
2 BEAMS (SHEAR, MOMENT, DEFLECTION)

3
4
5 17TH STREET CANAL FLOODWALL
6 A.6
7

8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
10 AS COUNTERCLOCKWISE.

11
12
13 THE MAXIMUM DEFLECTION IS 0.23 INCHES AND OCCURS AT MEMBER COORDINATE
14 13.60 FT.

15
16
17
18 Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.
19
20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21
22
23 CALCULATED EXTERNAL LOADS

24
25 DISTANCE FROM TYPE OF MAGNITUDE OF
26 REFERENCE (FT) LOAD LOAD
27
28 -0.92 POINT LD 0.00 LBF
29 -0.92 COUPLE -2.11 LBF-FT

30
31
32 INPUTTED LOADS

33
34 DISTANCE FROM TYPE OF MAGNITUDE OF
35 REFERENCE (FT) LOAD LOAD
36
37 13.60 CONTN LD 0.00 LBF/SQ FT
38 12.60 CONTN LD 62.50 LBF/SQ FT
39 11.60 CONTN LD 125.00 LBF/SQ FT
40 10.60 CONTN LD 187.50 LBF/SQ FT
41 9.60 CONTN LD 250.00 LBF/SQ FT
42 8.60 CONTN LD 312.50 LBF/SQ FT
43 7.60 CONTN LD 375.00 LBF/SQ FT
44 6.60 CONTN LD 437.50 LBF/SQ FT
45 6.50 CONTN LD 443.75 LBF/SQ FT
46 6.50 CONTN LD 0.00 LBF/SQ FT

47 6.50 CONTN LD -556.25 LBF/SQ FT
 48 5.50 CONTN LD -613.75 LBF/SQ FT
 49 4.50 CONTN LD -671.25 LBF/SQ FT
 50 3.50 CONTN LD -728.75 LBF/SQ FT
 51 3.50 CONTN LD -728.75 LBF/SQ FT
 52 2.50 CONTN LD -769.25 LBF/SQ FT
 53 1.78 CONTN LD -798.35 LBF/SQ FT
 54 1.02 CONTN LD 0.00 LBF/SQ FT
 55 -0.92 CONTN LD 2029.14 LBF/SQ FT
 56 -0.92 CONTN LD 0.00 LBF/SQ FT

57
 58
 59 Z-22 PROPERTIES ARE AS FOLLOWS.

60
 61
 62 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
 63 CROSS SECTIONAL AREA= 6.47 SQ IN.
 64 ELASTIC MODULUS= 29000000. LBF/SQ IN.
 65 DEFLECTION REFERENCE IS AT -0.923

66
 67
 68 THE MAXIMUM BENDING MOMENT IS 5778.52 LBF-FT AND OCCURS AT 3.99
 69 WHICH HAS THE SHEAR FORCE OF 0.18 LBF.

70					DEFLECTION
71					FROM TANG.
72					THRU DEFLE
73					REFERENCE
74	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	
75	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	(INCHES)
76	13.600	0.0	0.0	0.0	0.2269
77	13.599	0.0	0.0	0.0	0.2269
78	13.000	11.2	1.7	2.2	0.2120
79	12.000	80.0	12.4	42.7	0.1872
80	11.000	211.2	32.7	183.1	0.1624
81	10.000	405.0	62.6	486.0	0.1378
82	9.000	661.2	102.2	1013.9	0.1136
83	8.000	980.0	151.5	1829.3	0.0901
84	7.000	1361.2	210.4	2994.7	0.0679
85	6.500	1575.3	243.5	3728.2	0.0576
86	6.000	1290.0	199.4	4445.2	0.0479
87	5.000	676.2	104.5	5433.1	0.0310
88	4.000	5.0	0.8	5778.5	0.0178
89	3.993	0.2	0.0	5778.5	0.0178
90	3.000	-721.6	-111.5	5424.3	0.0087
91	2.000	-1490.9	-230.5	4321.4	0.0033
92	1.019	-1969.3	-304.4	2552.3	0.0008

93	1.017	-1969.3	-304.4	2548.3	0.0008
94	1.000	-1969.1	-304.4	2515.4	0.0008
95	0.000	-1427.8	-220.7	729.8	0.0001
96	-0.922	-2.1	-0.3	2.1	0.0000
97	-0.923	0.0	0.0	0.0	0.0000

98

99

100

101 *RUN COMPLETED*

102

EDT..

STA. 625+25 To STA. 635+00

LIST 070

Station	Material	Quantity	Unit	Value 1	Value 2
1 10001	17TH STREET CANAL FLOODWALL				
2 10002	STA. 625+25 TO STA. 635+00				
3 10003		3		0.14100000E+02	0.00000000E+01
4 10004		3		0.13100000E+02	0.62500000E+02
5 10005		3		0.12100000E+02	0.12500000E+03
6 10006		3		0.11100000E+02	0.18750000E+03
7 10007		3		0.10100000E+02	0.25000000E+03
8 10008		3		0.91000000E+01	0.31250000E+03
9 10009		3		0.81000000E+01	0.37500000E+03
10 10010		3		0.75000000E+01	0.41250000E+03
11 10011		3		0.75000000E+01	0.00000000E+01
12 10012		3		0.75000000E+01	-0.58750000E+03
13 10013		3		0.65000000E+01	-0.64500000E+03
14 10014		3		0.55000000E+01	-0.70250000E+03
15 10015		3		0.45000000E+01	-0.76000000E+03
16 10016		3		0.45000000E+01	-0.76000000E+03
17 10017		3		0.35291519E+01	-0.81582376E+03
18 10018		3		0.28082889E+01	0.00000000E+01
19 10019		3		0.10670714E+01	0.19705916E+04
20 10020		4		0.10670714E+01	0.00000000E+01
21 10021		0		0.10670714E+01	0.00000000E+01
22 10022				0.10670714E+01	-0.33769675E+01
					0.24161299E+01

EOT..

Required tip El. -5.0 based on 2.5:1 penetration
 to lead ratio
 Use tip El -5.0

1	100	1	14.1	1.0671	1	1.0671	0	-1
2	200		PZ-22					
3	300		29000000	6.4691	84.3818			

EOT..

CORPS

LIST 0700

1
2 BEAMS (SHEAR, MOMENT, DEFLECTION)

3
4
5 17TH STREET CANAL FLOODWALL
6 A.6

7
8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
10 AS COUNTERCLOCKWISE.

11
12
13 THE MAXIMUM DEFLECTION IS 0.14 INCHES AND OCCURS AT MEMBER COORDINATE
14 14.10 FT.

o.k.

15
16
17
18 Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19
20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21
22
23 CALCULATED EXTERNAL LOADS

24	25	26	27
DISTANCE FROM	TYPE OF	MAGNITUDE OF	
REFERENCE(FT)	LOAD	LOAD	
28	1.07 POINT LD	3.38 LBF	
29	1.07 COUPLE	-2.42 LBF-FT	

30
31
32 INPUTTED LOADS

34	35	36	37	38
DISTANCE FROM	TYPE OF	MAGNITUDE OF		
REFERENCE(FT)	LOAD	LOAD		
39	14.10 CONTN LD	0.00 LBF/SQ FT		
40	13.10 CONTN LD	62.50 LBF/SQ FT		
41	12.10 CONTN LD	125.00 LBF/SQ FT		
42	11.10 CONTN LD	187.50 LBF/SQ FT		
43	10.10 CONTN LD	250.00 LBF/SQ FT		
44	9.10 CONTN LD	312.50 LBF/SQ FT		
45	8.10 CONTN LD	375.00 LBF/SQ FT		
46	7.50 CONTN LD	412.50 LBF/SQ FT		
	7.50 CONTN LD	0.00 LBF/SQ FT		
	7.50 CONTN LD	-587.50 LBF/SQ FT		

47 6.50 CONTN LD -645.00 LBF/SQ FT
 48 5.50 CONTN LD -702.50 LBF/SQ FT
 49 4.50 CONTN LD -760.00 LBF/SQ FT
 50 4.50 CONTN LD -760.00 LBF/SQ FT
 51 3.53 CONTN LD -815.82 LBF/SQ FT
 52 2.81 CONTN LD 0.00 LBF/SQ FT
 53 1.07 CONTN LD 1970.59 LBF/SQ FT
 54 1.07 CONTN LD 0.00 LBF/SQ FT
 55
 56

57 Z-22 PROPERTIES ARE AS FOLLOWS.

58
 59
 60 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
 61 CROSS SECTIONAL AREA= 6.47 SQ IN.
 62 ELASTIC MODULUS= 29000000. LBF/SQ IN.
 63 DEFLECTION REFERENCE IS AT 1.067

64
 65
 66 THE MAXIMUM BENDING MOMENT IS 4469.15 LBF-FT AND OCCURS AT 5.41
 67 WHICH HAS THE SHEAR FORCE OF 6.95 LBF.
 68
 69

DEFLECTION

70						FROM TANG.
71						THRU DEFLE
72	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	REFERENCE	
73	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	(INCHES)
74	14.100	0.0	0.0	0.0	0.1403	
75	14.099	0.0	0.0	0.0	0.1403	
76	14.000	0.3	0.0	0.0	0.1387	
77	13.000	37.8	5.8	13.9	0.1217	
78	12.000	137.8	21.3	96.5	0.1047	
79	11.000	300.3	46.4	310.3	0.0878	
80	10.000	525.3	81.2	717.9	0.0712	
81	9.000	812.8	125.6	1381.8	0.0551	
82	8.000	1162.8	179.7	2364.4	0.0400	
83	7.500	1361.2	210.4	2994.7	0.0331	
84	7.000	1060.3	163.9	3600.7	0.0267	
85	6.000	415.3	64.2	4343.3	0.0158	
86	5.409	6.9	1.1	4469.2	0.0107	
87	5.000	-287.2	-44.4	4412.2	0.0079	
88	4.000	-1047.2	-161.9	3749.8	0.0030	
89	3.000	-1698.2	-262.5	2328.0	0.0007	
90	2.809	-1719.0	-265.7	2001.5	0.0005	
91	2.807	-1719.0	-265.7	1998.1	0.0005	
92	2.000	-1349.3	-208.6	710.0	0.0001	

$$f_b = \frac{4469.15 \times 12}{18.1}$$

18.1

$$= 2.962 \text{ C } (18.5 \text{ O.K.})$$

93	1.068	-5.4	-0.8	2.4	0.0000
94	1.067	0.0	0.0	0.0	0.0000
95					
97					
98	*RUN COMPLETED*				
99					
EOT..					

ORLEANS SIDE

D-File (R-7A) 17th ST. OUTFALL CANAL

STA. 635+00 to 642+00 B/L

1	10001	17TH STREET CANAL FLOODWALL			
2	10002	STA.635+00 TO STA.642+00			
3	10003		3	0.141000000E+02	0.000000000E+01
4	10004		3	0.131000000E+02	0.625000000E+02
5	10005		3	0.121000000E+02	0.125000000E+03
6	10006		3	0.111000000E+02	0.187500000E+03
7	10007		3	0.101000000E+02	0.250000000E+03
8	10008		3	0.950000000E+01	0.287500000E+03
9	10009		3	0.950000000E+01	0.000000000E+01
10	10010		3	0.950000000E+01	-0.912500000E+03
11	10011		3	0.850000000E+01	-0.967000000E+03
12	10012		3	0.82817371E+01	-0.97889533E+03
13	10013		3	0.75807167E+01	0.000000000E+01
14	10014		3	0.64877983E+01	0.15261363E+04
15	10015		4	0.64877983E+01	0.000000000E+01
16	10016		0	0.64877983E+01	0.000000000E+01
17	10017	0.64877983E+01		0.14901161E-07	0.13822856E+02

EOT..

Required tip EI 0.0 based on 2.5:1 penetration to lead ratio

Use tip EI 0.0

LIST PZ22

1	100	1	14.1	6.4877	1	6.4877	0	-1
2	200			PZ-22				
3	300	290000000	6.4691	84.3818				

EOT..

CCORPSLLIST 071

1 10001 17TH STREET CANAL FLOODWALL

2 10002 STA.635+00 TO STA.642+00

3 10003	3	0.14100000E+02	0.00000000E+01
4 10004	3	0.13100000E+02	0.62500000E+02
5 10005	3	0.12100000E+02	0.12500000E+03
6 10006	3	0.11100000E+02	0.18750000E+03
7 10007	3	0.10100000E+02	0.25000000E+03
8 10008	3	0.95000000E+01	0.28750000E+03
9 10009	3	0.95000000E+01	0.00000000E+01
10 10010	3	0.95000000E+01	-0.91250000E+03
11 10011	3	0.85000000E+01	-0.96700000E+03
12 10012	3	0.82817371E+01	-0.97889533E+03
13 10013	3	0.75807167E+01	0.00000000E+01
14 10014	3	0.64877983E+01	0.15261363E+04
15 10015	4	0.64877983E+01	0.00000000E+01
16 10016	0	0.64877983E+01	0.00000000E+01
17 10017	0.64877983E+01	0.14901161E-07	0.13822856E+02

EOT..

LIST 0710

1

2 BEAMS (SHEAR, MOMENT, DEFLECTION)

3

4

5 17TH STREET CANAL FLOODWALL

6 A.6

7

8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT

9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS

10 AS COUNTERCLOCKWISE.

11

12

13 THE MAXIMUM DEFLECTION IS 0.01 INCHES AND OCCURS AT MEMBER COORDINATE

14 14.10 FT.

15

16

17

18 Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19

20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21

22

23 CALCULATED EXTERNAL LOADS

O.K.

24
 25 DISTANCE FROM TYPE OF MAGNITUDE OF
 26 REFERENCE(FT) LOAD LOAD
 27
 28 6.49 POINT LD 0.00 LBF
 29 6.49 COUPLE -13.82 LBF-FT
 30
 31

32 INFUTTED LOADS

33
 34 DISTANCE FROM TYPE OF MAGNITUDE OF
 35 REFERENCE(FT) LOAD LOAD
 36
 37 14.10 CONTN LD 0.00 LBF/SQ FT
 38 13.10 CONTN LD 62.50 LBF/SQ FT
 39 12.10 CONTN LD 125.00 LBF/SQ FT
 40 11.10 CONTN LD 187.50 LBF/SQ FT
 41 10.10 CONTN LD 250.00 LBF/SQ FT
 42 9.50 CONTN LD 287.50 LBF/SQ FT
 43 9.50 CONTN LD 0.00 LBF/SQ FT
 44 9.50 CONTN LD -912.50 LBF/SQ FT
 45 8.50 CONTN LD -967.00 LBF/SQ FT
 46 8.28 CONTN LD -978.90 LBF/SQ FT
 47
 47 7.58 CONTN LD 0.00 LBF/SQ FT
 48 6.49 CONTN LD 1526.14 LBF/SQ FT
 49 6.49 CONTN LD 0.00 LBF/SQ FT
 50

51 Z-22 PROPERTIES ARE AS FOLLOWS.

52
 53
 54
 55 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
 56 CROSS SECTIONAL AREA= 6.47 SQ IN.
 57 ELASTIC MODULUS= 29000000. LBF/SQ IN.
 58 DEFLECTION REFERENCE IS AT 6.488

59
 60
 61 THE MAXIMUM BENDING MOMENT IS 1250.12 LBF-FT AND OCCURS AT 8.78
 62 WHICH HAS THE SHEAR FORCE OF -7.90 LBF.

63
 64 DEFLECTION
 65 FROM TANG.
 66 THRU DEFLE
 67 REFERENCE
 68 DISTANCE SHEAR FOR SHEAR STR BENDING MOM REFERENCE
 69 (FEET) (LBF) (LBF/SQIN) (LBF-FT) (INCHES)
 69 14.100 0.0 0.0 0.0 0.0123

By Inspection
f_b < 18.5
D.K.

70	14.099	0.0	0.0	0.0	0.0123
71	14.000	0.3	0.0	0.0	0.0121
72	13.000	37.8	5.8	13.9	0.0096
73	12.000	137.8	21.3	96.5	0.0071
74	11.000	300.3	46.4	310.3	0.0048
75	10.000	525.3	81.2	717.9	0.0026
76	9.500	661.2	102.2	1013.9	0.0017
77	9.000	198.2	30.6	1229.3	0.0010
78	8.782	-7.9	-1.2	1250.1	0.0008
79	8.000	-711.2	-109.9	954.0	0.0002
80	7.582	-834.0	-128.9	622.3	0.0001
81	7.580	-834.0	-128.9	620.6	0.0001
82	7.000	-598.5	-92.5	182.7	0.0000
83	6.489	-1.4	-0.2	13.8	0.0000
84	6.488	0.0	0.0	0.0	0.0000

85

86

87

88 *RUN COMPLETED*

89

EQT..

Q-File

Q-72

(17th St. OUTFALL CANAL)

ORLEANS SIDE

STA. 642+00 To STA. 663+00

1	10001	17TH STREET CANAL FLOODWALL		
2	10002	STA. 642+00 TO STA. 663+00		
3	10003		3	0.14600000E+02 0.00000000E+01
4	10004		3	0.13600000E+02 0.62500000E+02
5	10005		3	0.12600000E+02 0.12500000E+03
6	10006		3	0.12000000E+02 0.16250000E+03
7	10007		3	0.12000000E+02 0.00000000E+01
8	10008		3	0.12000000E+02 -0.10375000E+04
9	10009		3	0.11666735E+02 -0.10556629E+04
10	10010		3	0.11395166E+02 0.00000000E+01
11	10011		3	0.11015017E+02 0.14777430E+04
12	10012		4	0.11015017E+02 0.00000000E+01
13	10013		0	0.11015017E+02 0.00000000E+01
14	10014	0.11015017E+02	0.89406967E-07	0.61046655E+02

EOT..

Required tip El. 0.0 based on 2.5:1 penetration to lead ratio.

Use tip El. 0.0

LIST PZ22

1	100	1	14.6	11.015	1	11.015	0	-1
2	200			PZ-22				
3	300	29000000	6.4691	84.3818				

EOT..

LIST 0720

1
2 BEAMS (SHEAR, MOMENT, DEFLECTION)

3
4
5 17TH STREET CANAL FLOODWALL
6 A.6
7

8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
10 AS COUNTERCLOCKWISE.

11
12
13 THE MAXIMUM DEFLECTION IS 0.00 INCHES AND OCCURS AT MEMBER COORDINATE
14 14.60 FT.
15
16

17
18 Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19
20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.
21

22
23 CALCULATED EXTERNAL LOADS

24
25 DISTANCE FROM TYPE OF MAGNITUDE OF
26 REFERENCE(FT) LOAD LOAD
27
28 11.02 POINT LD 0.00 LBF
29 11.02 COUPLE -61.05 LBF-FT
30

31
32 INPUTTED LOADS

33
34 DISTANCE FROM TYPE OF MAGNITUDE OF
35 REFERENCE(FT) LOAD LOAD
36
37 14.60 CONTN LD 0.00 LBF/SQ FT
38 13.60 CONTN LD 62.50 LBF/SQ FT
39 12.60 CONTN LD 125.00 LBF/SQ FT
40 12.00 CONTN LD 162.50 LBF/SQ FT
41 12.00 CONTN LD 0.00 LBF/SQ FT
42 12.00 CONTN LD -1037.50 LBF/SQ FT
43 11.67 CONTN LD -1055.66 LBF/SQ FT
44 11.40 CONTN LD 0.00 LBF/SQ FT
45 11.02 CONTN LD 1477.74 LBF/SQ FT
46 11.02 CONTN LD 0.00 LBF/SQ FT

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80
EOT..

Z-22 PROPERTIES ARE AS FOLLOWS.

MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
CROSS SECTIONAL AREA= 6.47 SQ IN.
ELASTIC MODULUS= 29000000. LBF/SQ IN.
DEFLECTION REFERENCE IS AT 11.015

THE MAXIMUM BENDING MOMENT IS 202.83 LBF-FT AND OCCURS AT 11.74
WHICH HAS THE SHEAR FORCE OF -59.50 LBF.

DEFLECTION
FROM TANG.
THRU DEFLE
REFERENCE

DISTANCE (FEET)	SHEAR FOR (LBF)	SHEAR STR (LBF/SQIN)	BENDING MOM (LBF-FT)	DEFLECTION FROM TANG. THRU DEFLE REFERENCE (INCHES)
14.600	0.0	0.0	0.0	0.0005
14.599	0.0	0.0	0.0	0.0005
14.000	11.2	1.7	2.2	0.0004
13.000	80.0	12.4	42.7	0.0002
12.000	211.2	32.7	183.1	0.0000
11.741	-59.5	-9.2	202.8	0.0000
11.396	-280.9	-43.4	132.5	0.0000
11.394	-280.9	-43.4	132.0	0.0000
11.016	-1.5	-0.2	61.0	0.0000
11.015	0.0	0.0	0.0	0.0000

RUN COMPLETED

*By inspection Pb < 18.5
O.K.*

ORLEANS SIDE

Q-FILE - Q-73 (17th ST. OUTFALL CANAL)

STA. 663+00 To STA. 670+63

LIST Q73

1	10001	17TH STREET CANAL FLOODWALL			
2	10002	STA. 663+00 TO STA. 670+63			
3	10003		3	0.146000000E+02	0.000000000E+01
4	10004		3	0.136000000E+02	0.625000000E+02
5	10005		3	0.126000000E+02	0.125000000E+03
6	10006		3	0.116000000E+02	0.187500000E+03
7	10007		3	0.110000000E+02	0.225000000E+03
8	10008		3	0.110000000E+02	0.000000000E+01
9	10009		3	0.110000000E+02	-0.975000000E+03
10	10010		3	0.10229542E+02	-0.10169900E+04
11	10011		3	0.97923208E+01	0.000000000E+01
12	10012		3	0.90832776E+01	0.16492565E+04
13	10013		4	0.90832776E+01	0.000000000E+01
14	10014		0	0.90832776E+01	0.000000000E+01
15	10015	0.90832776E+01	0.29802322E-07	0.48744224E+01	

EOT..

Required tip EI. 0.0 based on 2.5 : 1 penetration to
 lead ratio
 use tip EI 0.0

LIST PZ22

1	100	1	14.6	9.0832	1	9.0832	0	-1
2	200			PZ-22				
3	300	290000000	6.4691	84.3818				

EOT..

LIST 0730

1
2 BEAMS (SHEAR, MOMENT, DEFLECTION)

3
4
5 17TH STREET CANAL FLOODWALL
6 A.6
7

8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
10 AS COUNTERCLOCKWISE.

11
12
13 THE MAXIMUM DEFLECTION IS 0.00 INCHES AND OCCURS AT MEMBER COORDINATE
14 14.60 FT.
15
16
17

18 Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.
19
20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.
21
22

23 CALCULATED EXTERNAL LOADS

24
25 DISTANCE FROM TYPE OF MAGNITUDE OF
26 REFERENCE(FT) LOAD LOAD
27
28 9.08 POINT LD 0.00 LBF
29 9.08 COUPLE -4.87 LBF-FT
30
31

32 INPUTTED LOADS

33
34 DISTANCE FROM TYPE OF MAGNITUDE OF
35 REFERENCE(FT) LOAD LOAD
36
37 14.60 CONTN LD 0.00 LBF/SQ FT
38 13.60 CONTN LD 62.50 LBF/SQ FT
39 12.60 CONTN LD 125.00 LBF/SQ FT
40 11.60 CONTN LD 187.50 LBF/SQ FT
41 11.00 CONTN LD 225.00 LBF/SQ FT
42 11.00 CONTN LD 0.00 LBF/SQ FT
43 11.00 CONTN LD -975.00 LBF/SQ FT
44 10.23 CONTN LD -1016.99 LBF/SQ FT
45 9.79 CONTN LD 0.00 LBF/SQ FT
46 9.08 CONTN LD 1649.26 LBF/SQ FT

47 9.08 CONTN LD 0.00 LBF/SQ FT

48
4
50 Z-22 PROPERTIES ARE AS FOLLOWS.

51
52
53 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
54 CROSS SECTIONAL AREA= 6.47 SQ IN.
55 ELASTIC MODULUS= 29000000. LBF/SQ IN.
56 DEFLECTION REFERENCE IS AT 9.083

57
58
59 THE MAXIMUM BENDING MOMENT IS 569.27 LBF-FT AND OCCURS AT 10.57
60 WHICH HAS THE SHEAR FORCE OF -20.35 LBF.

61					DEFLECTION
62					FROM TANG.
63					THRU DEFLE
64					REFERENCE
65	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	REFERENCE
66	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	(INCHES)
67	14.600	0.0	0.0	0.0	0.0030
68	14.599	0.0	0.0	0.0	0.0030
69	14.000	11.2	1.7	2.2	0.0025
70	13.000	80.0	12.4	42.7	0.0017
71	12.000	211.2	32.7	183.1	0.0009
72	11.000	405.0	62.6	486.0	0.0003
73	10.569	-20.4	-3.1	569.3	0.0002
74	10.000	-534.5	-82.6	399.2	0.0000
75	9.793	-584.7	-90.4	281.8	0.0000
76	9.791	-584.7	-90.4	280.7	0.0000
77	9.084	-1.5	-0.2	4.9	0.0000
78	9.083	0.0	0.0	0.0	0.0000

79
80
81
82 *RUN COMPLETED*
83
EDT..

By inspection $t_b < 18.50$ o.k.