**MEMORANDUM THRU** 

Area Engineer, New Orleans Area Office
Chief, Construction Division ATTN: Contr Adm Br

FOR Chief, Engineering Division

SUBJECT: Contract No. DACW29-97-C-0029, Lake Pontchartrain, Louisiana and Vicinity, High Level Plan, Hurricane Protection Project, Orleans Avenue Canal Flood Protection, Phase II-A Floodwall, Orleans Parish, Louisiana, Narrative Completion Report

- 1. The subject contract dated 11 March 1997 was awarded to Maharrey-Houston Construction Company, P. O. Box 70250, Memphis, Tennessee 38107-0250. The Notice to Proceed was issued 8 April 1997, with construction to commence no later than 18 April 1997. The completion date was set for 13 May 1998 with the contract amount being \$2,885,733.99.
- 2. The contract provided for the construction of approximately 8,352 linear feet of steel reinforced concrete I-wall. In conjunction with the floodwall, 126,001 square feet of steel sheet piling was driven. The contract also required degrading the existing levee, backfilling next to the I-wall, and fertilizing, seeding, and mulching all disturbed areas.
- 3. There was one subcontractor on the project, Louisiana Erosion Control, 35952 Highway 75, Plaquemines, Louisiana 70764. This subcontractor was responsible for fertilizing, seeding, and mulching.
- 4. The preconstruction conference was held 8 April 1997 at the New Orleans Area Office. The on-site prework coordination and safety meetings were held 22 April 1997.
- 5. The contract had 6 major construction phases:
  - a. clearing and grubbing
  - b. structural excavation and backfill
  - c. degrading of the existing levee
  - d. piling, steel sheet, type PZ-22 and PSA-23
  - e. reinforced concrete floodwalls
  - f. fertilizing, seeding, and mulching
- 6. The contractor began mobilizing equipment to the site 22 April 1997. The office trailer were brought to the site 30 April 1997, and the contractor began constructing the equipment staging areas and safety fence at this time.
- 7. The first phase of work was clearing and grubbing. The preparatory meeting for this phase was held 20 May 1997 with work commencing the same day. A John Deere 450 dozer and Caterpillar 416 backhoe were used to remove all vegetation and debris from the work areas. This phase of work was ongoing throughout the contract period and was finally completed 22 May 1998.
- 8. The second phase of work was driving steel sheet piles. The preparatory meeting for this phase was held 28 May 1997 with work commencing 29 May 1997. This phase of work consisted of driving 126,001 square feet of PZ-22 and PSA-23 steel sheet piling. A John Deere 790D backhoe equipped with an approved vibratory hammer was used to drive the piles to the prescribed elevations. All piles were driven plumb and interlocked with the adjoining sheets. Cathodic protection was provided on the sheets by welding #6 rebar to the top of the piles except at the monolith joints where approved bonding cables were installed. This phase of work was completed 12 December 1997.

- 9. The third phase of work was degrading of the existing levees. The preparatory meeting for this phase was held 20 May 1997 with work commencing 12 June 1997. The levee was degraded as specified. Interim flood protection per the contract drawings was left in place until construction in the area was completed. Degrading of the existing levees was ongoing throughout the contract period and was finally completed 22 May 1998.
- 10. The fourth phase of work was reinforced concrete floodwall. The preparatory meeting for this phase was held 10 June 1997 with work commencing 17 June 1997. This phase of work consisted of constructing 8,350 linear feet of steel reinforced concrete I-wall. Formwork was a combination of wood and steel with an inner fractured fin molded rubber lining. The forms were checked for proper line and grade prior to concrete placement. Prior to and during each concrete placement, the QC manager with QA representative present performed slump and air tests on the concrete to assure compliance with the contract specifications. Four compression test cylinders were molded for each pour three were delivered to the Corps of Engineers New Orleans District for testing, and one was sent for testing at an independent lab by the contractor. Once cured, the I-wall was painted with a base coat of thoroseal compound followed by two coats of thorosheen paint in the color prescribed in the contract specifications. This phase of work was completed 6 May 1998.
- 11. The fifth phase of work was structural excavation and backfill. The preparatory meeting for this phase was held 19 August 1997 with work commencing 26 August 1997. This phase of work consisted of excavation for sheet pile driving and I-wall construction as well as backfilling against the wall after the proper curing period. Backfill was placed in lifts not greater than 8 inches, and each lift was compacted to 95% compaction. A John Deere 450 dozer and 790 excavator were used in this operation. Structural excavation and backfill was ongoing throughout the project and was finally completed 22 May 1998.
- 12. Fertilizing, seeding, and mulching was the final phase of work. The preparatory meeting for this phase was held 28 October 1997 with work commencing the same day. This phase consisted of fertilizing, seeding, and mulching all disturbed areas with a fertilizer containing a minimum of 60 pounds nitrogen, 60 pounds phosphorus, and 60 pounds potash per acre. Seed was in the quantities and of the type prescribed for the time of year that the seeding took place. Fertilizing, seeding, and mulching was ongoing throughout the project and was finally completed 21 May 1998.
- 13. Both the contractor and subcontractor were cooperative in performing their respective work. The contractor's Accident Prevention Program was adequately enforced on the project, and there were no lost time accidents.
- 14. The personnel working on the project were skilled and performed their duties in a professional manner. The equipment used at the job site was inspected, found to be in good condition, and was maintained throughout the project.
- 15. All contractor surveying was performed by the contractor's own survey party. All surveys were plotted and submitted to the government with the survey books. The surveys assured contract compliance.
- 16. Included herewith is a comparison of the initial contract quantities and actual contract quantities. A copy of "As-Built" drawings is also attached.

<u>Item</u>	Description	Contract Oty	Unit Price	Actual Qty	Actual Amt
0001	Mobilization and Demobilization	Lump Sum			48,000.00
0002	Clearing and Grubbing	Lump Sum			41,210.43
0003	Structural Excavation and Backfill	Lump Sum			46,000.00

<u>ltem</u>	Description	Contract Qty	Unit Price	Actual Qty	Actual Amt
0004	Degrading Existing Levee	12,430 CY	3.00	12,430 CY	37,290.00
0005	Erosion Control	310 LF	4.00	310 LF	1,240.00
0006	Piling, Steel Sheet Types PZ-22 and PSA-23				
0006A	Materials	125,480 SF	6.58	126,001 SF	829,086.58
0006B	Installation	125,480 SF	4.42	126,001 SF	556,924.42
0006C	Cut Offs - Cut	EA	10.00	4	40.00
0006D	Cut Offs - Material	SF	7.70	45.5	350.35
0007	Reinforced Concrete Floodwalls	Lump Sum			1,312,190.00
0008	Fertilizing, Seeding, and Mulching	Lump Sum			22,000.00
0009	Remove Ramp	Lump Sum			6,500.00

- 17. There were 11 modifications to the contract. A summary of the four work related contract modifications follows:
- a. A00001, CIN-02, dated 8 July 1997 to increase the limits of levee degrading. This modification gave the contractor permission to advance an additional 800 feet, with Contracting Officer approval, provided that the existing levee is not degraded beyond the limit of the floodside crown.
- b. A00002, CIN-03, dated 22 December 1997 to change the geometry of the special I-wall monolith at the 30" water line. This modification impacted the Steel Sheet Piling and Reinforced Concrete Floodwall payment items.
- c. A00004, CIN-04, dated 27 March 1998 to remove and dispose of dead trees within the right-of-way. This modification impacted the Clearing and Grubbing payment item.
- d. A00005, CIN-05, dated 4 June 1998 to remove the earthen ramp at station 20+40. This modification added Item No. 0009 to the Bid Schedule.
- 18. The contract was substantially complete 26 May 1998. A final inspection will be scheduled in early July 1998.

Construction Representative

New Orleans Area Office

CF:

Proj Engr (Lauto)
Const Rep (Carr)
Ofc Engr w/As-Built
CEMVN-CD-Q w/As-Built

CEMVN-CT

CEMVN-ED-C

CEMVN-CD-B

CEMVN-CD-CS

CEMVN-PP File

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**CEMVN-CT** 

CEMVN-ED-C

CEMVN-CD-B

CEMVN-CD-CS

CEMVN-PP

File

## PLANS FOR LAKE PONTCHARTRAIN, LOUISIANA AND VICINITY HIGH LEVEL PLAN ORLEANS PARISH, LA.

# PARALLEL PROTECTION PHASE II-A (EAST SIDE) PROTECTION

1996



AS BUILT

THE SOLICITATION NUMBER ON THESE DRAWINGS IS CHANGED FROM DACW29-96-B-0096 TO DACW29-97-B-0026

DRAWINGS IN THIS FOLIO
HAVE BEEN REDUCED ONE
HALF THE ORIGINAL SCALE



Safety is a Part of Your Contract

### GENERAL NOTES:

- I. AZIMUTHS SHOWN ARE MEASURED CLOCKWISE FROM THE SOUTH.
- 2. ELEVATIONS ARE IN FEET AND REFER TO NATIONAL GEODETIC VERTICAL DATUM (N.G.V.D.).
- 3. DIMENSIONS AND/OR ELEVATIONS MARKED THUS (±) ARE APPROXIMATE. CONTRACTOR SHALL VERIFY ACTUAL DIMENSIONS IN THE FIELD.
- 4. DIMENSIONS AND/OR ELEVATIONS MARKED THUS (N.T.S.) ARE NOT SHOWN TO SCALE.
- 5. DRAWINGS ARE GENERALLY TO SCALE, BUT SHOULD NOT BE SCALED. N.T.S. IS SHOWN ONLY WHERE DRAWING IS OBVIOUSLY SCALED. N.T.S. OUT OF SCALE.
- 6. BENCH MARKS AND BASE LINES HAVE BEEN ESTABLISHED AT THE SITE BY THE GOVERNMENT.
- 7. FOR BORING LOGS, SEE DWG. 27 THRU 31

### STEEL NOTES:

- I. ALL STRUCTURAL STEEL SHALL BE ASTM A36, UNLESS OTHERWISE
- 2. TO PREVENT CORROSION BY MOISTURE BETWEEN STEEL SURFACES IN CONTACT, ALL SUCH CONTACTS SHALL BE SEALED WATERTIGHT BY RUNNING A CONTINUOUS 1/8" FILLET WELD ALONG ALL EDGES OF THE CONTACT, UNLESS OTHERWISE NOTED.
- 3. ALL WELDING SHALL BE ELECTRIC WELDING. WORKMANSHIP AND TECHNIQUE, WHERE APPLICABLE, SHALL CONFORM TO THE AMERICAN WELDING SOCIETY (SEE SPECS.) STRUCTURAL WELDING CODE.
- 4. WELDING SYMBOLS SHOWN ARE THOSE ADOPTED BY THE AMERICAN WELDING SOCIETY AND INDICATE ONLY SIZE AND TYPE OF WELDS REQUIRED. DETAILED INFORMATION SHALL BE SHOWN ON THE SHOP DRAWINGS AND SUBMITTED BY THE CONTRACTOR FOR APPROVAL.
- 5. DIMENSIONS SHOWN OR CALLED FOR ARE THE FINAL DIMENSIONS; ALLOWANCES MUST BE MADE FOR MACHINING.
- 6. ITEMS MARKED C.R.S. SHALL BE CORROSION RESISTANT STEEL (STAINLESS STEEL), SEE SPECIFICATIONS.

### CONCRETE NOTES:

- 1. CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (F $_{\rm C}$ ) OF 3000 PSI AT 28 DAYS, 90 DAYS IF POZZOLAN IS USED, UNLESS OTHER WISE NOTED.
- 2. STABILIZATION SLAB CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (F'\_C) OF 2500 PSI AT 28 DAYS, 90 DAYS IF POZZOLAN IS USED.
- 3. REINFORCING STEEL SHALL HAVE A MINIMUM YIELD STRENGTH (Fy) OF 60,000 PSI.
- 4. REINFORCING SHALL BE SPACED TO MISS RECESSES FOR GATE LATCHES.
- 5. CONSTRUCTION JOINTS SHALL BE PROVIDED WHERE SHOWN. WHERE NOT SHOWN, CONSTRUCTION JOINTS SHALL BE PLACED AT LOCATIONS LEAST LIKELY TO IMPAIR THE INTERGRITY OF THE CONCRETE STRUCTURE. CONSTRUCTION JOINT LOCATIONS SHALL BE APPROVED BY THE CONTRACTING OFFICER.
- 6. UNLESS OTHERWISE NOTED, PROVIDE 3/4" CHAMFER AT ALL EXPOSED JOINTS, EDGES, EXTERNAL CORNERS, AND VERTICAL EXPANSION JOINTS.
- 7. ALL PRIMARY REINFORCEMENT SHALL HAVE A MINIMUM COVER OF 3" UNLESS OTHERWISE NOTED. THE COVER FOR SECONDARY REINFORCEMENT MAY BE REDUCED FROM THE ABOVE BY THE DIAMETER OF THE BAR.
- 8. ALL BENDS OF REINFORCEMENT AND ALL BAR SPACERS AND SUPPORTS SHALL BE IN ACCORDANCE WITH SP-66, AMERICAN CONCRETE INSTITUTE DETAILING MANUAL 1980.
- 9. REINFORCING BAR DESIGNATION NUMBERS CONFORM TO THE NUMBERING SYSTEM OF THE CONCRETE REINFORCING STEEL INSTITUTE.
- 10. REINFORCING BARS SHALL BE CONTINUOUS AT ALL CORNERS UNLESS OTHERWISE NOTED.
- II. REINFORCEMENT, WHERE NECESSARY TO AVOID OPENINGS, PIPES, EMBEDDED ITEMS AND OTHER OBSTRUCTIONS, SHALL BE BENT OR SHIFTED AS DIRECTED BY THE CONTRACTING
- 12. THE EMBEDMENT AND SPLICE TABLE SHALL BE USED IN DETERMINING LAP SPLICES AND EMBEDMENT LENGTHS WHERE LENGTHS ARE NOT OTHERWISE INDICATED. SPLICE LENGTHS SHALL BE BASED ON THE SMALLER BAR BEING LAPPED. THE CONTRACTOR WILL BE ALLOWED TO MAKE SPLICES IN ADDITION TO THOSE INDICATED IN THE DRAWINGS, WHERE ESSENTIAL TO CONSTRUCTIBILITY, SUBJECT TO APPROVAL BY THE CONTRACTING OFFICER. SPLICES OTHER THAN THOSE SHOWN ON THE DRAWINGS AND OTHER THAN ANY ADDITIONAL SPLICES REQUIRED BY THE CONTRACTING OFFICER, WILL BE AT THE CONTRACTOR'S EXPENSE.
- 13. ALL EXTERIOR FORMED SURFACES NOT COVERED BY BACKFILL SHALL BE CLASS "A" FINISH AND SURFACES COVERED BY BACKFILL SHALL BE CLASS "D" FINISH, UNLESS OTHERWISE

	REIN	NFORCEME	NT EMB	EDMENT	AND SPL	ICE TABL	ES	
		BASIC	TABLE		ALTERNATE TABLE			
BAR SIZE	MINIMUM EMBEDMENT LENGTH, INCHES		MINIMUM LAP LENGTH INCHES		MINIMUM EMBEDMENT LENGTH, INCHES		MINIMUM LAP LENGT	
-	TOP	OTHER	TOP	OTHER	TOP	OTHER	TOP	OTHER
3	16	12	21	16	16	12	21	16
4	21	16	28	21	21	16	28	21
5	27	21	35	27	27	21	35	27
6	32	25	42	32	32	25	42	32
7	37	29	49	37	37	29	49	37
8	45	35	59	45	43	33	56	43
9	57	44	74	57	48	37	63	48
10	72	56	94	72	58	45	75	58
100	89	68	116	89	71	55	92	71

- I. USE THE BASIC TABLE IF ALL OF THE FOLLOWING CONDITIONS ARE MET:
  - A) CENTER TO CENTER BAR SPACING LATERALLY IS AT LEAST 4 BAR DIAMETERS
- B) CONCRETE COVER IS AT LEAST 2 BAR DIAMETERS, AND
- C) EDGE DISTANCE TO THE FIRST BAR IN A LAYER IS AT LEAST 2 BAR DIAMETERS.
- 2. THE ALTERNATE TABLE MAY BE USED IF ALL OF THE FOLLOWING CONDITIONS ARE MET: A) CENTER TO CENTER BAR SPACING LATERALLY IS AT LEAST 6 BAR DIAMETERS
  - B) CONCRETE COVER IS AT LEAST 2 BAR DIAMETERS, AND
  - C) EDGE DISTANCE TO THE FIRST BAR IN A LAYER IS AT LEAST 2.5 BAR DIAMETERS.
- 3. IF CONCRETE COVER OR EDGE DISTANCE IS LESS THAN 2 BAR DIAMETERS OR THE CENTER TO CENTER BAR SPACING LATERALLY IS LESS THAN 4 DIAMETERS, SEE ACI 318 FOR
- 4. TOP BARS ARE HORIZONTAL BARS AND BARS INCLINED LESS THAN 45 DEGREES WITH RESPECT TO A HORIZONTAL PLANE WHICH ARE PLACED SUCH THAT MORE THAN 12 INCHES OF CONCRETE IS CAST IN THE MEMBER BELOW THE BAR.
- 5. THE TABLES SHOWN ABOVE ARE FOR NORMAL WEIGHT CONCRETE AND UNCOATED REINFORCING BARS. IF LIGHTWEIGHT CONCRETE OR EPOXY COATED BARS ARE USED, SEE ACI 318 FOR FOR ADDITIONAL CONSIDERATIONS.

## **ABBREVIATIONS**

	U TERMITE CRICINO	
	= ALTERNATE SPACING	
	= AZIMUTH	
	= BASELINE = BOTTOM FACE	
	= BOTTOM LAYER	
-	_	
•	= CENTER = CATCH BASIN	
CB		
C.1.	= CAST IRON = CONSTRUCTION JOINT	
CJ	= CLEAR COVER	
CL OD F		
	= CENTER LINE = CORROSION RESISTANT STEEL	
	= DIAMETER	
Ø		
	= DROP INLET	
5	= DRAIN PIPE	
	= DWN STREAM	
D/S		
D.V.	= DRAIN VALVE MANIES E	
	= DRAIN VALVE MANHOLE = ELECTRICAL	
E	= EACH FACE	
EF		
EL.	= EQUALLY SPACED	
	= FIRE HYDRANT	
	= FAR FACE	
. ,	= FAR FACE = GAS	
G	= HIGH STRENGTH	
H.S. LP	= LIGHT POLE	
LS	= LIGHT STANDARD	
MH	= MANHOLE	
NF	= NEAR FACE	
0.C.	= ON CENTER	
	= OPTIONAL	
	= POWER	
P.C.	= POINT OF CURVATURE	
	= POINT OF TANGENCY	
P.T.		
S	= SEWER	
	= SPECIAL FABRICATED CONNECTION	
	= SUBBASELINE	
	= SEWER CLEANOUT	
STD. HK.		
STA.	= STATION	
T	= TELEPHONE	
TD	= TRENCH DRAIN	
TF	= TOP FACE	
TEL.M.H.	= TELEPHONE MANHOLE	
TL	= TOP LAYER	
TP	= TEST PILE	
U/S	= UP STREAM	
W	= WATER	



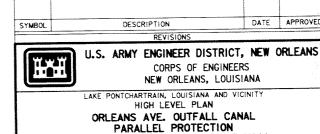
W/L

W.M.

W.V.

= WALL LINE = WATER METER

= WATER VALVE



PHASE II-A (EAST SIDE FLOODWALL)
STA. 3+60.00 E. B/L TO STA. 90+26.33 E. B/L

ORLEANS PARISH, LOUISIANA GENERAL NOTES

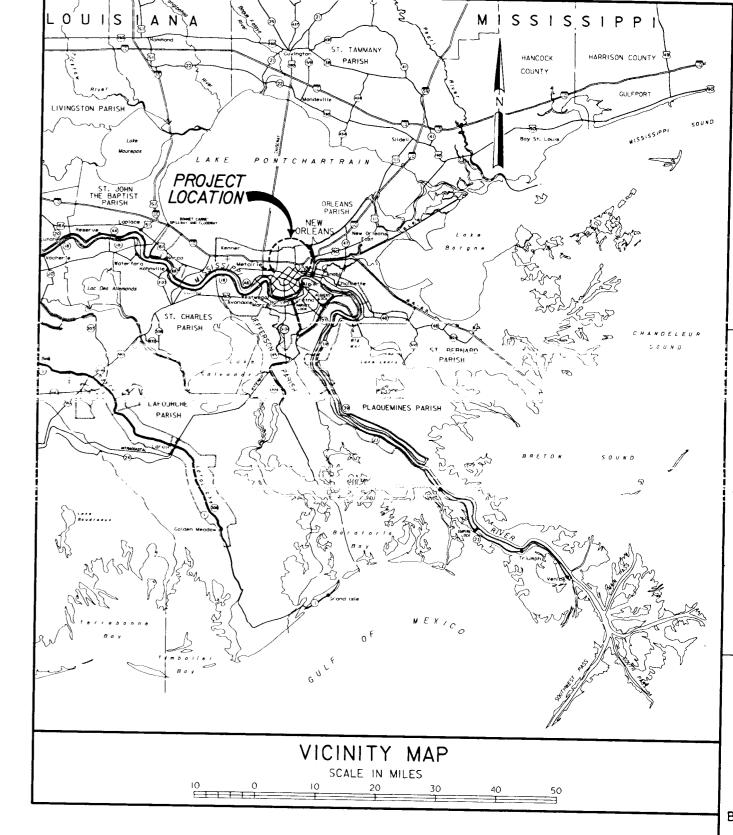
J. ROMERO DESIGN ENGINEER		SOLICITATION DACW29-	N NO. <b>96-B-0096</b>	<del></del>		 32
	M. DESAI B. DORCEY J. ROMERO	SEP. 96	1 1644A10.DGN	FILE	SEP. NO. 4-4	 ΔΔ
		DATE:	PLOT SCALE:	PLOT	DATE:	 



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THIS PROJECT WAS DESIGNED BY THE NEW ORLEANS DISTRICT OF THE U.S. ARMY CORPS OF ENGINEERS. THE INITIALS OR SIGNATURES AND REGISTRATION DESIGNATIONS OF INDIVIDUALS APPEARING ON THESE PROJECT DOCUMENTS ARE WITHIN THE SCOPE OF THEIR EMPLOYMENT AS REQUIRED BY ER 1110-1-8125.



1					
DWG.	TITIC		INDEX TO DRAWINGS		
D.1.0.	ITTLE	DWG.	TITLE	DWG.	TITLE
	INDEX, LOCATION AND VICINITY MAP	20	TYPICAL SECTIONS-STA. 103+25 W/L TO STA. 112+70 W/L		1116
2	GENERAL NOTES	21	TYPICAL SECTIONS-STA. 113+20 W/L TO STA. 126+32.36 W/L	A8	CROSS SECTIONS - STA. 53+00 E. B/L TO STA. 59+00 E. B/L
3	INDEX OF PLAN DRAWINGS	22	TYPICAL SECTIONS STA. 713+20 W/L 10 STA. 126+32.36 W/L	A9	CROSS SECTIONS - STA. 60+00 F. B/I TO STA 65+00 F. B/I
4	PLAN-STA. 0+00 W/L TO STA. 5+75 W/L	27	TYPICAL SECTIONS-STA. 200+00 W/L TO STA. 225+50 W/L	AIO	CROSS SECTIONS - STA. 66+00 E. B/L TO STA 72+00 F B/L
_ 5	PLAN-STA. 5+75 W/L TO 16+80 W/L	24	SHEET PILE AND MISCELLANEOUS DETAILS  I-WALL REINFORCING	All	CROSS SECTIONS - STA. 73+00 E. B/L TO STA. 79+00 E. B/L
6	PLAN-STA. 16+80 W/L TO 27+50 W/L	25		A12	CROSS SECTIONS - STA. 80+00 E. B/L TO STA. 86+00 E. B/L
	PLAN-STA. 27+50 W/L TO STA. 31+67.64 W/L	23	TYPICAL WALL JOINTS	AI3	CROSS SECTIONS - STA. 87+00 E. B/L TO STA. 90+00 E. B/L
1 '	STA. 100+00 W/L STA. 105+00 W/L	20	ARCHITECTURAL WALL TREATMENT		37 C. 67 E 10 STA. 90400 E. B/L
8	PLAN-STA. 105+00 W/L TO STA. 115+20 W/L	-21	SOIL BORINGS		
9	PLAN-STA. 115+20 W/L TO STA. 125+30 W/L	28	SOIL BORINGS	1	
	PLAN-STA. 125+30 W/L TO STA. 126+32.36 W/L	29	SOIL BORINGS		
10	STA. 200+00 W/L TO STA. 206+60 W/L	30	SOIL BORINGS	1	
11	PLAN-STA. 206+60 W/L TO STA. 219+30 W/L	1 31	SOIL BORINGS	1 -	
12	PLAN-STA. 219+30 W/L TO STA. 225+50 W/L	32	SOIL BORING LEGEND	1 [	TABULATION OF BENCH MARKS
13	PROFILE-STA. 0+00 W/L TO STA. 13+00 W/L	<del>↓</del>		1 }	
14	PROFILE-STA. 13+00 W/L TO STA. 27+00 W/L		ATTACHMENT DRAWINGS	1 [	DESIGNATION DESCRIPTION
<u> </u>	PROFILE STA. 13700 W/L 10 STA. 27400 W/L	ļ	ATTACHMENT DRAWINGS	1 1	
15	PROFILE-STA. 27+00 W/L TO STA. 31+67.64 W/L	Al	CROSS SECTIONS - STA. 4+00 E. B/L TO STA. 10+00 E. B/L	-	"CHRYSLER RM". AT NEW ORLEANS, ABOUT 0.45 MILES EAST ALONG
	STA. 100+00 W/L TO STA. 126+32.36 W/L	A2	CROSS SECTIONS - STA. 11+00 E. B/L TO STA. 17+00 E. B/L	1 1	LAKESMURE DRIVE FROM JUNCTION OF CANAL ROLLEVARD LOS VARDS
16	PROFILE-STA. 200+00 W/L TO STA. 225+50 W/L	A3	CROSS SECTIONS - STA. 18+00 E. B/L TO STA. 24+00 E. B/L	4	I NUKTHWEST OF THE NORTHWEST CORNER OF LAKESHORE DRIVE BRIDGE
17	PLAN AND SECTION VICINITY 30" WATER LINE	A4	CROSS SECTIONS - STA. 25+00 E. B/L TO STA. 31+00 E. B/L	1	OVER ORLEANS CANAL, SET IN THE TOP OF THE CONCRETE SEA WALL ALONG SHORE OF LAKE PONTCHARTRAIN, 33 FEET NORTHEAST OF THE
18	30" WATER LINE-SLURRY TRENCH AND SEEPAGE COLLAR DETAILS	A5	CROSS SECTIONS - STA. 32+00 E. B/L TO STA. 38+00 E. B/L	4	NORTH ONE OF A CROUP OF PALM TREES, 66 FEET WEST OF CHRYSLE
1 19	TYPICAL SECTIONS-STA. 0+00 W/L TO STA. 31+67.64 W/L	A6	CROSS SECTIONS - STA. 39+00 E. B/L TO STA. 38+00 E. B/L	1 1	STATION DESCRIBED, 270 FEET NORTH OF THE CENTER LINE OF
1		1	1 511000 DECTIONS DIA. JUTOU F. HALLON C. DA		I AMERICAN DELICATION OF THE OUT OF

CROSS SECTIONS - STA. 4+00 E. B/L TO STA. 10+00 E. B/L CROSS SECTIONS - STA. 11+00 E. B/L TO STA. 17+00 E. B/L CROSS SECTIONS - STA. 18+00 E. B/L TO STA. 24+00 E. B/L CROSS SECTIONS - STA. 25+00 E. B/L TO STA. 31+00 E. B/L CROSS SECTIONS - STA. 32+00 E. B/L TO STA. 38+00 E. B/L CROSS SECTIONS - STA. 39+00 E. B/L TO STA. 45+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E.

T	ABULATION OF BENCH MARKS	
DESIGNATION	DESCRIPTION	ELEVATION
"CHRYSLER RM".	AT NEW ORLEANS, ABOUT 0.45 MILES EAST ALONG LAKESHORE DRIVE FROM JUNCTION OF CANAL BOULEVARD, 125 YARDS NORTHWEST OF THE NORTHWEST CORNER OF LAKESHORE DRIVE BRIDGE OVER ORLEANS CANAL, SET IN THE TOP OF THE CONCRETE SEA WALL ALONG SHORE OF LAKE PONTCHARTRAIN, 33 FEET NORTHEAST OF THE NORTH ONE OF A GROUP OF PALM TREES, 66 FEET WEST OF CHRYSLER STATION DESCRIBED, 270 FEET NORTH OF THE CENTER LINE OF LAKESHORE DRIVE AND ABOUT 2 FEET ABOVE THE LEVEL OF THE	EL. 7.11 N.G.V.D. (1983 EPOCH)

		1	
SYMBOL	DESCRIPTION	DATE	APPROVED
	REVISIONS		
HAH	U.S. ARMY ENGINEER DISTRICT, CORPS OF ENGINEER NEW ORLEANS, LOUISI	S Ana	ORLEANS
	LAKE PONTCHARTRAIN, LOUISIANA AND VIC HIGH LEVEL PLAN ORLEANS AVE. OUTFALL CAN PARALLEL PROTECTION		

PARALLEL PROTECTION
PHASE II-A (EAST SIDE FLOODWALL)
STA. 3+60.00 E. B/L TO STA. 90+26.33 E. B/L
ORLEANS PARISH, LOUISIANA
INDEX, LOCATION. AND VICINITY MAP

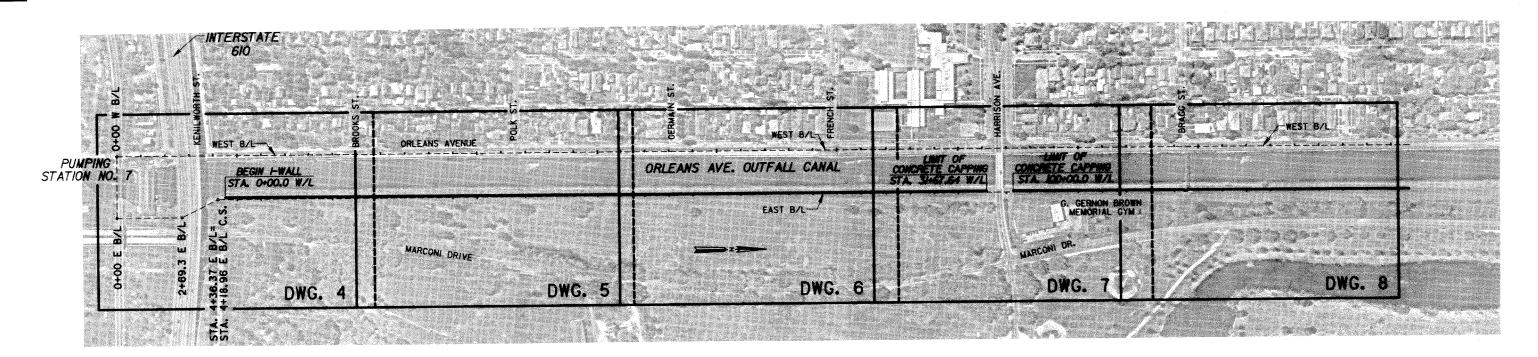
INDEA, LUCA	ATTON, AND VIC	INLLT	MAP
ESIGNED BY: M. DESAI	SOLICITATION NO.	CADD FILE: 4	4644A08.DGN
RAWN BY: B. DORCEY	DACW29-96-B-0096	PLOT DATE:	PLOT SCALE:
HECKED BY: J. ROMERO	APPROVED BY	SEP. 96	4800
ATE: SEP. 96	CHIEF, ENGINEERING QVISION	FILE NO.	14044
JBMTTTED BY: "	APPROYED BY	H-4-4	14644

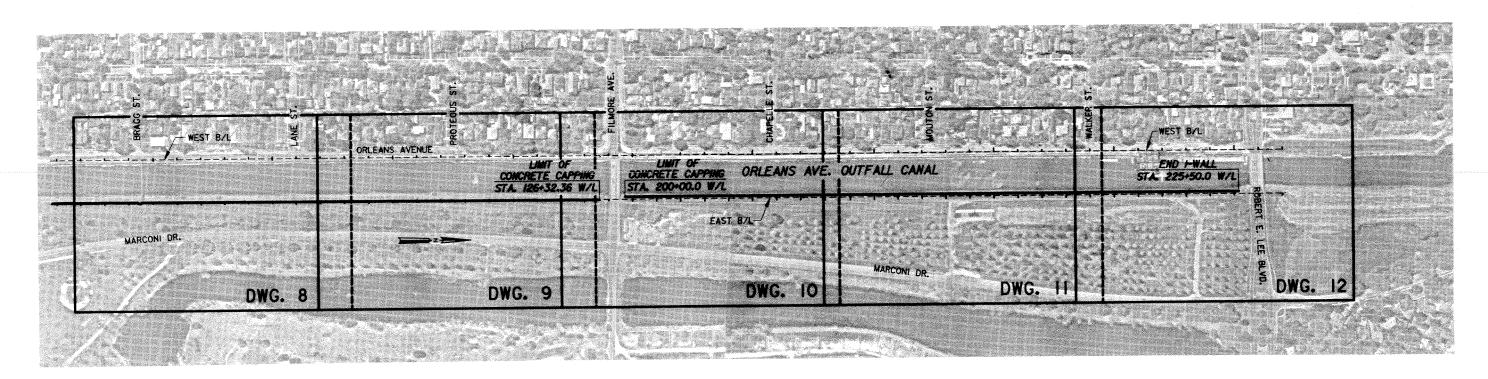
TYPICAL SECTIONS-STA. 0+00 W/L TO STA. 31+67.64 W/L
STA. 100+00 W/L TO STA. 102+75 W/L

CROSS SECTIONS - STA. 46+00 E. B/L TO STA. 52+00 E. B/L

Safety is a Part of Your Contract

5





PLAN

SCALE: I'' = 200'

200' 0 200' 400' 600' 80



SYMBOL DESCRIPTION DATE APPROVED
REVISIONS

HAH

U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS

CORPS OF ENGINEERS

NEW ORLEANS, LOUISIANA

NEW ORLEANS, LOUISIANA

LAKE PONTCHARTRAIN, LOUISIANA AND VICINITY
HIGH LEVEL PLAN

ORLEANS AVE. OUTFALL CANAL
PARALLEL PROTECTION
PHASE II-A (EAST SIDE FLOODWALL)

STA. 3+60.00 E. B/L TO STA. 90+26.33 E. B/L
ORLEANS PARISH, LOUISIANA

### INDEX OF PLAN DRAWINGS

DESIGNED BY: J. ROMERO DRAWN BY: B. DORCEY CHECKED BY: J. ROMERO	SEP. 96	<b>2400</b> 644A14.DGN	FILE NO.	. 96 AAG	:44
SUBMITTED BY:  J. ROMERO  DESIGN ENGINEER	SOLICITATION	N NO. 96-B-0096			32

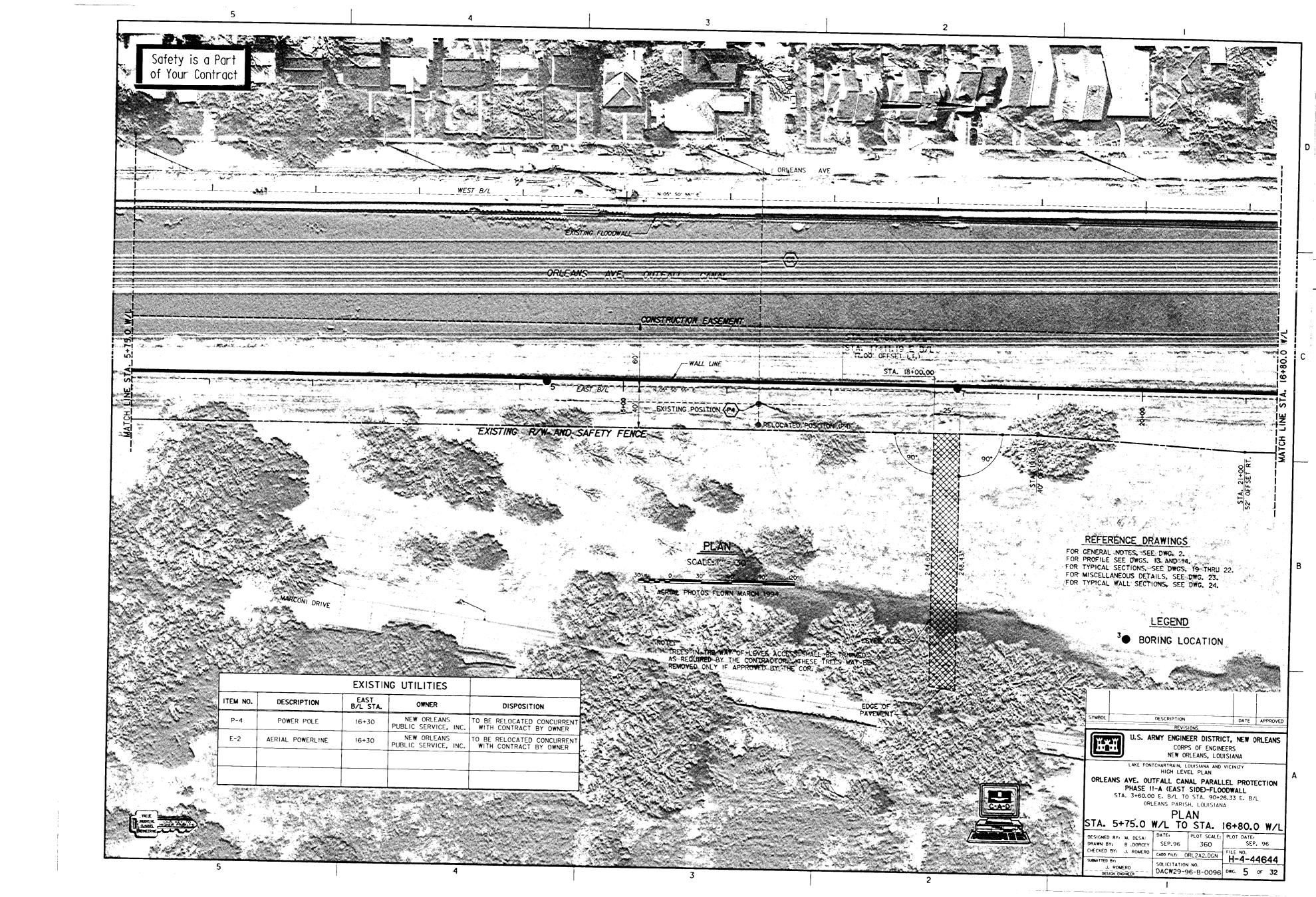


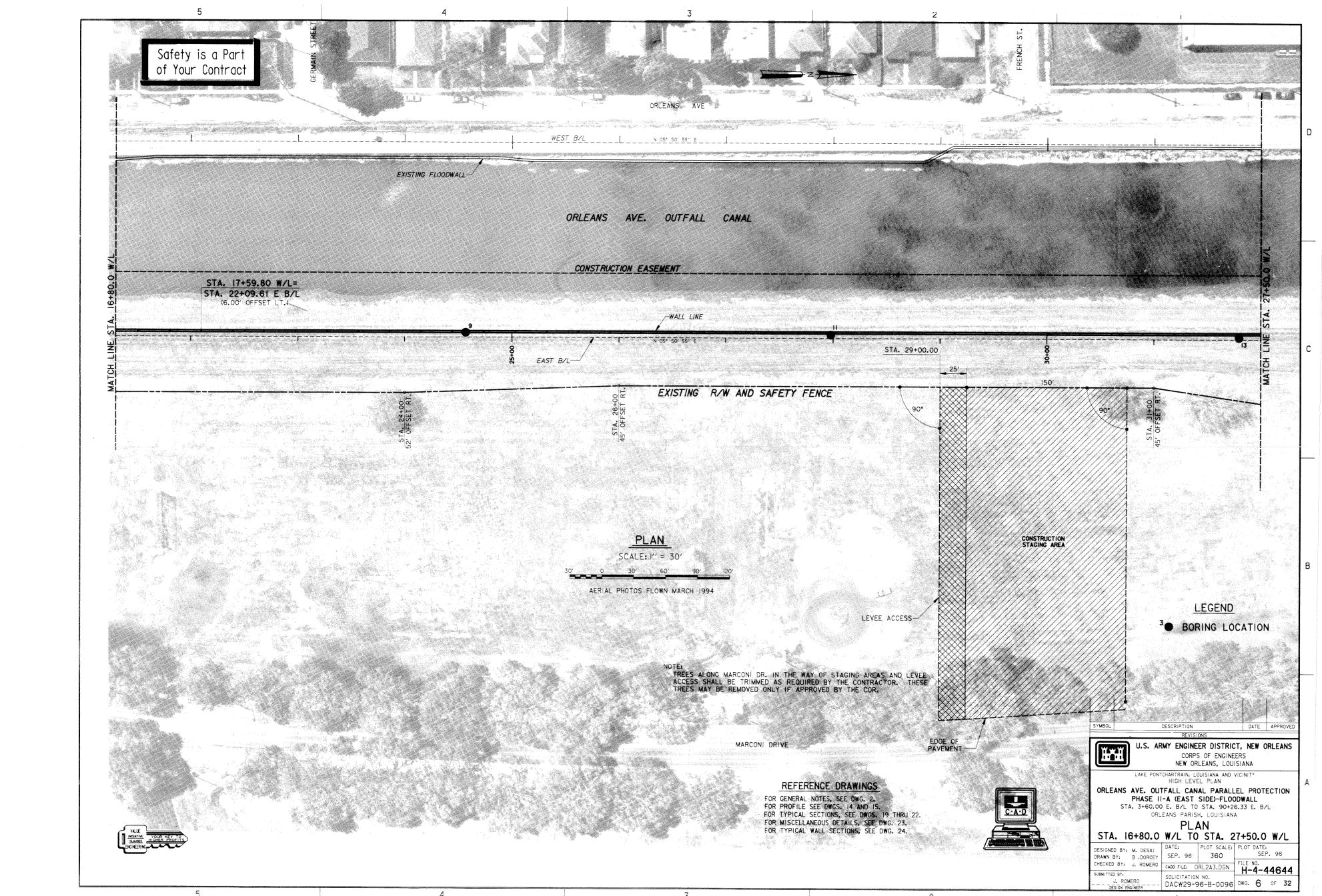
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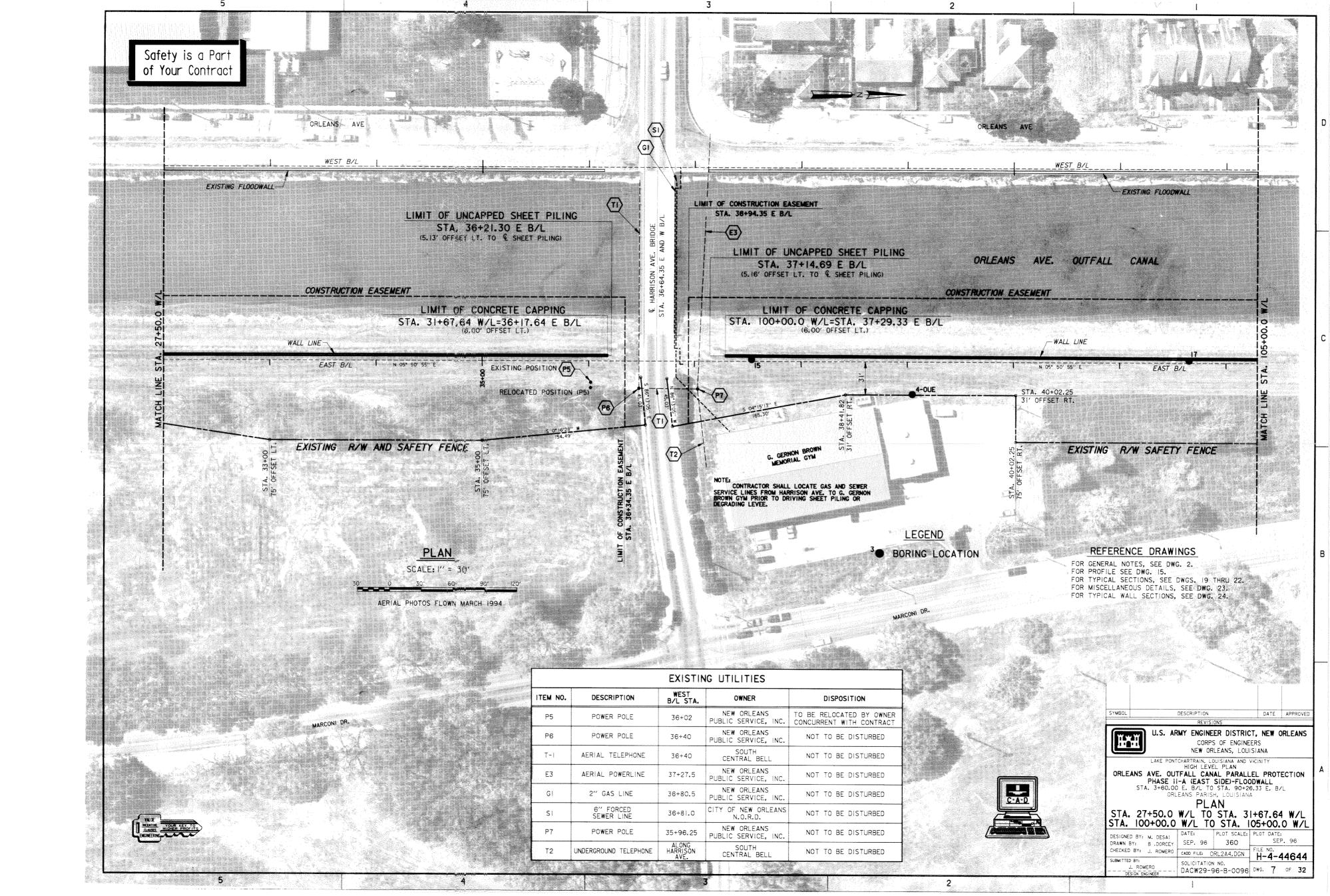
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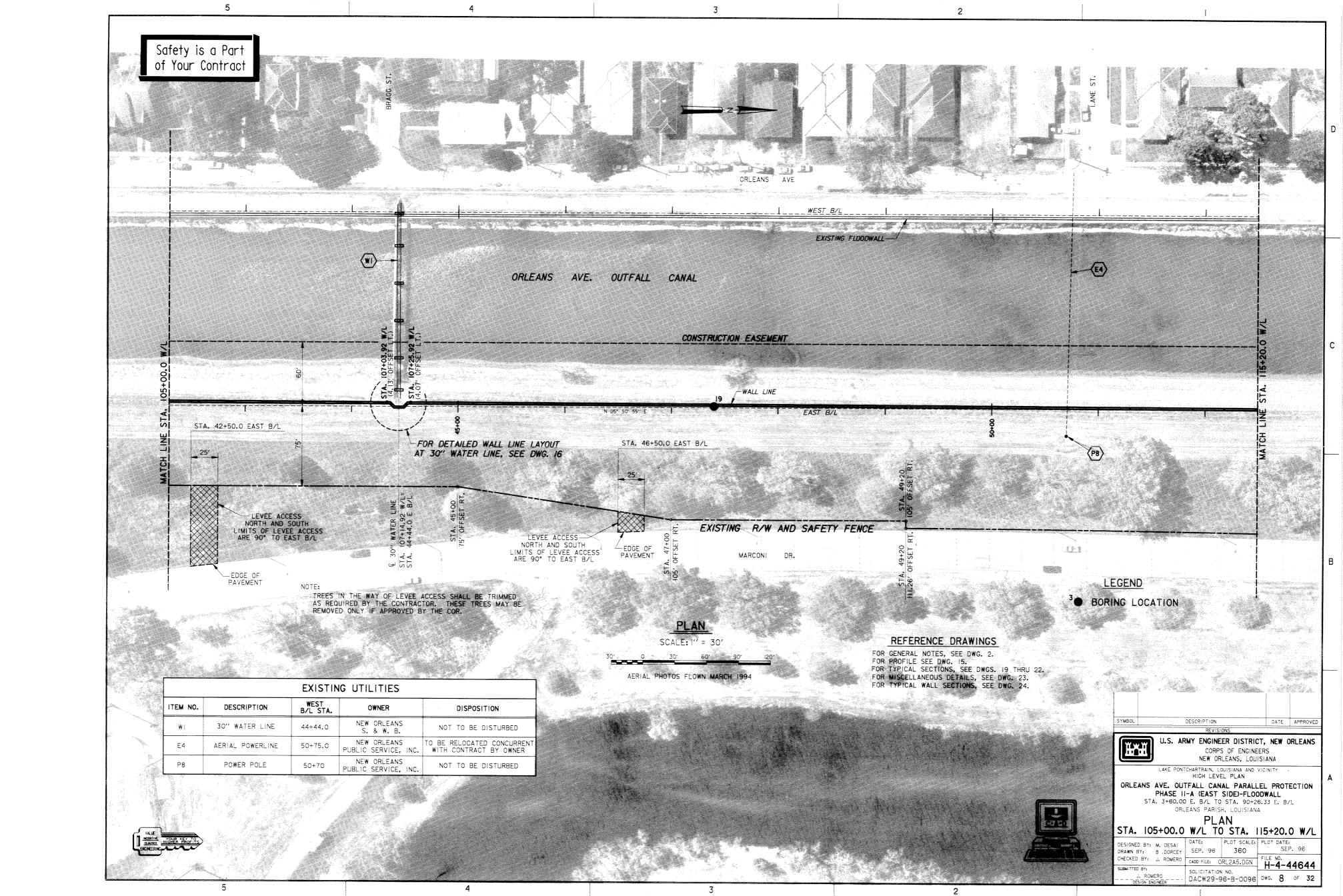
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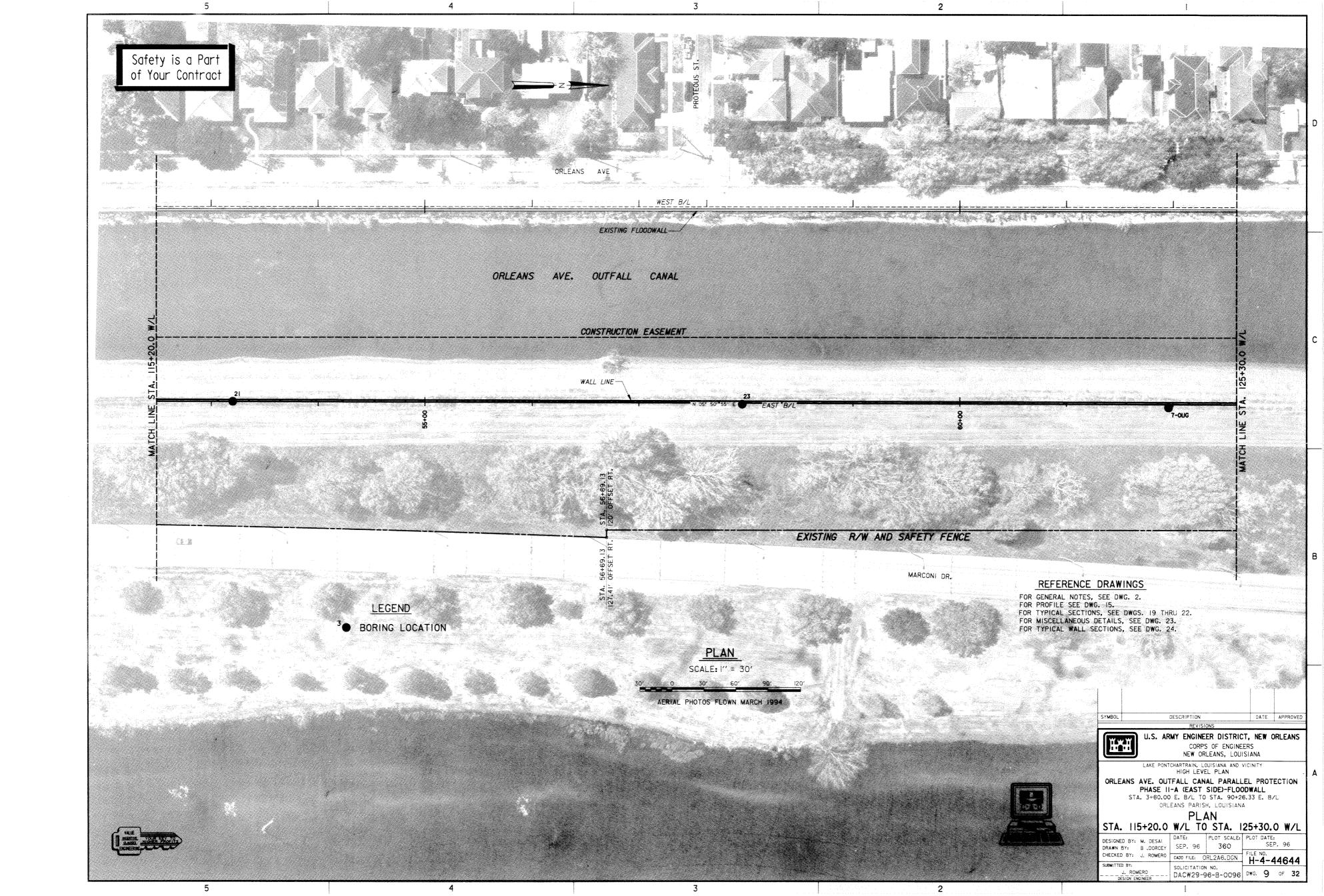


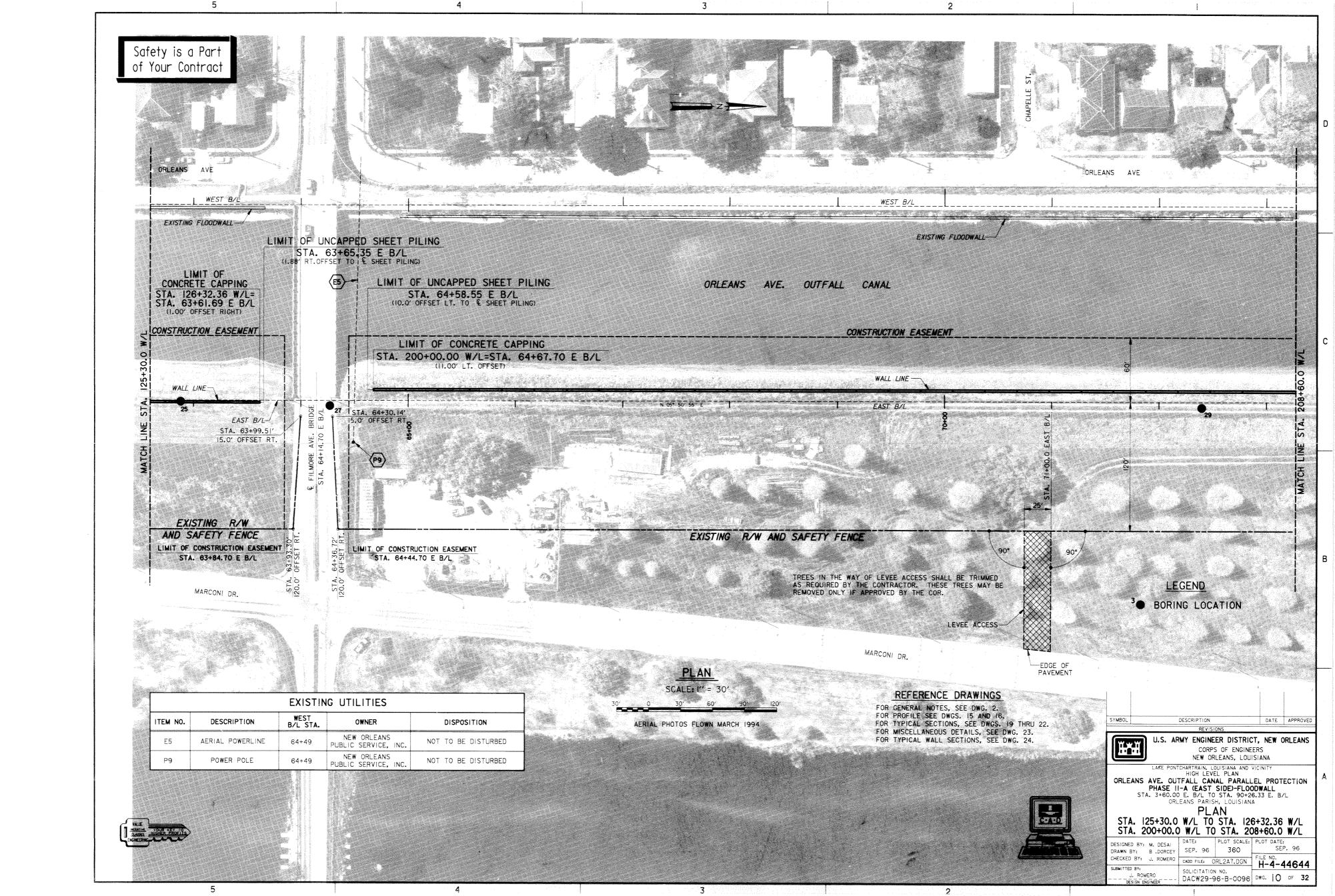




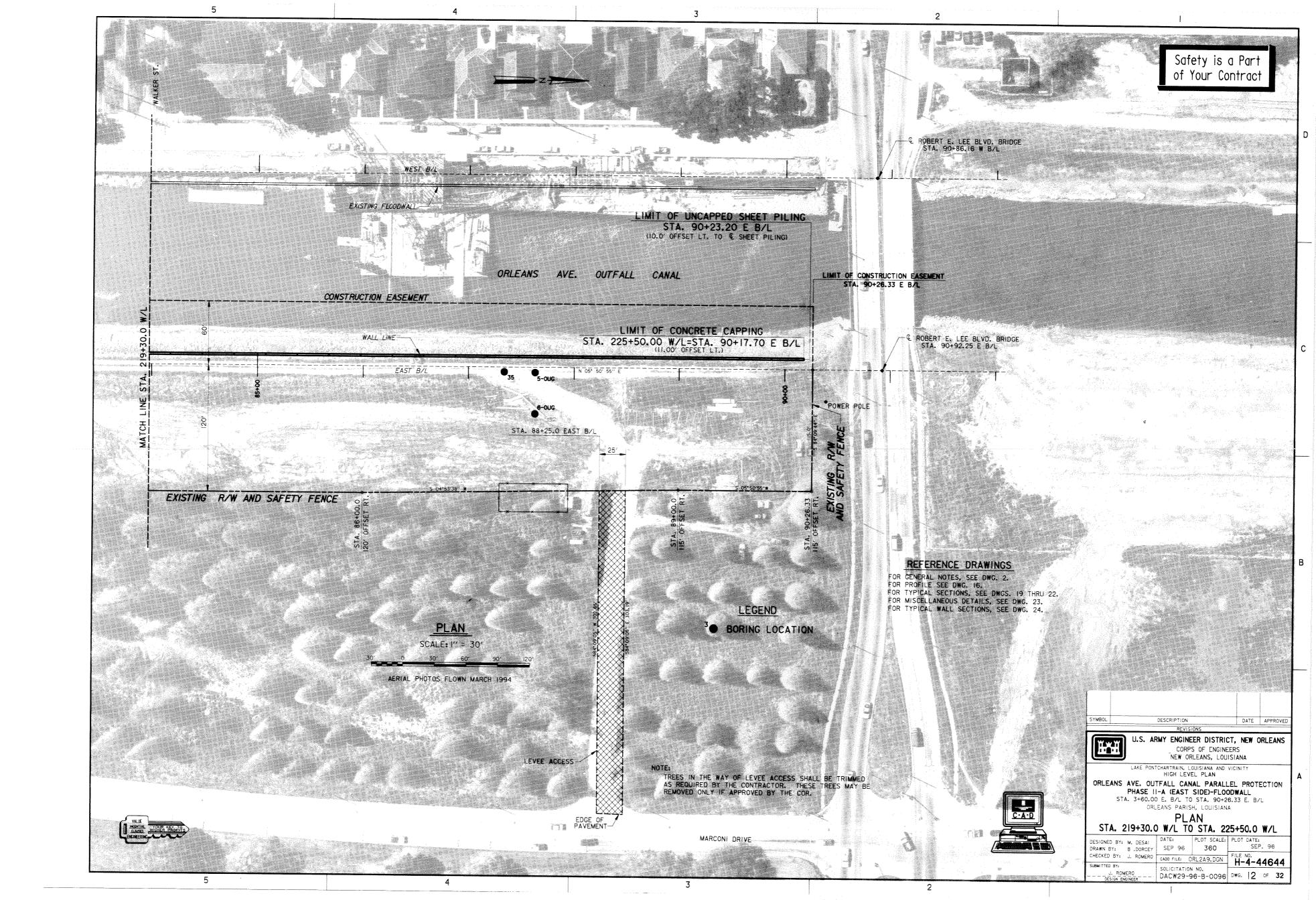


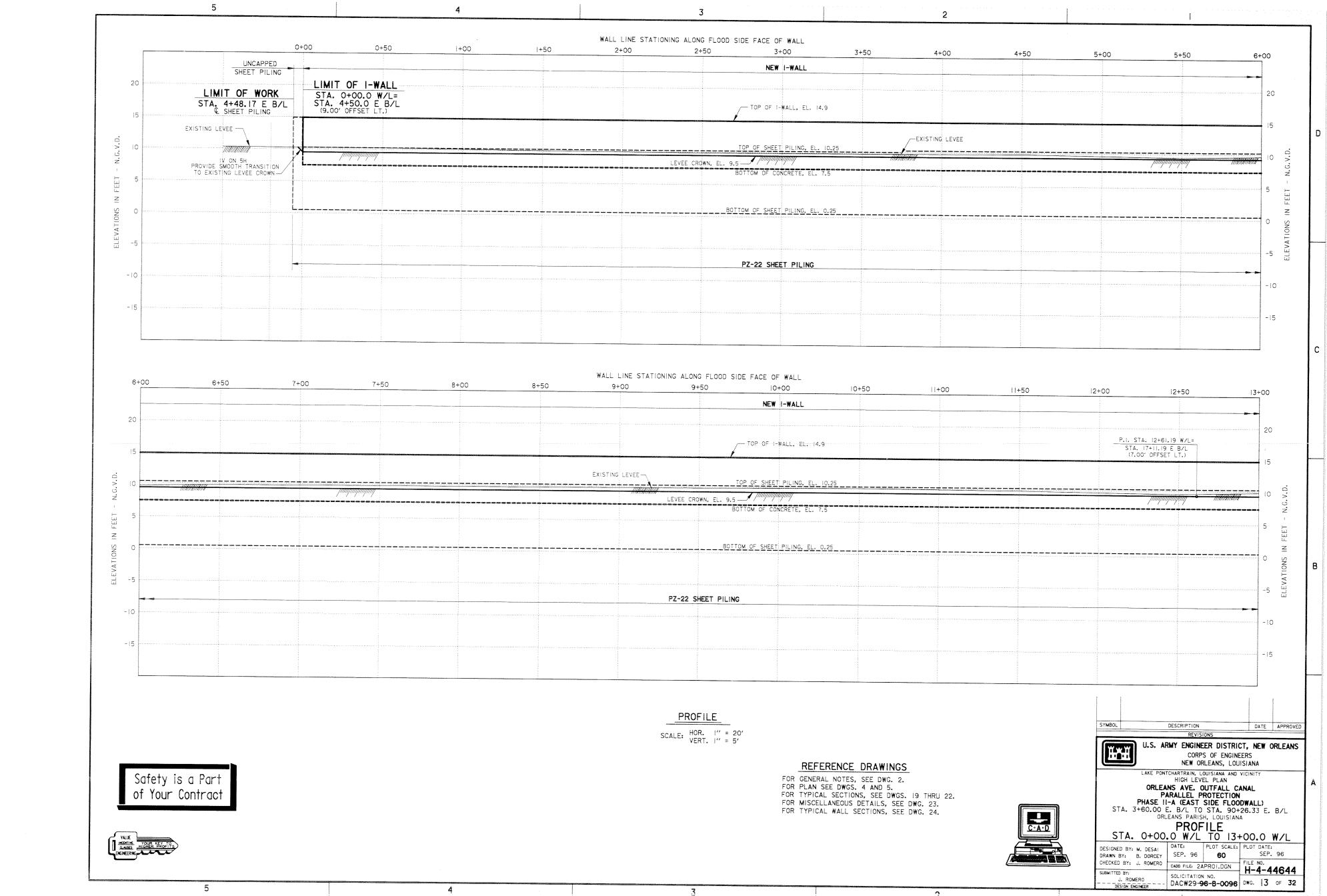


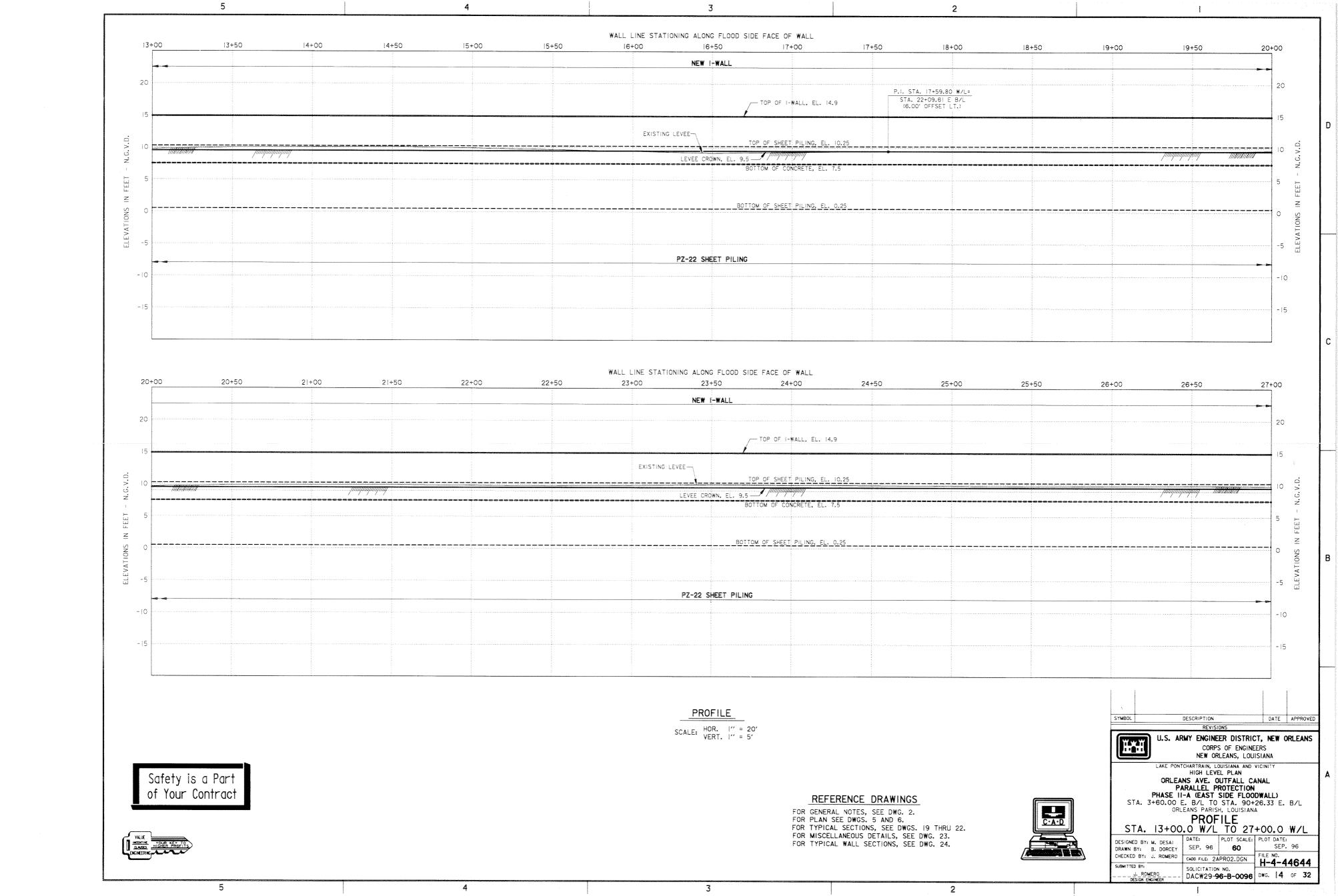


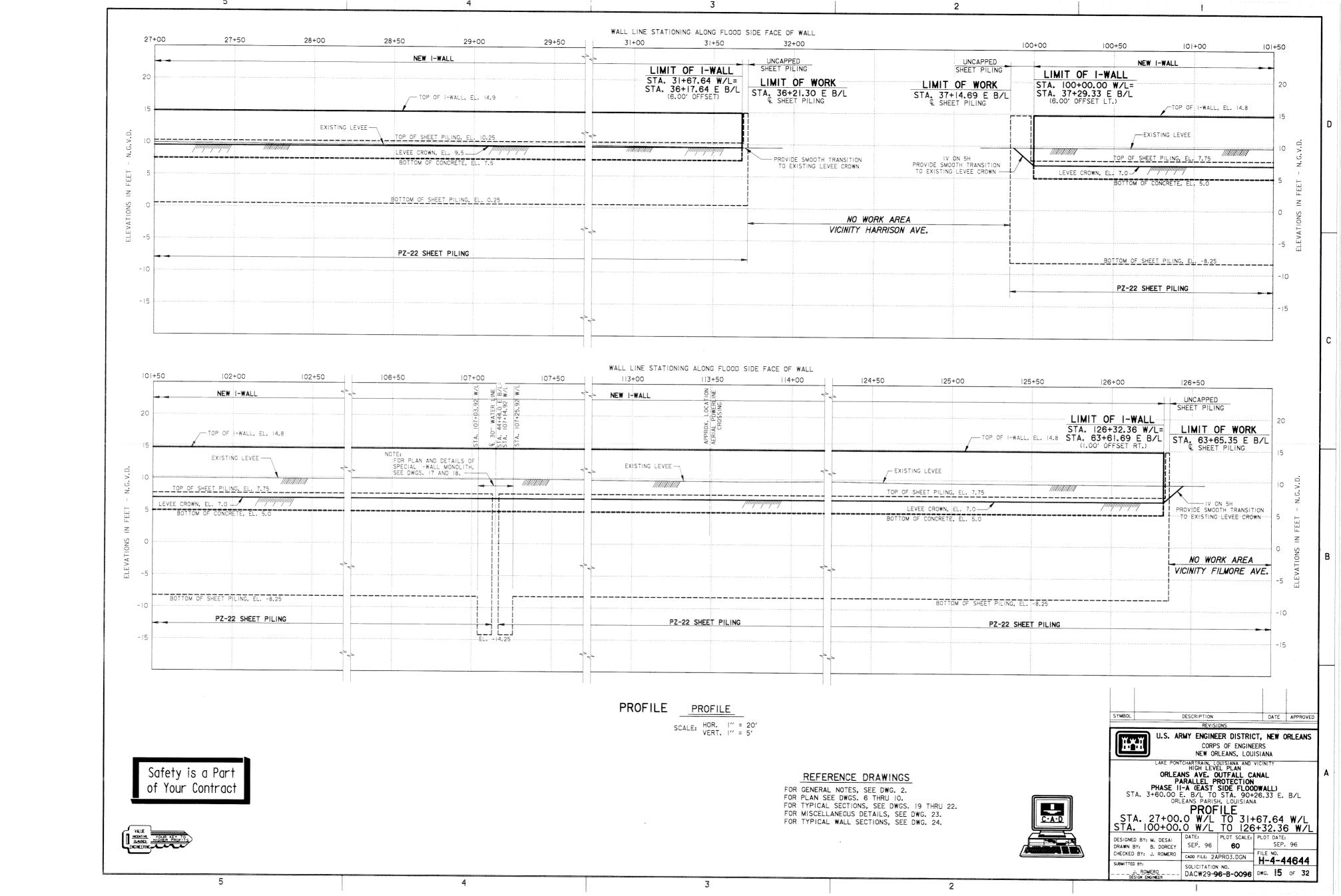


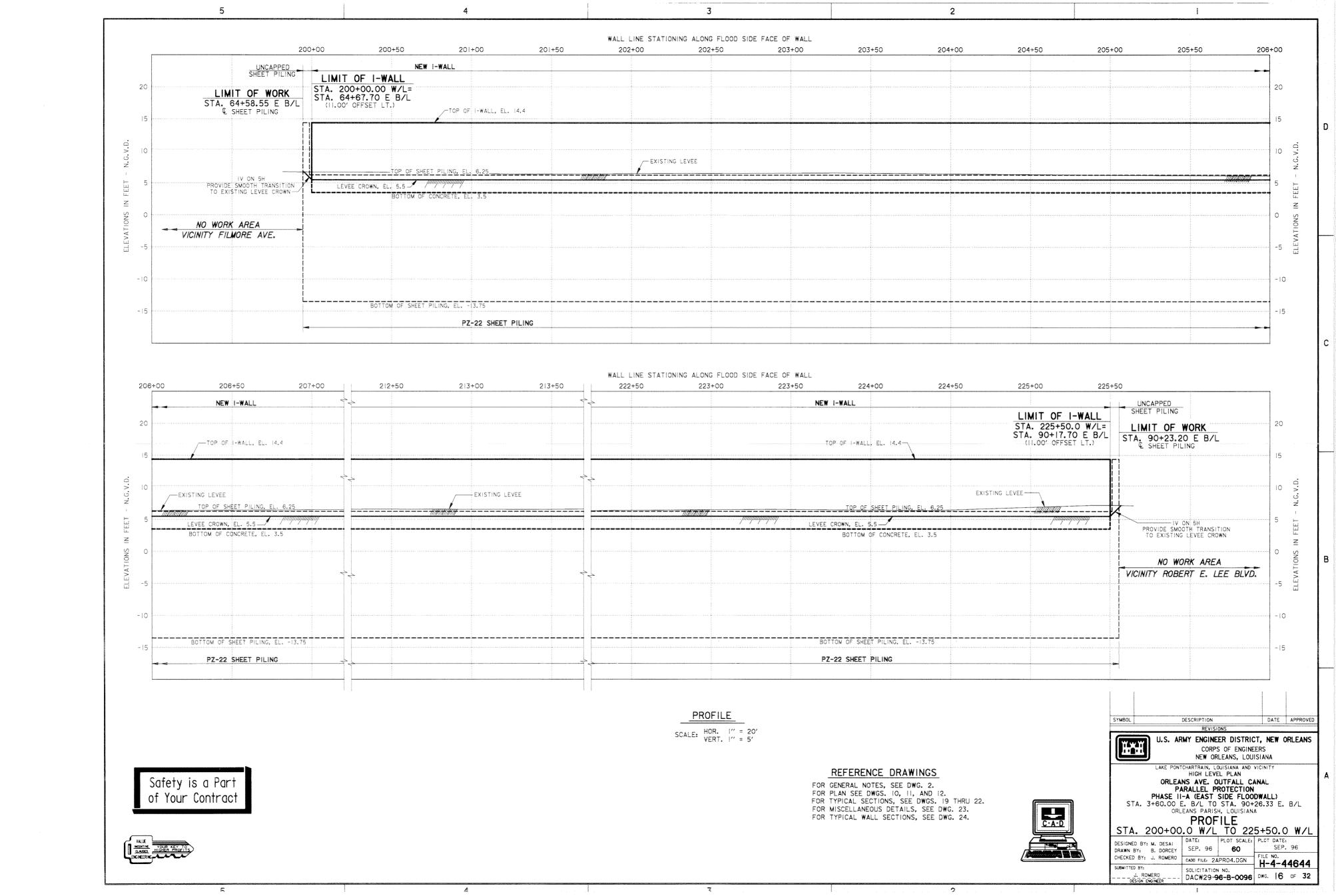


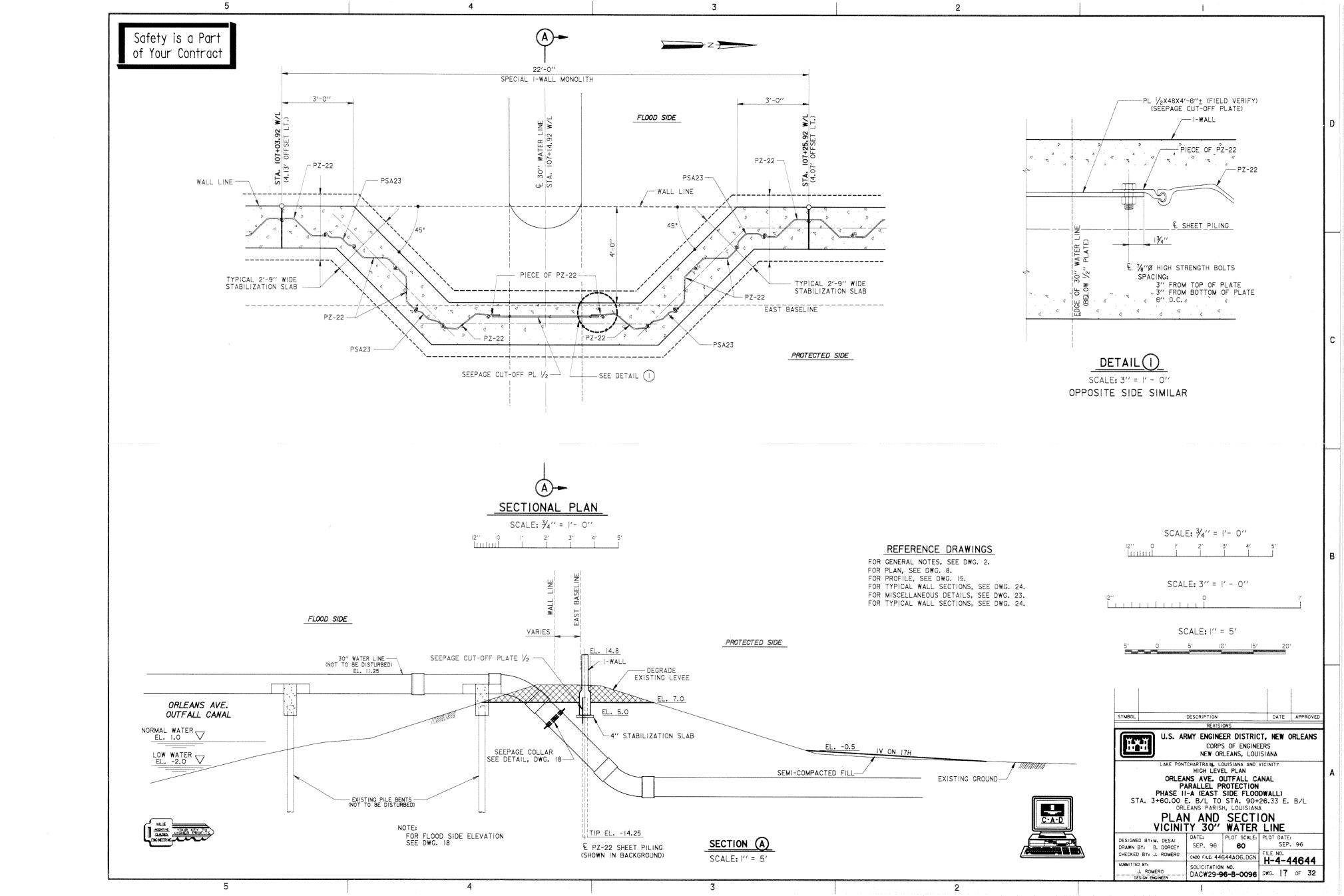


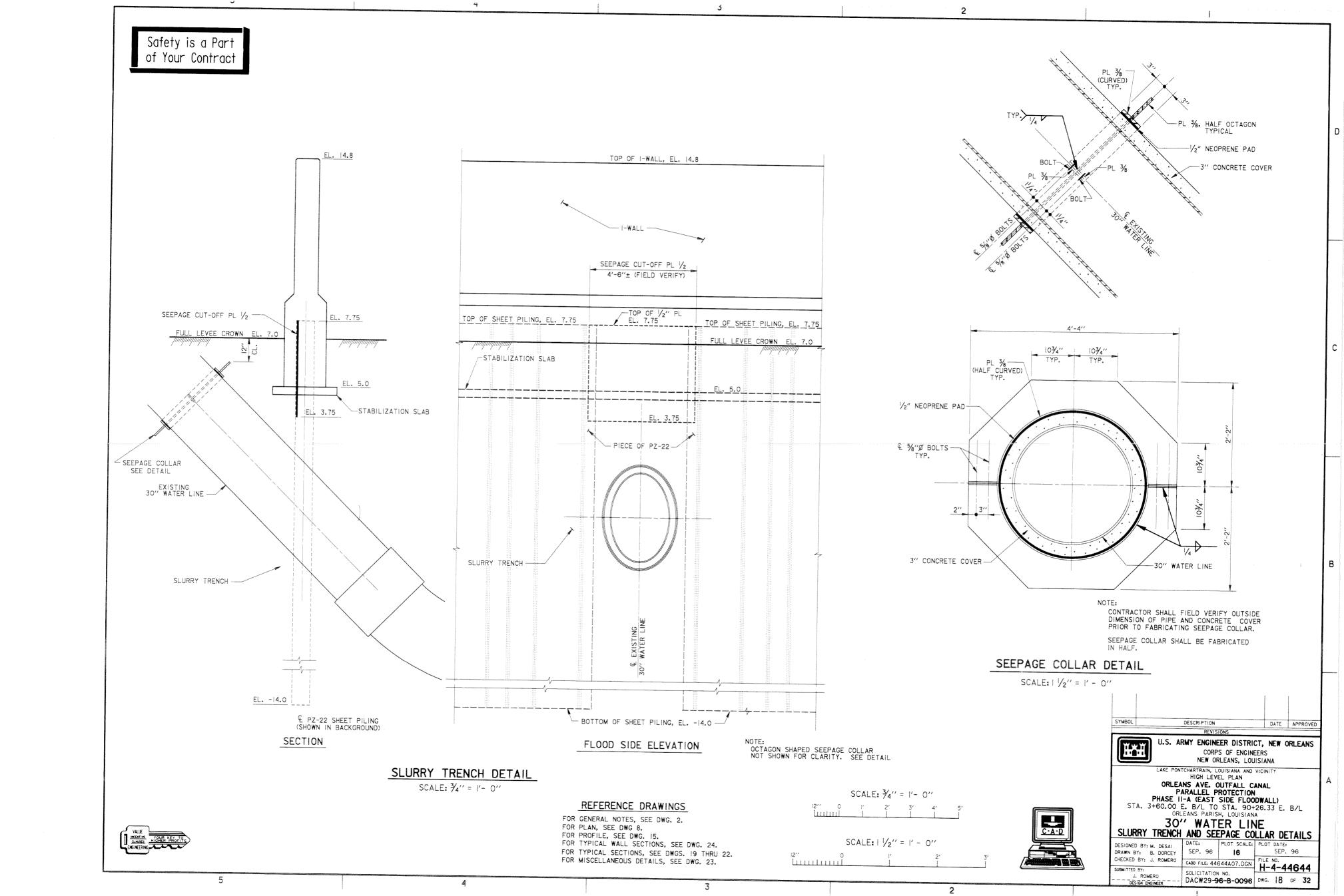


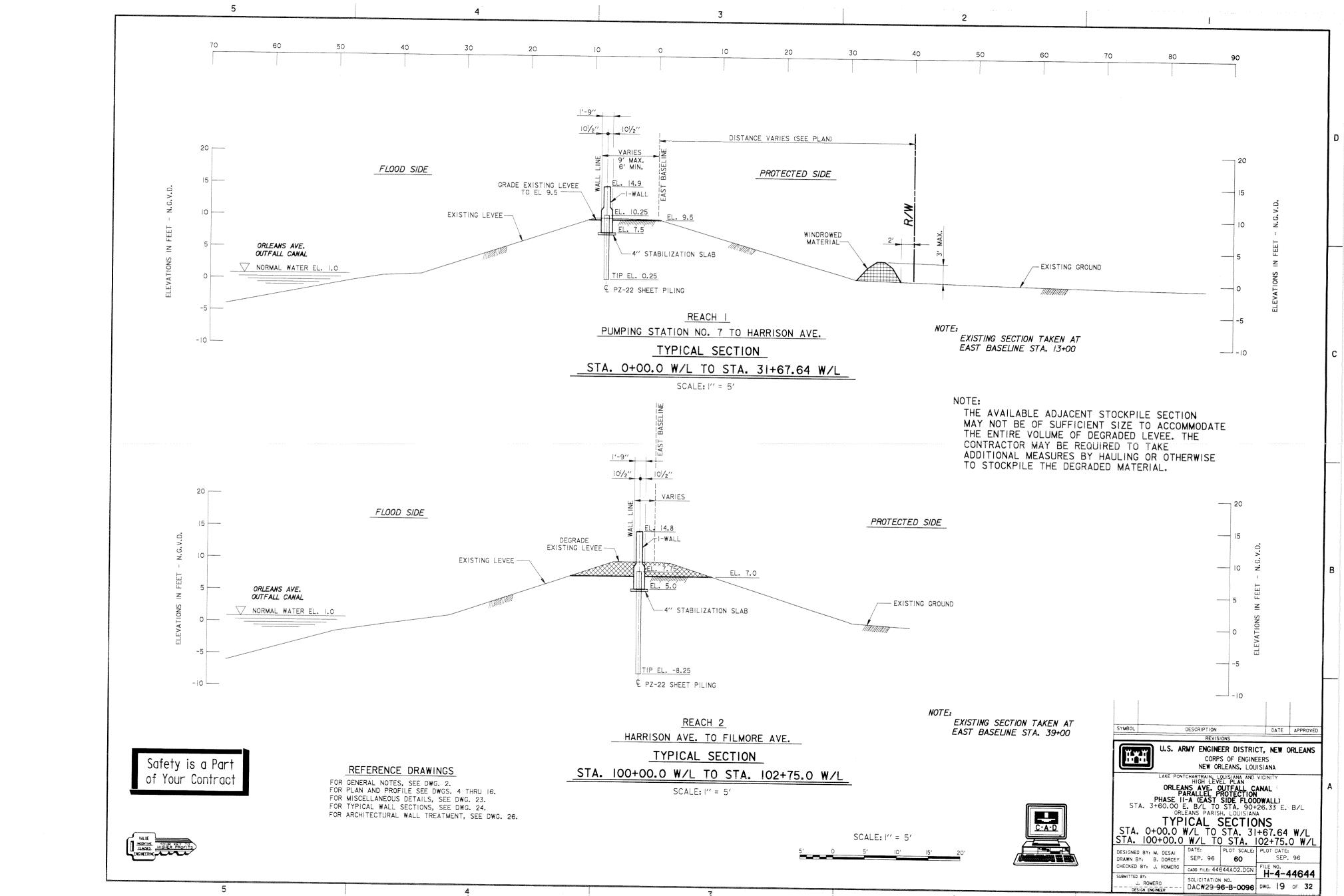


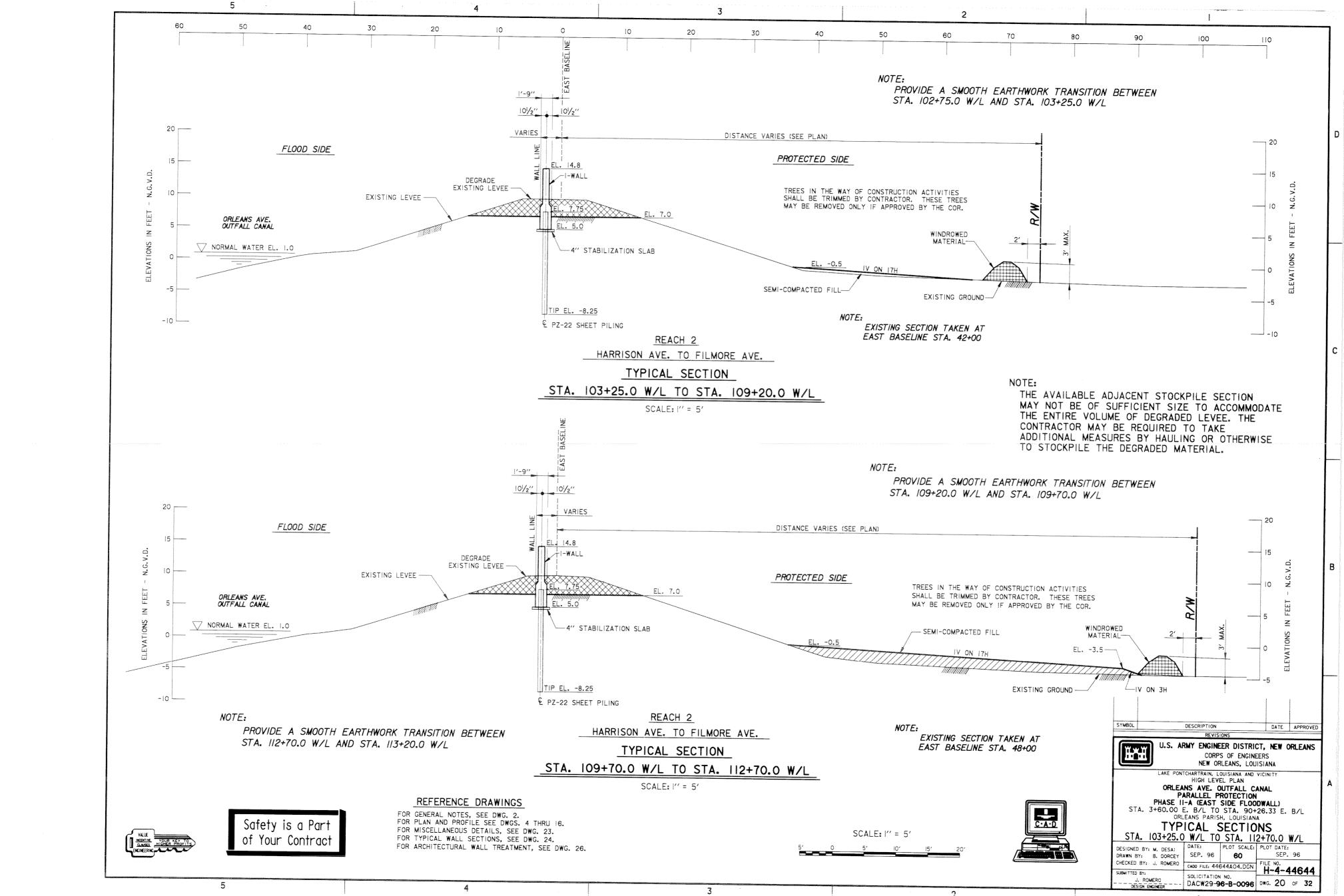


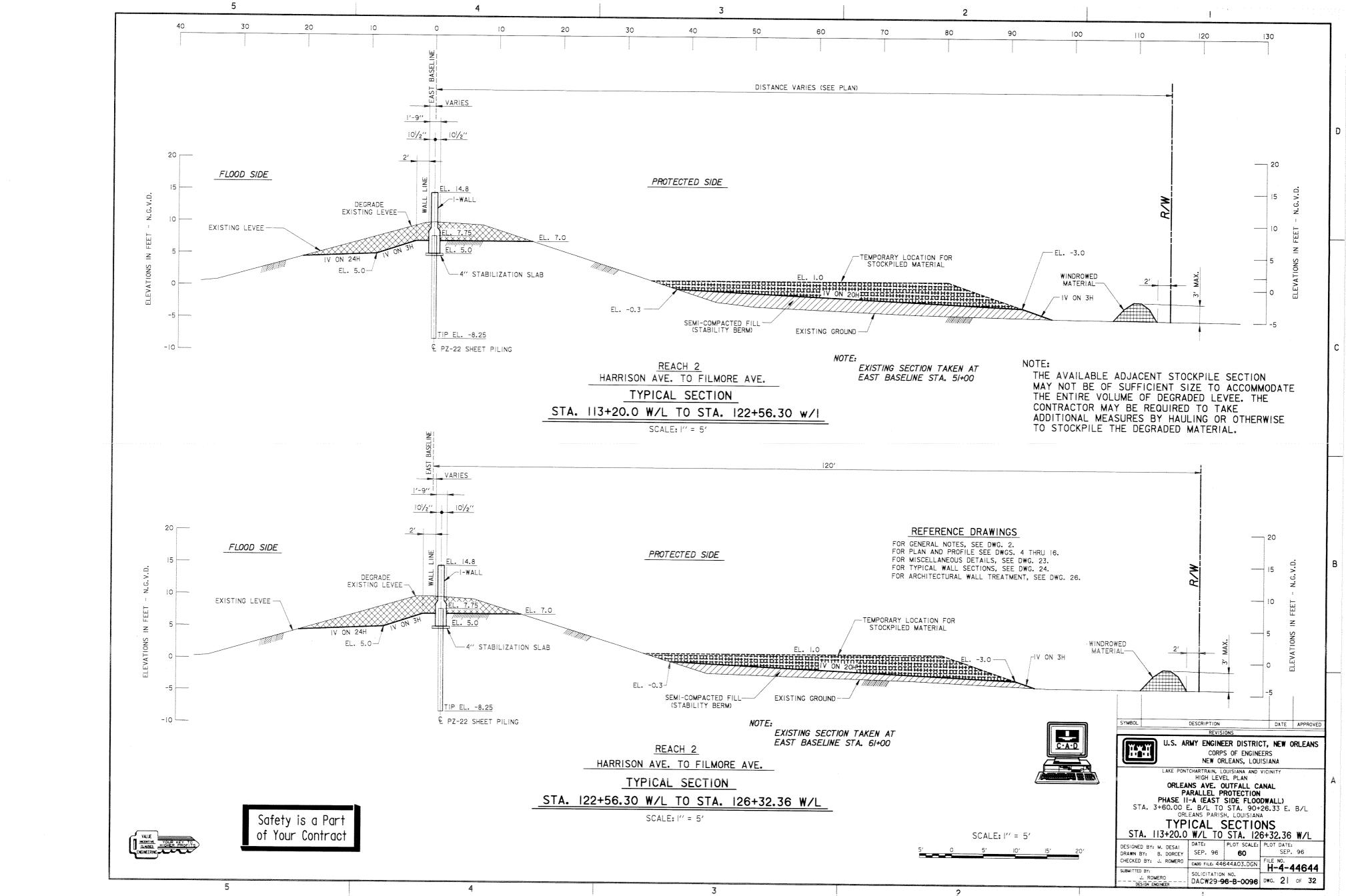


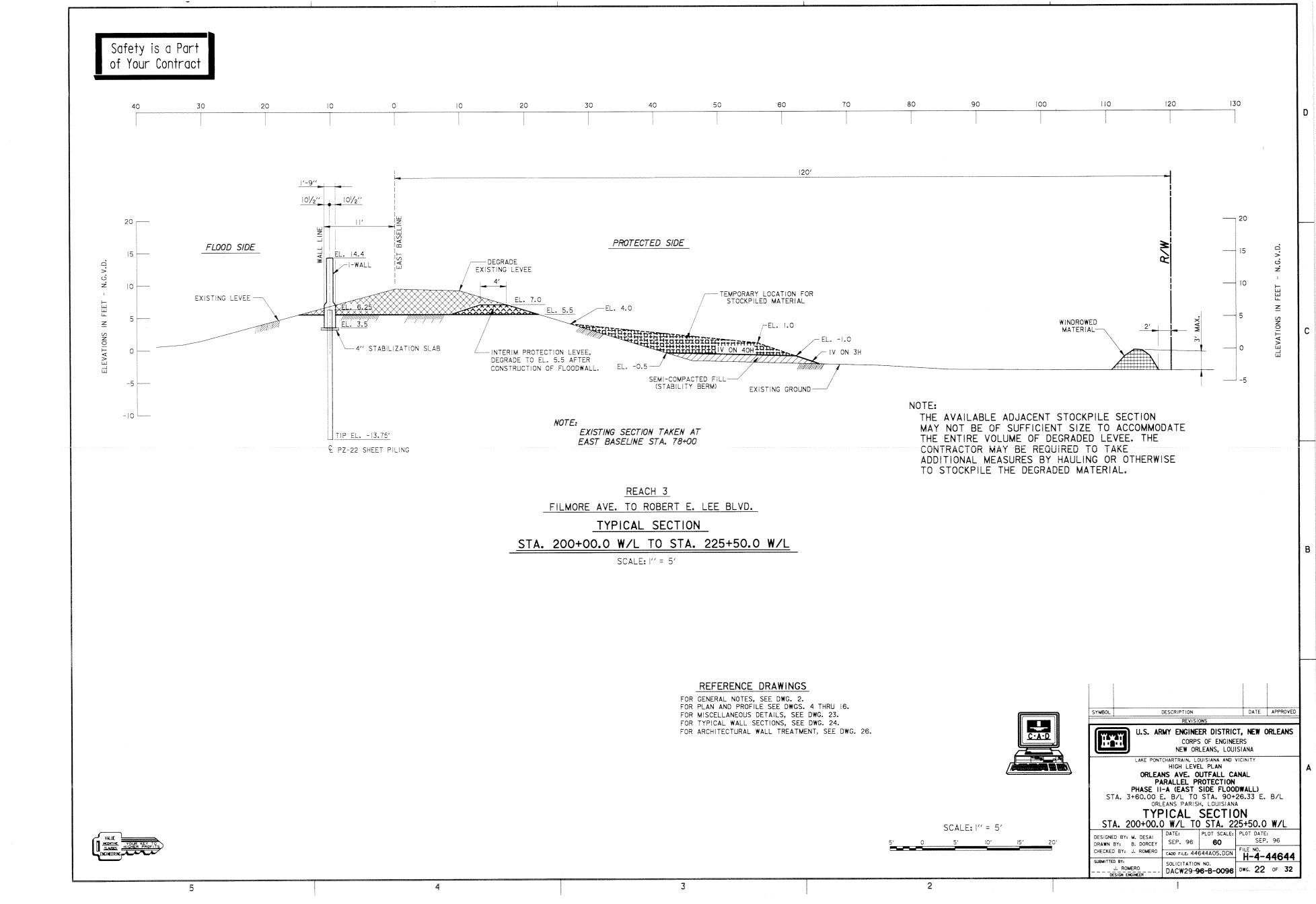


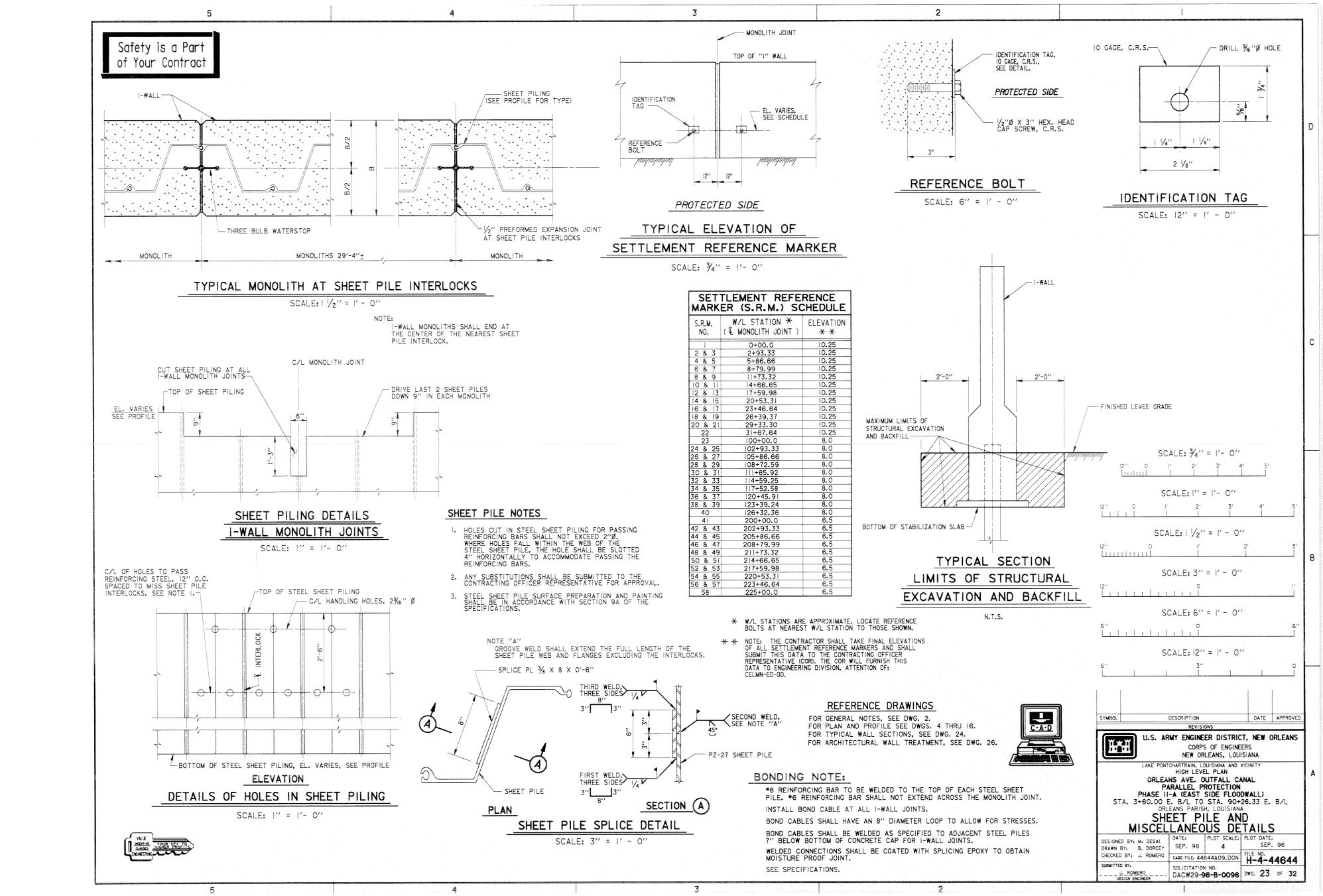




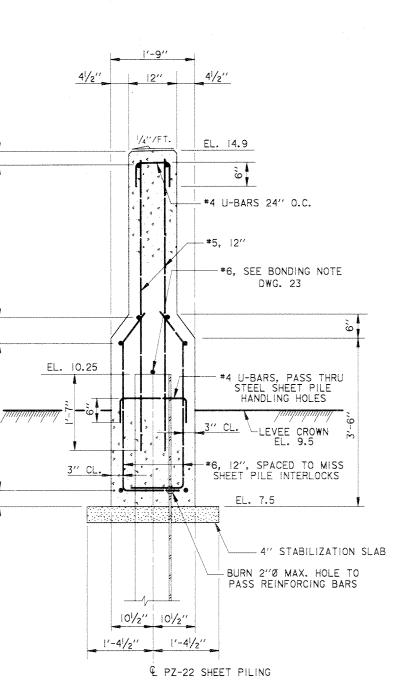






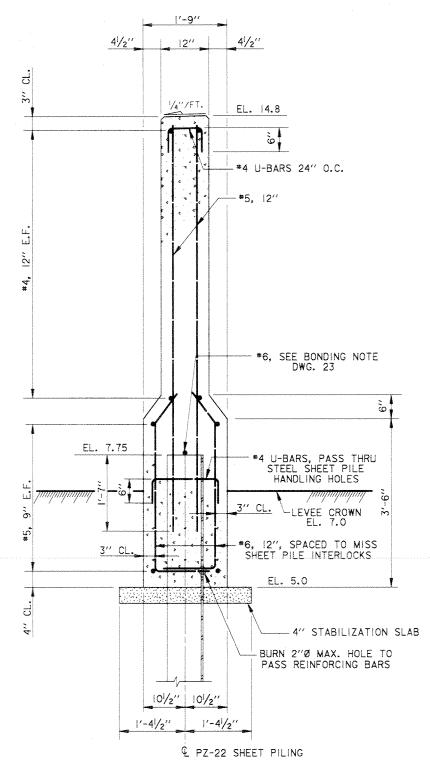


5 Safety is a Part of Your Contract



REACH I PUMPING STATION NO. 7 TO HARRISON AVE. STA, 0+00.0 W/L TO STA. 31+67.64 W/L

SCALE: |" = |- 0"

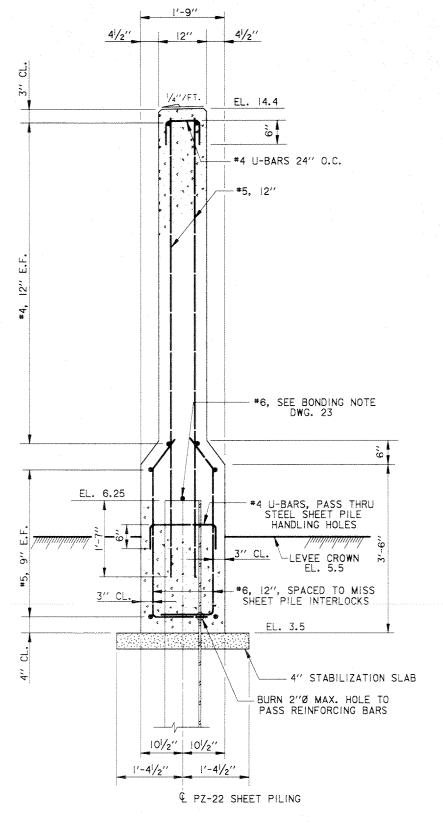


3

REACH 2 HARRISON AVE. TO FILMORE AVE.

STA, 100+00.0 W/L TO STA. 126+32.36 W/L

SCALE: |" = |- 0"



REACH 3 FILMORE AVE. TO ROBERT E. LEE BLVD. STA, 200+00.0 W/L TO STA. 225+50.0 W/L

SCALE: |" = |'- 0"



U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS

CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA

NEW ORLEANS, LOUISIANA

LAKE PONTCHARTRAIN, LOUISIANA AND VICINITY
HIGH LEVEL PLAN

ORLEANS AVE. OUTFALL CANAL
PARALLEL PROTECTION
PHASE II-A (EAST SIDE FLOODWALL)

STA. 3+60.00 E. B/L TO STA. 90+26.33 E. B/L
ORLEANS PARISH, LOUISIANA

I-WALL REINFORCING

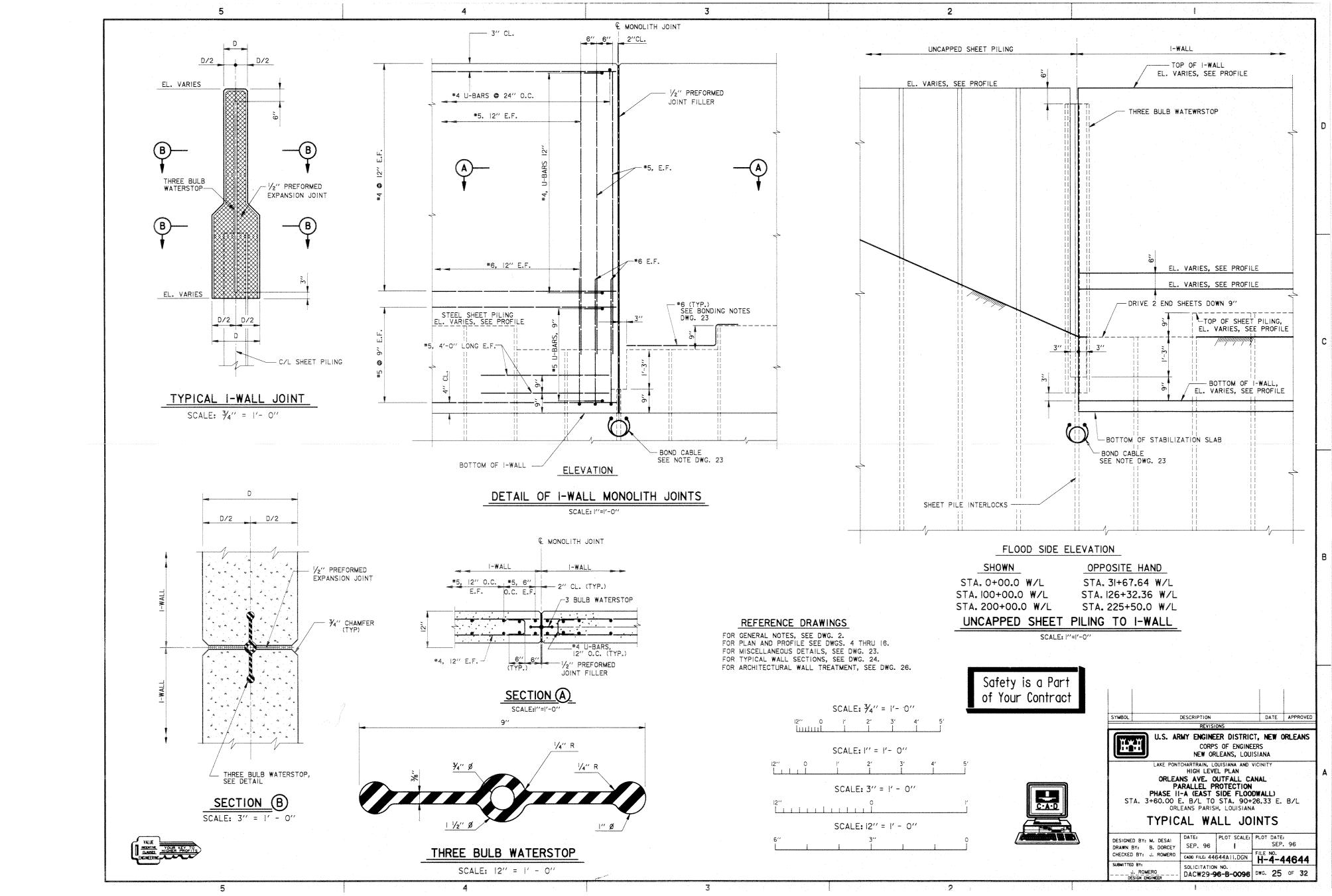
DESIGNED BY: M. DESAI DRAWN BY: B. DORCEY CHECKED BY: J. ROMERO SEP. 96 12 CADD FILE: 44644A01.DGN H-4-44644 SOLICITATION NO. DACW29-96-B-0096 DWG. 24 OF 32

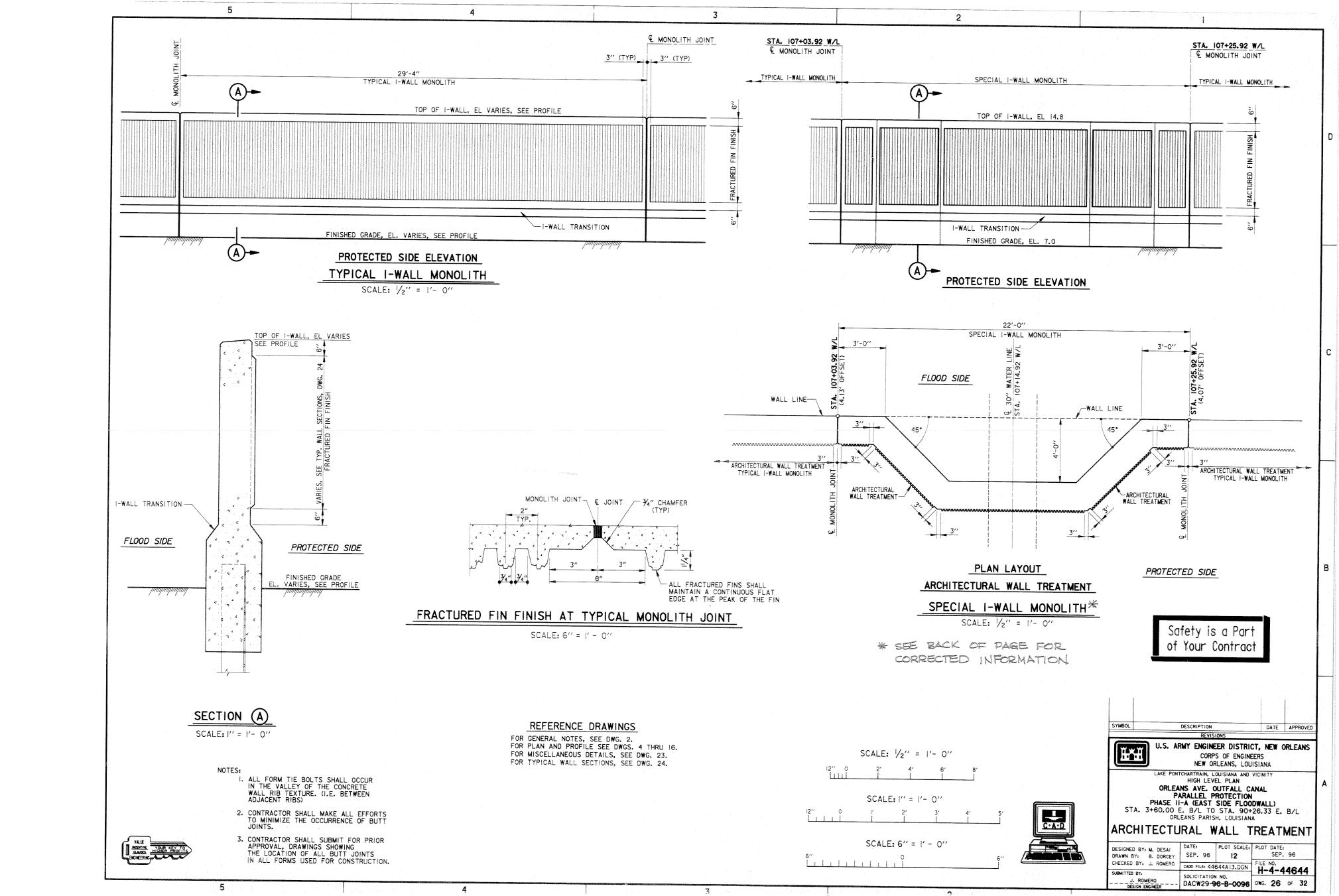
REFERENCE DRAWINGS

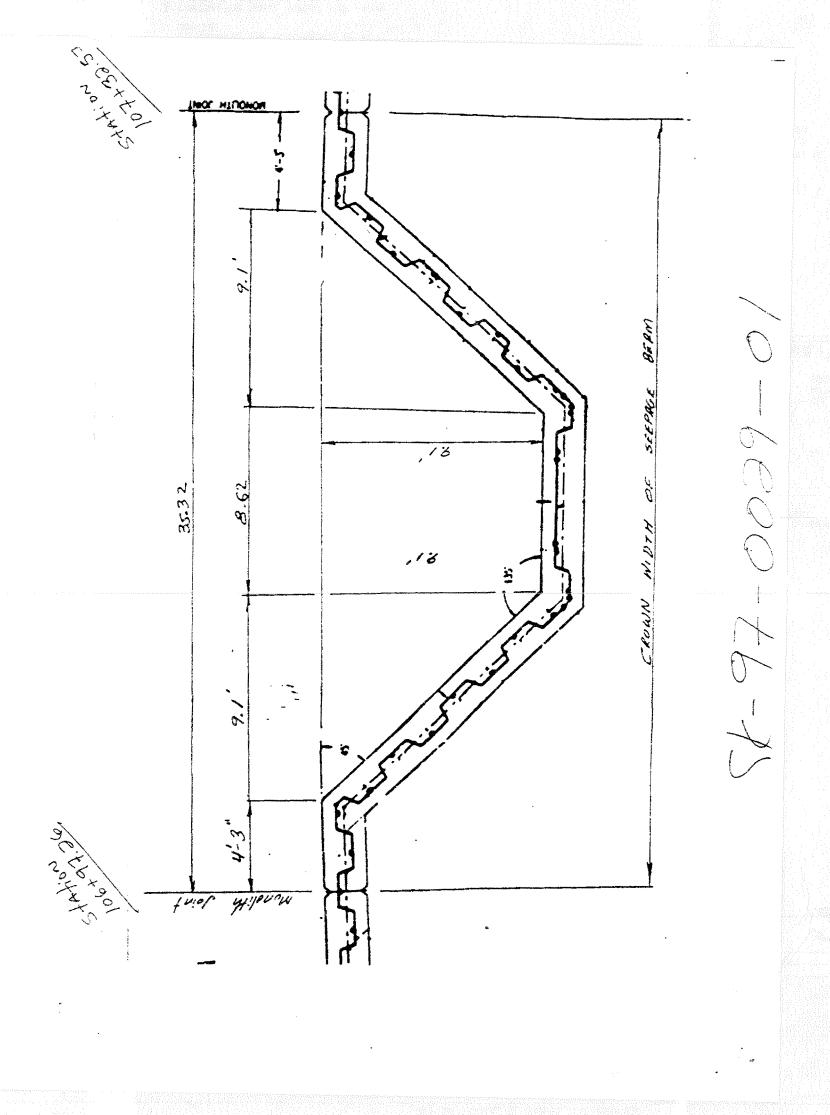
FOR GENERAL NOTES, SEE DWG. 2.
FOR PLAN AND PROFILE SEE DWGS. 4 THRU 16.
FOR MISCELLANEOUS DETAILS, SEE DWG. 23.
FOR ARCHITECTURAL WALL TREATMENT, SEE DWG. 26.

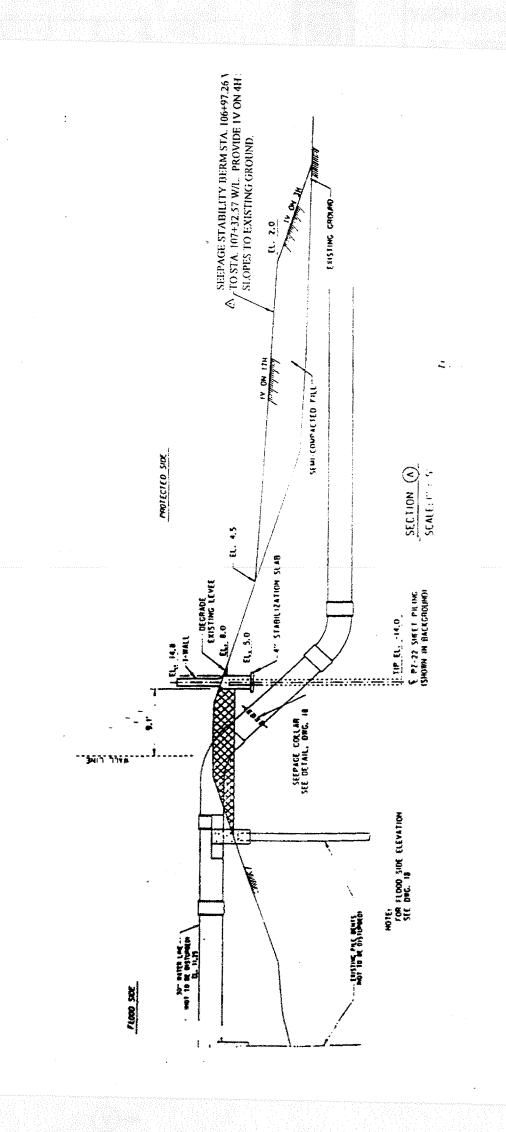
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SCALE: |" = |'- 0"









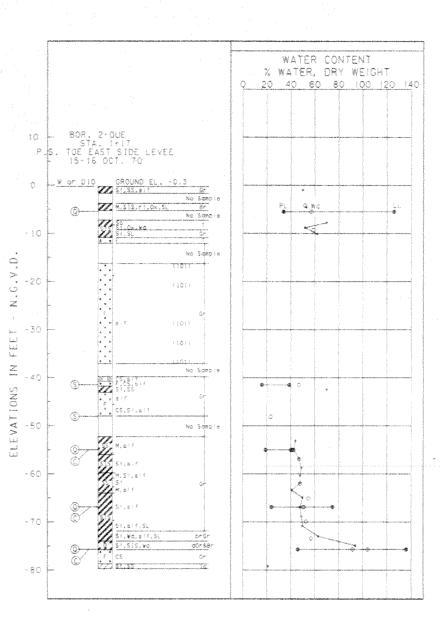
80-6800-th-75

E.B. STA. 1+17, P.S. TOE

E.B. STA. 4+06, 20' LT.

E.B. STA, 8+61. 5' LT.

E.B. STA. 14+26. 4' LT.



ame	of Proje	at	Drlaen	s Laves	District, Orleans Avenue Outfall Canal			
					No. 2048-0304, New Orleans, Louisians		commence of the contract of th	
re	The Bo	erd of	Leve	: Commi	ssioners of the Orleans Levec District,	Nev	Orleans	La.
nipapaan			Des	igh Brig	rincering, Inc., Metairie, Louisiana		4/	
orting	No. I		Solf Tect	nician _	A. J. Neyeux Date 17	Sep 3	enber 19	5
roun	d Elev	9.9	) 4		Datum NGVD Gr. Water Depth	Sec	Text	97
iampie	SA. Septor	- 500	5257%	STRATUS		1	WEARDARD	¬
×6.	Frank	Fit.	Proper	75			7527	
1	2.0	2.5	0.0	4.0	Stiff twn & gray silty clay w/silt	Ļ		
			-	-	DOC 12 TS	·	-	- 30
2	5.0	5.5	4.0	7.0	Stiff gray & tan clay w/silt pockets			
					8 fill	-	; <del></del>	-
3	8.0	8.5	7.0	10.0	Modium stiff gray & ten clay w/organic		<u> </u>	
-	2 C A				matter	ļ.,		40
5	11.0	11.5	10.0	12.5	Soft gray sandy clay w/organic matter	-		-
3	14.0	14.5	12.5	16.0	Medium stiff gray clay w/organic metter	ļ	-	
6					£ wood	ļ		
-	47.0	17.5	16.0	18,5	Medium stiff gray clay w/some organic			S0
7	5 05 0	900 20		02. 0	京島さとかす	-	·	-
-					Dense gray sand	Acres and	43	į.
9	23.5	22.5	41.0	Color olimbia commi	Very dense gray sand		50=10	1 2
9 1		25. G	nana and an and he	entires went man	Bit to	graining.	50×10*1	60
1		30.0		28.0	Bitto	(face a speciment	50=9"	
2		35.0	and the state of the party of	37.5	Medium dense gray sand	2	22	1 1
3		40.0			Ditto	8	CALL AVENABRATION	
4		45.0			Dense gray sand w/shell fragments Ditto	arrend, real	48	10
5		50.0		55,0			22	
6		53.5		27,57			38	
-		22.3	33.0		Medium dense gray sand w/shell fragments & clay layers	÷.	. 1.5	
7	58.5	60.0		62.0	Pitto	-	1.4	50
8	Active was a substitute of the	65.0	62.0		Medium stiff gray clay w/sund peckets	1	E4	
1			A A V V		& shell fragments	*		J.
9	69.0	69.5			944xo			
0		74.5		78 0	Medium stiff gray clay w/surs/ pockets	; 1 (		80.
7					5 shell fragments		o, a prima promotion, the a transmit	
3	79.0	79.5	78.0	84.0	Stiff gray clay w/silt lenses	en.en, i reem.ee		
2	84.0	54.5	84.0	86.0	Stiff gray clay w/organic matter	a construction	faginger i commence in a co	
3					Stiff greenish-gray & tam clas w/sami		and the field of the field of the field of the	-10.1

25 98.0 99.5 96.0 100.0 Compact tan & gray sandy silt 8 26

Name of Project Orleans Levee District, Orleans Avenue Out[all Canal) OLB Project No. 2018-0304, New Orleans, Louisiana
For: The Board of Lowee Commissioners of the Orleans Leves District, New Orleans, La Boring No 3 Soil Technolen A J Maybux Case 3 September 1985
Ground Elev 10.94 Cosum MAND Gr. Water Deptember 2005

The Park To New To Technolen A D September 1985

1 2.0 3.0 0.0 4.0 Compact tun 8 gray clayey sitt 1 2.0 5.0 0.0 4.0 Compact tan 8 gray clayey slit
2 5.0 6.0 4.0 7.5 Medium stiff gray & tan slity clay #/some organic matter

3 8.0 9.0 7.5 10.0 (Lose dark brown clayey slit w/sand
| layers & organic matter |

4 11.0 12.5 10.0 11.5 (Lose tan sand w/some clay (f)11)

5 14.0 15.0 11.5 16.0 Soft gray clay w/roots

6 16.0 17.5 16.0 18.0 Very danse gray sand | 12.50=10\*\*

7 18.5 20.0 18.0 Design matter 7 18.5 20.0 18.0 Dense gray said 11 55
8 21.0 22.5 Ditto 6 31
9 23.5 25.0 27.5 Ditto 10 42
10 28.5 30.0 27.5 32.5 Medium dense gray sand 5 22 11 33.5 35.0 32.5 38.0 Dense gray sand 12 38.5 40.0 38.0 Medium dense gray sand w/shell 

LOG OF BORING EUSTIS ENGINEERING COMPANY SOLAND FOUNDATION CONSULTANTS WETWIRE LA Name of Project: Orleans Levee District, Orleans Avenue Outfall Canal
OLB Project No. 2048-0304 New Orleans, Louisians For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans. Design Engineering, Inc., Metairie, Louisians 1 2.0 2.5 0.0 3.0 Medium stiff brown & gray fissured 2 5.0 5.5 5.0 7.0 Very stiff tan 6 gray clay w/silt | 11.0 | 11.5 | 10.0 | 12.0 | Loose tail silty sond w/clay | 4 | 14.0 | 14.5 | 12.0 | 16.0 | Soft gray clay w/organic matter & wood | 5 16.0 17.5 16.0 19.5 Nood w/s/me clay
6 19.5 21.0 19.5 Dense gray sand w/shell fragments
7 22.0 23.5 Dense gray sand w/shell fragments
8 25.0 26.5 28.0 Ditto
9 28.5 30.0 28.0 31.0 Very dense gray sand
10 33.5 35.0 51.0 Dense gray sand w/shell fragments 10 33.5 35.0 51.0 Dense gray sand w/shell fragments
11 38.5 40.0 Ditto

### NOTES:

- \*NUMBER IN FIRST COLUMN INDICATES NUMBER OF BLOWS OF 140-LB HAMMER DROPPED 30 IN. REQUIRED TO SEAT 2-IN.

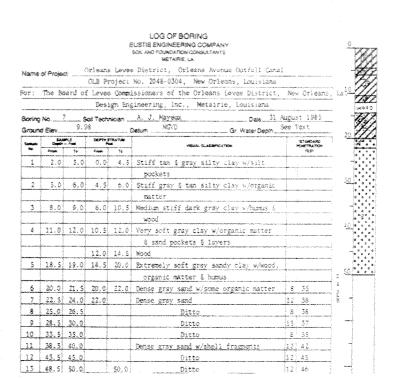
  O.D. SPLITSPOON SAMPLER 6 IN. NUMBER IN SECOND COLUMN INDICATES NUMBER OF BLOWS OF 140-LB, HAMMER DROPPED 30 IN, REQUIRED TO DRIVE 2-IN. O.D. SPLITSPOON SAMPLER I FT. AFTER SEATING 6 IN.
- 2. FOR SOIL BORINGS 2-OUE, 4-OUE, 5-OUE, 5-OUG, 6-OUG, 7-OUG, 1-OAW, & 2-OAW SEE LEGEND DRAWING 32.
- 3. FOR EUSTIS ENGINEERING BORINGS FURTHER INFORMATION IS

  AVAILABLE IN EUSTIS ENGINEERING APPENDICES DATED 19 JUNE 1989

  "GEOTECHNICAL INVESTIGATION ORLEANS AVENUE OUTFALL CANAL OLB
  PROJECT NO. 2048-0304 NEW ORLEANS, LOUISIANA.
- 4. WHILE THESE LOGS OF BORINGS ARE CONSIDERED TO BE REPRESENTATIVE OF SUBSURFACE CONDITIONS AT ITS RESPECTIVE LOCATION ON THE DATE SHOWN, IT IS NOT WARRANTED THAT IT IS REPRESENTATIVE OF SUBSURFACE CONDITIONS AT OTHER LOCATIONS

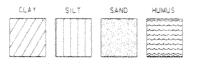
E.B. STA. 18+22, 5' LT.

E.B. STA. 24+57, 4.5' LT.



Name Cr:	of Proje	maria unum	OLB Pt	Levec Toject Commi	LOG OF BORING EUSTIS ENGINEERING GOMPARY SOR AND POLINATOR COMBUTANTS METABBE LA DISTRICT, OTHERNS Avenue Outfall Canal No. 2048-0304, New Offens, Louisiana astoners of the Orleans Levce District,		**************************************	0 
	and the second	9			A. J. Spycus Communication A. J. Spycus Communication Science Comm	nt or	dana 1066	- <i>377</i> ).
	No Kritiev	9.8		ynicien	537575		Yext	96
Groun	C CIEV.	1001.4	DOFTX	STRATIS	Datum Gr. Water Deptin	-	*574#0A#0.	5 20 <b>///</b>
Tearning No.	Sugar.	To To	Free	To the	erations, duality-from reper-	ł	AR WELLEY LACK LETT.	755
1	2.0	2.5	0,0	<u> </u>	Stiff tan & gray clay w/silt & sand		***************************************	
Bern Lawrence				1	polikets	T		
2	5.0	5.5			Ditto	1	-	
3	8.0	8.5		9.0	Ditto	7	* Politica per a contraction a contraction and a	
4	11.0	11.5	9.0	12.5	Kedus stiff gray silty clay w/organic		2	
					MOTES:		1	
5	14.0	14.3	12.5		Soft gray clay w/organic matter & wood	Ī	- Marie Control of the Control of th	-:.::
6	19.0	19.5		21.0	Bitto			
7	23.0	23.5	21.0	23.5	Soft gray sandy clas w/organic matter	T	-	
					& roots	-		
8	23.5	25.0	23.5	25.5	Demoe gray sand w/shell fragments	1.7	36	-
9	25.0	27.5	25.5		Medium dense gray sand w/shell	6	26	*
					fragmonts			7 # 4
10	28.5	30.0			Medium dense gray sond w/some organic	1.7	- 23	
					Matter	3		
11	33,5	35.0			Ditto	] 5	17	September 1
12	38.5	40.0			Ditto	1 5	13	
13	43.5	45.0		46.0	Ditto	13	16	
14	48.5	50.0	46.0	50.0	Medium dense gray sand w/shall fragments	4	16	1

LEGEND FOR BORINGS 1-35



PREDOMINANT TYPE SHOWN HEAVY MODIFYING TYPE SHOWN LIGHT.

•				a trade or server to be trade or server to the server to t		
	SYMBOL	DES	CRIPTION		DATE	APPROVED
			REVISIONS			
		U.S. ARMY		DISTRICT, F ENGINEERS UNS, LOUISIA	S	DRLEANS
A CALL OF THE PARTY OF THE PART		AVE. OUTFA PHASE II-A 3+60.00 E.	EAST SIC	PLAN PARALLEI DE FLOODW TA. 90+26	PRO	

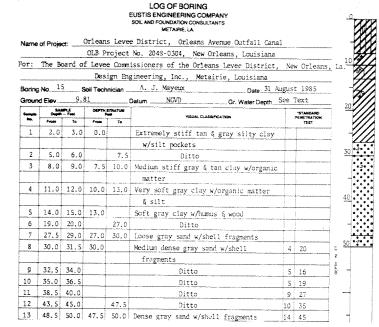
DESIGNED BY: VOJKOVICH
DRAWN BY: WOODS
CHECKED BY: RICHARDSON CADO FILE: fmocow.agr | H-4-44644 DACW29-96-8-0096 DWG 27 OF 32

SOIL BORINGS

13 43.5 45.0 43.5 Very dense gray fine sand w/shell 14 50=7" fragments

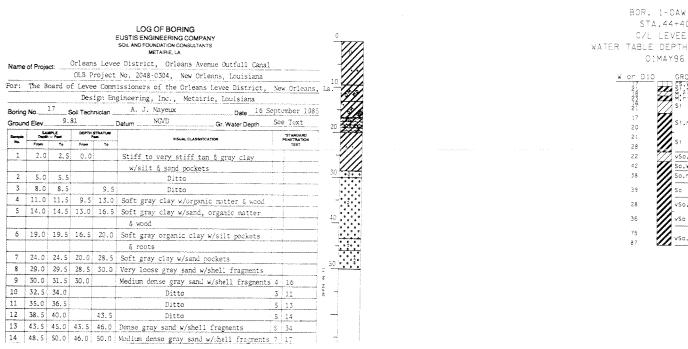
E.B. STA. 37+54, 2' LT.

2



E.B. STA. 41+65, 2' LT.

E.B. STA. 44+40. C/L LEVEE



14 48.5 50.0 S0.0

STA.44+40 C/L LEVEE WATER TABLE DEPTH 16.0 FT. O:MAY96 W or D10 GROUND EL. 9.4 

E.B. STA. 44+50, C/L LEVEE

E.B. STA. 47+40. 1.5' LT.

E.B. STA. 53+20, 0.5' LT.

BOR. 2-DAW STA.44+50 CZL LEVEE WATER TABLE DEPTH 14.0 FT. 02MAY96 ZZ2 §

					EUSTIS ENGINEERING COMPANY SOIL AND FOUNDATION CONSULTANTS METAIRIE, LA.			<u>(</u>	
Name	of Proje	ct:	Orlean	s Leve	e District, Orleans Avenue Outfall Canal				1
					No. 2048-0304, New Orleans, Louisiana				
or:	The B	oard o			issioners of the Orleans Levee District,	Ne	w Orleans	, Lal	3
			Des	ign En	gineering, Inc., Metalile, Louisiana				4
Boring	No			inician _	A. J. Mayeux Date 28 Au	gus	t 1985	_	1
Groun	d Elev	10	.01		Datum NOVD Gr. Water Depth S	ee	Text	- 20	汝
Sample	SA	MPLE Foot	DEPTH	STRATUM	VISUAL CLASS/PICATION	T	"STANDARD PENETRATION	40	2
<b>20</b> 0.	From	To	From	Te		İ	TEST		woo
1	2.0	3.0	0.0		Stiff tan & gray silty clay w/silt	1	ļ		-
					pockets	1		- 50	٠٠٠
2	5.0		ļ	ļ	Ditto	1		4	
3	8.0			9.0	**************************************	-			
4		12.0			Very soft dark gray clay w/humus & wood	4			•
5	14.0	15.0	13.0	17.0	Extremely soft brown & gray clay w/silt	1	<u> </u>	40	
					layers, organic matter & wood	<u> </u>		1 **	
6	19.0	20.0	17.0	21.0	Very soft black organic clay w/humus	ļ.,	<u> </u>		
					& wood	_	<u> </u>	-	
		25.0			Wood w/organic matter & clay	-		50	• • • •
7		29.0		30.0	Very loose gray sand w/shell fragments	1	4	E 300	1
8		31.5	30.0		Loose gray sand w/shell fragments	3	9	Нидж.	
9	32.5				Ditto	2	8	ž -	1
10		36.5			Ditto	4	6	į	
11		40.0		42.5	Ditto	3	9	ļ	4
12	43.5	45.0	42.5		Medium dense gray sand w/shell	6	1.7	4	
					fragments			-	1
13	48.5	50.0		50.0	Ditto	7	18		1

LOG OF BORING Name of Project: Orleans Levee District, Orleans Avenue Outfall Canal OLB Project No. 2048-0304, New Orleans, Louisiana
For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, Design Engineering, Inc., Metairie, Louisiana

Boring No. 21 Soil Technician A. J. Mayeux Date 16 September 1985

Ground Elev 9.71 Datum NGVD G. Warranger Son Technician 4 11.0 11.5 10.5 13.0 Soft gray clay w/organic matter, silt 5 14.0 14.5 13.0 17.0 Medium stiff gray sandy clay w/organic 6 19.0 19.5 17.0 21.5 Soft gray clay w/wood & organic matter 7 24.0 24.5 21.5 Medium stiff gray sandy clay w/some 8 29.0 29.5 32.5 Ditto 9 32.5 34.0 32.5 35.0 Medium dense gray sand w/shell fragments

10 35.0 36.5 35.0 Loose gray sand w/shell fragments 

W or D10 GROUND EL. 0.9

| St. Wd.rt.cc.SIS dBr | M.Wd.rt.SIS Br&dBr | St. Wd.rt.SIS dBr | St. Wd.rt.SIS Br&dBr | ① vso, vd, r1, s15 0 0 CS,SI,sif ®----0.12

E.B. STA. 39+05. P.S. TOE

WATER CONTENT % WATER, DRY WEIGHT

0 20 40 60 80 100 120 140

Safety is a Part of Your Contract

REFER TO NOTES ON DRAWING 27.



U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA

LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN ORLEANS AVE. OUTFALL CANAL PARALLEL PROTECTION PHASE II-A (EAST SIDE FLOODWALL)
STA. 3+60.00 E. B/L TO STA. 90+26.33 E. B/L
ORLEANS PARISH, LA. SOIL BORINGS

DESIGNED BY: VOJKOVICH DATE: PLOT SCALE: DRAWN BY: WOODS SEP 96 20 DRAWN BY: WUUUS
CHECKED BY: RICHARDSON CADD FILE: fmodowo.dgn FILE NO. H-4-44644 SOLICITATION NO. DACW29-96-B-0096 DWG. 28 OF 32

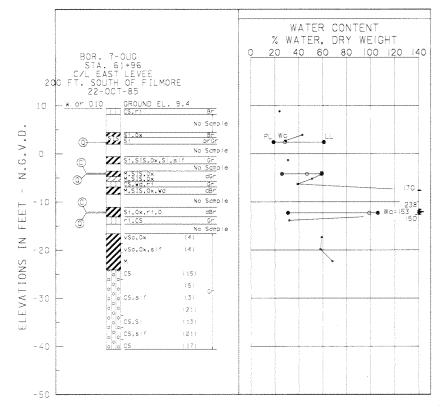
# E.B. STA. 61+96, C/L

# E.B. STA. 62+88, 1.5' RT.

## E.B. STA. 64+27, 5' RT.

LOG OF BORING

					LOG OF BORING EUSTIS ENGINEERING COMPANY SOILAND FOUNDATION CONSULTANTS METAIRIE LA			
lame	of Proje	ct:	Orlean	s Leve	e District, Orleans Avenue Outfall Cana			_
			OLB P	roject	No. 2048-0304, New Orleans, Louisiana			_
or:	The Bo	pard o	E Leve	e Comm	issioners of the Orleans Levee District,	Nev	v Orleans	, L
			Des	ign Eng	gineering, Inc., Matairie, Louisiana			-
Boring	No	23 s	icil Tech	nician _	A. J. Mayeux Date 27 At	igust	1985	
Groun	d Elev	9	. 56		Datum NGVD Gr. Water Depth	Sec 1	ext	_
Sample	SAI Dogen	aFt.E Feest	DEPTH	STRATUE	YISUAL CLASSIFICATION	Τ.	STANDARD PENETRATION	7
340.	From	To	Frank	Yes			TEST	
1	2,0	3.0	0.0	4.5	Very stiff tan & gray silty clay	+		-
					w/silt pockets	<u> </u>		
2	5.0	6.0	4.5	7.0	Very stiff gray & tan clay w/silt			-
					pockets	<u> </u>		_
3	8.0	9.0	7.0	10.0	Stiff gray clay w/some organic matter		)  -  -	
4	11.0	12.0	10.0		Soft gray clay w/silt, organic matter			
					& wood			1
5	14.0	15.0		15.0	Ditto			1
6	16.0	17.0	15.0	18.0	Very soft gray clay w/humus layers	1		
					§ wood	i		į
			18.0	22.0	Wood w/organic matter & some clay			į
7	24.0	25.0	22.0	26.0	Loose gray clayey silt w/wood			1
8	29.0	30.0	26:0	33.0	Soft gray clay w/silt layers			100
9	33.5	35.0	33.0	36.0	Medium dense gray sand w/shells	7	21	j
10	36.0	37.5	36.0		Very loose gray sand w/shells	3	- 5	1
11	38.5	40.0		41.0	Ditto	2	3	]
12	41.0	42.5	41.0		Medium dense gray sand w/shell	5	21	]
					fragments			1
13	43.5	45.0		47.5		5	21	
14	*0 =	50.0	47 5	E0 0	Loose gray sand w/shell fragments		9	-



					LOG OF BORING EUSTIS ENGINEERING COMPANY SOIL AND FOUNDATION CONSULTANTS METAIRIE, LA	£ 2			0	<b>%</b>
Name	of Proje	ect:	Orlea	ins Levi	ee District, Orleans Avenue Outfall Cana	1			-1	W
			OLB	Projec'	t No. 2048-0304, New Orleans, Louisiana			-	10	H)
For:	The	Board	of Lev	ree Com	missioners of the Orleans Levee District,	Ne	w Orleans	- 5, La	10	44
			De	sign E	ngineering, Inc., Metairie, Louisiana			_	1	7/4
Boring	No	25	Soil Tec	hnician	A. J. Mayeux Date 12 S	epte	ember 198	5	+	44
Grour	d Elev	9.	51		Datum NGVD Gr. Water Depth	See	e Text		20	7
Same		MPLE N France	DEPT	H STRATUM	T T T T T T T T T T T T T T T T T T T	T	'STANDARD PEMETRATION	7	20	$\mathcal{Z}$
20.	Frank	To	From	Te .	VISUAL CLASSIFICATION		TEST	1	ļ	1
1	2.0	2.5	0.	0	Very stiff tan & gray silty clay				4	H
					w/clayey silt pockets				70	
2	5.0	5.5		7.0	Ditto					<i>777</i>
3	8.0	8.5	7.0	0 10.0	Medium stiff gray & tan clay w/silt			_		///
		<u> </u>	ļ		pockets				-	.3 abda
4	11.0	11.5	10.0	0	Soft gray clay w/organic matter, humas			Ι.	40	7//
		ļ	-		& wood					$\mathcal{U}$
5	<del></del>	14.5	<del> </del>	15.0	Ditto				1	
- 6	19.0	19.5	15.0	21.0	Soft black organic clay w/humas & wood			j	7	• • •
7	24.0	24.5	21.0	25.0	Very soft gray silty clay w/organic	1			sn i	$\cdots$
					matter & wood			2	~	m
8	<del></del>	29.5	25,0	29.5	Soft gray sandy clay	1	!	3 5	E	977
9	32.0	<del> </del>			Soft gray clay			OEPTM	-/	44
10	33.5	35.0	33.5	36.0	Medium dense gray sand w/shell	7	21		sa 🗸	
					fragments					
11	·	37.5		1	Very loose gray clayey sand w/shells	1	2		-	///
12	·	40.0	<del></del>	41.0	Ditto	1	3		-	44
13	41.0	42.5	41.0	1	Medium dense gray sand w/shell	6	15	,	10 V	
				ļ	fragments				7	44
14		45.0		-	Ditto	3	13		1	HZ.
15	48.5			50.5		3	11		7	
16	53.5			Participation involute consu	Medium stiff gray clay w/sand layers	. 2	5	8	o .	
17	59.0	59.5	56.0		Medium stiff gray clay w/clayey sand	<u>                                     </u>			7	44
					pockets & shell fragments	-			1	///
18		64.5		66.0	Ditto	1			1	44
19					Medium stiff gray clay		1000 P	9	o P	///
20	74.0	74.5	71.0	74.5	Medium stiff greenish-gray clay				1	44
					w/organic matter & shells				1	///
21	79.0	79.5	74.5	81.5	Very stiff greenish-gray clay w/silt				1	
				ļļ	pockets			10	0 8	
22					Stiff greenish-gray & tan-sandy clay	-	************			
23				91.0	Stiff tan & gray clay w/sand layers	-				
24		94.5			Stiff tan & gray clay w/silt lenses					
25	99.0	99.5		100.0	Ditto					

Name of Project: Orleans Levee District, Orleans Avenue Outfall Canal OLB Project No. 2048-0304, New Orleans, Louisiana For: The Board of Levee Commissioners of the Orleans Levee District, New Orleans, Design Engineering, Inc., Metairie, Louisiana Boring No. 27 Soil Technician A. Croal, Jr. Date 31 August 1985
Ground Elev. 9.06 Datum NGVD Gr. Water Depth. See Text

| Search | Depth Plant 30 1 0.0 0.5 0.0 0.5 Medium stiff brown & gray clay w/fine sand pockets & grass roots

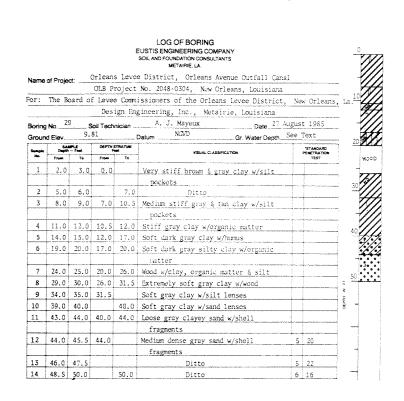
2 1.7 2.5 0.5 3.0 Medium stiff tan & gray clay w/many fine sand pockets & lenses 3 4.7 5.5 3.0 5.5 Medium compact tan & gray sandy silt w/thin clay layers
4 7.7 8.5 5.5 9.0 Medium stiff tan & gray clay w/thin clay layers 5 10.7 11.5 9.0 Stiff gray clay w/few clayey silt pockets 6 13.7 14.5 17.5 Stiff gray clay w/trace of organic 7 18.2 19.0 17.5 19.0 Loose gray clayey silt w/organic clay & humais layers 8 23.2 24.0 19.0 25.0 Loose brown humus w/roots & organic clay layers

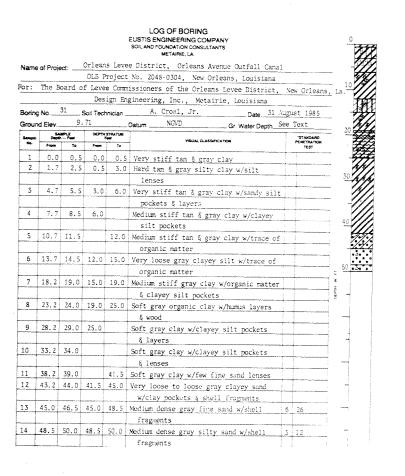
9 28.2 29.0 25.0 Soft gray clay w/clayey silt lenses & 11 38.2 39.0 38.0 Dense gray silty sand w/clay pockets 14 46.0 47.5 46.0 Medium dense gray silty sand w/shell 6 14 15 48.5 50.0 50.0 Ditto 3 11

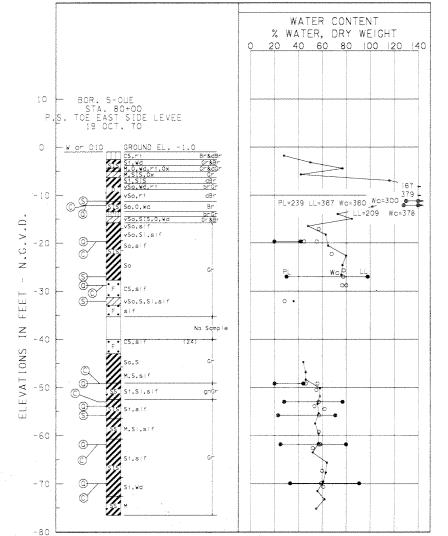
E.B. STA. 72+40. 5' RT.

E.B. STA. 77+27, 5.5' RT.

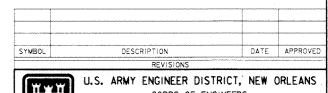
E.B. STA. 80+00, P.S. TOE







REFER TO NOTES ON DRAWING 27.



u.s.

CORPS OF ENGINEERS
NEW ORLEANS, LOUISIANA

LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN

ORLEANS AVE. OUTFALL CANAL PARALLEL PROTECTION
PHASE II-A (EAST SIDE FLOODWALL)
STA. 3+60.00 E. B/L TO STA. 90+26.33 E. B/L
ORLEANS PARISH, LA.

201F	BU	TIN	G S	>

DESIGNED BY: VOUKOVICH DRAWN BY: WOODS CHECKED BY: RICHARDSON CADD FILE: fmoocwo.dgn FILE NO. H-4-44644

SUBMITTED BY: SOLICITATION NO. DACW29-96-B-0096 DWG. 29 OF 32

O INCENTIVE YOUR KEY TO Z CLASSES O HIGHER PROFITS

4

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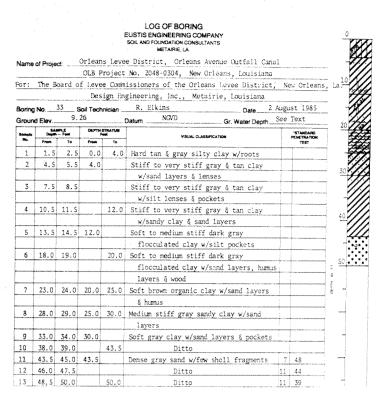
5

E.B. STA. 82+90, 6' RT.

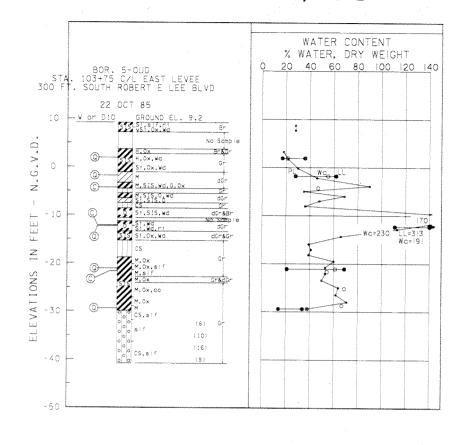
E.B. STA. 87+34, 4.5 RT.

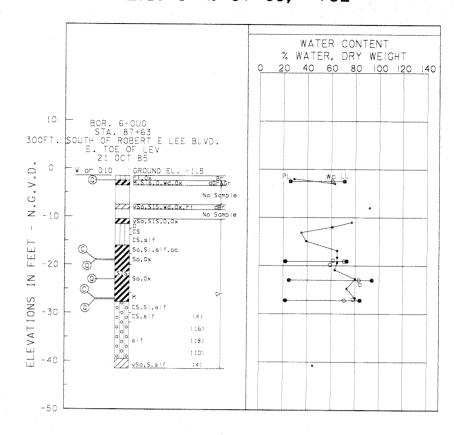
E.B. STA. 87+63, C/L

E.B. STA. 87+63. TOE



			0		LOG OF BORING EUSTIS ENGINEERING COMPANY SOIL AND FOUNDATION CONSULTANTS METAIRIE, LA			
Name	of Proje	ct:		CONTRACTOR CONTRACTOR	e District, Orleans Avenue Outfall Canal			,
ior.	The P	and -			No. 2048-0304, New Orleans, Louisiana			10
01:	ine B	oard o			issioners of the Orleans Levee District,	Nev	v Orleans	, La.
		c			gineering, Inc., Metairio, Louisiana			
	No3		Soil Tech 16	nnician				'
Groun	d Elev.	~~			Datum NGVD Gr. Water Depth S	ee :	ext	_ 20
Vompte No.	Depth	Pest	,	STRATURE	WISHAL CLASSIFICATION		"STANDARD PENETRATION	_
	From	Te .	Frant	10		-	YE57	
2	0.0	0.5	0.0	<del></del>	Stiff tan & gray clay w/grass roots	-	<u> </u>	
	1.7	2.5		3.0	Stiff tan & gray clay w/clayey silt	ļ	ļ	- 30
3	4.7	5.5	7.0	ļ	pockets	+		-
	4./	3.3	3.0		Stiff gray clay w/clayey silt pockets	***		-
Δ	7.7	0.5		-	& silty sand layers	-		j -
		8.5			Stiff gray clay w/clayey silt pockets	ļ	!	40
5		11.5		12.0	Ditto	<u> </u>		4
5	13,/	14.5	12.0	15.0	Soft dark gray clay w/humus pockets	-		
					& organic matter	ļ,		j ~
7	18.2	19.0	15.0	19.0	Soft dark gray silty clay w/clayey silt			50
					pockets	-		-
8	23.2	24.0	19.0	24.0	Soft brown & gray organic clay w/humus	-		1 HIA
					layers & few roots	1		100
9				29.0	Very loose gray clayey silt	1		
10	33.2	34.0	29.0		Soft to medium stiff gray clay w/few	-		-
					clayey silt lenses & shells		*****************	
11	38.2	39.0		41.0	Soft to medium stiff gray clay w/few	-	and an exercise of the second	. 1
					silty sand pockets			
12	43.2	44.0	41.0	44.0	The state of the s	1		
					pockets & shell fragments			
13	45.0	46.5	44.0		Medium dense gray fine sand w/shell	5	18	1
					fragments	1		
14	48.5	50.0		50.0	Ditto	5	15	





SUMMARY OF LABORATORY TEST RESULTS

BORING 1

Sam- ple	Depth In		Water Content	F	sity CF	Unconfined Compressive Strength
No.	Feet	Classification	Percent	Dry	Wet	PSF
2	5.0	Stiff brown & gray clay w/silt pockets	20.5	93.7	112.9	3210▼
4	11.0	Soft gray sandy clay w/trace of organic matter	25.2	95.9	120.1	705
6	17.0	Medium stiff gray clay w/roots	56.6	65.7	102.8	1640

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

SUMMARY OF LABORATORY TEST RESULTS

BORING 3

Sam- ple No.	Depth In Feet	Classification	Water Content Percent		sity CF Wet	Unconfined Compressive Strength PSF
1	2 0	Compact tan & gray clayey silt	16.9	100.8	117.8	2735*
, 2	5.0	Medium stiff tan & gray silty clay w/large sandy silt pockets & shells	38.9	72.3	100.5	1405
3	8.0	Loose dark brown clayey silt w/organic matter & sand	21.4	79.6	96:7	535*
5	14.0	Soft gray clay w/sand pockets & roots	43.1	75.4	107.9	830

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

SUMMARY OF LABORATORY TEST RESULTS

BORING 5

		ADDITE A LEG D				
1	2.0	Medium stiff brown & gray fissured clay w/many silt	24.0	89.1	110.5	1085*
2	5.0	pockets Very stiff tan & gray clay w/silt pockets	25.1	98.2	122.9	6295
3	8.0	Medium stiff dark gray clay w/silt pockets	33.8	78.0	104.4	1370*
4	14.0	Soft gray clay w/roots & trace	84.5	49.6	91.6	730

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

SUMMARY OF LABORATORY TEST RESULTS BORING 7

Sam- ple	Depth In		Water Content		sity	Unconfined Compressive Strength
No.	Feet	Classification	Percent	Dry	Wet	PSF
1	2.0	Stiff tan & gray silty clay w/partings	18.9		****	
3	8.0	Medium stiff dark gray clay w/silty sand layers	40.0	74.0	103.6	1020*
4	11.0	Very soft gray clay w/silty sand layers	50.3	66,1	99.4	495*
5	18.5	Extremely soft gray sandy clay w/roots	44.0	77.7	111.9	115

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

SUMMARY OF LABORATORY TEST RESULTS

		BORING 9				
	2.0	Stiff tan & gray clay w/silt pockets	24.6	95.4	118.9	3545*
i	8.0	Stiff gray & tan clay w/silt	28.6	86.8	111.6	2705*
	11.0	Medium stiff dark gray silty clay w/trace of organic matter	37.3	74.4	102.2	1265*
	14.0	Soft gray clay w/organic clay layers & wood	140.6	31.2	75.1	760*
•	23.0	Soft gray sandy clay w/organic matter & roots	35.9	81.3	110.5	760*

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

SUMMARY OF LABORATORY TEST RESULTS

BORING 11

Sam-	Depth In		Water Content		sity CF	Unconfined Compressive Strength
Vo.	Feet	Classification	Percent	Dry	Wet	PSF
2	5.0	Stiff tan & gray clay w/silt	25.0	95.0	118.7	3100*
. 3	8.0	Medium stiff dark gray clay w/organic matter & sand pockets	50.0	64.1	96.1	1135
4	11.0	Soft gray clay w/silty sand	54.5	66.1	102.1	930

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

SUMMARY OF LABORATORY TEST RESULTS

BORING 13

Sam- ple	Depth In		Water Content	P	sity	Unconfined Compressive Strength
No.	Feet	Classification	Percent	Dry	Wet	PSF
1	1.5	Medium stiff brown clay w/clayey silt pockets	24.1	90.0	111.6	1585*
2	4.5	Very soft tan & gray clay	34.7	78.6	105.8	315
3	7,5	Soft brown clay w/sand pockets & trace of organic matter (fill)	53.6	63.2	97.1	545
4	10.5	Soft dark gray clay w/clayey silt pockets & trace of sand	44.4	72.1	104.2	860
5	13.5	Medium stiff gray clay w/clayey sand pockets	45.4	73.0	106.2	1045

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

REFER TO NOTES ON DRAWING 27.

			***************************************					
			[					
			(					
	SYMBOL	DECADIOTION						
	3 HWOUL	DESCRIPTION	DATE	APPROVED				
1	REVISIONS							

HAH

U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS CORPS OF ENGINEERS
NEW ORLEANS, LOUISIANA

CAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN
ORLEANS AVE. OUTFALL CANAL PARALLEL PROTECTION PHASE II-A (EAST SIDE FLOODWALL)
STA. 3+60.00 E. B/L TO STA. 90+26.33 E. B/L
ORLEANS PARISH, LA. SOIL BORINGS

				1
DESIGNED BY: VOJKOVICH	DATE:	PLOT SCALE:	PLOT DATE:	1.
DRAWN BY: WOODS	SEP 96	20	SEP 96	L
CHECKED BY: RICHARDSON	CADD FILE: fmod	icwo.dgn	FILE NO. H-4-44644	1
SUBMITTED BY:	SOLICITATION			-
Design Florings	1 DACW29-9	96-B-0096	DWG. 30 OF 32	1

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#### SUMMARY OF LABORATORY TEST RESULTS

#### BORING 15

Sam- ple	Depth	(Table ) (Table )	Water Content	. P	sity CF	Unconfined Compressive Strength
No	Feet	Classification	Percent	Dry	Wet	PSF
1	2.0	Extremely stiff tan & gray silty clay	15.5	99.7	115.2	8425*
3	8.0	Medium stiff tan & gray clay w/silt pockets	28.5	86.6	111.3	1520*
4	11.0	Very soft gray clay w/organic matter	52.7	66.4	101.5	410
5	14.0	Soft gray clay w/organic matter	94.1	45.2	87.7	635

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

### SUMMARY OF LABORATORY TEST RESULTS

#### BORING 23

Sam- ple	Depth In		Water Content		sity	Unconfined Compressive Strength
No.	Feet	Classification	Percent	Dry	Wet	PSF
1	2.0	Very stiff tan & gray silty	14.4	101.2	115.8	6305*
2	5.0	Very stiff gray ξ tan clay w/silt pockets	25.8	95.7	120.3	5460
3	8.0	Stiff gray clay w/silt pockets	33.3	86.9	115.8	2945
4	11.0	Soft gray clay w/organic matter & silt pockets	45.5	69.0	100.3	500
5	14.0	Soft gray clay w/clayey silt lenses, layers & organic matter	37.8	78.9	108.8	755*
6	16.0	Very soft gray clay w/organic clay layers	97.1	44.4	87.4	490
7	24.0	Loose gray clayey silt w/roots	35.1	82.5	111.4	630
8	29.0	Soft gray clay w/silty sand pockets & shell fragments	53.7	68.4	105.1	735

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

## SUMMARY OF LABORATORY TEST RESULTS

#### BORING 31

Sam- ple	Depth In		Water Content		sity CF	Unconfined Compressive Strongth
No.	Feet	Classification	Percent	Dry	Wet	PSF
2	1.7	Hard tan & gray silty clay	12.0			
4	7.7	Medium stiff tan & gray clay w/silt pockets	26.9	84.9	107.8	1530*
6	13.7	Very loose gray clayey silt w/trace of organic matter	39.2	76.7	106.8	455*
8	23.2	Soft gray organic clay w/decayed wood & clay layers	161.5	29.1	76.0	655
10	33.2	Soft gray clay w/clayey silt lenses & layers	49.8	70.6	105.7	740
12	43.2	Very loose gray clayey sand w/shell fragments	29.4	93.2	120.6	420*

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

### SUMMARY OF LABORATORY TEST RESULTS

#### BORING 17

Sam- ple	Depth In		Water Content	F	sity CF	Unconfined Compressive Strength
No.	Feet	Classification	Percent	Dry	Wet	PSF
2	5.0	Very stiff tan & gray clay w/silt pockets	25.8	96.5	121.4	4965
4	11.0	Soft dark gray clay w/silt	55.2	61.9	96.0	595
S	14.0	Soft gray clay w/organic matter layers & sand pockets	73.6	53.6	93.1	755
6	19.0	Soft brown organic clay w/silt pockets & roots	191.1	25. 2	73.4	670
7	24.0	Soft gray clay w/many sand	36.7	81.2	111.0	695

### SUMMARY OF LABORATORY TEST RESULTS

### BORING 25

Sam- ple No.	Depth In Feet	Classification	Water Content Percent		nsity PCF Wet	Unconfined Compressive Strength PSF
2	5.0	Very stiff tan & gray silty clay w/clayey silt pockets	25.2	93.3	116.9	4165*
3	8.0	Medium stiff tan & gray clay w/clayey silt pockets	28.2	86.4	110.8	1000*
4	11.0	Soft dark gray silty clay w/organic matter & roots	43.2	61.2	87.6	* * * *
6	19.0	Soft black organic clay w/humus, roots & wood	198.7	24.4	73.0	\$40
7	24.0	Soft gray silty clay w/much organic matter & wood	76.4	50.3	88.6	500
9	32.0	Soft gray clay w/silt pockets	63.4	61.0	99.6	655
1.7	59.0	Medium stiff gray clay w/clayey sand pockets & shell fragments	53.8		102.9	1350
19	69.0	Medium stiff gray clay	50.6	69.3	104.3	1125
21	79.0	Very stiff greenish-gray clay w/clayey silt pockets	19.5	105.7		4505
23	89.0	Stiff tan & gray clay w/silt pockets	33.3	86.1	114.8	2000*
25	99.0	Stiff greenish-gray & tan clay w/silt lenses	37.9	82.5	113.7	2510

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

### SEMMARY OF LABORATORY TEST RESULTS

### BORING 33

Sam- ple No.	Depth In Feet	Classification	Water Content Percent		nsity XCF Wet	Unconfined Compressive Strength PSF		erbe		
1	1.5	Hard tan & gray silty clay w/roots	12.7							
3	7.5	Very stiff gray & tan clay w/silt lenses & pockets	23.5	96.9	119.6	4600*				
5	13.5	Soft dark gray flocculated clay w/silt pockets	46.5 .	70.1	102.7	980				
7	23.0	Soft brown organic clay w/silty clay layers	130.1	34.8	80.0	500	118	32	86	
9	33.0	Soft gray clay w/clayey silt layers, lenses, pockets & decayed	53.1	67.4	103.2	745				

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

#### SUMMARY OF LABORATORY TEST RESULTS

#### BORING 19

Sam- ple No.	Depth In Feet	Classification	Water Content Percent		nsity PCF Wet	Unconfined Compressiv Strength PSF
1	2.0	Stiff tan & gray silty clay	21.2	98.7	119.6	***************************************
2	5.0	Ditto	32.3	83.5	110.5	3945*
3	8.0	Stiff tan & gray silty clay w/silt pockets & lenses	29.5	89.7	116.1	2725* 2365*
4	11.0	Very soft dark gray clay w/sand pockets & organic matter	50.7	62.4	94.0	410*
5	14.0	Extremely soft brown & gray clay w/large sand pockets	51.4	67.0	101.4	160
6	19.0	Very soft black organic clay w/humus layers	196.8	24.4	72.5	725

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

#### SUMMARY OF LABORATORY TEST RESULTS

### BORING 27

Sam-	Depth In		Water		nsity	Unconfined Compressive
No.	Feet	Classification	Percent	Dry	Wet	Strength PSF
2	1.7	Medium stiff tan & gray clay w/silty sand lenses, layers & roots	22.5	94.4	115.6	1005
3	4.7	Medium compact ran & gray sandy silt w/silty clay layers	22.2	93,6	114.4	1090*
4	7.7	Medium stiff gray & tan clay w/silt pockets	31.3	87.1	114.4	1275
6 7	13.7	Stiff gray clay w/silt pockets	29.6	90.0		
7	18.2	Loose gray clayey silt	37.2	82.3	116.6	21.45
8	23.2	Loose brown humus w/organic clay layers & roots	235.8	19.7	113.0 66.0	840* 745
9	28.2	Soft gray clay w/sandy silt pockets & few shell fragments	56.1	65.7	102.5	710
2	42.2	Dense gray silty sand w/trace of clay & few shell fragments	26.6	99.2	125.6	3695*

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

### SUMMARY OF LABORATORY TEST RESULTS BORING 35

Sam- ple No.	Depth In Feet	Classification	Water Content Percent		nsity CF Wet	Unconfined Compressive Strength PSF
2	1.7	Stiff tan & gray clay w/clayey	26.2	91.9	115.9	2290*
		silt layers & pockets	20.2	31.3	113.9	2290*
4	7.7	Stiff gray clay w/clayey silt	22.6	95.7	117.3	2440*
-		layers & lenses				2770
5	10.7	Ditto	30.1	89.1	115.9	2560*
7	18.2	Soft dark gray silty clay	70.0	52.8	89.8	
		w/organic matter	,0.0	34.0	09.8	640
9	28.2	Very loose gray clayey silt	47.0	<b>7</b> 2 0		
		w/silty clay layers	47.0	71.8	105.5	385
11	38.2	Medium stiff gray clay	70.0			
		Bidy Clay	70.9	57.3	98.0	1105

\*Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

### SUMMARY OF LABORATORY TEST RESULTS

### BORING 21

Sam- ple	Depth In		Water Content	Density PCF		Unconfined Compressiv Strength	
No.	Feet	Classification	Percent	Dry	Wet	PSF	
1	2.0	Medium stiff gray & tan clay w/clayey sand layers	34.8	82.7	111.5	1000*	
3	8.0	Stiff gray & tan clay w/silt pockets	29.5	87.2	113.0	2255	
4	11.0	Soft gray clay w/organic matter & trace of sand	52.6	60.5	92.4	515	
5	14.0	Medium stiff gray sandy clay w/silty clay layers	25.8	98.7	124.1	1995*	
. 6	19.0	Soft gray clay w/organic matter, roots & wood	117.3	38.7	84.1	585	
8	29.0	Medium stiff gray sandy clay w/shell fragments	29.1	93.2	120.4		

\*Unconsolidated Undrained Triavial Compression Test - One Specimen; Confined at the approximate overburden pressure.

### SUMMARY OF LABORATORY TEST RESULTS

### BORING 29

Sam- ple No.	Depth In Feet	Classification	Water Content Percent	1	sity CF Wet	Unconfined Compressive Strength
		The state of the s	Annual Services and Services	Dry	wet	PSF
1	2.0	Very stiff brown & gray clay w/clayey silt pockets &	24.7	96.4	120.2	4445
3		trace of organic matter				
_	8.0	Medium stiff gray clay w/clayey silt pockets	25.8	89.8	113.0	1905*
4	11.0	Stiff gray clay w/many clayey silt lenses, layers & pockets	29.0	89.3	115.3	3340*
5	14.0	Soft dark gray clay w/trace of organic matter	50.9	66.0	99.6	720
6	19.0	Soft dark gray silty clay w/organic matter & decayed wood	56.4	60.3	94.4	715
8	29.0	Extremely soft gray clay w/silt pockets, shell fragments & roots	47.3	73.5	108.3	245
.0	39.0	Soft gray clay w/silt lenses	69.9	57.7	98.1	835

'Unconsolidated Undrained Triaxial Compression Test - One Specimen; Confined at the approximate overburden pressure.

## REFER TO NOTES ON DRAWING 27.

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U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA

LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN
ORLEANS AVE. OUTFALL CANAL PARALLEL PROTECTION PHASE II-A (EAST SIDE FLOODWALL)
STA. 3+60.00 E. B/L TO STA. 90+26.33 E. B/L
ORLEANS PARISH, LA. SOIL BORINGS

DESIGNED BY: DRAWN BY: CHECKED BY:	WOODS	DATE: SEP 96	PLOT SCALE:	SEP 96	-
		CADD FILE: fmoacwo.dgn		H-4-44644	
SUBMITTED BY:		SOLICITATIO	N NO.		-

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DESIGN ENGINEER DACW29-96-B-0096 DWG. 31 OF 32

of Your Contract

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				JNI	F	ED SOIL CLASSIFICATION
	MAJOR	DIVISION	TYPE	LETTER SYMBO		TYPICAL NAMES
	Λ je	40 C	CLEAN GRAVEL	GW	0.0	GRAVEL, Well Graded, gravel-sand mixtures, little or no fines
Constitution of the	SOIL.S	/ELS half action on No.	(Little or No fines)	GP		GRAVEL, Poorly Graded, gravel-sand mixtures, little or no fines
-		GRAVELS More than half o coarse fraction larger than No sieve size.	GRAVEL WITH FINES	GM	• ::	SILTY GRAVEL,gravel-sand-silt mixtures
	GKAINED of material eve size.		(Appreciable Amount of Fines)	GC		CLAYEY GRAVEL,gravel-sand-clay mixtures
	4- 10	0. 4 0. 4	CLEAN SAND	SW	000	SAND, Well-Graded, gravelly sands
11.0	AMSE Than half	SANDS than hall the fraction or than N	(Little or No Fines)	SP		SAND,Poorly-Graded,gravelly sands
The Co	COARSE More than than No. 20	SANDS More than half o coarse fraction smaller than No. sleve size.	SANDS WITH FINES (Appreciable Amount of Fines)	SM	0000	SILTY SAND, sand-silt mixtures
		Mor cog smg sfev		SC	%	CLAYEY SAND, sand-clay mixtures
	SOIL S terial		SILTS AND	ML		SILT & very fine sand, slity or clayey fine sand or clayey slit with slight plasticity
1	0.0		CLAYS	CL		LEAN CLAY, Sandy Clay, Silty Clay, of low to medium plasticity
	GRAINED half the m		< 50)	OL	The second	ORGANIC SILTS, and organic silty clays of low plasticity
-			SILTS AND	MH	Щ	SILT, fine sandy or silty soil with high plasticity
	FINE More than is smaller		CLAYS	CH		FAT CLAY, inorganic clay of high plasticity
	T ON S	5	> 50)	OH	11/1/2	ORGANIC CLAYS of medium to high plasticity,organic silts
-	HIGH	LY ORGANIC	SOILS	P†		PEAT, and other highly organic soil
		WOOD		Wd		MOOD
L	***************************************	SHELLS		SI		SHELLS
			NS		No Sample Retrieved	
L	***************************************	***************************************				
_	***************************************	12 T CONTROL OF THE STREET OF				
-					<u> </u>	
_	NOTE:	Soils po	ssessing	g char	acter	istics of two groups are designated by combinations of group symbols.

COLOR			riptive syn			***************************************	
COLOR	SYMBOL		CONSISTENCY FOR COHESIVE SOILS		MODIFICATIONS		
	1				MODIFICATION	SYMBOL	
TAN	T	CONSISTENCY	COHESION IN LBS./SQ.FT. FROM	SYMBOL	Traces	Tr	
YELLOW	Y		UNCONFINED COMPRESSION TEST		Fine	F	
RED	R	VERY SOFT	< 250	vSo	Medium	М	
BLACK	8K	SOFT	250-500	So	Cogrse	С	
GRAY	Gr	MEDIUM	500-1000	М	Concretions	cc	
LIGHT GRAY	lGr	STIFF	1000-2000	S†	Rootlets	rt	
DARK GRAY	dGr	VERY STIFF	2000-4000	vSt	Lignite fragments	lg	
BROWN	Br	HARD	> 4000	H	Shale fragments	sh	
LIGHT BROWN	IBr				Sandstone fragments	. sds	
DARK BROWN	dBr	60			Shell fragments	sif	
BROWNISH-GRAY	brGr	INDEX	July 1		Organic matter	0	
GRAYISH-BROWN	дуВг		"J" OH "A"		Clay strata or lenses	cs	
GREENISH-GRAY	gnGr	PLASTICITY	CH &		Silt strata or lenses	SIS	
GRAYISH-GREEN	gyGn	S			Sand strata or lenses	SS	
GREEN	Gn				Sandy	S	
BLUE	ВІ	•	MH on OH		Gravelly	G	
BLUE-GREEN	BiGn	a 7 - 1	CL-ML OR OL		Boulders	В	
WHITE	Wh	00	20 40 60 80 100		Slickensides	SL	
MOTTLED	Mot		L.LLIQUID LIMIT		Wood	Wd	
					0×idized	0×	
		OCO MATERIAL PROPERTY AND A STATE OF THE STA	PLASTICITY CHART				
		For classification	of fine-grained soils in accordance with .	ASTM D 2487			

	RES TO LEFT OF BORING UNDER COLUMN " W OR DIO"
Are	natural water contents in percent dry weight
When	underlined denotes D <sub>10</sub> size in mm*
FIGUR	ES TO LEFT OF BORING UNDER COLUMNS " LL" AND " PL"
Are li	quid and plastic limits, respectively
SYMB	OLS TO LEFT OF BORING
	Ground-water surface and date observed
0	Denotes location of consolidation test**
\$	Denotes location of consolidated-drained direct shear test**
®	Denotes location of consolidated-undrained triaxial compression test**
0	Denotes location of unconsolidated-undrained triaxial compression test $^st$
Ō	Denotes location of sample subjected to consolidation test and each of the above three types of shear test**
FW	Denotes free water encountered in boring or sample
FIGUR	ES TO RIGHT OF BORING
	lues of cohesion in lbs./sq.ft. from unconfined compression tests

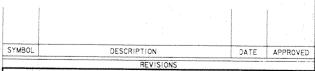
Where underlined with a solid line denotes laboratory permeability in centimeters per second of undisturbed sample

Where underlined with a dashed line denotes laboratory permeability in centimeters per second of sample remoulded to the estimated natural void ratio

- \*The D  $_{
  m 10}$  size of a soil is the grain diameter in millimeters of which 10% of the soil is finer, and 90% coarser than  $D_{10}$  .
- $^{**}$ Results of these tests are available for inspection in the U.S. Army Engineer District Office, if these symbols appear beside the boring logs on the drawings.

### TYPICAL NOTES:

- i. While the borings are representative of subsurface conditions at their respective locations and for their respective vertical reaches, local variations characteristic of the subsurface materials of the region are anticipated and, if encountered, such variations will not be considered as differing materially within the purview of the contract clause entitled "Differing Site Conditions".
- 2. Ground-water elevations shown on the boring logs represent ground-water surfaces encountered in such borings on the dates shown. Absence of water surface data on certain borings indicates that no ground-water data are available from the boring but does not necessarily mean that ground-water will not be encountered at the locations or within the vertical reaches of such borings.
- 3. Consistency of cohesive soils shown on the boring logs is based on driller's log and visual examination and is approximate, except within those vertical reaches of the borings where shear strengths from unconfined compression tests are shown.
- 4. Unless otherwise noted:
- a. Undisturbed borings, indicated by the letter "U", are taken with a 5" I.D. Piston Type Sampler.
- b. General type borings are taken with a 1  ${\it V_8}''$  i.D. Tube Sampler and/or a 1.3/8" I.D. Split Spoon Sampler.



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U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA

LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN
ORLEANS AVE. OUTFALL CANAL PARALLEL PROTECTION PHASE II-A (EAST SIDE FLOODWALL)
STA. 3+60.00 E. B/L TO STA. 90+26.33 E. B/L ORLEANS PARISH, LA.

# SOIL BORING LEGEND

DESIGNED BY: VOJKOVICH DRAWN BY: WOODS	DATE: SEP 96	PLOT SCALE:		SEP	96	
CHECKED BY: RICHARDSON	CADD FILE: orleand.DGN		FILE H-	NO. 4-4	46	44
SUBMITTED BY: X DESIGN ENGINEER	SOLICITATION DACW29-9	1 NO. 96-B-0096				32

