

Dunbar, J.

MISSISSIPPI RIVER LEVEES AND BANKS

MILE 66 TO MILE 10

SOIL REPORT - PART I



NEW ORLEANS

Myrtle Grove

Phoenix

Pointe à la Hache

Port Sulphur

Empire

Buras

Venice

Head of Passes

15

SOIL DATA
VOLUME 2-WEST BANK



AUGUST 1971

DEPARTMENT OF THE ARMY
NEW ORLEANS DISTRICT, CORPS OF ENGINEERS
NEW ORLEANS, LOUISIANA

MISSISSIPPI RIVER LEVEES AND BANKS

MILE 66 TO MILE 10

SOIL REPORT

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PREFACE

This report presents a compilation of available geology and soil data for the west bank of the Mississippi River from the vicinity of Jesuit Bend to Venice, Louisiana, a distance of about 56 river miles. The data presented include soil boring logs; results of laboratory tests; soil profiles; and soil stratification and shear strengths selected for design.

The design soil stratification and shear strengths presented in this report were selected by personnel of the New Orleans District and were reviewed and approved by representatives of the Mississippi River Commission.

The soil borings were made by field personnel of the New Orleans District under the supervision of Messrs. Roy V. Bankston (now retired) and Wayne W. Weiser, Field Investigations Section.

This report was prepared in the Structures Foundation Section by Mr. Rodney P. Picciola under the direction of Messrs. Herman A. Huesmann and Krum J. Cannon, Foundations and Materials Branch, New Orleans District. Geologic data presented were prepared by Mr. E. Burton Kemp III, Geologic Section.

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I - INTRODUCTION

1. Through the years, a voluminous amount of geologic and soil data has been collected from borings made on the banks of the Mississippi River below New Orleans, Louisiana, for revetment, levee, and various miscellaneous projects. In recent years, additional borings and soil data were obtained for use in study of hurricane protection projects.

2. This report presents a compilation of all available geologic and soil data for the west (right descending) bank of the Mississippi River between about river miles 10 and 66 AHP¹. This data was used in design of the Mississippi River and Tributaries Project (MR&T) levees; the New Orleans to Venice, Louisiana, Mississippi River Hurricane Protection Project levees; and bank revetments required within this stretch of the Mississippi River.

¹All mileages in this report are those above Head of Passes of the Mississippi River. This report was originally prepared to present soil data between river miles 10 and 66 AHP; however, the borings and soil profile within the limits of three additional miles (miles 66 to 69 AHP) have been included.

II - GEOLOGY

Physiography

3. The study area is located within the Central Gulf Coastal Plain. Specifically, the area is located on the modern subdelta which projects gulfward from the deltaic plain of the Mississippi River. It is a region of extremely low relief. Dominant physiographic features are the natural levees of the Mississippi River and abandoned distributaries, and the marshlands and inland bodies of water that lie between the natural levee ridges. Elevations range from a maximum of about 5 feet along the crests of the natural levees to a minimum of sea level or slightly lower in the marshlands between the natural levee ridges. The numerous inland bodies of water vary in depth from 1 to 6 feet. The Mississippi River channel varies in depth from 65 to 190 feet below sea level.

General Geology

4. Only the geologic history since the end of the Pleistocene Epoch is significant for this project. At that time, with sea level about 450 feet below its present level, the Mississippi River began to aggrade the final entrenchment which it had cut to the west of the project area during the last glacial period. Initial alluvial sedimentation was confined to the central portion of the alluvial valley. This sedimentation was accompanied by downwarping of the Pleistocene Prairie surface and some faulting resulting in a gulfward dip of the Prairie surface averaging about 3 feet per mile and increasing towards the coastline. Only minor amounts of dissection occurred on the Pleistocene as a result of estuaries and small streams. Sedimentation was insignificant in the study area prior to the time sea level reached about 200 feet below its present elevation. Most of the study area stood above sea level and only coarse fluvial materials were deposited

in the deep entrenchment to the west of the study area. The continued rise in sea level resulted in the reworking and redepositing of minor amounts of fluvial sediments in the study area. When sea level reached within tens of feet of its present level, the first marine and fluvial marine sediments of any significance were carried into the study area. Deltaic marine sediments were first introduced into the study area about 3,500 years ago when the Mississippi occupied the Teche course to the west of the study area. The first major advance of sediments occurred approximately 2,800 years ago when the Mississippi River shifted eastward and began to develop the La Loutre-St. Bernard Delta. About 1,500 years ago, the Mississippi River shifted westward to the Lafouche course and for a period of several hundred years the study area was subjected to only minor amounts of sedimentation and deltaic deterioration and subsidence became important. When the river again shifted eastward about 1,200 years ago and began to occupy the present Plaquemine course, sedimentation again became the predominant process in the study area. With the construction of levees along the Mississippi River, floodwaters have been eliminated from most of the area and at present only a small amount of sediments is being introduced into the area. The land mass along the edges of the study area is decreasing as a result of subsidence and erosion by wave action.

Subsidence and Erosion

5. Progressive subsidence and downwarping have been occurring in the study area since the end of the Pleistocene Epoch. The surface of the Pleistocene deposits have been downwarped toward the south and west to a maximum of about 500 feet at the edge of the continental shelf, which is 15 to 25 miles south of Venice, Louisiana. At present, the rate of subsidence in the study area varies between 0.5 and 1.0 ft per century.

6. As a result of subsidence and wave erosion, the seaward facing edges of the shoreline and the shorelines of the ponds, lakes, and bays within the marshlands are retreating.

Mineral Resources

7. Extensive oil, gas, and sulphur production are found in the general vicinity of the study area. Exploration and production of these mineral resources will not adversely affect the levee and bank stabilization projects, nor will the projects adversely affect the production of these resources.

III - SOIL DATA

8. Considerable soil and related data were obtained from the west bank of the Mississippi River between river miles 10 and 66 AHP to determine the characteristics of the subsurface materials for use in levee and bank revetment design. Undisturbed soil borings were made at numerous locations and supplemented with general type borings. Samples from the undisturbed borings were subjected to various laboratory tests to determine the consolidation and strength characteristics of the subsurface soils. The method used and the tests performed in the field and laboratory for collection of the soil data are discussed in the following paragraphs.

Borings

9. A total of 247 general type and undisturbed soil borings have been made on the west bank of the Mississippi River within the study area for the various levee and revetment projects. The type of project for which the soil data was required determined the depth to which each boring was made. Depths ranged down from the ground surface to about elevation -40 to -220 feet mean sea level² for borings made for levee projects, and down to about -110 to -210 for revetment projects. The undisturbed borings were made with a 5-inch diameter steel tube piston-type sampler and the general type borings with a 1-7/8 inch I.D. core barrel sampler. In addition to the 247 soil borings mentioned above, 13 general type borings were made in the river bottom between river miles 22 and 24 AHP to locate a possible source of sand borrow. Eight of the borings were made by the Louisiana Department of Highways and 5 by the Corps of Engineers. Also, a total of 54 auger borings were made to depths of 10 to 15 feet on the west bank batture between Empire and Venice, Louisiana, for investigations of this area as a possible source of borrow material for use in levee construction.

²Hereinafter, all elevations stated refer to feet mean sea level.

The borrow borings located on the batture were made by hand with a 4-inch diameter post hole auger. The locations of all the borings presented in this report are shown in plan on plates 2 to 15. In addition, a tabulated list of the locations of the general type and undisturbed borings is presented in Table 1. Tabulation of the borrow boring locations is not presented.

Boring Numbers

10. Through the years, various numbering systems were used to identify the borings for the various projects. As a guide in identifying the different borings included within this report, the following should prove helpful:

- a. All undisturbed borings are identified by the letter "U." (Example: 2-U or 29-MHU). Boring numbers without the letter "U" indicate general type borings or auger borings.
- b. The letters "H" or "MH" indicate borings made for the Mississippi River Hurricane Project. (Example: 9-UH or 25-MHU).
- c. The letter "R" as a prefix to the boring number indicates the boring was made for a revetment project. (Example: R-32.3-LU).
- d. As a suffix, the letter "W" or "R" indicates that the boring was made on the west (right descending) bank. (Example: 17-UW or R-43.9-RU). The letter "T" indicates a boring made at the levee toe. (Example: 25-MHUT).
- e. The letter "B" indicates a borrow boring. (Example: 4-B, B-7, or 3-BC).

Boring Logs

11. The borings made on the west bank of the river consisted of 187 general type, 60 undisturbed, and 54 auger borings. The detailed logs of the west bank general type borings are shown on plates 16 to 36 and the undisturbed borings on plates 37 to 86. The logs of the 13 general type borings made in the river bottom to locate a source of sand borrow are shown on plates 87 and 88. The auger type borrow borings are shown on plates 89 to 91.

Laboratory Tests

12. Visual classifications and water content determinations were

made on all samples from the borings. Unconfined-compression (UC) shear tests were made on typical clay samples. Unconsolidated-undrained (Q) and consolidated-undrained (R) triaxial compression tests; consolidated-drained (S) direct shear tests; and consolidation tests were performed on selected samples from the undisturbed borings. In addition, Atterberg limit determinations were performed on each sample subjected to a shear or consolidation test. Results of all laboratory tests performed are shown on the boring logs, plates 16 through 91. Detailed laboratory test data sheets for the (Q), (R), and (S) shear tests are shown in Appendix A to this report.

Foundation Conditions

Soil Profiles

13. Generalized profiles of the subsurface soils along the west bank of the river in the study area are shown on plates 92 to 94. It was impossible to show all available borings used in determining the profile in some areas because of the necessity for confining the horizontal scale; therefore, the profiles are based on the results of selected levee and revetment borings. The boring number of each boring used is shown on the profiles.

14. The subsurface on the west bank, as shown on plates 92 and 93, consists of Recent deposits varying in thickness from 90 feet at mile 69 AHP near the upstream end of the project to 260 feet at mile 10 AHP, the downstream end of the project. The Recent deposits are underlain by Pleistocene materials. Generally, the Recent consists of a surface layer of soft to stiff natural levee clays with layers and lenses of silt, varying in thickness from a minimum of 3 to 5 feet in the vicinity of stations 650+00 (mile 69.2), 2390+00 (mile 37.0), and between stations 3650+00 and 3800+00 (mile 12.9-10.8) to a maximum of 20 to 22 feet in the general vicinity of stations 1040+00 (mile 61.3), 1165+00 (mile 59.8), and 1295+00 (mile 57.5). In the vicinity of stations 967+00 through 1015+00 (mile 63.5-62.9); 1155+00 through

1255+00 (mile 59.9-58.4); 1330+00 through 1575+00 (mile 56.8-52.9); 1920+00 through 2117+00 (mile 45.7-41.9); 2177+00 through 2466+00 (mile 40.7-35.5); 2650+00 through 2770+00 (mile 31.5-29.6); 2895+00 through 3005+00 (mile 27.2-25.3); and 3223+00 through 3277+00 (mile 21.5-20.6), the natural levee deposits are underlain by a discontinuous layer of very soft marsh clays with peat and organic matter. The marsh deposits vary in thickness from 2 feet in the vicinity of stations 1450+00 (mile 54.7), 2177+00 (mile 40.7), and 2700+00 (mile 30.6), to 7 to 9 feet between stations 1155+00 and 1255+00 (mile 59.9-58.4). Between stations 690+00 (mile 68.6) and 840+00 (mile 65.8); 925+00 (mile 66.3) and 947+00 (mile 63.9); 2117+00 (mile 41.9) and 2466+00 (mile 35.5); 2650+00 (mile 31.5) and 3066+00 (mile 23.7); 3135+00 (mile 22.8) and 3195+00 (mile 21.8); and 3370+00 (mile 18.5) and 3797+64 (mile 10.8), the natural levee and marsh deposits are underlain by soft, alternating intradelta clays and silts with layers of silty sand and sand. The thickness of the intradelta deposits varies from 25 feet between stations 3157+00 (mile 22.3) and 3195+00 (mile 21.8) to about 97 feet in the general vicinity of station 2685+00 (mile 30.8). The reaches of natural levee and marsh deposits between stations 650+00 (mile 69.2) and 690+00 (mile 68.6); 860+00 (mile 65.4) and 907+00 (mile 64.6); 967+00 (mile 63.5) and 1015+00 (mile 62.6); 1155+00 (mile 59.9) and 1255+00 (mile 58.4); 1280+00 (mile 57.8) and 1575+00 (mile 52.9); 1717+00 (mile 49.6) and 1765+00 (mile 48.6); 1920+00 (mile 45.7) and 2117+00 (mile 41.9); 3210+00 (mile 21.6) and 3230+00 (mile 21.3); and 3242+00 (mile 21.1) and 3310+00 (mile 19.7) are underlain by very soft to soft interdistributary clays with lenses and layers of silt and silty sand. The interdistributary deposits vary in thickness from 20 feet in the general vicinity of station 2080+00 (mile 42.4) to about 78 feet in the vicinity of station 3300+00 (mile 19.8). The remaining reaches of natural levee and marsh deposits are underlain by point bar silts, silty sands, and sands with layers of clay between stations 1015+00 (mile 62.6) and 1155+00 (59.9); 1575+00 (mile 52.9) and 1717+00 (mile 49.6); 1765+00 (mile 48.6) and 1920+00 (mile 45.7); 2466+00

(mile 35.5) and 2650+00 (mile 31.5); and 3310+00 (mile 19.7) and 3370+00 (mile 18.5); and by abandoned distributary silty sands and sands with clay layers between stations 840+00 (mile 65.8) and 860+00 (mile 65.4); 907+00 (mile 64.6) and 925+00 (mile 66.3); 947+00 (mile 63.9) and 967+00 (mile 63.5); 1255+00 (mile 58.4) and 1280+00 (mile 57.8); 3077+00 (mile 23.7) and 3135+00 (mile 22.8); 3195+00 (mile 21.8) and 3210+00 (mile 21.6); and 3230+00 (mile 21.3) and 3242+00 (mile 21.1). The point bar deposits vary in thickness from 66 feet in the vicinity of station 1015+00 (mile 62.6), where the deposits extend down to elevation -73.5, to 127 feet near stations 1765+00 (mile 49.6) and 2600+00 (mile 32.4) where these deposits extend down to elevations -130 and -136.5, respectively. The abandoned distributary deposits vary in thickness from 52 feet in the vicinity of station 1255+00 (mile 58.4), where the deposits extend down to elevation -59.5, to 138 feet in the vicinity of station 3195+00 (mile 21.8) where these deposits extend down to elevation -145. The intradelta, interdistributary, point bar, and abandoned distributary deposits are underlain along the entire levee alignment by medium to stiff prodelta clays, except between stations 840+00 (mile 65.8) and 860+00 (mile 65.4), and stations 947+00 (mile 63.9) and 967+00 (mile 63.5), where abandoned distributary deposits lie directly over Pleistocene and nearshore deposits, respectively; and between stations 1015+00 (mile 62.6) and 1155+00 (mile 59.9); 1575+00 (mile 52.9) and 1717+00 (mile 49.6); and 1765+00 (mile 48.6) and 1920+00 (mile 45.7), where point bar deposits lie directly over Pleistocene and nearshore deposits. The thickness of the prodelta clays varies from 8.5 feet (to elevation -145) beneath the point bar deposits located between station 2466+00 (mile 35.5) and 2650+00 (mile 31.5), to 135 feet (to elevation -230) in the vicinity of station 3797+64 (mile 10.8). The prodelta deposits are underlain along the entire levee alignment by nearshore sands with shell and shell fragments except between stations 650+00 (mile 69.2) and 907+00 (mile 64.6) where the prodelta lies directly over Pleistocene materials. The nearshore deposits vary in thickness from 3 feet in the vicinity of station 2685+00 (mile 30.8)

to 34 feet in the vicinity of station 1650+00 (mile 50.9). The entire sequence of Recent deposits, including a 60 ft wedge of estuarine sands, silts and clays with shell fragments that underlie the nearshore deposits between stations 1300+00 (mile 57.4) and 1450+00 (mile 54.6), is underlain by stiff to very stiff Pleistocene clays at elevations varying between -83.5 at station 650+00 (mile 69.2) to -260.0 at station 3797+64 (mile 10.8).

Soil Stratification

15. To facilitate levee and bank revetment designs, the bank of the river was divided into soil reaches. The extent of each was determined from the soil profiles and was based on the stratification and types of subsurface soils and the concept that each reach was generally uniform in composition and stratification. The reaches selected are shown on the soil profiles, plates 92 and 93. In addition, the soil stratification for use in bank revetment design at Fort Jackson, Louisiana, was based on the soil profile shown on plate 94.

Shear Strength Data

16. The results of all laboratory shear tests performed are shown on the boring logs. The shear strengths selected for use in levee and bank revetment design are shown on the undisturbed boring logs, plates 37 to 86. Also, the design shear strengths, together with the soil stratification selected for each soil reach, are shown tabulated in Table 2. The design shear strengths and soil stratification for groups of reaches were reviewed and approved by representatives of the Mississippi River Commission, progressively as completed.

Clay Strengths

17. The shear strengths for clay soils selected for use in levee and bank revetment designs were based on the results of the unconsolidated-undrained (Q) triaxial compression shear tests. The preconsolidation pressures shown on the undisturbed boring logs agree closely with the overburden pressure, thus indicating that the clay soils are

normally consolidated. It was determined that the undrained (Q) shear strengths of the normally consolidated clays above elevation -10 to -40 are essentially constant and range between 0.15 and 0.40 ton per sq ft beneath existing levee fill, and between 0.10 and 0.25 ton per sq ft beyond the influence of levee fill. Below these elevations, the undrained shear strength increases with depth according to the relation defined by the ratio $c/\bar{p}=0.25$, where c is the undrained shear strength in tons per square foot and \bar{p} is the effective overburden pressure in tons per square foot. Based on the above c/\bar{p} ratio and the results of the (Q) shear tests, it was determined that with few exceptions, the undrained shear strength of the Recent clay soils below elevation -10 to -40 increases at a rate of 0.005 ton per sq ft per ft of depth.

Silt Strength

18. A shear strength value of $\phi=15^\circ$, $c=0.10$ ton per sq ft, was selected for silt (ML). This value is based on the results of the unconsolidated-undrained (R) shear tests. In determining the shear strength, the strength envelopes used were based on the deviator stresses at maximum positive pore pressures.

Sand Strength

19. The shear strength selected for silty sand (SM) and sand (SP) was based on the results of the consolidated-drained (S) shear tests. A value of $\phi=30^\circ$, $c=0$ was selected for levee and bank revetment designs.

TABLE 1
SOIL BORING LOCATIONS

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
WEST BANK

SOIL BORING LOCATIONS

SOIL BORING NO.	LEVEE B/L		SOIL BORING NO.	LEVEE B/L	
	STATION	LOCATION		STATION	LOCATION
87-MHU	654+00	on C/L	R-60.9 R	✓ 1098+50	'78' R.S.
86-MH	677+00	66' R.S. @ Toe	MS-27	✓ 1101+67	1120' L.S. C/L
85-MH	699+00	57' R.S. @ Toe	69-MH	✓ 1111+00	49' L.S. C/L
84-MH	721+00	on C/L	R-60.4 R	✓ 1125+50	135' R.S.
83-MH	747+00	52' R.S. @ Toe	68-MH	✓ 1137+00	C/L
82-MHUT	773+00	62' L.S.	67-MHUT	✓ 1164+00	70' L.S. C/L
81-MH	799+00	C/L	R-59.7 R	✓ 1176+00	125' R.S.
R-66.7 UR	804+50	346' R.S.	66-MH	✓ 1189+00	50' R.S. C/L
R-66.4 R	818+50	332' R.S. C/L	R-59.5U R	✓ 1195+61.8	63' R.S. C/L
80-MH	825+00	55' R.S. @ Toe	8 N.P.	✓ 1196+16.8	404' R.S. C/L
R-65.9-R	✓ 849+40	64' R.S. C/L	65-MH	✓ 1215+00	C/L
79-MH	✓ 851+00	50' L.S. @ Toe	R-58.8 RU	✓ 1232+50	140' R.S. C/L
R-65.6 R	✓ 856+50	114' R.S.	64-MH	✓ 1241+00	63' L.S. C/L
78-MH	✓ 877+00	C/L	63-MH	✓ 1267+00	53' R.S. C/L
77-MH	✓ 903+00	50' L.S. C/L	R-57.7 R	✓ 1286+12	145' R.S. C/L
R-64.5. R	✓ 909+00	78' R.S.	62-MHU	✓ 1293+00	C/L
76-MHUT	✓ 929+20	65' R.S. C/L	61-MH	✓ 1319+00	60' L.S. C/L
75-MH	✓ 955+00	C/L	60-MH	✓ 1345+00	C/L
R-63.5R	✓ 969+00	175' R.S.	59-MH	✓ 1371+00	48' R.S. C/L
74-MH	✓ 981+00	55' L.S. C/L	R-55.9 R	✓ 1379+00	180' R.S.
R-62.9R	✓ 1001+50	200' R.S.	58-MH	✓ 1397+00	70' L.S. @ Toe
73-MH	✓ 1007+00	47' R.S. C/L	57-MHU	✓ 1423+00	C/L
72-MH	✓ 1033+00	C/L	57-MHUT	✓ 1423+00	70' L.S. C/L
1	✓ 1041+00	C/L	56-MH	✓ 1449+00	62' L.S. @ Toe
71-MH	✓ 1059+00	53' L.S. C/L	R-54.5 RU	✓ 1453+50	185' R.S. C/L
R-61.6R	✓ 1067+50	84' R.S.	55-MH	✓ 1476+00	50' R.S. @ Toe
70-MH	✓ 1085+00	C/L	54-MH	✓ 1501+00	C/L

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
WEST BANK

SOIL BORING LOCATIONS

SOIL BORING NO.	LEVEE B/L		SOIL BORING NO.	LEVEE B/L	
	STATION	LOCATION		STATION	LOCATION
R-53.3R ✓	1522+50	110' R.S.	38-MH	1956+00	C/L
53-MH ✓	1527+00	51' L.S. @ Toe	37-MH	1982+00	60' R.S. C/L
52-MH ✓	1555+50	50' R.S. @ Toe	36-MHU	2008+00	C/L
MS-31 ✓	1579+19	C/L	36-MHUT	2008+00	56' L.S. C/L
52-UW ✓	1587+00	C/L	R-43.9 RU	2014+50	283' R.S. C/L
52-TUW ✓	1587+00	80' L.S. C/L	35-MH	2034+00	C/L
51-MH	1618+00	54' R.S. C/L	34-MH	2064+00	58' R.S. @Toe
50-MH	1644+00	C/L	33-MH	2087+00	38' L.S. @TOE
R-50.9 R	1650+00	190' R.S.	42-UW	2112+00	C/L
49-MH	1670+00	55' L.S. @Toe	42-UWT	2112+00	111' L.S.
R-50.2 R	1682+00	128' R.S.	32-MH	2141+00	50' L.S. @ Toe
48-MH ✓	1696+00	48' R.S. C/L	31-MH	2167+00	C/L
R-49.7 RU	1709+50	185' R.S.	30-MH	2193+00	52' L.S. @ TOE
47-MH	1722+00	C/L	29-MHUT	2212+75	70' R.S. C/L
R-49.2 R	1739+50	145' R.S.	29-MHU	2219+00	C/L
46-MH	1748+00	65' L.S. @Toe	R-39.8R	2235+00	112' R.S.C/L
R-48.6 R	1766+00	110' R.S.	28-MH	2245+00	45' L.S. @Toe
45-MH	1774+00	50' R.S. @Toe	6	2257+35	100' R.S.
R-48.0 R	1796+00	200' R.S.	1-U	2272+00	57' R.S.
44-MH	1800+00	C/L	5	2272+90	240' L.S.
1-U	1808+15	65' R.S.C/L	3	2273+80	142' R.S.
43-MHUT	1826+00	45' L.S. @Toe	R-38.6 R	2288+00	262' R.S.C/L
42-MH	1852+00	38' R.S. @Toe	27-MH	2299+00	C/L
R-46.7 R	1861+00	327' R.S.	R-38.3R	2311+50	305' R.S.C/L
41-MH	1878+00	C/L	26-MH	2326+93	50' R.S. @Toe
40-MH	1904+00	45' L.S. @Toe	R-38.0 R	2331+50	230' R.S.C/L..
39-MH	1930+00	62' R.S. @Toe	1	2338+00	49' L.S.C/L

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
WEST BANK

SOIL BORING LOCATIONS

SOIL BORING NO.	LEVEE B/L		SOIL BORING NO.	LEVEE B/L	
	STATION	LOCATION		STATION	LOCATION
2	2348+00	45'L.S.C/L	1-U	2555+00	C/L
1-U	2357+50	C/L	1-UT	2555+00	82'L.S.C/L
1-UT	2357+50	55'L.S.@Toe	8	2565+00	56'L.S.C/L
3	2368+00	57'L.S.C/L	9	2576+00	40'L.S.C/L
R-37.2 R	2372+50	208'R.S.C/L	10	2588+00	58'L.S.C/L
4	2378+00	55'L.S.C/L	R-32.4 UR	2597+00	710'R.S.
37.0	2388+50	70'R.S.C/L	11	2598+00	49'L.S.C/L
R-36.6 UR	2406+00	185'R.S.C/L	23-MH	2605+00	45'R.S.@Toe
1	2415+00	58'L.S.C/L	12	2608+00	50'L.S.C/L
2	2425+00	60'L.S.C/L	1	2620+00	44'L.S.C/L
R-36.2 R	2428+50	302'R.S.C/L	R-32.0 R	2624+00	135'R.S.
1-U	2435+00	C/L	2	2630+00	63'L.S.C/L
1-UT	2435+00	@L.S. Toe	3	2640+00	50'L.S.C/L
3	2444+00	59'L.S.C/L	1-U	2654+00	C/L
R-35.6 R	2452+00	424'R.S.C/L	1-UT	2654+00	70'L.S.C/L
4	2455+00	62'L.S.C/L	4	2660+00	72'L.S.C/L
25-MH	2470+00	37'R.S.@Toe	5	2670+00	58'L.S. C/L
R-35.4 R	2474+50	440'R.S.C/L	22-MH	2675+00	46'R.S.@Toe
1	2485+15	35'L.S. C/L	6	2680+00	44'L.S. C/L
2	2494+00	40'L.S.C/L	R-30.9UR	2683+58	140'R.S.
R-34.9 R	2498+50	136'R.S.C/L	R-30.8	2695+00	21'R.S.C/L
3	2505+00	54'L.S.C/L	21-MH	2703+00	C/L
4	2515+00	60'L.S.C/L	R-30.3 R	2723+00	227'R.S.
5	2525+00	42'L.S.C/L	20-MH	2730+00	40'R.S.@Toe
24-MH	2530+00	38'R.S.@Toe	19-MH	2755+00	50'L.S.@Toe
6	2536+00	60'L.S.C/L	18-MHU	2781+00	C/L
7	2545+00	50'L.S.C/L	17-MH	2804+00	44'R.S.@Toe

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
WEST BANK

SOIL BORING LOCATIONS

SOIL BORING NO.	LEVEES B/L		SOIL BORING NO.	LEVEE B/L	
	STATION	LOCATION		STATION	LOCATION
R-28.55 R	2822+00	70' R.S.	R-23.4	3097+76.3	130' R.S.
16-MH	2833+00	50' L.S. @ Toe	5-MHU	3113+25	C/L
R-28.05-R	2849+50	165' R.S. C/L	5-MHUT	3113+25	75' L.S. C/L
15-MH	2859+00	C/L	5-MH	3113+50	53' R.S. @ Toe
R-27.6UR	2874+25	63' R.S. C/L	R-23.05 RU	3114+00	475' R.S. C/L
14-MH	2885+00	48' L.S. @ Toe	4-MH	3143+00	54' L.S. @ Toe
R-27.3 R	2894+75	110' R.S. C/L	3-MHU	3169+00	C/L
13-MH	2911+00	43' R.S. @ Toe	3-MHUT	3169+00	75' L.S. C/L
R-26.8 R	2917+00	168' R.S. C/L	.5	3181+51	5' R.S.
12-MH	2935+00	C/L	5-A	3181+75	60' R.S.
R-26.45 R	2940+50	100' R.S. C/L	4	3201+10	50' R.S.
1	2952+55	125' R.S.	3	3211+45	60' R.S. C/L
11-MH	2961+00	37' R.S. C/L	1-U	3216+00	235' R.S.
R-25.8 R	2975+75	96' R.S. C/L	2-U	3218+00	650' R.S. B/L
10-MHUA	2987+00	C/L	2-MH	3221+00	43' L.S. @ Toe
10-MHUT	2987+00	93' L.S. C/L	2	3232+20	27.5' R.S.
2	2991+50	90' R.S.	1	3241+50	290' R.S. C/L
3	3009+15	100' R.S.	1-MH	3247+00	C/L
9-MH	3013+00	C/L	1	3251+00	30' L.S. C/L
4	3021+75	125' R.S.	R-20.8-R	3252+00	125' R.S. C/L
R-24.75 R	3029+00	150' R.S. C/L	2	3261+00	43' L.S. C/L
5	3037+00	180' R.S.	3	3271+00	42' L.S. C/L
8-MH	3039+00	40' R.S. @ Toe	1-U	3282+00	C/L
R-24.3 UR	3050+00	167' R.S.	1-UT	3282+00	42' L.S. @ Toe
R-24.0	3063+60	242' R.S.	R-20.2-R	3289+00	1235' R.S. C/L
7-MH	3065+00	44' L.S. @ Toe	4	3291+00	45' L.S. C/L
6-MH	3091+00	C/L	5	3302+00	50' L.S. C/L

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
WEST BANK

SOIL BORING LOCATIONS

SOIL BORING NO.	LEVEES B/L		SOIL BORING NO.	LEVEES B/L	
	STATION	LOCATION		STATION	LOCATION
R-19.6-R	3311+00	1118' - N50°35'W	1-U	3784+00	56' R.S. B/L
6	3312+00	58' L.S. C/L	1-UT	3784+00	30' L.S. B/L
7	3326+00	56' L.S. C/L	15	3787+00	100' L.S.
8	3340+00	46' L.S. C/L	16	3797+00	100' L.S.
9	3354+00	70' L.S. C/L			
9-UH	3355+00	C/L			
9-UHT	3355+35	50' R.S. C/L			
1	3396+00	100' L.S.C/L			
2	3421+00	100' L.S.C/L			
17-UW	3439+75	C/L			
17-UWT	3439+75	72' L.S. C/L			
R-16.9 RU	3439+75	90' R.S. C/L			
3	3478+50	100'L.S. C/L			
4	3504+00	100'L.S. C/L			
5	3531+50	100'L.S. C/L			
6	3560+00	100;L.S. C/L			
7	3587+50	100' L.S.			
8	3626+00	100' L.S.			
9-UT	3650+60	100' L.S.C/L			
10	3674+50	100' L.S.			
11	3697+50	100' L.S.			
R-11.5-RU	3726+00	200' R.S.C/L			
12	3726+00	100' L.S.			
12-UH	3727+00	C/L			
13	3749+50	100' L.S.			
2-UT	3767+56	25' L.S.B/L			
14	3774+00	100' L.S.			

TABLE 2
DESIGN SHEAR STRENGTHS
AND
SOIL STRATIFICATION
(SOIL REACHES A THROUGH V-2)

WEST	LEVEE	REACH NO.	A	LOCATION	STA 828+00 to 1020+00
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UNDIST. BORING NO. 87-MHU UNDIST. BORING NO. 82-MHUT

BENEATH C/L OF LEVEE				BENEATH TOE OF LEVEE			
SOIL TYPE	*ELEV. (M.S.L.)	STRENGTH PARAMETERS		SOIL TYPE	*ELEV. (M.S.L.)	STRENGTH PARAMETERS	
		τ (P.C.F.)	C AVG.			(P.S.F.) BASE	ϕ
CL-ML	6.0	115	600	-	0	-	-
CH	0.0	115	600	-	0	CH	0.0
CH	-6.0	53	600	-	0	CH	-6.0
CH	-20.0	43	600	600	0	CH	-20.0
ML	-26.0	55	200	660	15	ML	-26.0
CH	-36.0	43	710	760	0	CH	-36.0
ML	-40.0	55	200	800	15	ML	-40.0
CH	-80.0	43	1000	1200	0	CH	-80.0
CH	-93.0	50	1265	1330	0	CH	-93.0
SP-F	-102.0	60	0	-	30	SP-F	-102.0
PLEIST	-	60	1320	-	0	PLEIST	-

*Elevations denote bottom of each soil strata.

*Elevations denote bottom of each soil strata.

WEST LEVEE REACH NO. B LOCATION STA. 1020+00 TO 1153+30

UNDIST. BORING NO. 62-MHU

UNDIST. BORING NO. 67-MHU; 70' L.S.

BENEATH C/L OF LEVEE				BENEATH TOE OF LEVEE				STRENGTH PARAMETERS			
SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS		SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS		σ_c (P.C.F.)	C AVG,	(P.S.F.) BASE	ϕ
		(P.C.F.)	Avg.			(P.S.F.)	Base				
CH	0.0	114	800	0	CH	0.0	110	500	-	-	0
CH	-2.0	52	800	0	CH	-2.0	48	500	500	500	0
ML	-5.0	55	200	600	15	ML	-5.0	55	200	500	15
CH	-22.0	43	600	600	0	CH	-22.0	40	500	500	0
ML	-25.0	55	200	600	15	ML	-25.0	55	200	500	15
CH	-38.0	43	600	600	0	CH	-38.0	43	500	500	0
ML	-49.0	55	200	690	15	ML	-49.0	55	200	500	15
CL	-57.0	43	730	770	0	CL	-57.0	43	530	570	0
SM-SP	-68.0	60	0	880	30	SM-SP	-68.0	60	0	680	30
CH	-71.0	43	895	910	0	CH	-71.0	43	695	710	0
SP-F	-80.0	60	0	1000	30	SP-F	-80.0	60	0	800	30
CH	-97.0	48	1085	1170	0	CH	-97.0	43	885	970	0
SM	-102.0	60	0	1220	30	SM	-102.0	60	0	1020	30
CH	-122.0	48	1320	1420	0	CH	-122.0	43	1120	1220	0
Pleist.	-	60	1500	-	0	Pleist.	-	60	1500	-	0

LEVEE WEST

REACH NO. C-1

LOCATION STA. 1153* 30T0 1255+00

UNDIST. BORING NO.

62

67-NHTR: 2015

67-MHO; 701 L.S.

UNDIST. BORING NO.

WEST	LEVEE	REACH NO.	LOCATION	STA.
		C-2		1255+00 TO 1334+00

INDIST BOBING NO 62 MINI

67-MHU5; 70, 11.S.

LOCATION STA. 12 BORING NO. 11011

INDI ST BOPING NO

WEST LEVEE

BENEATH C/L OF LEVEE		STRENGTH PARAMETERS				BENEATH TOE OF LEVEE			
SOIL TYPE	ELEV. (M.S.L.)	σ_c (P.C.F.)	C AVG.	(P.S.F.) BASE	ϕ	SOIL TYPE	ELEV. (M.S.L.)	σ_c (P.C.F.)	(P.S.F.) BASE
CH	5.0	114	800	-	0	CH	0.0	110	500
CH	0.0	110	800	-	0	CH	-2.0	48	500
CH	-2.0	48	800	600	0	CH	-18.0	40	500
CH	-18.0	43	600	-	0	CHO	-23.0	28	500
CHO	-23.0	32	600	-	0	CHO	-30.0	43	500
CH	-30.0	43	600	600	0	ML	-40.0	55	500
ML	-40.0	55	200	600	15	CH	-50.0	43	500
CH	-50.0	43	650	700	0	CH	-60.0	43	550
CH	-60.0	43	750	800	0	Pleist.	-117.0	43	885
CH	-117.0	48	1085	1370	0	Pleist.	-	60	1170
Pleist.	-	60	1500	-	0				0

WEST	LEVEE	REACH NO.	D	LOCATION	STA. 1334+00 TO 1575+00
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UNDIST. BORING NO.: 57-MHU
UNDIST. BORING NO.: 57-MHUT; 70' L.S.

57-MHUT; 701 L.S.

UNDIST. BORING NO.

57-MHU

WEST	LEVEE	REACH NO.	E	LOCATION
				STA. 1575+00 TO 1678+00

UNDIST. BORING NO. 52-UW 52-TTW: 80' I.s.

32-UW

STA. 1575+00 TO 1678+00

52-111W: 80' T.L.S.

INDISTINCT BOPING NO

LOCATION STA. 1575+00 TO 1678+00

52-PIW: 80' I.S.

INDIST BOBLING NO
32-FWU: 88, L.S.

WEST LEVEE REACH NO. H LOCATION Sta. 1890+00 to 1920+00
 UNDIST. BORING NO. 52-UW UNDIST. BORING NO. 43-MHUT; 45' L.S.

BENEATH C/L OF LEVEE				BENEATH TOE OF LEVEE			
SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS		SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS	
		σ (P.C.F.)	C AVG.			σ (P.C.F.)	C AVG.
CH	3.0	112	800	500	0	-	-
CH	0.0	112	500	-	0	CH	0.0
CH	-17.0	50	500	500	0	CH	-17.0
ML	-23.0	55	200	500	15	ML	-23.0
CH	-30.0	43	500	-	0	CH	-30.0
CH	-35.0	43	500	500	0	CH	-35.0
ML	-41.0	55	200	500	15	ML	-41.0
CH	-50.0	43	500	500	0	CH	-50.0
ML	-55.0	55	200	-	15	ML	-55.0
SP-F	-100.0	60	0	1000	30	SP-F	-100.0
CH	-115.0	53	1075	1150	0	CH	-115.0
SP-F	-128.0	60	0	1500	30	SP-F	-128.0
PLEIST	-	60	1500	-	0	PLEIST.	-

WEST	LEVEE	REACH NO.	I	LOCATION	STA. 1920+00 TO 2093+00
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UNDIST. BORING NO. 36-MHU UNDIST. BORING NO. 36-MHUT; 56' L.S.

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BENEATH C/L OF LEVEE				BENEATH TOE OF LEVEE			
SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS		SOIL TYPE (M.S.L.)	ELEV. (M.S.L.)	STRENGTH PARAMETERS	
		σ (P.C.F.)	C (P.S.F.)			ϕ	(P.C.F.)
CH	2.0	110	600	300	CH	2.0	
CH	0.0	94	300	0	CH	0.0	
CH	-10.0	32	300	0	CH	-10.0	
CH	-25.0	48	500	0	CH	-25.0	
CHO	-30.0	38	600	-	CHO	-30.0	
CH	-40.0	45	600	-	CH	-40.0	
CH	-50.0	45	600	600	CH	-50.0	
CH	-122.0	45	960	1320	0	CH	-122.0
SP-F	-132.0	60	0	1320	30	SP-F	-132.0
Pleist.	-	60	1320	-	0	Pleist.	-
							60
							1320
							-
							0

WEST

LEVEE

REACH NO.

LOCATION

Sta.: 2093+00 to 2130+00

UNDIST. BORING NO.

10 1117

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42 - UW

420 U.S. 111.

BENEATH C/L OF LEVEE				STRENGTH PARAMETERS				BENEATH TOE OF LEVEE				STRENGTH PARAMETERS			
SOIL TYPE	ELEV. (M.S.L.)	σ (P.C.F.)	C (P.S.F.)	σ (P.C.F.)	C (P.S.F.)	σ (P.C.F.)	C (P.S.F.)	ELEV. (M.S.L.)	σ (P.C.F.)	C (P.S.F.)	σ (P.C.F.)	C (P.S.F.)	σ (P.C.F.)	C (P.S.F.)	
CH	3.0	110	800	500	0	CH	0.0	110	200	-	0	0	1390	0	
CH	0.0	110	500	-	0	CH	0.0	110	200	-	0	0	200	0	
CH	-10.0	48	500	500	0	CH	-10.0	48	200	200	0	0	200	0	
ML	-16.0	55	200	560	15	ML	-16.0	55	200	260	15	0	1500	30	
CL	-19.0	48	575	590	0	CL	-19.0	48	275	290	0	0	290	0	
ML	-32.0	55	200	720	15	ML	-32.0	55	200	420	15	0	420	15	
CH	-35.0	48	735	750	0	CH	-35.0	48	435	450	0	0	450	0	
ML	-38.0	55	200	-	15	ML	-38.0	55	200	-	15	0	-	15	
SM	-46.0	60	0	860	30	SM	-46.0	60	0	560	30	0	560	30	
CH	-50.0	48	880	900	0	CH	-50.0	48	580	600	0	0	600	0	
SP-F	-90.0	60	0	1000	30	SP-F	-90.0	60	0	1000	30	0	1000	30	
CH	-129.0	53	1195	1390	0	CH	-129.0	53	1195	1390	0	0	1390	0	
SP-F	-135.0	60	0	1500	30	SP-F	-135.0	60	0	1500	30	0	1500	30	
PLEIST.	-	60	1500	-	0	PLEIST.	-	60	1500	-	0	0	-	0	

WEST LEVEE REACH NO. K LOCATION Sta. 2130+00 to 2338+00

UNDIST. BORING NO. 29-MHU UNDIST. BORING NO. 29-MHU

BENEATH C/L OF LEVEE				BENEATH TOE OF LEVEE				STRENGTH PARAMETERS			
SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS		SOIL TYPE	ELEV. (M.S.L.)	(P.C.F.)		(P.C.F.)	C (P.S.F.)	C (P.S.F.)	φ
		σ (P.C.F.)	C AVG.			φ	BASE		AVG,	BASE	φ
CH-CL	3.0	110	800	400	0						
CH	0.0	110	400	-	0	CH	0.0	100	350	-	0
CH	-8.0	48	400	-	0	CH	-8.0	38	350	-	0
CHO	-13.0	43	400	400	0	CHO	-13.0	38	350	350	0
ML	-16.0	55	200	400	15	ML	-16.0	55	200	350	15
CH	-20.0	48	400	400	0	CH	-20.0	38	350	-	0
CH	-27.0	43	600	-	0	CH	-27.0	38	350	350	0
CH	-44.0	43	600	600	0	CH	-44.0	38	600	600	0
ML	-50.0	55	200	600	15	ML	-50.0	55	200	600	15
CH	-60.0	48	600	600	0	CH	-60.0	48	600	600	0
CH	-80.0	48	700	800	0	CH	-80.0	48	700	800	0
CH	-100.0	48	900	1000	0	CH	-100.0	43	900	1000	0
CH	-135.0	48	1175	1350	0	CH	-135.0	48	1175	1350	0
SIP-F	-140.0	60	0	1400	30	SP-F	-140.0	60	0	1400	30
PLEIST.	-	60	1400	-	0	PLEIST.	-	60	1400	-	0

REACH NO.	M	LOCATION	Sta. 2383+00 To 2457+00
LEVEE	West		

UNDIST. BORING NO. 1-U INDIST BORING NO 1-UT; 46' L.S.

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DENGAN C/L OF LEVEE BENEATH TOE OF LEVEE

SOIL	ELEV.	SITE	STRENGTH PARAMETERS
JINSHIJI FANGMILENG	1000	SOIL 1	E=1000

TYPE	(M.S.R.)	G	C	(P.S.F.)	TYPE	(M.S.L.)	G	C	(P.S.F.)
	(M.S.R.)	(M.S.E.)	(M.S.E.)	(M.S.E.)		(M.S.L.)	(M.S.E.)	(M.S.E.)	(M.S.E.)

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CH 3.0 120 800 500 0

CH 0.0 110 500 - 0 CH 0.0 110 400 - 0

CH -20.0 43 500 - 0 CH -20.0 38 200 200 0

CH -35.0 43 300 500 0 CH -35.0 38 275 350 0

ML -41.0 55 200 50 15 ML -41.0 55 200 410 15

CH = -30.0 48 810 830 0 CH = -30.0 48 455 500 0

CH -140.0 48 1150 1550 0 CH -140.0

SP-F -150.0 60 0 1550 30 SP-F -150.0 60 0 1550 30

CH -165.0 48 1600 1650 0 CH -165.0

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LEVEE _____

REACH NO. _____ N-1

LOCATION

Sta. 2457+00 to 2570+00

UNDIST. BORING NO.

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INNIST BOP INC NO

L-Unit: 82, T.S.

BENEATH C/L OF LEVEE		STRENGTH PARAMETERS						BENEATH TOE OF LEVEE				STRENGTH PARAMETERS			
SOIL TYPE	ELEV. (M.S.L.)	σ (P.C.F.)	C AVG.	(P.S.F.) BASE	ϕ	SOIL TYPE	ELEV. (M.S.L.)	σ (P.C.F.)	C AVG., BASE	(P.S.F.) BASE	ϕ				
CH	3.0	115	600	400	0	CH	0.0	115	400	400	-	0			
CH	0.0	115	400	-	0	CH	-4.0	53	400	53	400	400	0		
CH	-4.0	53	400	400	0	CH	-11.0	200	400	15	ML	-11.0	55	200	
ML	-11.0	55	200	400	15	ML	-18.0	48	400	0	CH	-18.0	48	300	
CH	-18.0	48	400	400	0	CH	-138.0	60	0	SP-F	-138.0	30	300	0	
SP-F	-138.0	60	0	1380	30	SP-F	-158.0	48	1480	1580	0	1380	30		
CH	-158.0	48	1480	1580	0	CH	-158.0	-	Pleist.	-	60	1580	-	0	
Pleist.	-	60	1580	-	0	Pleist.	-								

UNDIST. BORING NO.	LEVEE	REACH NO.	0-1	LOCATION	Sta. 2635+00 To 2765+00
UNDIST. BORING NO.	1-U			UNDIST. BORING NO.	1-UT; 70' L.S.
BENEATH C/L OF LEVEE					
STRENGTH PARAMETERS					
SOIL TYPE	ELEV. (M.S.L.)	σ (P.C.F.)	C AVG.	(P.S.F.) BASE	ϕ
SITE	ELEV. (M.S.L.)	σ (P.C.F.)	C AVG.	(P.S.F.) BASE	ϕ
CH	2.0	114	550	-	0
CH	0.0	110	550	-	0
CH	-6.0	48	550	-	0
CHO	-13.0	33	550	550	0
SM&SP	-23.0	60	0	550	30
CH	-30.0	48	550	-	0
CH	-35.0	48	550	550	0
CH	-50.0	43	625	700	0
CH	-75.0	48	825	950	0
CH	-100.0	53	1075	1200	0
SP-F	-111.0	60	0	1200	30
CH	-165.0	53	1450	1700	0
Pleist.	-	60	1700	-	0
BENEATH TOE OF LEVEE					
STRENGTH PARAMETERS					
SOIL TYPE	ELEV. (M.S.L.)	σ (P.C.F.)	C AVG.	(P.S.F.) BASE	ϕ
SITE	ELEV. (M.S.L.)	σ (P.C.F.)	C AVG.	(P.S.F.) BASE	ϕ
CH	2.0	114	550	-	0
CH	0.0	110	550	-	0
CH	-6.0	48	550	-	0
CHO	-13.0	33	550	550	0
SM&SP	-23.0	60	0	550	30
CH	-30.0	48	550	-	0
CH	-35.0	48	550	550	0
CH	-50.0	43	625	700	0
CH	-75.0	48	825	950	0
CH	-100.0	53	1075	1200	0
SP-F	-111.0	60	0	1200	30
CH	-165.0	53	1450	1700	0
Pleist.	-	60	1700	-	0

West

EVENT

BEACH M 0-2

LOCATION

Sta. 2765+00 To 2850+00

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10-MAY-1950

WEST	LEVEE	REACH NO.	0-3	LOCATION	Sta. 2850+00 To 3000+00
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UNDIST. BORING NO. _____ UNDIST. BORING NO. _____
18-MHU _____ 10-MHUT; 52' L.S. _____

BENEATH C/L OF LEVEE				BENEATH TOE OF LEVEE				STRENGTH PARAMETERS			
SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS		SOIL TYPE (M.S.L.)	ELEV. (M.S.L.)	STRENGTH PARAMETERS		σ (P.C.F.)	C AVG.	(P.S.F.)	φ
		σ (P.C.F.)	C AVG.			σ (P.C.F.)	C AVG.				
CH	2.0	114	600	-	0	CH	0.0	110	300	-	0
CH	0.0	110	600	-	0	CH	-6.0	48	300	-	0
CH	-6.0	48	600	-	0	CH	-12.0	28	300	-	0
CHO	-12.0	33	600	600	0	CHO	-12.0	28	300	300	0
ML	-19.0	55	200	-	15	ML	-19.0	55	200	-	15
SP&SM	-30.0	60	0	600	30	SP&SM	-30.0	60	0	300	30
CH	-40.0	48	600	-	0	CH	-40.0	43	350	400	0
CH	-50.0	38	600	600	0	CH	-50.0	38	450	500	0
SP&SM	-56.0	60	0	-	30	SP&SM	-56.0	60	0	-	30
ML	-66.0	55	200	760	15	ML	-66.0	55	200	660	15
CH	-185.0	48	1355	1950	0	CH	-185.0	48	1255	1850	0
SP-F	-205.0	60	0	2100	30	SP-F	-205.0	60	0	2100	30
Pleist.	-	60	2100	-	0	Pleist.	-	60	2100	-	0

WEST

LEVEE

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REACH M

LOCATION

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INDIST BOBTNG NO

5-METT

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UNLIST. BORING NO
5-MHUIT; 48' L.S.

BENEATH C/L OF LEVEE				BENEATH TOE OF LEVEE				STRENGTH PARAMETERS			
SOIL TYPE	ELEV. (M.S. L.)	STRENGTH PARAMETERS		SOIL TYPE	ELEV. (M.S. L.)	STRENGTH PARAMETERS		σ (P.C.F.)	C (P.S.F.)	σ (P.C.F.)	C (P.S.F.)
		σ (P.C.F.)	C (P.S.F.)			σ (P.C.F.)	C (P.S.F.)				
CH&CL	1.0	110	700	400	0	CH&CL	0.0	110	250	-	0
CH&CL	0.0	110	400	-	0	CH&CL	-15.0	48	250	250	0
CH&CL	-15.0	48	400	400	0	CH&CL	-25.0	48	300	350	0
CH	-25.0	48	450	500	0	CH	-32.0	60	0	-	30
SP-F	-32.0	60	0	-	30	SP-F	-32.0	60	0	-	30
ML	-35.0	55	200	600	15	ML	-35.0	55	200	450	15
CL	-45.0	48	650	700	0	CL	-45.0	48	500	550	0
SP-F	-74.0	60	0	800	30	SP-F	-74.0	60	0	800	30
CH	-95.0	48	800	800	0	CH	-95.0	48	800	800	0
CH	-190.0	53	1275	1750	0	CH	-190.0	53	1275	1750	0
SP-F	-210.0	60	0	1950	30	SP-F	-210.0	60	0	1950	30
Pleist.	-	60	1950	-	0	Pleist.	-	60	1950	-	0

WEST	LEVEE	REACH NO.	Q	LOCATION	STA. 3100+00 TO 3130+00
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UNDIST. BORING NO. _____ 5-MHU UNDIST. BORING NO. 5-MHUT; 40' L.S.

BENEATH C/L OF LEVEE

BENEATH C/L OF LEVEE				BENEATH TOE OF LEVEE			
SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS		SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS	
		σ (P.C.F.)	C AVG.			σ (P.S.F.)	ϕ
CH&CL	1.0	110	700	400	0	-	-
CH&CL	0.0	110	400	-	0	CH&CL	0.0
CH&CL	-15.0	48	400	400	0	CH&CL	-15.0
CH&CL	-23.0	48	440	480	0	CH&CL	-23.0
ML	-33.0	55	200	-	15	ML	-33.0
SP-F	-110.0	60	0	950	30	SP-F	-110.0
CH	-198.0	53	1390	1830	0	CH	-198.0
SP-F	-209.0	60	0	1940	30	SP-F	-209.0
Pleist.	-	60	1940	-	0	Pleist.	-

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LIMITS TO OPTIMIZATION

3-MHUT 3-MHUT

BENEATH C/L OF LEVEE

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WEST	LEVEE	REACH NO.	S	LOCATION	Sta. 3218+00 to 3299+50
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UNDIST. BORING NO. _____ 1-U UNDIST. BORING NO. _____ 1-IT: 42' T.S.

WEST

HEYER

LOCATION

STA. 3299+50 to 3370+00

UNDIST.: BORING NO.

HII-6

BENEATH C/L OF LEVEE		BENEATH TOE OF LEVEE					
SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS				STRENGTH PARAMETERS	
		σ (P.C.F.)	C (P.C.F.)	(P.S.F.) AVG.	(P.S.F.) BASE	σ (P.C.F.)	C (P.C.F.)
CH-ML	2.0	110	550	350	0	-	-
CH	0.0	105	350	-	0	CH	0.0
CH	-10.0	43	350	350	0	CH	-10.0
ML	-14.0	55	200	350	15	ML	-14.0
CH	-20.0	43	350	-	0	CH	-20.0
CH	-25.0	43	350	350	0	CH	-25.0
CH	-32.0	43	385	420	0	CH	-32.0
SP-F	-62.0	60	0	720	30	SP-F	-62.0
CH	-70.0	43	760	800	0	CH	-70.0
CH	-85.0	43	800	800	0	CH	-85.0
SP-F	-93.0	60	0	-	30	SP-F	-93.0
ML	-97.0	55	200	920	15	ML	-97.0
CH	-203.0	48	1450	1980	0	CH	-203.0
SP-F	-209.0	60	0	2040	30	SP-F	-209.0
PLEIST	-	60	2040	-	0	PLEIST	-

WEST

LEVEE

REACH NO. U-1

Sta. 3370+00 to 3450+00

UNDIST. BORING NO.

17-UW

UNDIST. BORING NO. 17-UW; 72' L.S.

BENEATH C/L OF LEVEE

STRENGTH PARAMETERS

SOIL TYPE	ELEV. (M.S.L.)	σ (P.C.F.)	C (AVG.)	(P.S.F.)	φ	SOIL TYPE	ELEV. (M.S.L.)	σ (P.C.F.)	C (AVG.)	(P.S.F.)	φ
CH	1.0	110	600	400	0	CH	0.0	0	110	350	-
CH	0.0	110	400	-	0	CH	-9.0	-9.0	48	350	-
CH	-9.0	48	400	-	0	CHO	-13.0	-13.0	33	350	-
CHO	-13.0	38	400	-	0	CH	-16.0	-16.0	40	350	350
CH	-16.0	40	400	400	0	ML	-25.0	-25.0	55	200	-
ML	-25.0	55	200	-	15	ML	-32.0	-32.0	60	0	470
SM&SP	-32.0	60	0	520	30	SM&SP	-32.0	-32.0	60	0	30
CL	-43.0	53	575	630	0	CL	-43.0	-43.0	53	525	580
CH	-70.0	48	765	900	0	CH	-70.0	-70.0	48	715	850
CH	-205.0	53	1575	2250	0	CH	-205.0	-205.0	53	1525	2200
SP-F	-225.0	60	0	2400	30	SP-F	-225.0	-225.0	60	0	2400
PLEIST.	-	60	2400	-	0	PLEIST.	-	-	60	2400	-

WEST

LEVEE

REACH NO.

LOCATION

Sta. 3450+00 to 3640+00

UNDIST. BORING NO.

17-UW

UNDIST. BORING NO. 17-UWT; 72' L.S.

BENEATH C/L OF LEVEE

SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS			SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS		
		σ (P.C.F.)	C AVG.	(P.S.F.) BASE			σ (P.C.F.)	C AVG.	(P.S.F.) BASE
CL	2.0	110	600	400	0	0	0.0	110	350
CH	0.0	110	400	-	0	CH	0.0	48	350
CH	-6.0	48	400	-	0	CH	-6.0	-	-
CHO	-12.0	38	400	400	0	CHO	-12.0	33	350
ML	-16.0	55	200	-	15	ML	-16.0	55	200
SM&SP	-26.0	60	0	460	30	SM&SP	-26.0	60	0
CL	-43.0	53	545	630	0	CL	-43.0	53	495
CH	-70.0	48	765	900	0	CH	-70.0	48	715
CH	-205.0	53	1575	2250	0	CH	-205.0	53	1525
SP-F	-225.0	60	0	2400	30	SP-F	-225.0	60	0
PILEIST.	-	60	2400	-	0	PILEIST.	-	60	2400

BENEATH TOE OF LEVEE

SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS			SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS		
		σ (P.C.F.)	C AVG.	(P.S.F.) BASE			σ (P.C.F.)	C AVG.	(P.S.F.) BASE
CL	2.0	110	600	400	0	CH	0.0	110	350
CH	0.0	110	400	-	0	CH	0.0	48	350
CH	-6.0	48	400	-	0	CH	-6.0	-	-
CHO	-12.0	38	400	400	0	CHO	-12.0	33	350
ML	-16.0	55	200	-	15	ML	-16.0	55	200
SM&SP	-26.0	60	0	460	30	SM&SP	-26.0	60	0
CL	-43.0	53	545	630	0	CL	-43.0	53	495
CH	-70.0	48	765	900	0	CH	-70.0	48	715
CH	-205.0	53	1575	2250	0	CH	-205.0	53	1525
SP-F	-225.0	60	0	2400	30	SP-F	-225.0	60	0
PILEIST.	-	60	2400	-	0	PILEIST.	-	60	2400

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STA 36110+00 TN 3778+EO

UNDIST. BORING NO.

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BENEATH C/L OF LEVEE				BENEATH TOE OF LEVEE			
SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS		SOIL TYPE (M.S.L.)	ELEV. (M.S.L.)	STRENGTH PARAMETERS	
		σ (P.C.F.)	C AVG.			ϕ	σ (P.C.F.)
CH	4.0	110	600	400	0	-	-
CH	0.0	102	400	-	0	CH	0.0
CH	-10.0	40	400	-	0	CH	-10.0
CHO	-15.0	33	400	-	0	CHO	-15.0
CH	-20.0	40	400	0	CH	-20.0	-
ML	-30.0	55	200	-	15	ML	-30.0
SM	-40.0	60	0	600	30	SM	-40.0
CH	-60.0	48	740	855	0	CH	-60.0
CH	-80.0	48	970	1085	0	CH	-80.0
CH	-100.0	48	1200	1315	0	CH	-100.0
CH	-120.0	48	1430	1545	0	CH	-120.0
CH	-200.0	48	2115	2685	0	CH	-220.0
SP-F	-245.0	60	0	2830	30	SP-F	-245.0
Pleist.	-	60	2830	-	0	Pleist.	-

LEVEE WEST

REACH NO

LOCATION STA 3778 E0 NO 3303; C1.

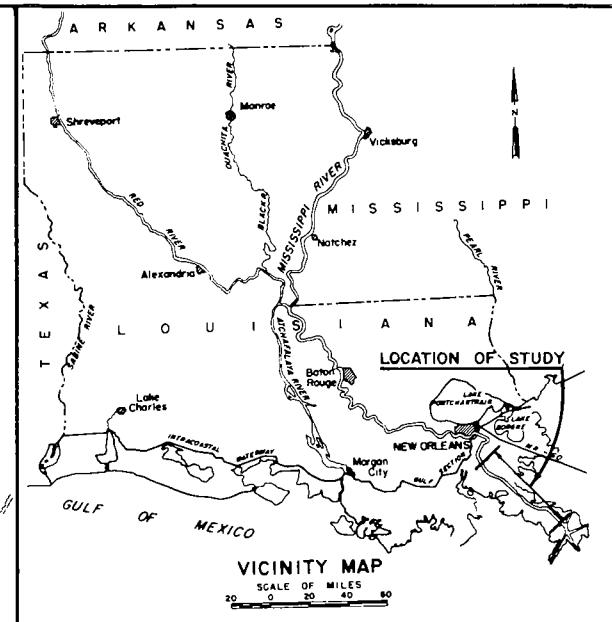
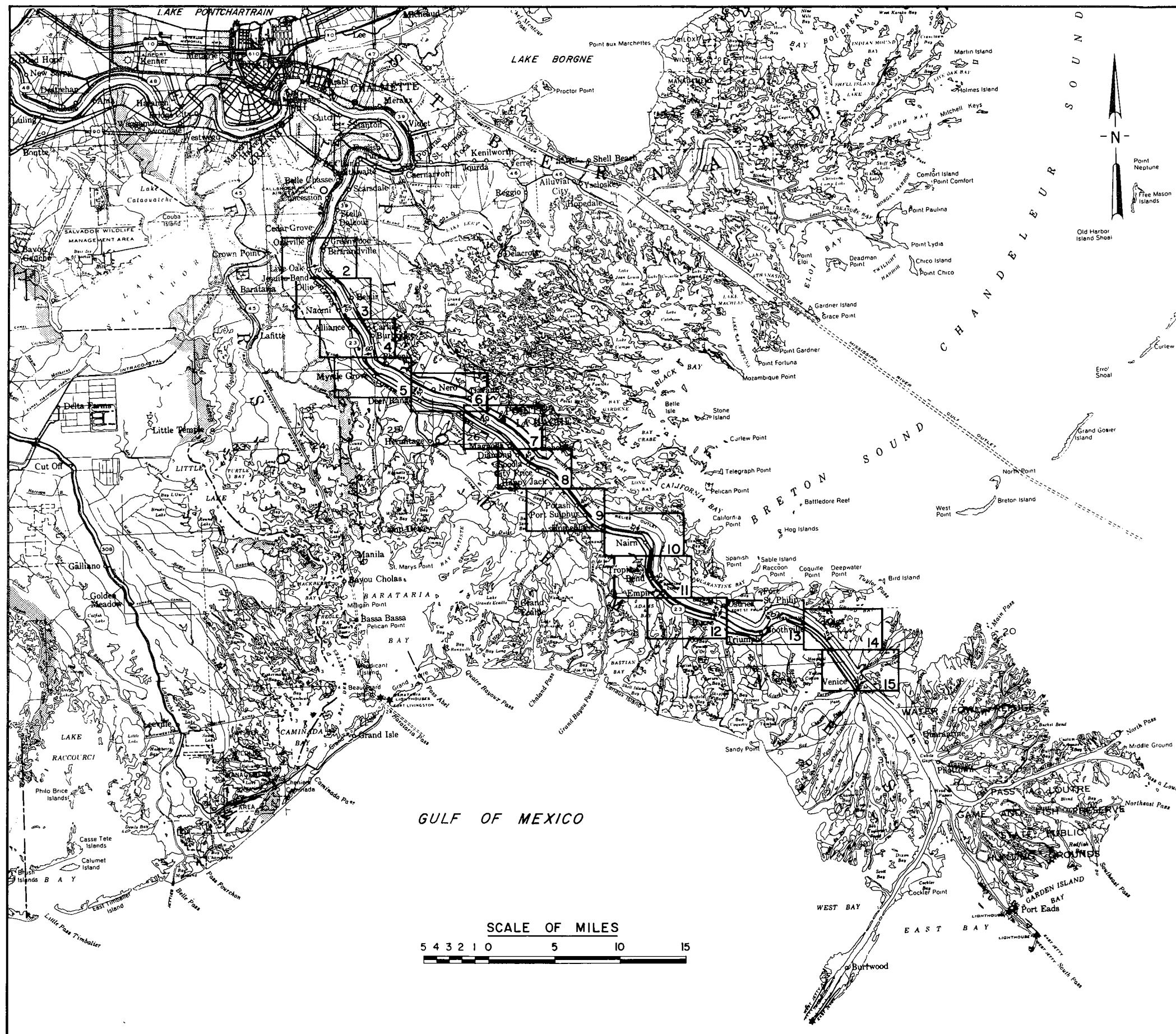
UNLISTED BORING NO

1-U

INDIST BOINC NO

1 III, 301 T S

BENEATH C/L OF LEVEE				BENEATH TOE OF LEVEE			
SOIL TYPE	ELEV. (M.S.L.)	STRENGTH PARAMETERS		STRENGTH PARAMETERS		C (P.S.F.) AVG,	ϕ °
		σ (P.C.F.)	C (P.C.F.) AVG.	σ (P.S.F.) BASE	C (P.C.F.) AVG.		
CH	0.0	110	400	350	0		
CL&CH	-10.0	48	350	-	0	CL&CH	-10.0
CH	-20.0	43	350	350	0	CH	-20.0
CH	-30.0	43	400	-	0	CH	-30.0
CH	-40.0	43	400	400	0	CH	-40.0
CH	-230.0	43	1350	2300	0	CH	-230.0
SP-F	-260.0	60	0	2600	30	SP-F	-260.0
PLEIST.	-	60	2600	-	0	PLEIST	-



MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
EAST AND WEST BANKS
SOIL BORING DATA
GENERAL MAP

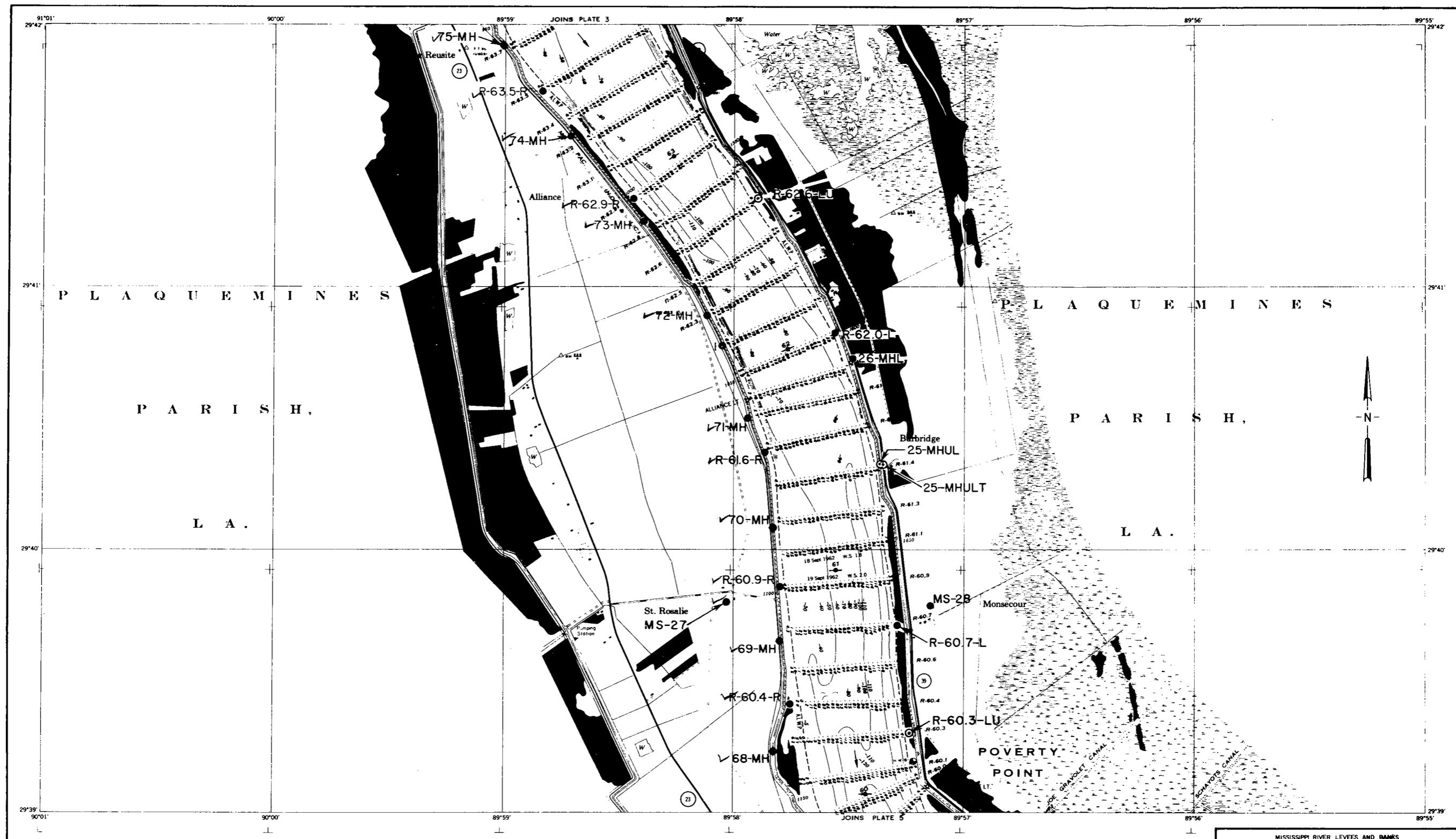
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FILE NO. H-2-25275

PLATE



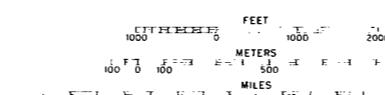




All elevations are expressed in feet and refer to Mean Sea Level
Contours below Average Low Water Plane are expressed in feet at 5 and 10 ft intervals
Contours above Average Low Water Plane are expressed in feet at 5 ft intervals
Photogrammetry from aerial photographs flown November 1962
—● Distances on Mississippi River above Head of Passes are shown at 1 mile intervals
1962 and 1942 surveys
Polyconic Projection, North American Datum

Planimetry from aerial photographs flown November 1962
— Distances on Mississippi: River above Head of Passes are shown at 1 mile intervals
1962 and 1964 surveys
Polyconic Projection North American Datum

SCAL



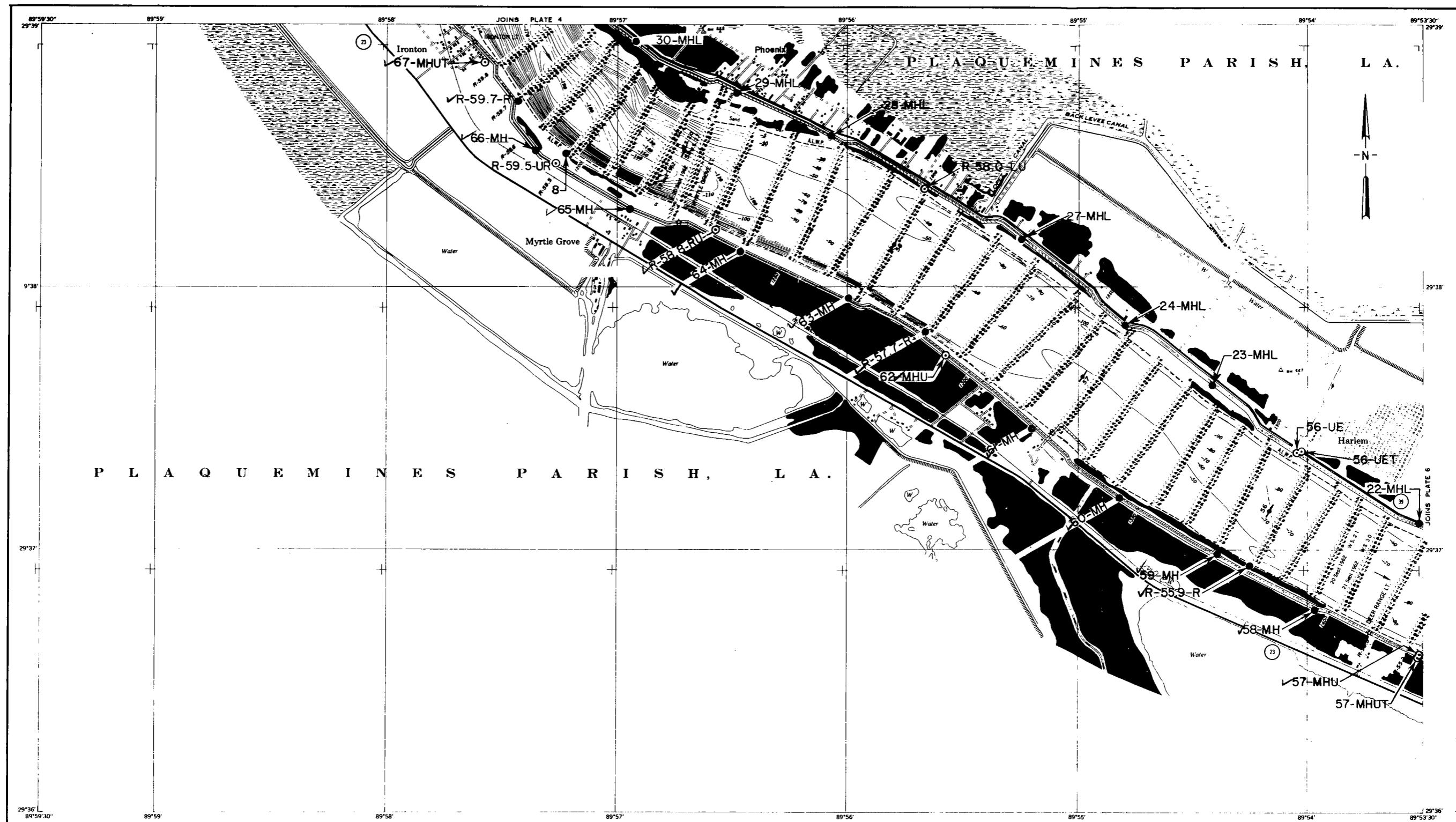
INDEX TO ADJOINING SHEETS

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
EAST AND WEST BANKS
SOIL BORING DATA
BORING LOCATIONS
MILE 63.7 TO MILE 60.0

**U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS**

AUGUST 1971

FILE NO. H-2-25275



All elevations are expressed in feet and refer to Mean Sea Level.
Contours below Average Low Water Plane are expressed in feet at 5 and 10 ft. intervals
Contours above Average Low Water Plane are expressed in feet at 5 ft. intervals
Planimetry from aerial photographs flown November 1962
→ Distances on Mississippi River above Head of Passes are shown at 1 mile intervals,
1962 and 1942 surveys.
Polyconic Projection, North American Datum
Polyconic Projection, Gulf Coast Datum is indicated by ticks →
A L W P --Average Low Water Plane

SCALE

FEET
1000 0 1000 2000
METERS
100 500 1000
MILES

MISSISSIPPI RIVER LEVEES AND BANKS

MILE 66 TO MILE 10
SOIL REPORT - PART I
EAST AND WEST BANKS
OIL BORING DATA

BORING LOCATIONS

MILE 60.0 TO MILE 55.2

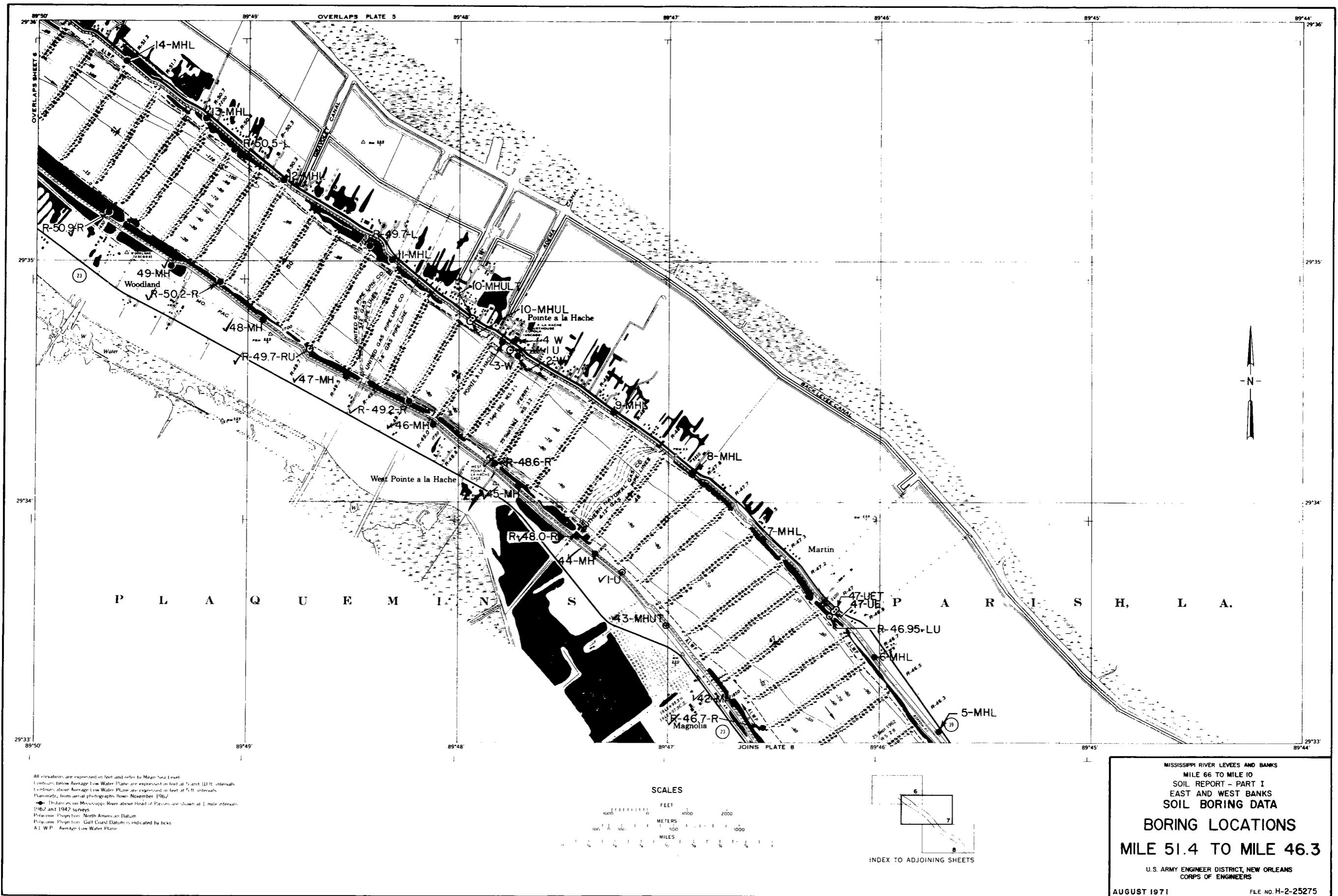
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS

AUGUST 1971

FILE NO. H-2-25275

PLATE 5







All elevations are expressed in feet and refer to Mean Sea Level.
Contours below Average Low Water Plane are expressed in feet at 5 ft. intervals.
Contours above Average Low Water Plane are expressed in feet at 5 ft. intervals.
Photometry from aerial photographs, November 1962.

— Distances on Mississippi River above Head of Passes are shown at 1 mile intervals
1962 and 1942 surveys.

Polyconic Projection North American Datum
Polyconic Projection Gulf Coast Datum is indicated by ticks
A L W P Average Low Water Plane

SCALE



INDEX TO ADJOINING SHEETS

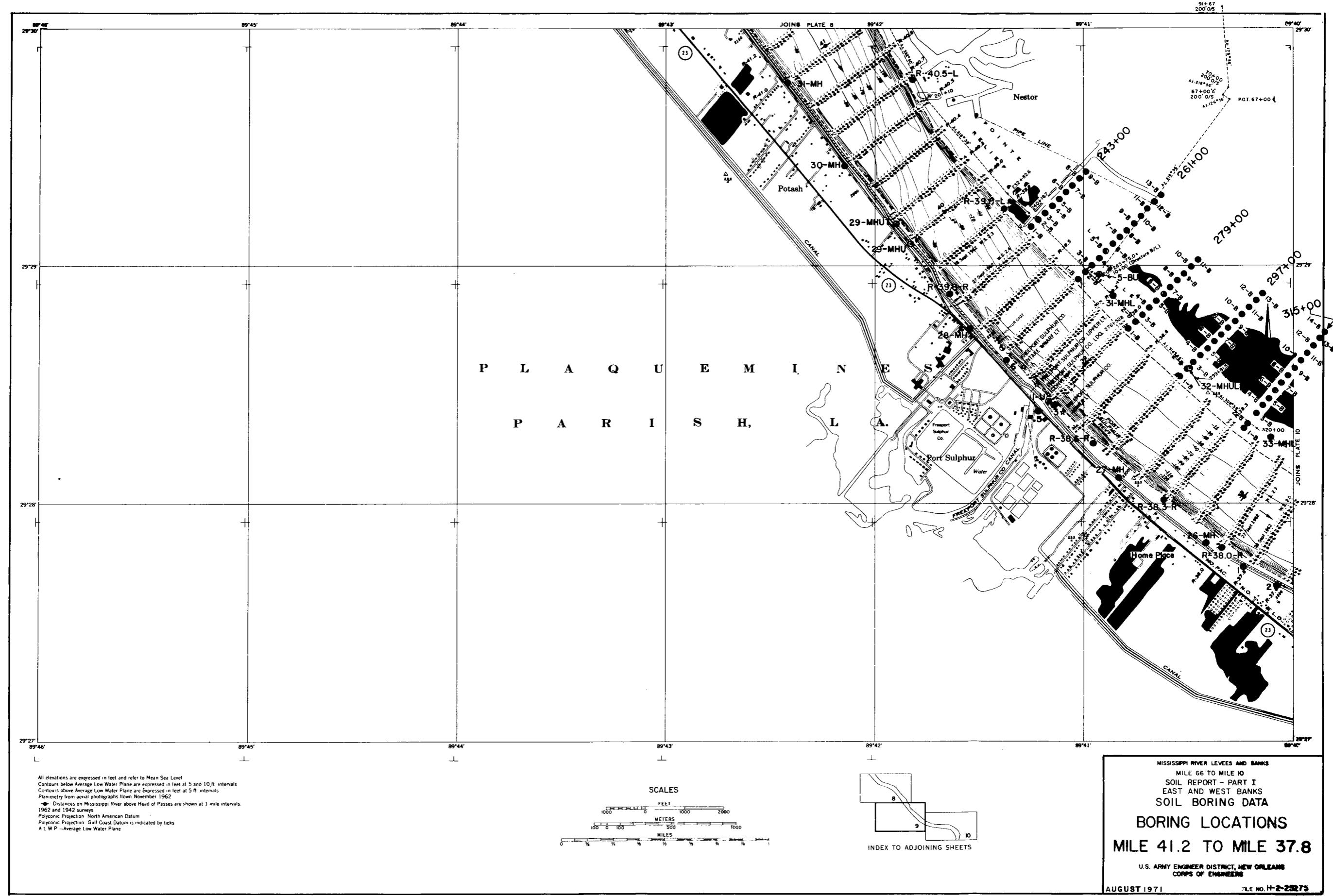
MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
EAST AND WEST BANKS
SOIL BORING DATA

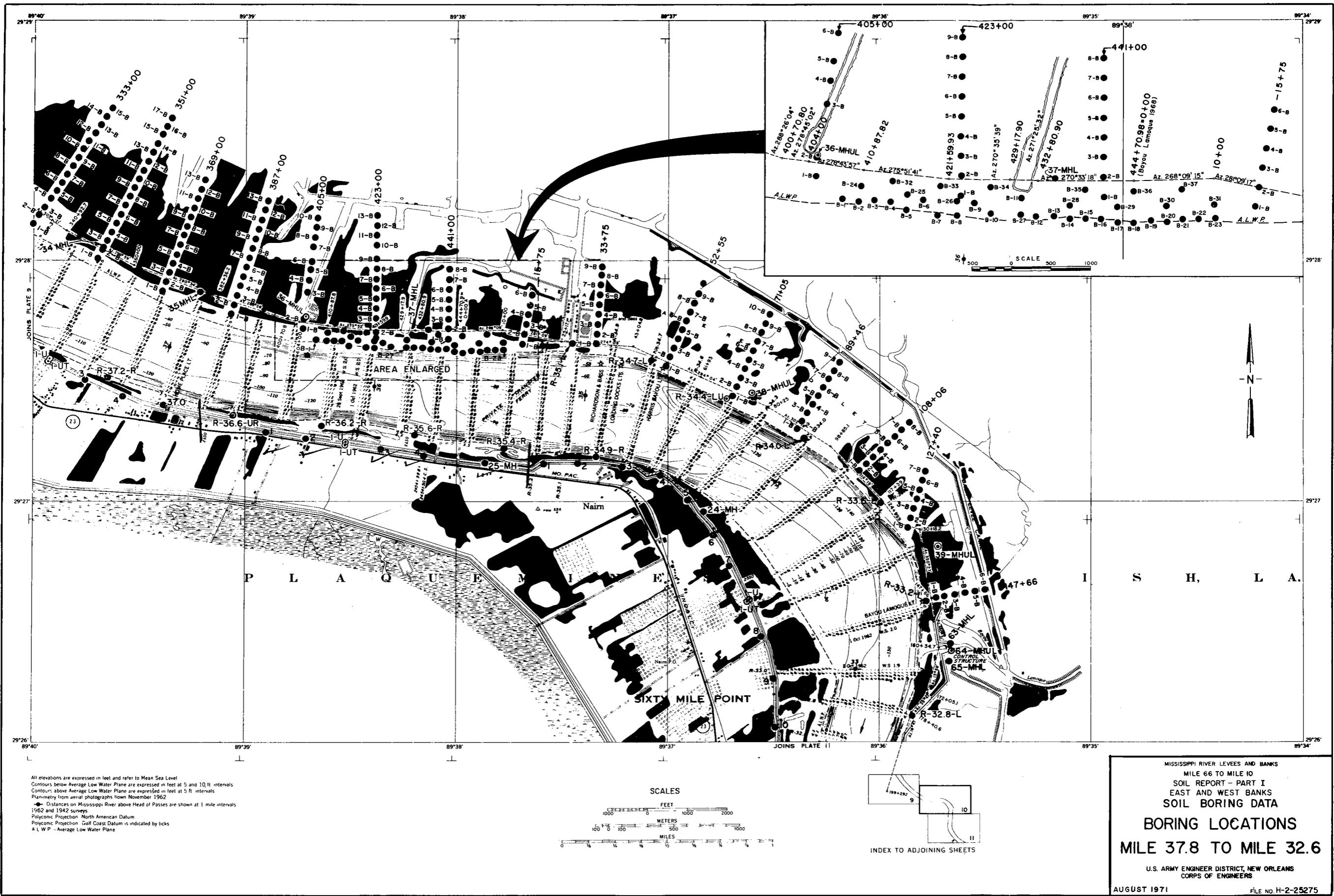
MILE 46.3 TO MILE 41.2

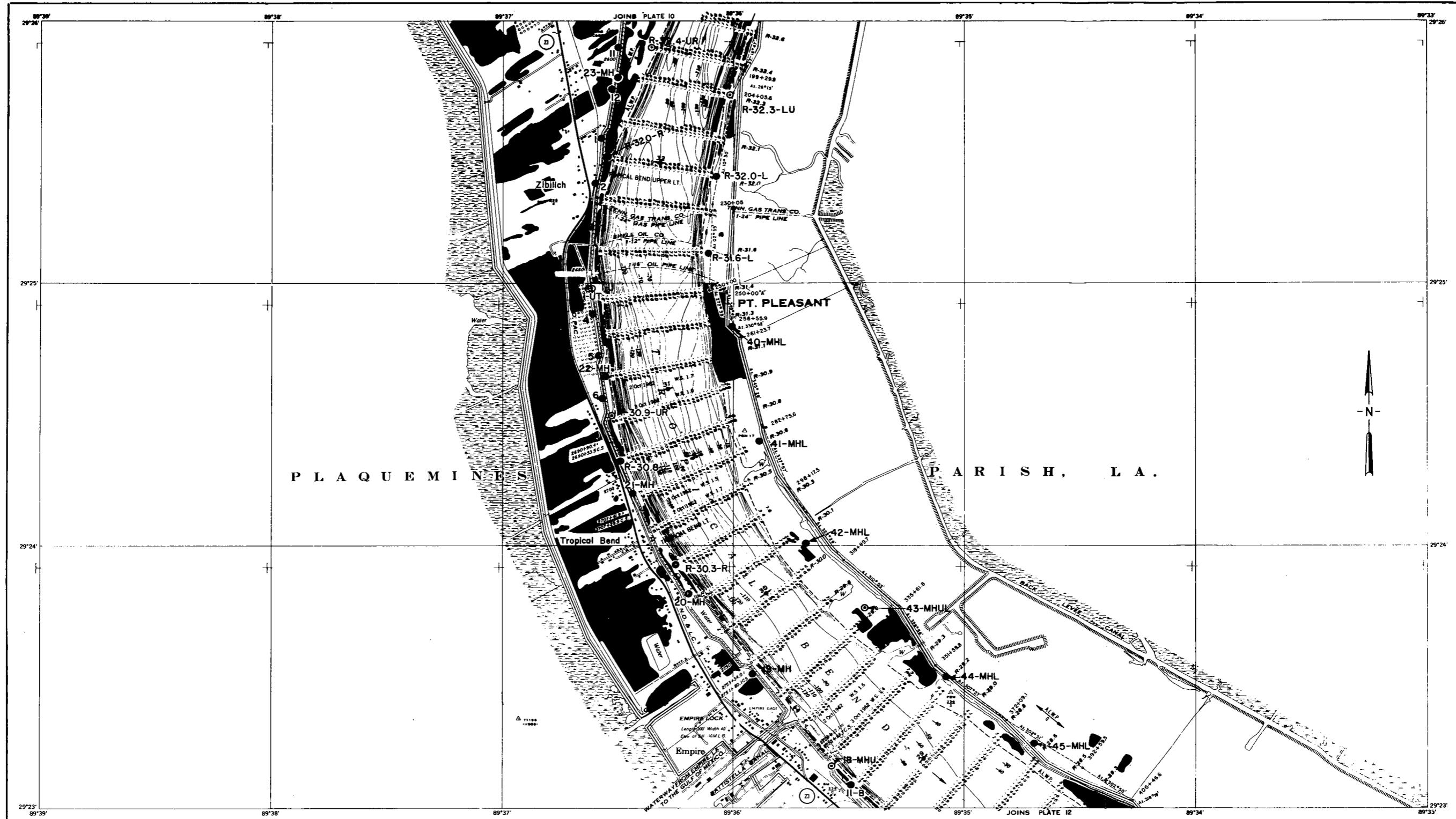
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS

AUGUST 1971

FILE NO. H-2-25275

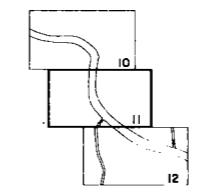






All elevations are expressed in feet and refer to Mean Sea Level
Contour below Average Low Water Plane are expressed in feet at 5 and 10 ft. intervals
Contours above Average Low Water Plane are expressed in feet at 5 ft. intervals
Planimetry from aerial photographs flown November 1962
→ Distances on Mississippi River above Head of Passes are shown at 1 mile interval
1962 and 1942 surveys
Polyconic Projection North American Datum
Polyconic Projection Gulf Coast Datum is indicated by ticks
A L W P - Average Low Water Plane

SCAL



MILE 32.6 TO MILE 28.5

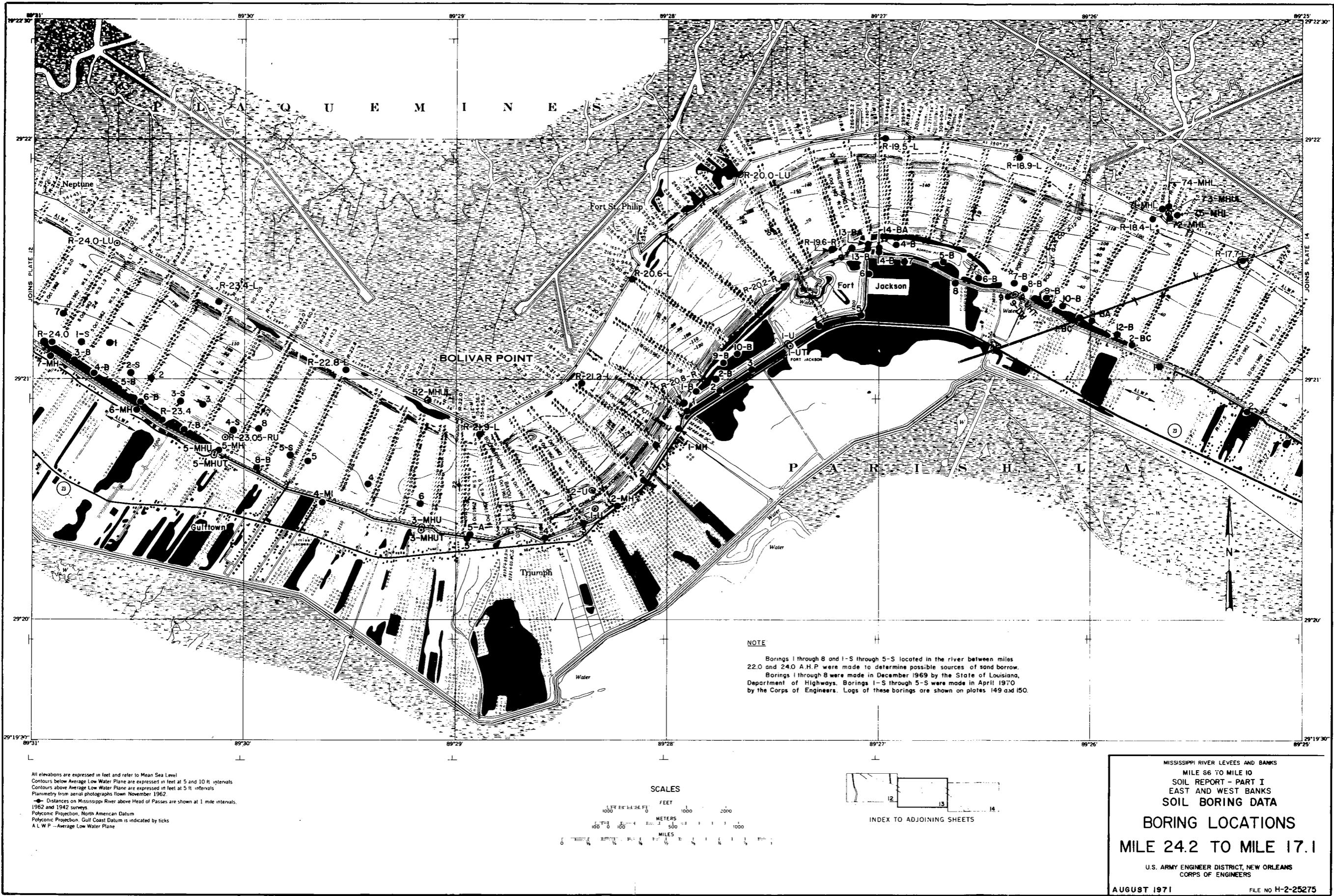
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS

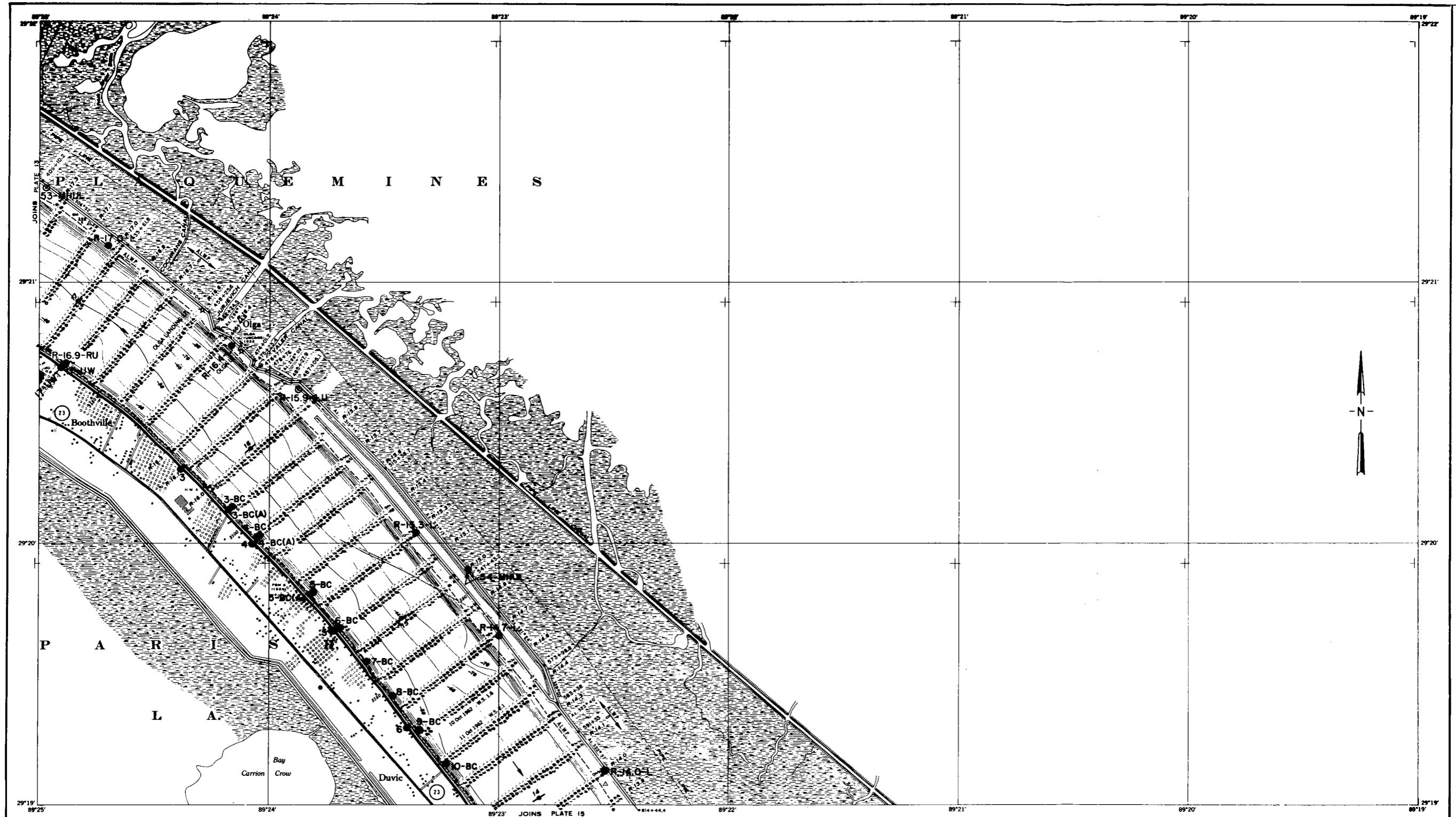
AUGUST 1971

FILE NO. H-2-25275

PLATE II







All elevations are expressed in feet and refer to Mean Sea Level
Contours below Average Low Water Plane are expressed in feet at 5 and 10 ft intervals
Contours above Average Low Water Plane are expressed in feet at 5 ft intervals
Planimetry from aerial photographs flown February 1963

— Distances on Mississippi River above Head of Passes are shown at 1 mile intervals.
1962 and 1942 surveys.

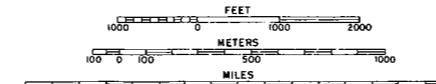
1962 and 1942 surveys
Polyconic Projection, North American Datum

Polyconic Projection, Gulf Coast Datum is indicated by ticks

Polyconic Projection. Gulf Coast Datum is indicated by ticks
A L W P -Average Low Water Plane

NEW! Microg EA Water Flame

SCALE



INDEX TO ADJOINING SHEETS

**MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
EAST AND WEST BANKS
SOIL BORING DATA**

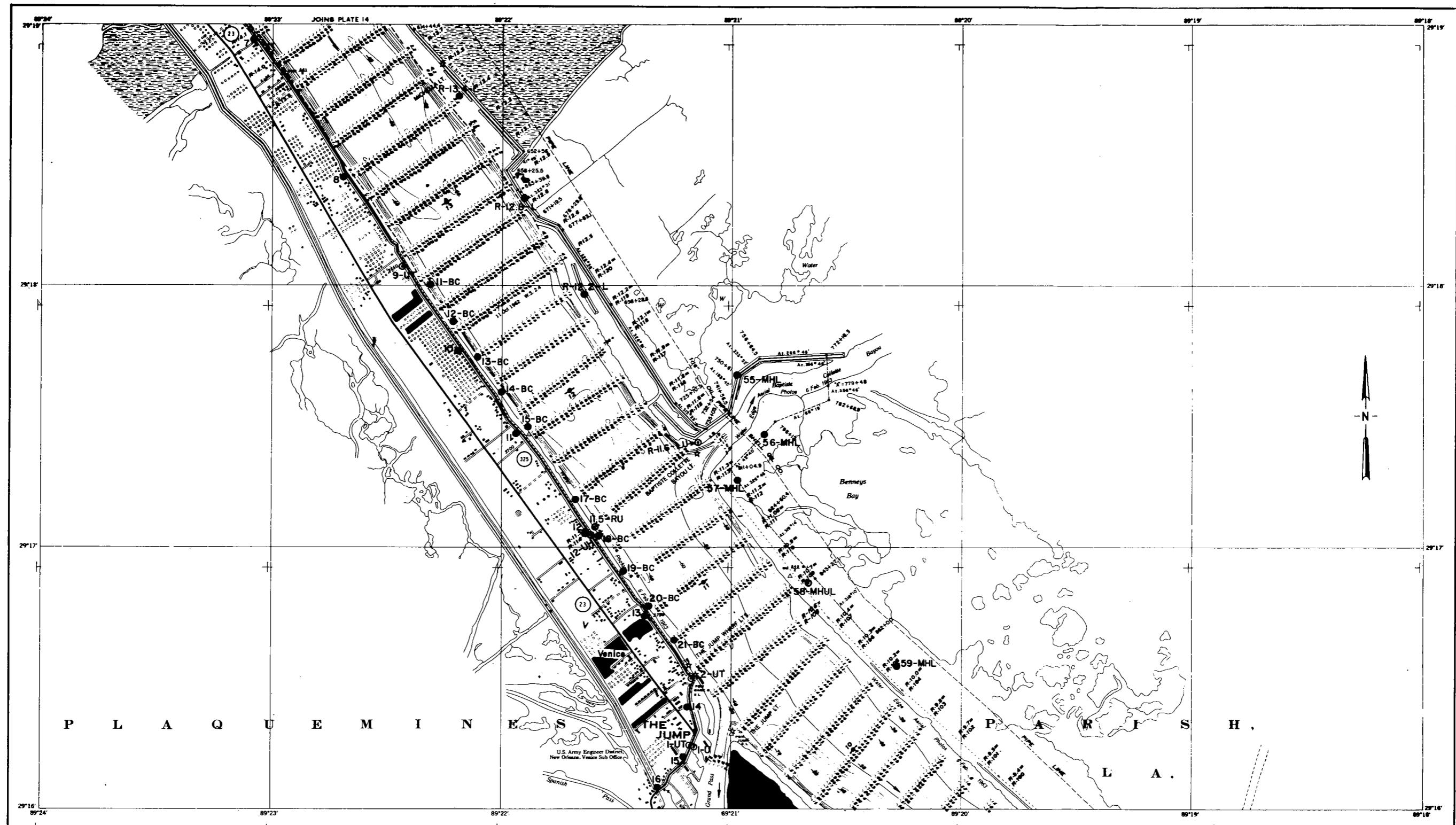
BORING LOCATIONS

MILE 17.1 TO MILE 14.0

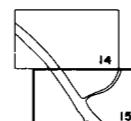
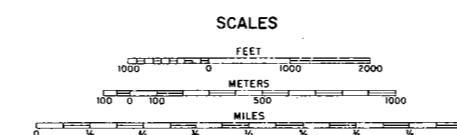
**U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS**

AUGUST 1971

LE NO. H-2-25275



All elevations are expressed in feet and refer to Mean Sea Level
 Contours below Average Low Water Plane are expressed in feet at 5 and 10 ft intervals
 Contours above Average Low Water Plane are expressed in feet at 5 ft intervals
 Planimetry from aerial photographs flown February 1963
 1962 and 1942 surveys
 Distances on Mississippi River above Head of Passes are shown at 1 mile intervals
 Polyconic Projection, North American Datum
 Polyconic Projection, Gulf Coast Datum is indicated by ticks
 A.L.W.P.—Average Low Water Plane



INDEX TO ADJOINING SHEETS

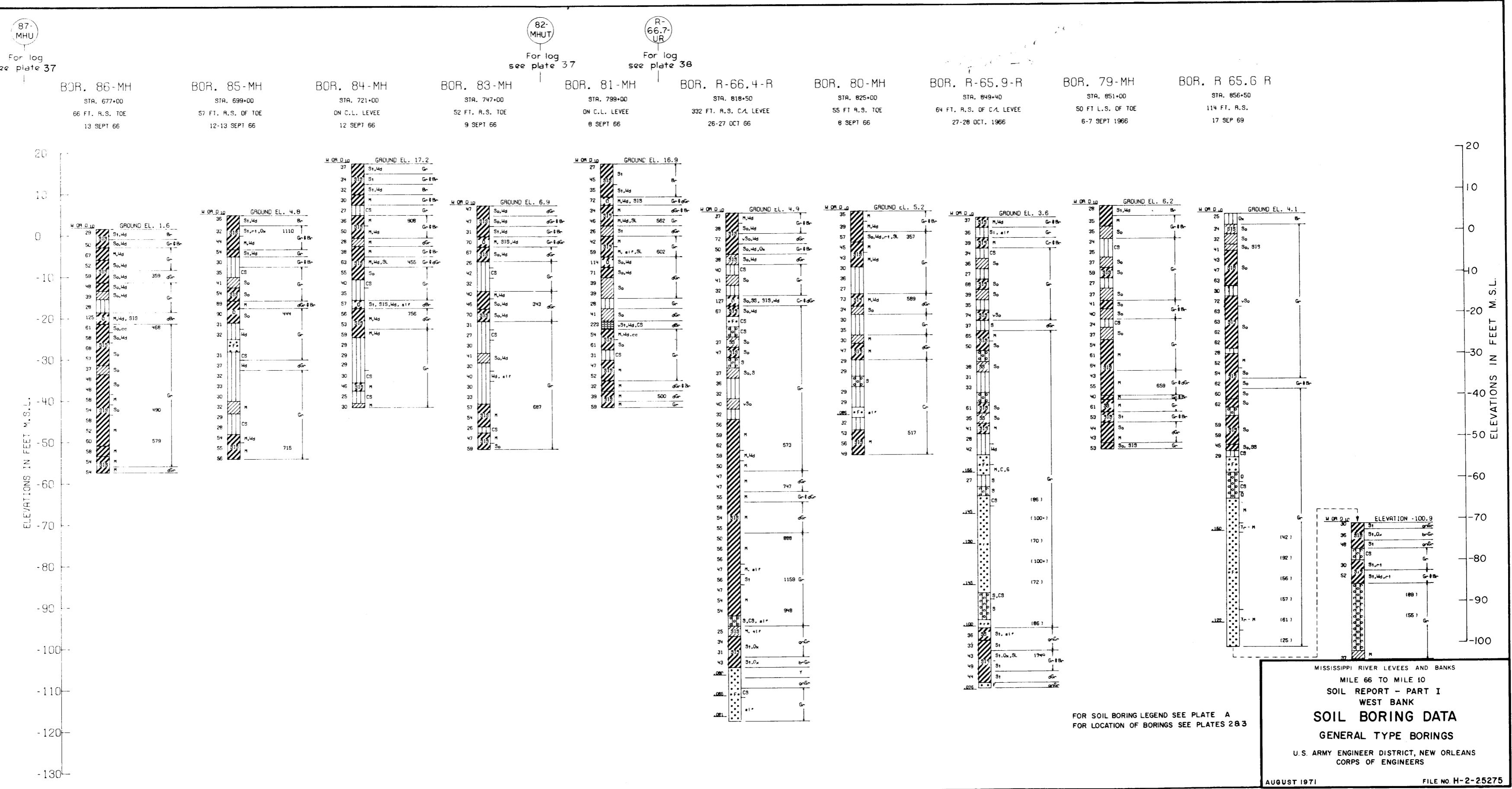
MISSISSIPPI RIVER LEVEES AND BANKS
 MILE 66 TO MILE 10
 SOIL REPORT - PART I
 EAST AND WEST BANKS
SOIL BORING DATA
BORING LOCATIONS
MILE 14.0 TO MILE 10.0

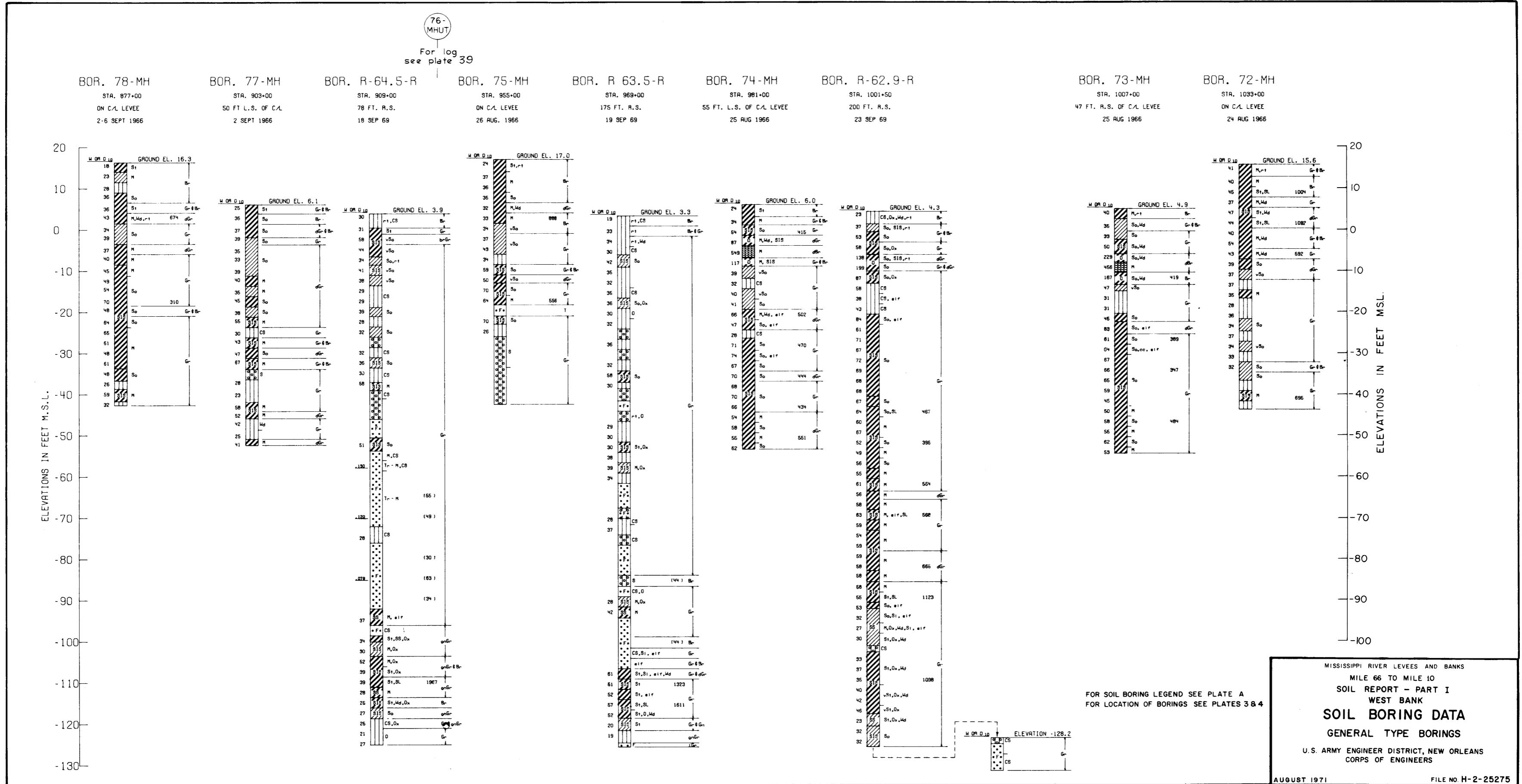
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
 CORPS OF ENGINEERS

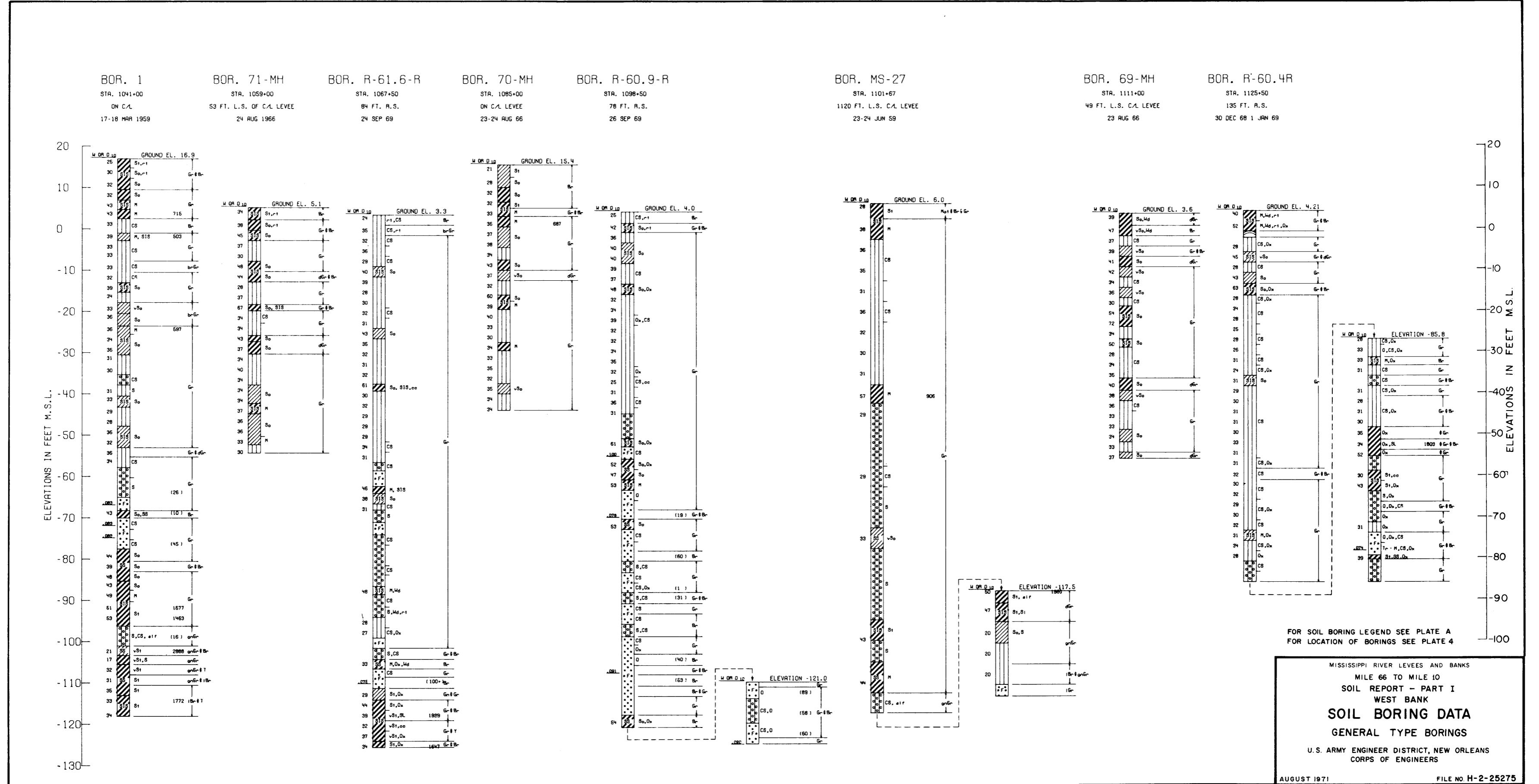
AUGUST 1971

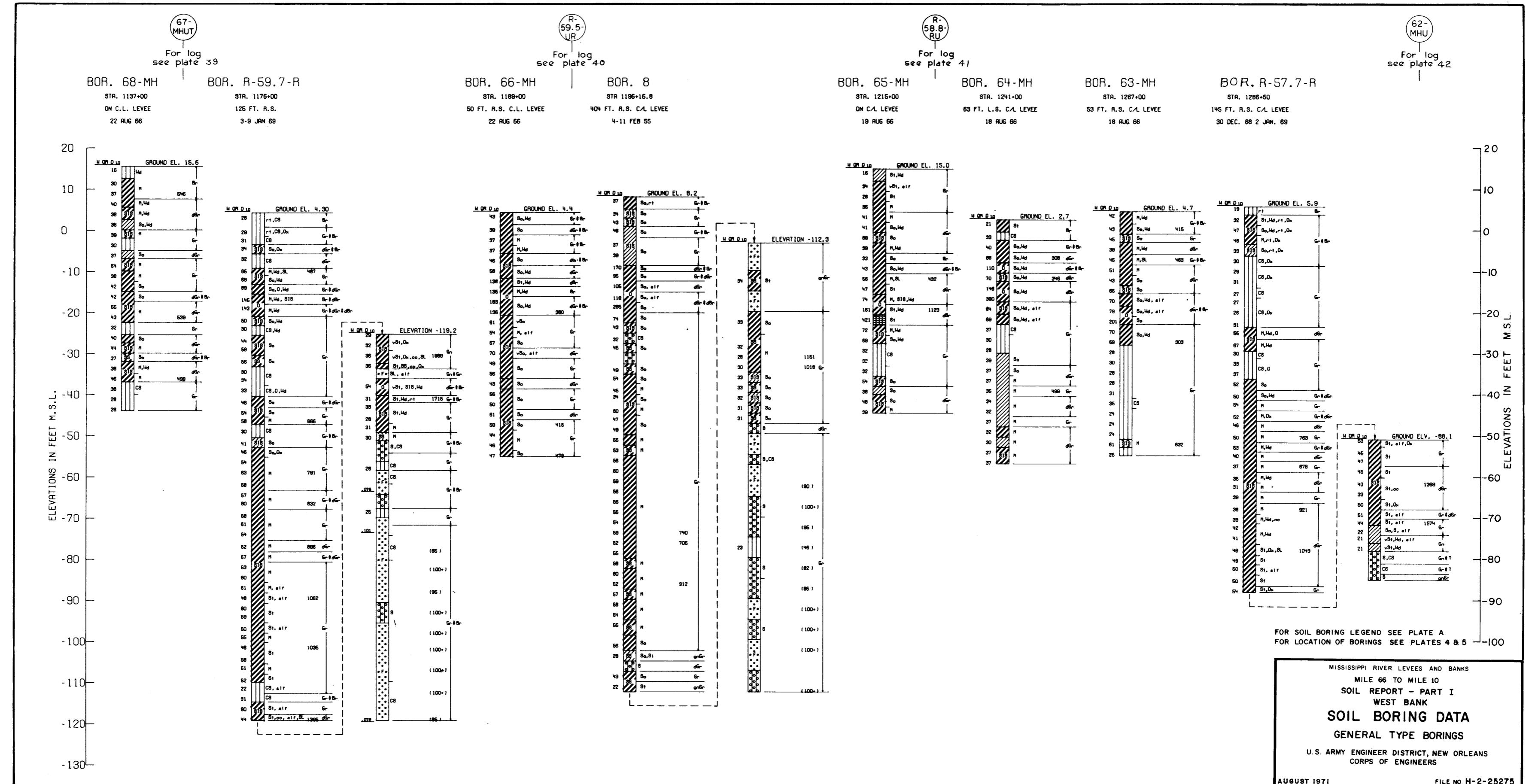
FILE NO. H-2-25275

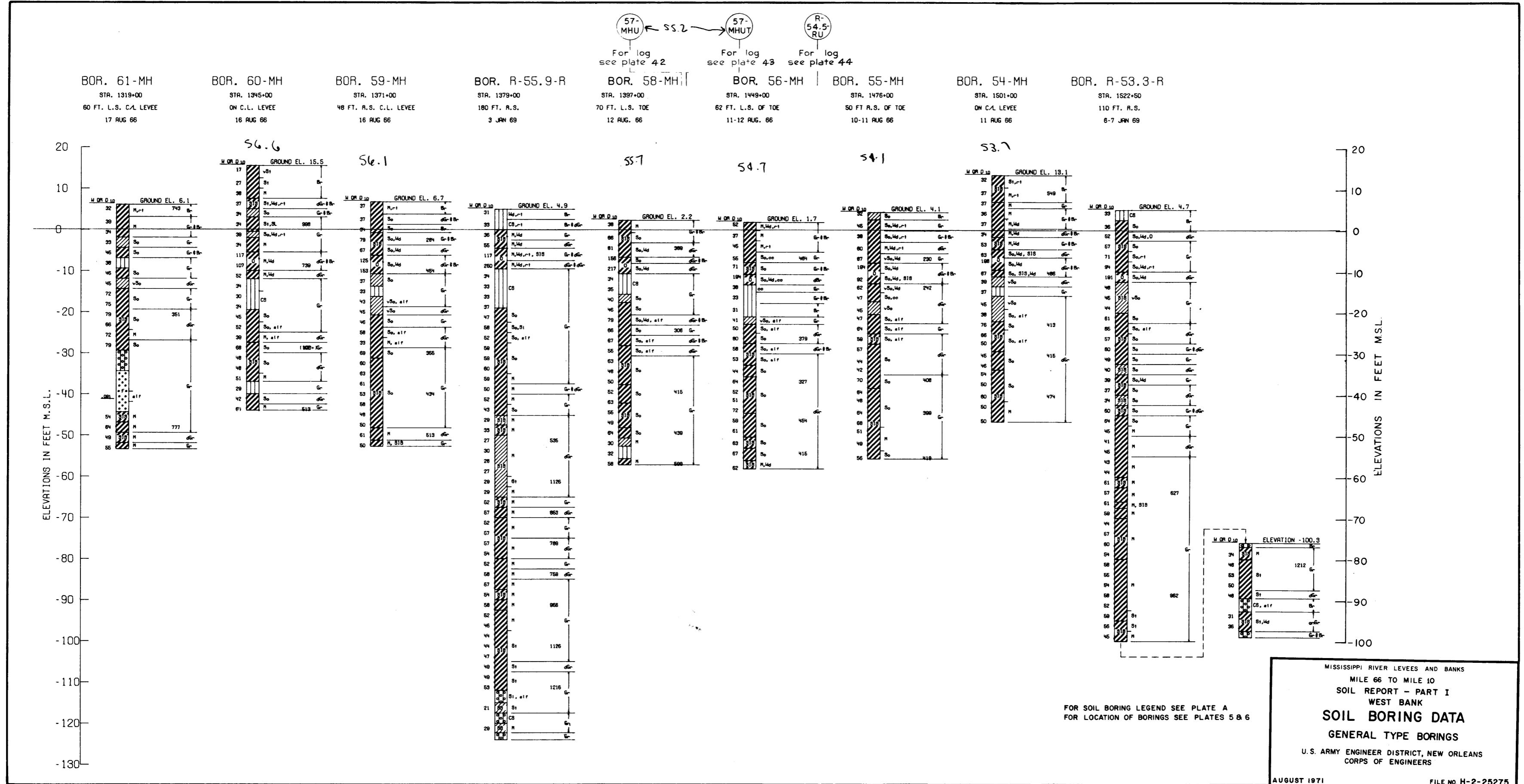
PLATE 15

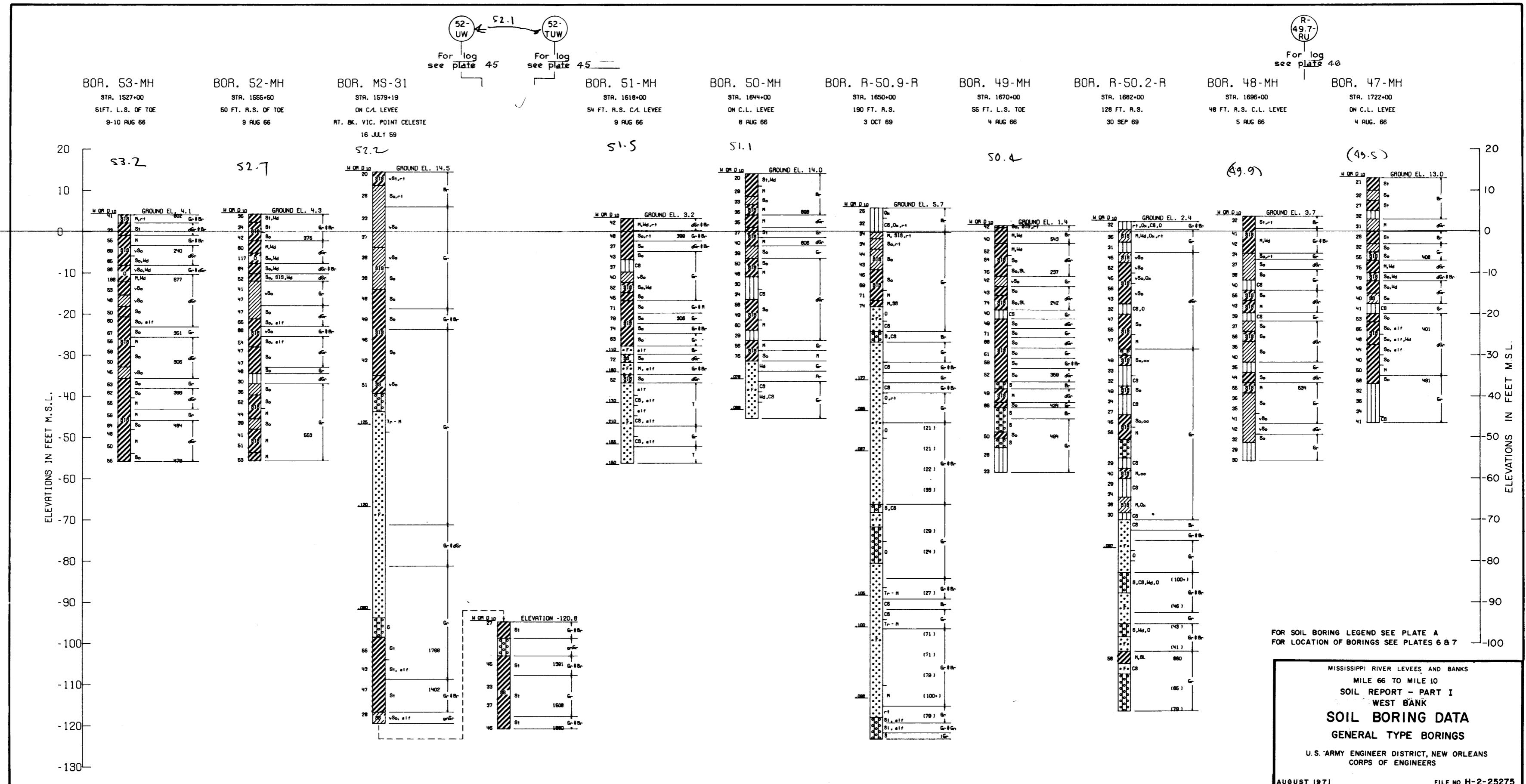












BOR. R-49.2-R
STA. 1739+50
145 FT. R.S.
6 OCT. 69

BOR. 46-MH
STA. 1748+00
65 FT. L.S. TOE
4 AUG. 66

BOR. R-48.6-R
STA. 1766+00
110 FT. R.S.
8 OCT. 69

BOR. 45-MH
STA. 1774+00
50 FT. R.S. TOE
4 AUG. 66

BOR. R-48.0-R
STA. 1796+00
200 FT. R.S.
8 OCT. 69

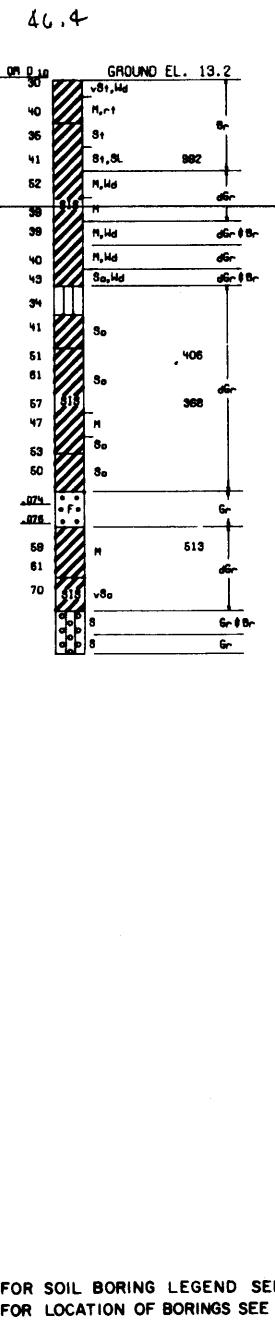
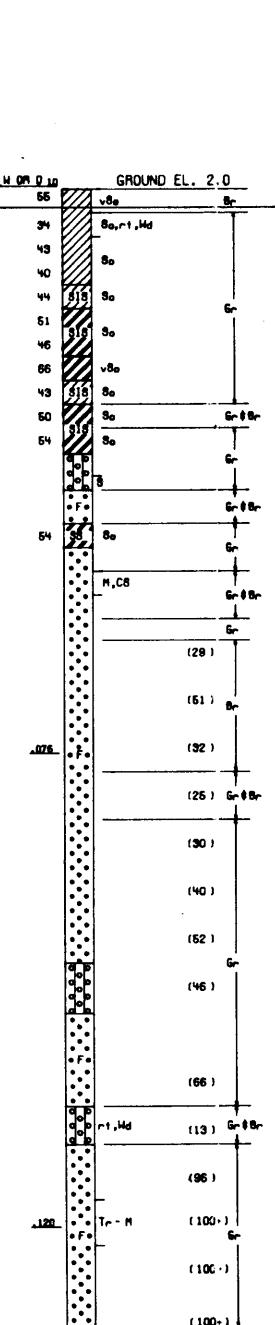
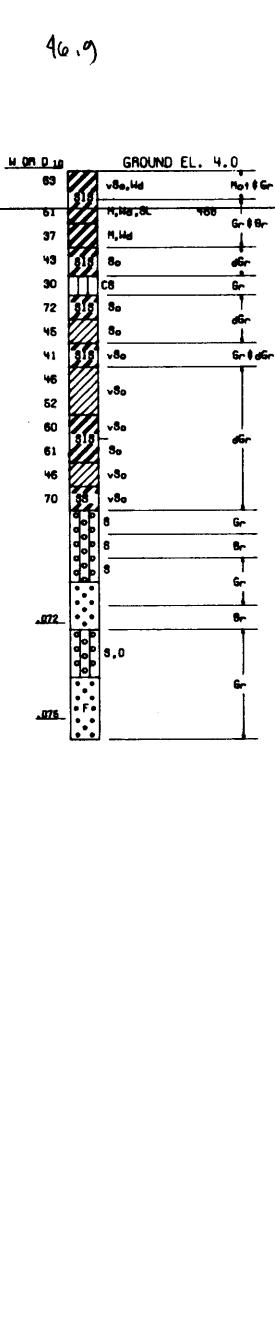
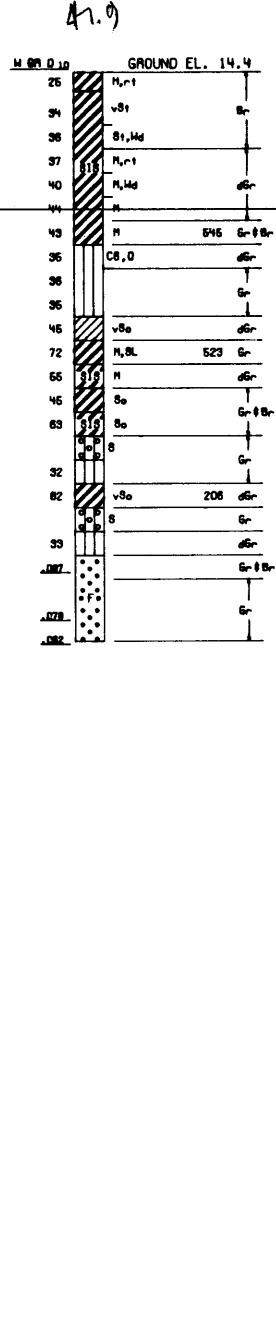
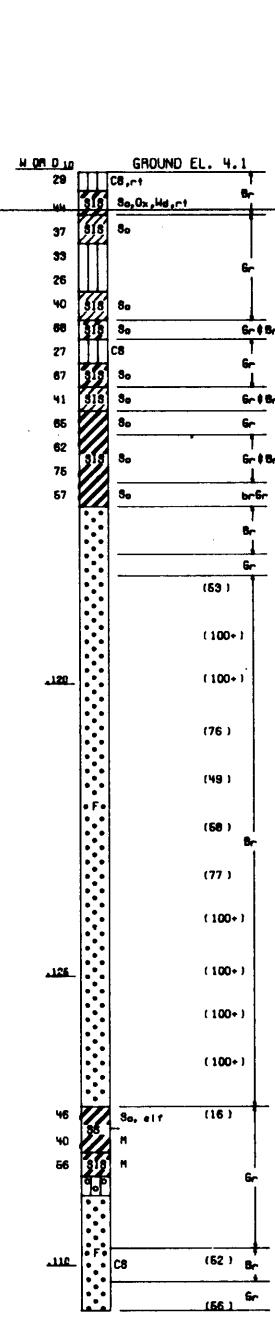
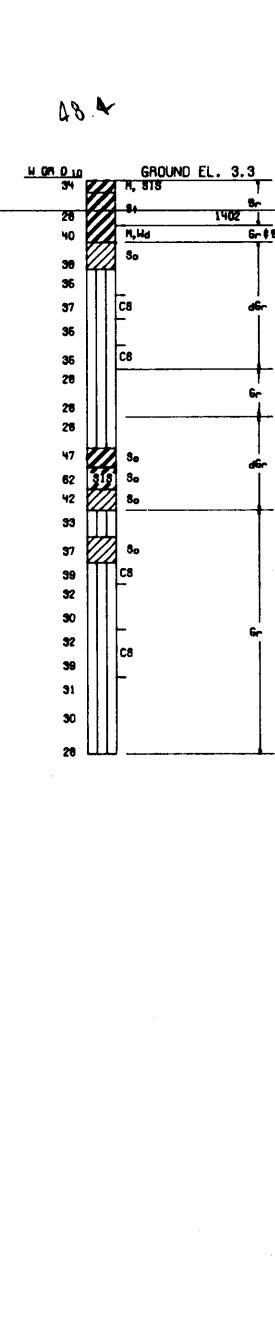
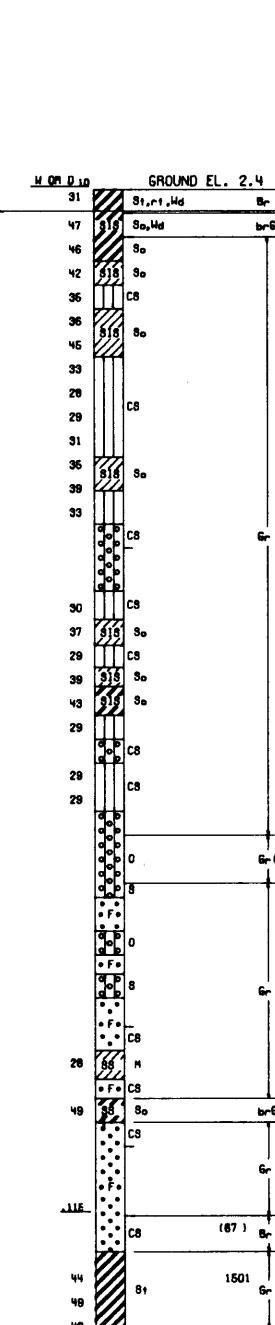
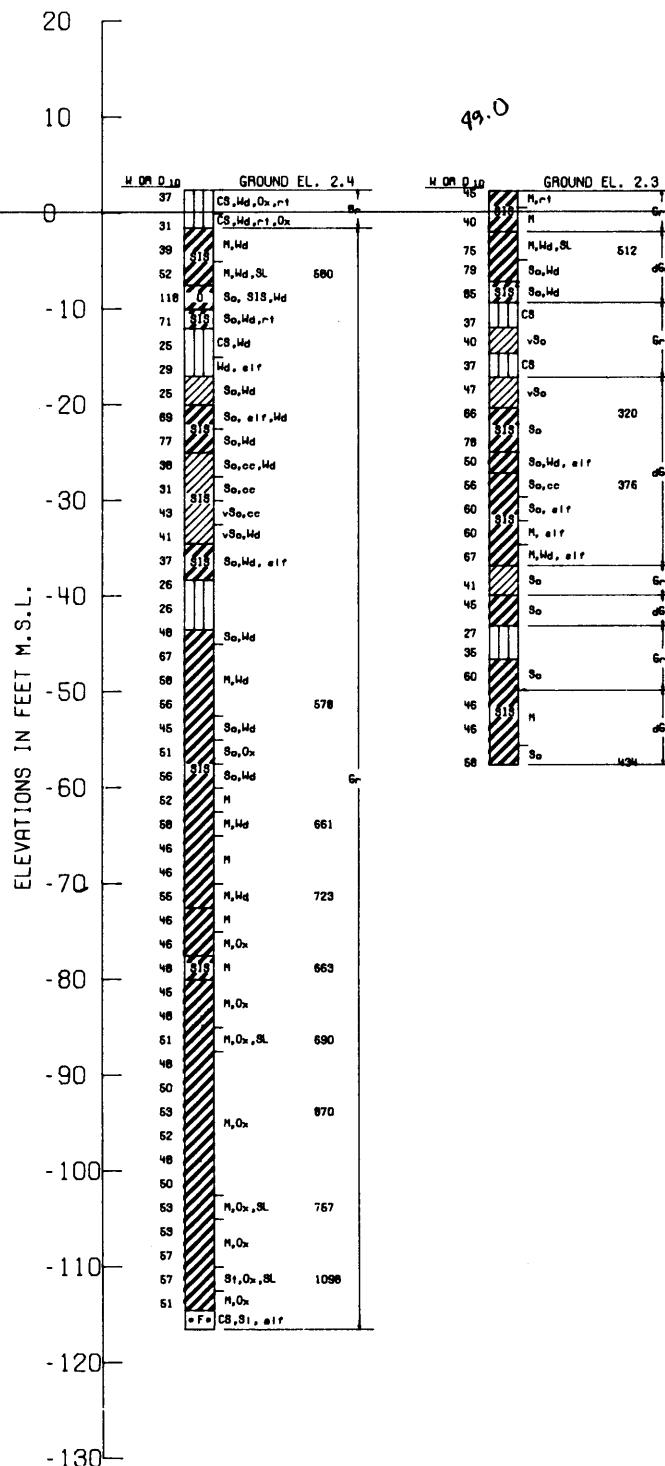
BOR. 44-MH
STA. 1800+00
ON C.L. LEVEE
4 AUG. 66

BOR. 42-MH
STA. 1852+00
38 FT. R.S. TOE
3 AUG. 66

BOR. R-46.7-R
STA. 1861+00
327 FT. R.S.
9 OCT. 69

BOR. 41-MH
STA. 1878+00
C.L. LEV.
3 AUG. 66

BOR. 40-MH
STA. 1904+00
45 FT. L.S. TOE
3 AUG. 66



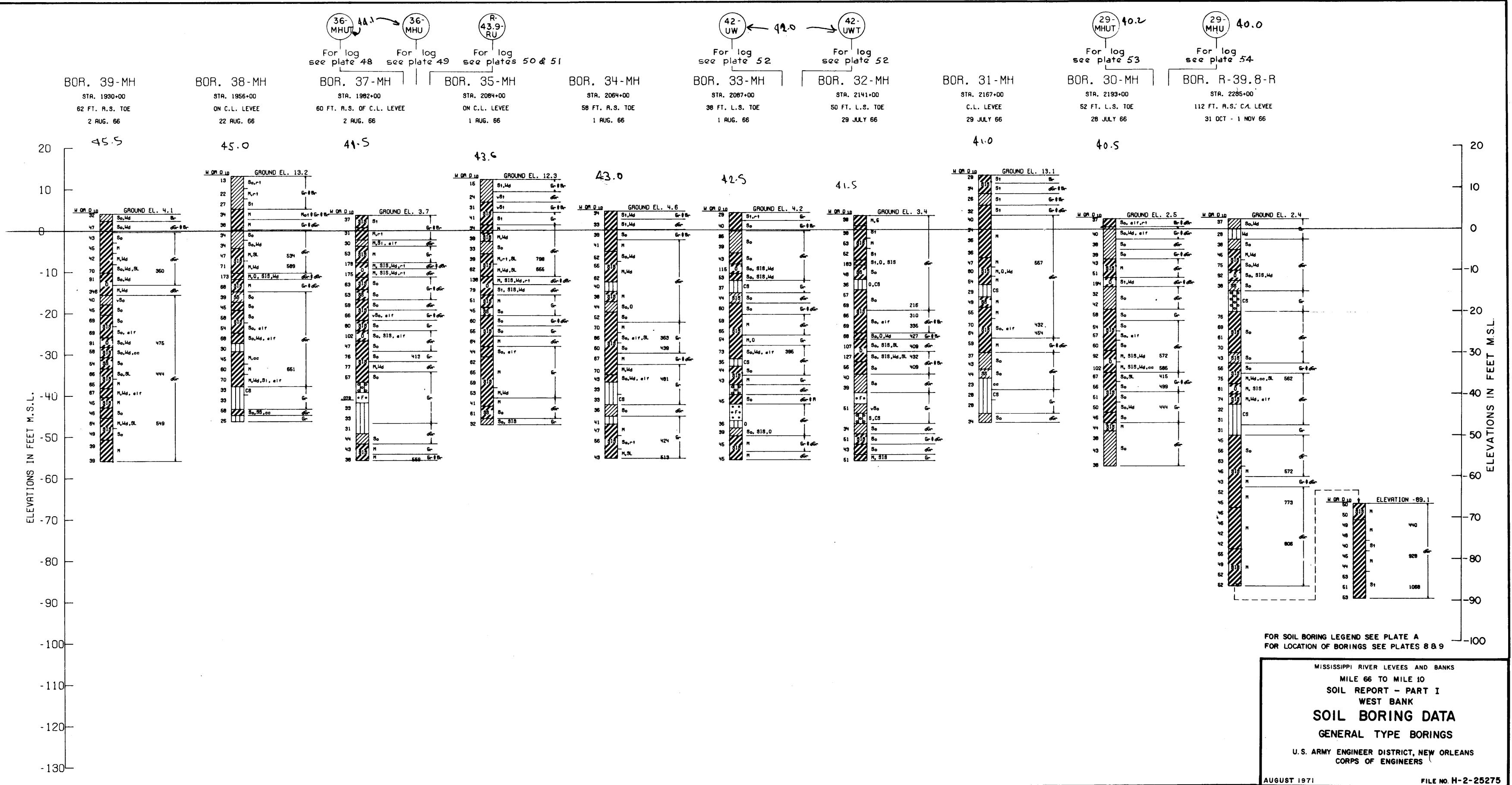
MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
WEST BANK
FOR SOIL BORING LEGEND SEE PLATE A
FOR LOCATION OF BORINGS SEE PLATES 7 & 8

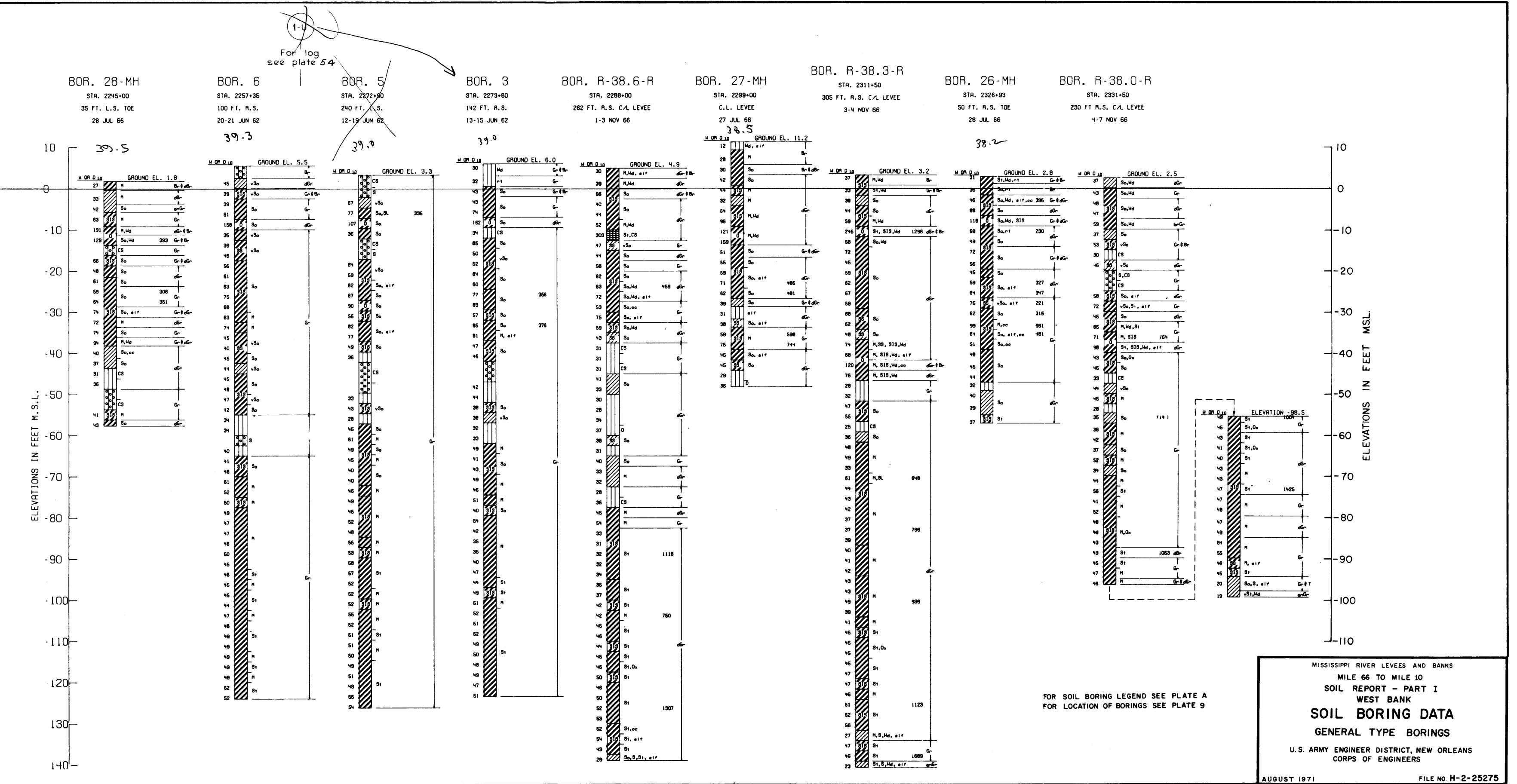
SOIL BORING DATA GENERAL TYPE BORINGS

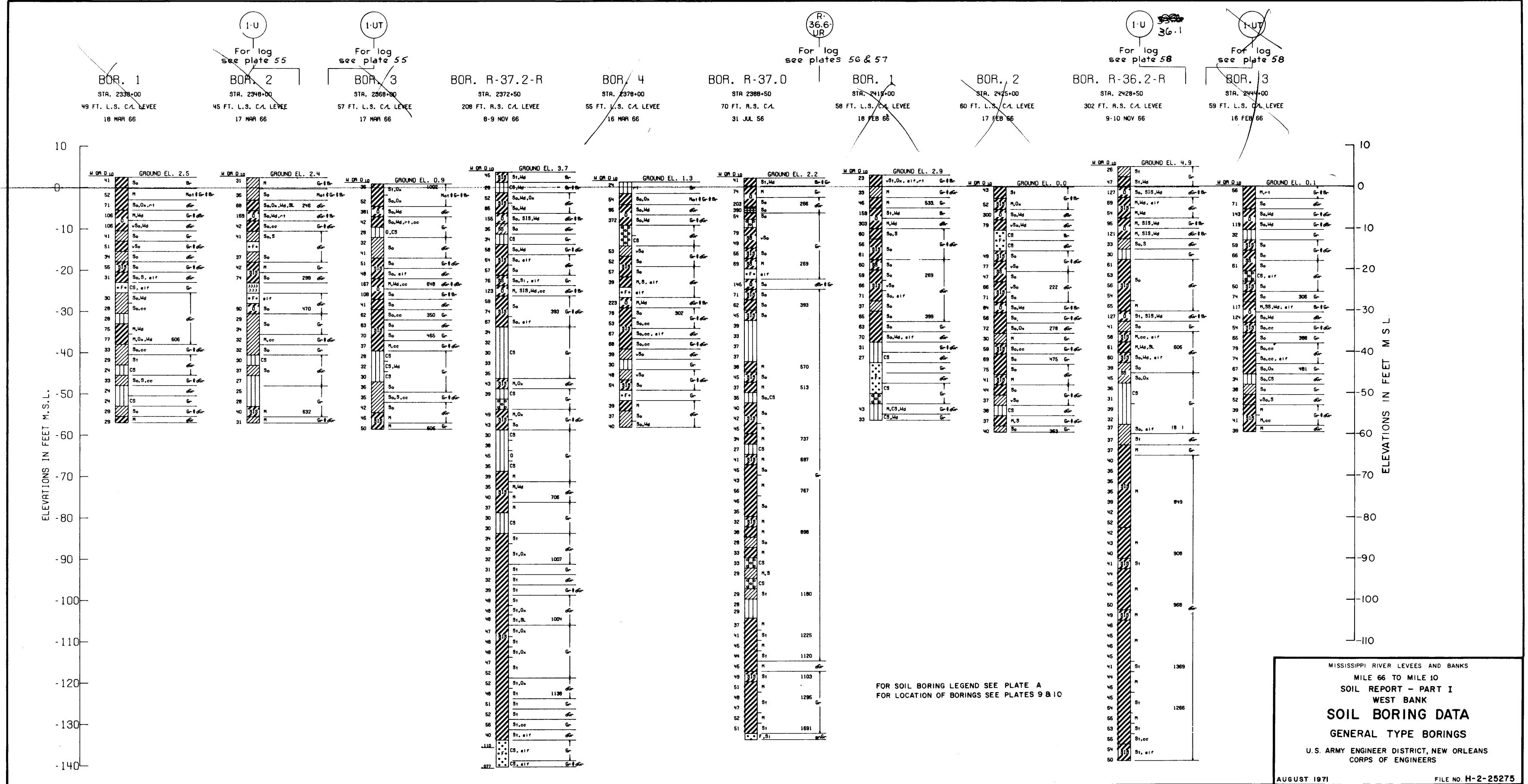
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS

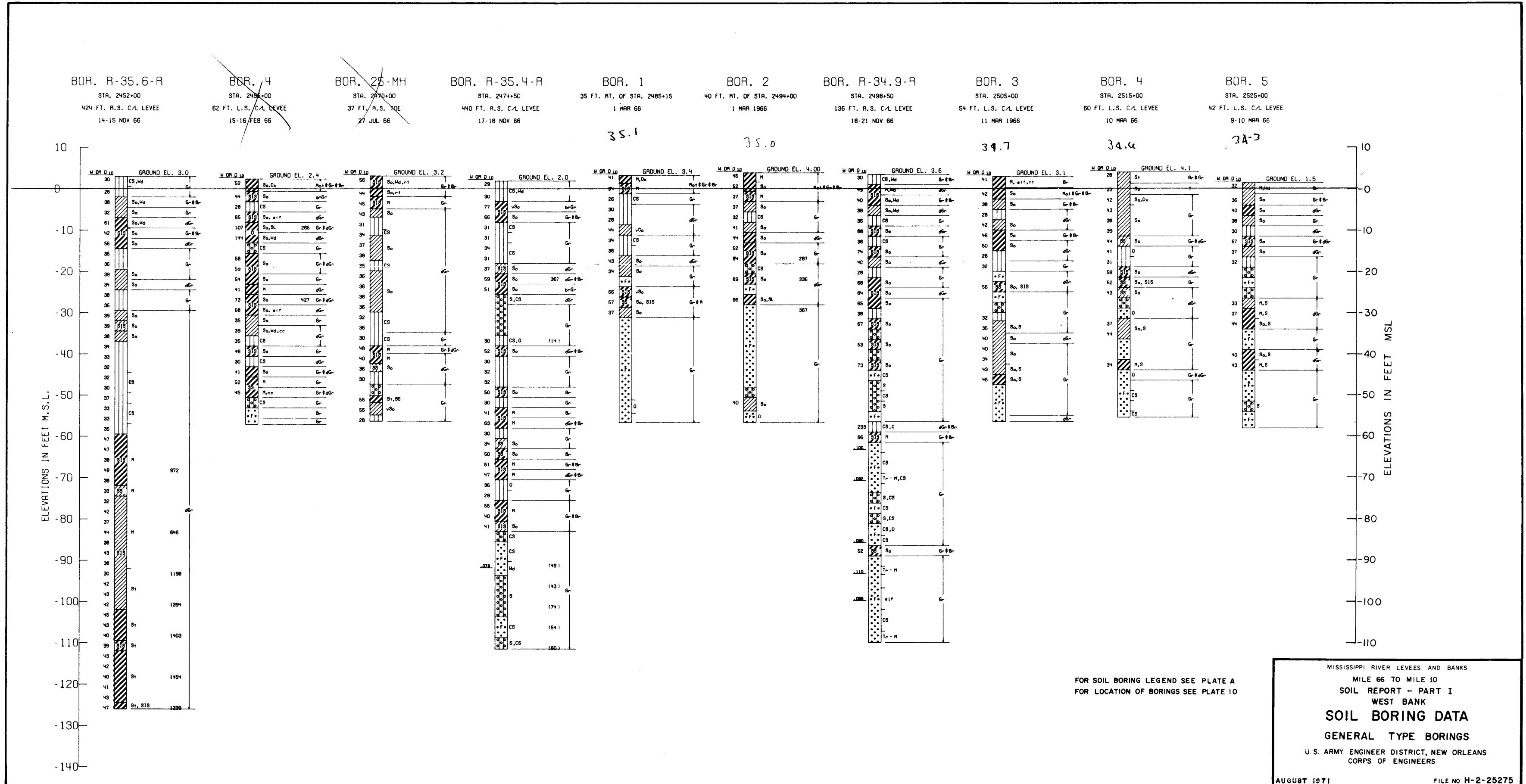
AUGUST 1971

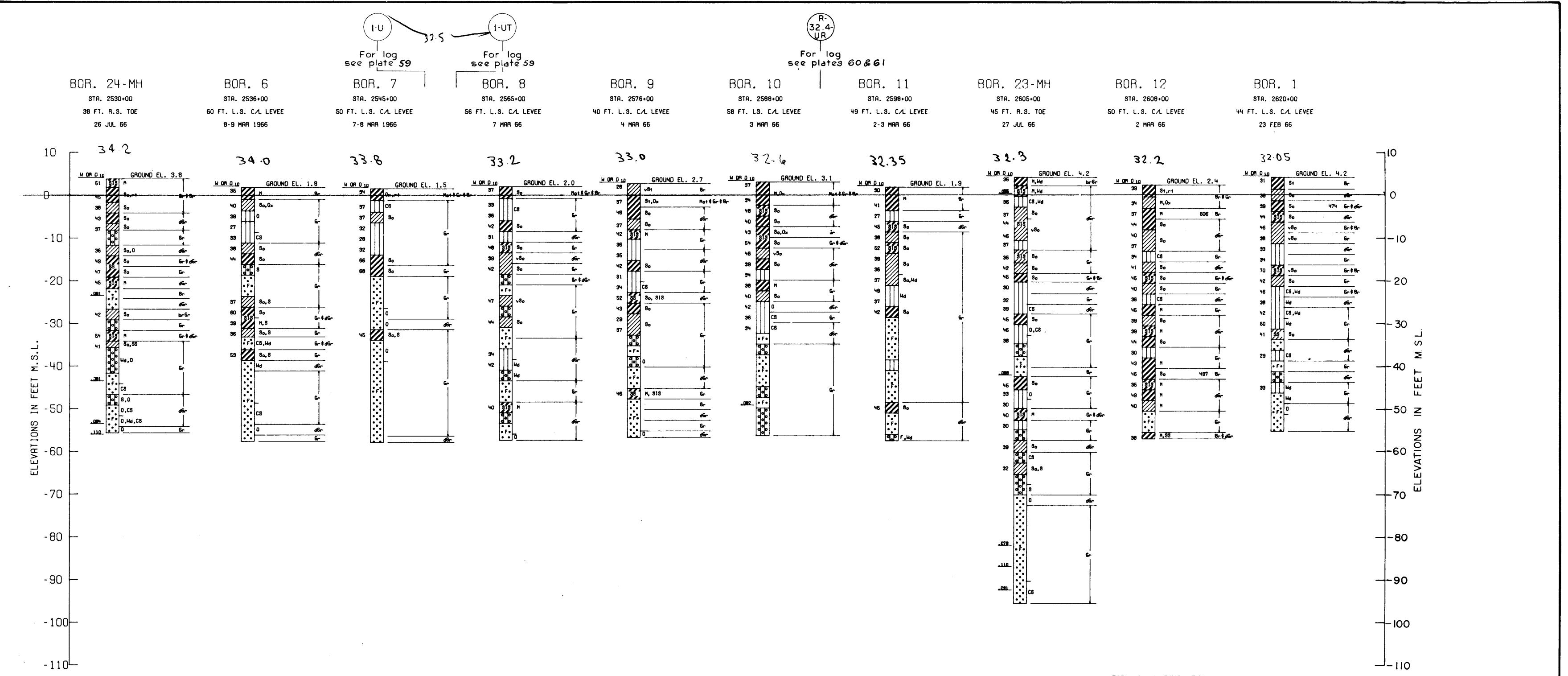
FILE NO. H-2-25275











FOR SOIL BORING LEGEND SEE PLATE A
FOR LOCATION OF BORINGS SEE PLATES 10 & II

MISSISSIPPI RIVER LEVEES AND BANKS

MILE 66 TO MILE 10
SOIL REPORT - PART I
WEST BANK

SOIL BORING DATA

GENERAL TYPE BORINGS

U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS

AUGUST 1971

FILE NO H-2-25275

PLATE 27

18-
IHU 293

For log
see plate 64

BOR. 20-MH
STA. 2730+00
40 FT. R.S. TOE
25 JUL 66

BOR. 19-MH
STA. 2755+00
50 FT. L.S. TOE
22 JUL 66

BOR. 17-MH
STA. 2804+00
44 FT. R.S. TOE
25 JULY 66

BOR. R-28.55-F
STA. 2022+00
70 FT. R.S.
13 FEB 68

BOR. 16-MH
STA. 2833+00
50 FT. L.S. TOE
21 JUL 66

BOR. R-28.05-R
STA 2849+50
165 R.S. C/L LEVEE
1-2 DEC 1966

BOR. 15-MH
STA. 2859+00
ON C.L. LEVEE
20 JUL 66

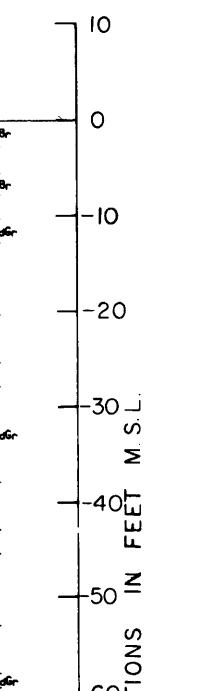
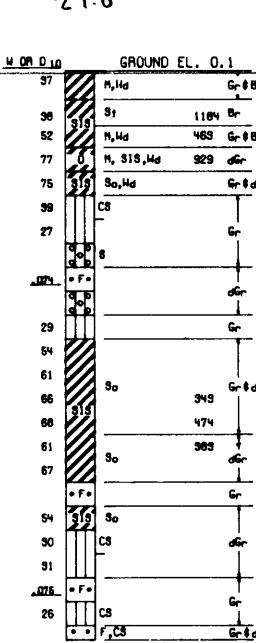
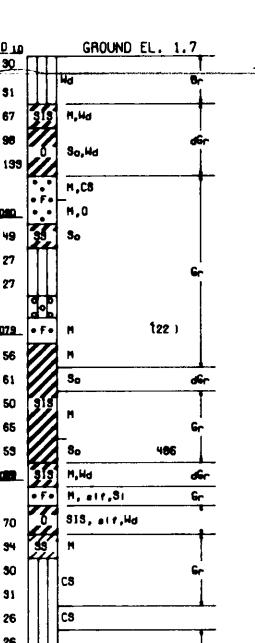
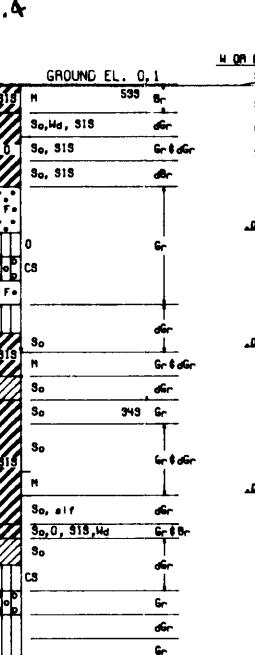
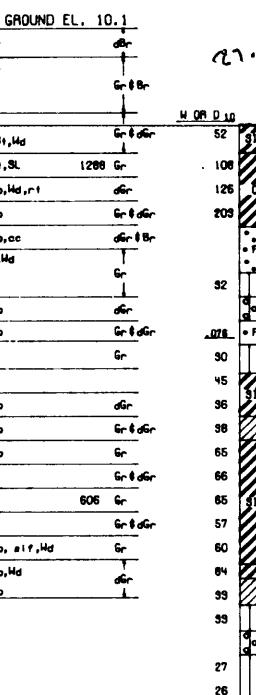
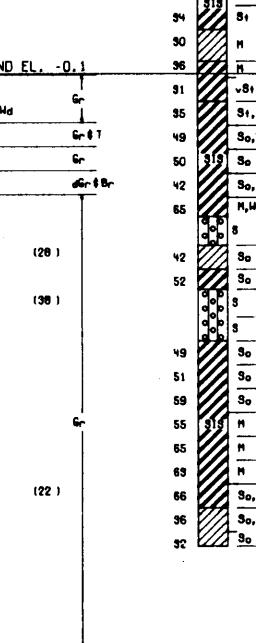
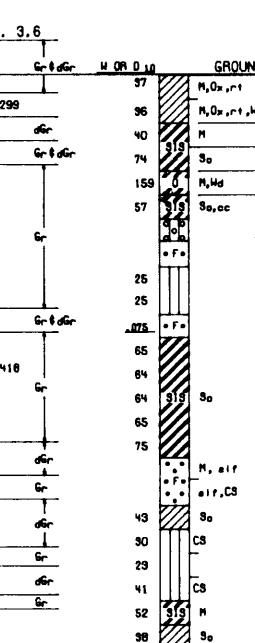
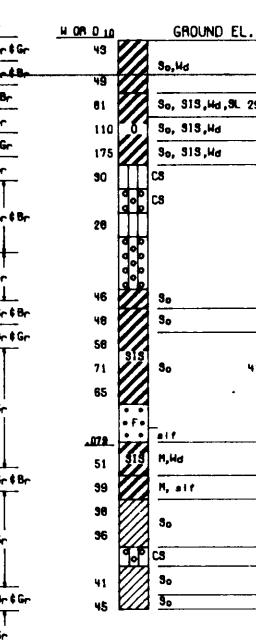
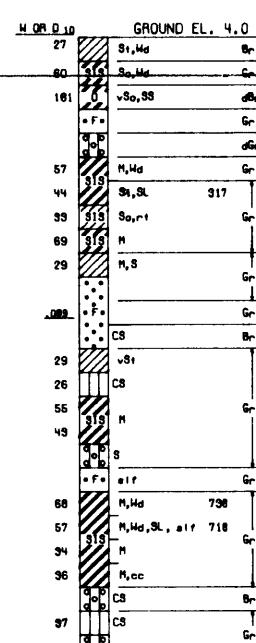
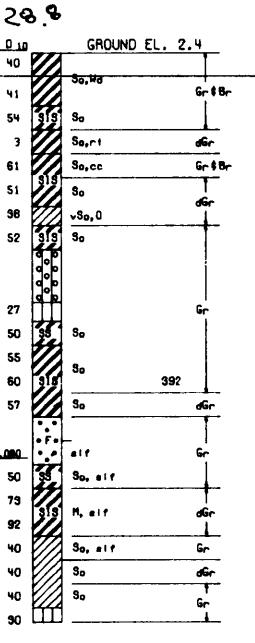
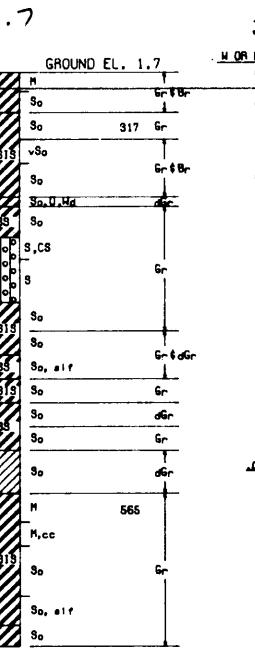
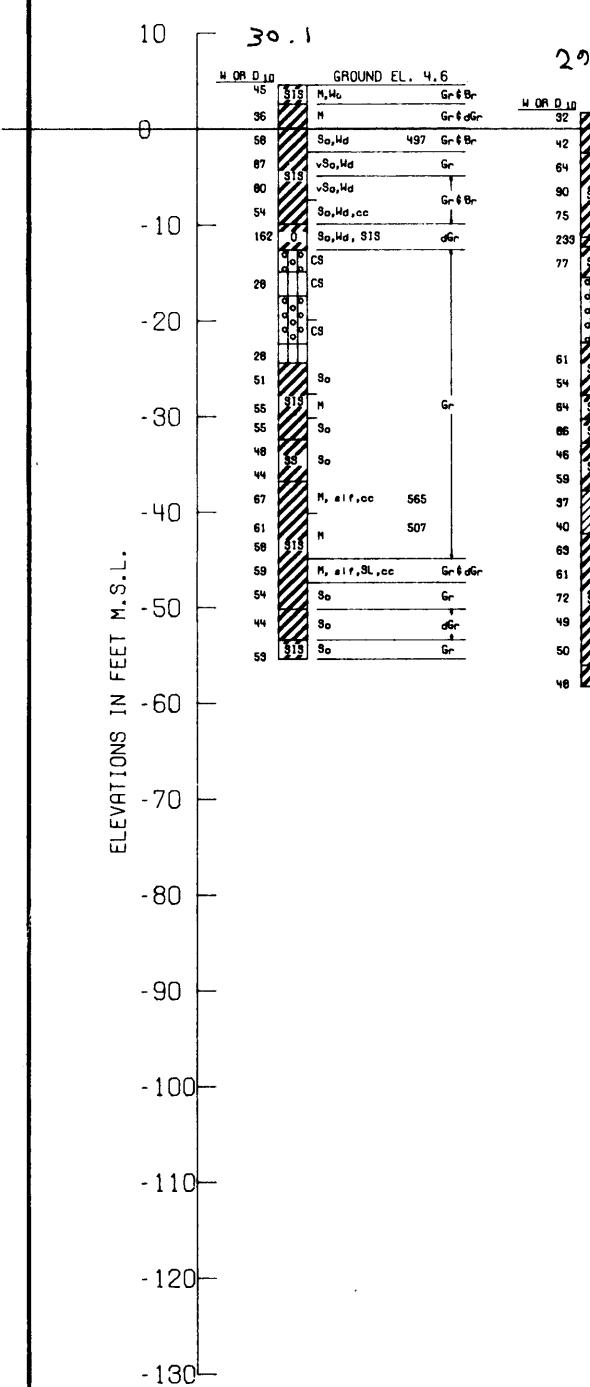
BOR. 14-MH
STA. 2885+00
48 FT. L.S. TOE
20 JUL 66

BOR. R-27.3-R
STA 2694+75
110 FT. R.S. C/L LEVEE
22 NOV 1966

BOR. 13-MH
STA. 2911+00
49FT. R.S. TOE
18 JUL 66

R-
27.6-
LIP

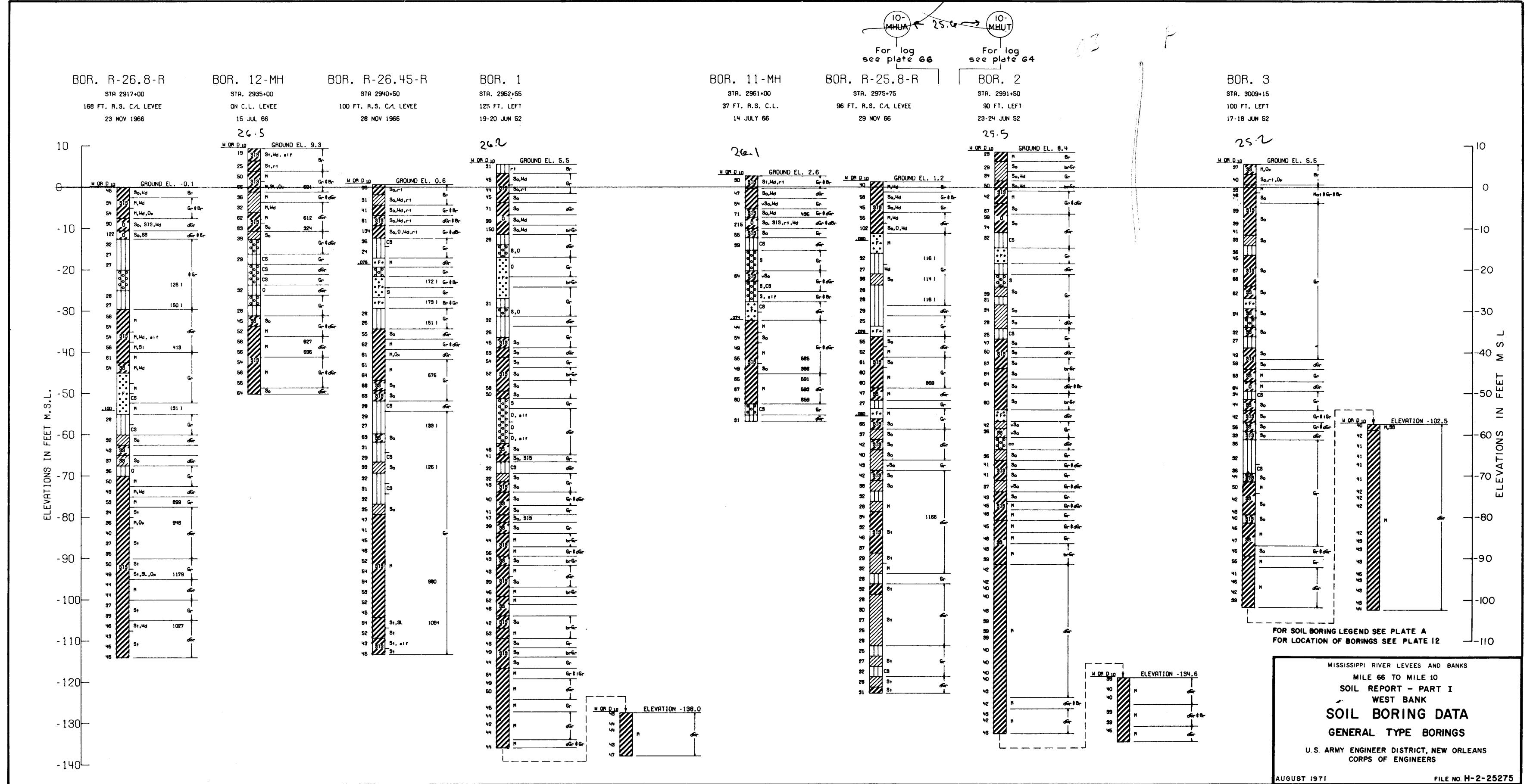
For log
see plate 65

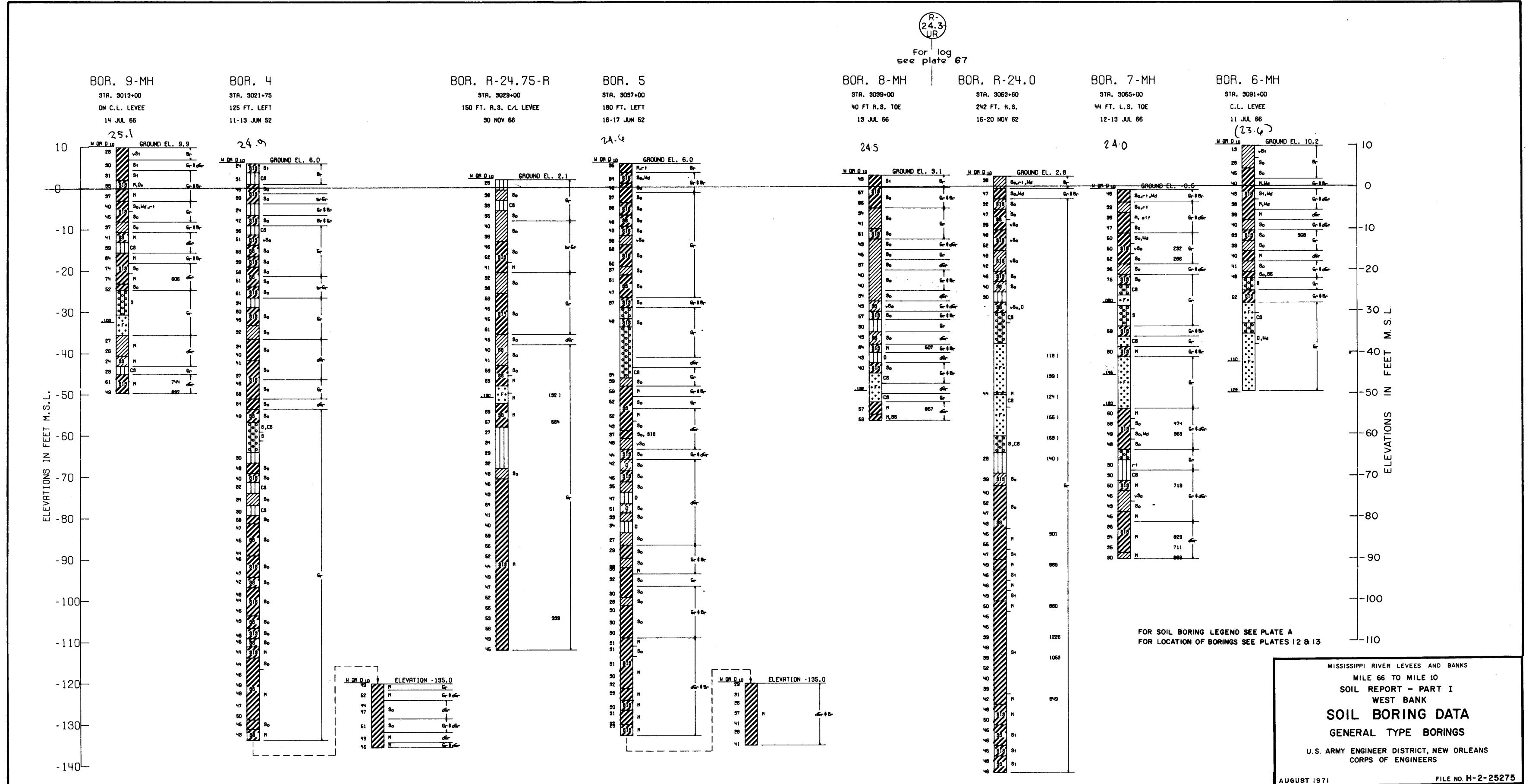


OR SOIL BORING LEGEND SEE PLATE A
OR LOCATION OF BORINGS SEE PLATES 11 & 12

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
WEST BANK

U. S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS



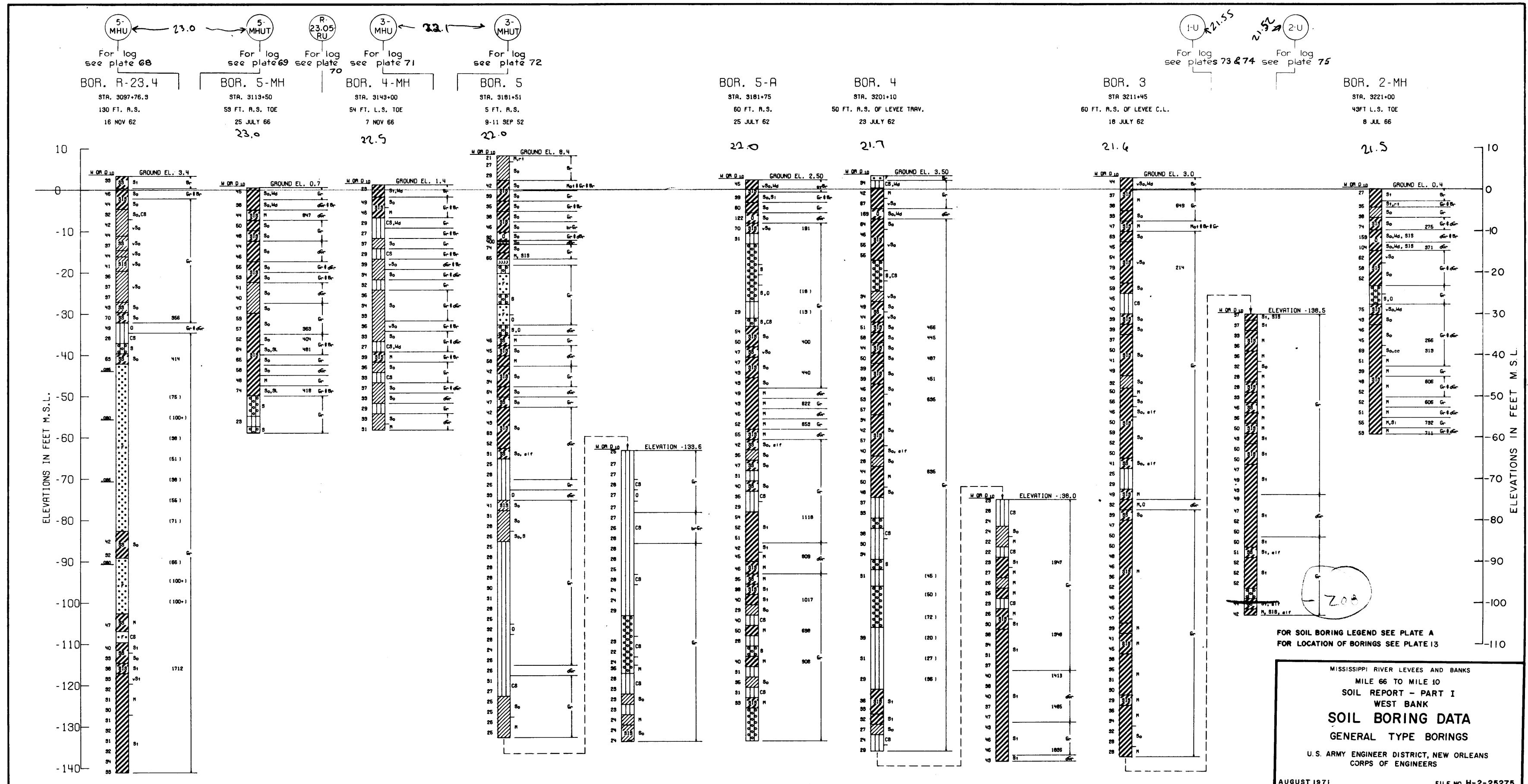


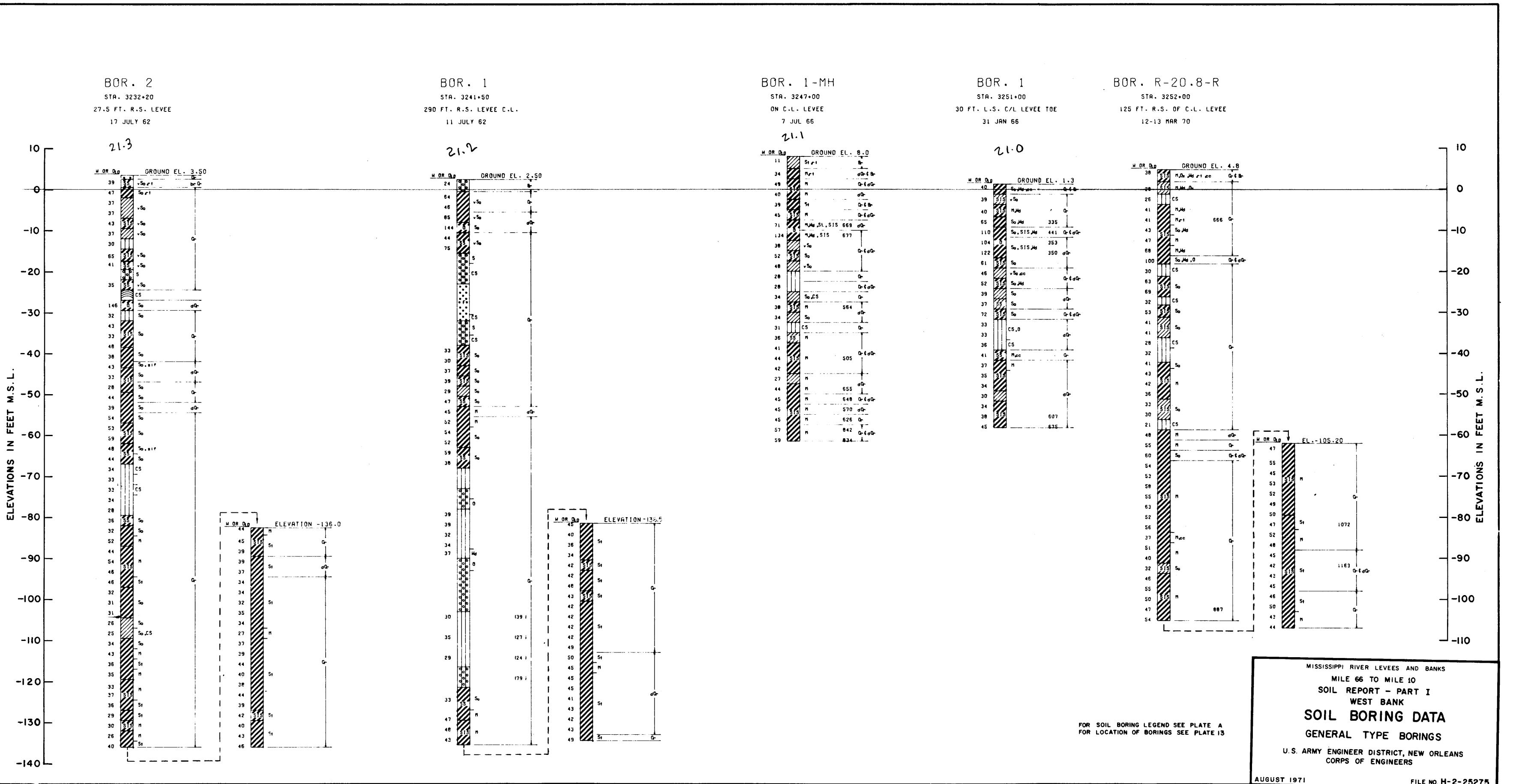
MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
WEST BANK
SOIL BORING DATA
GENERAL TYPE BORINGS
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS

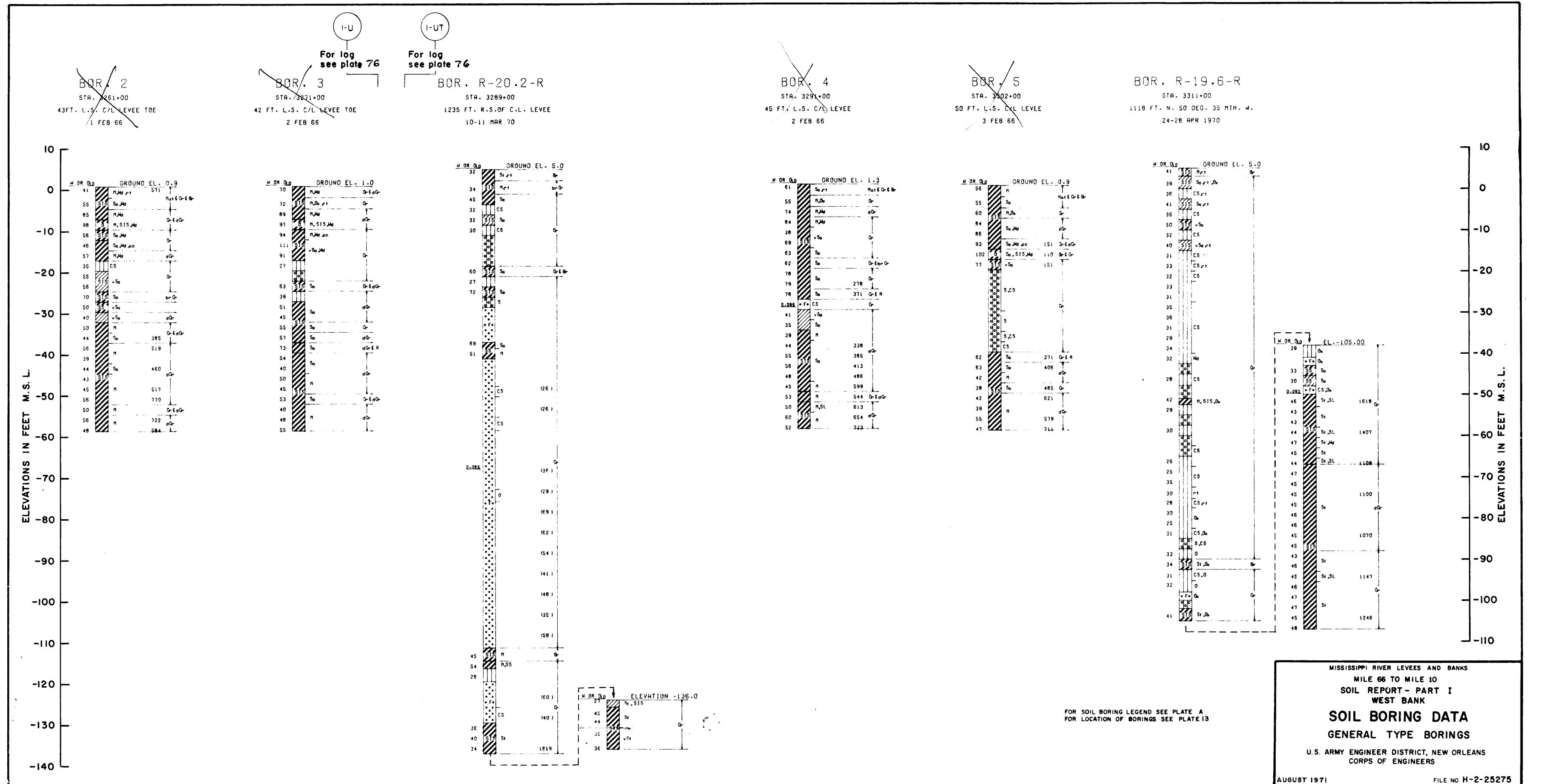
AUGUST 1971

FILE NO. H-2-25275

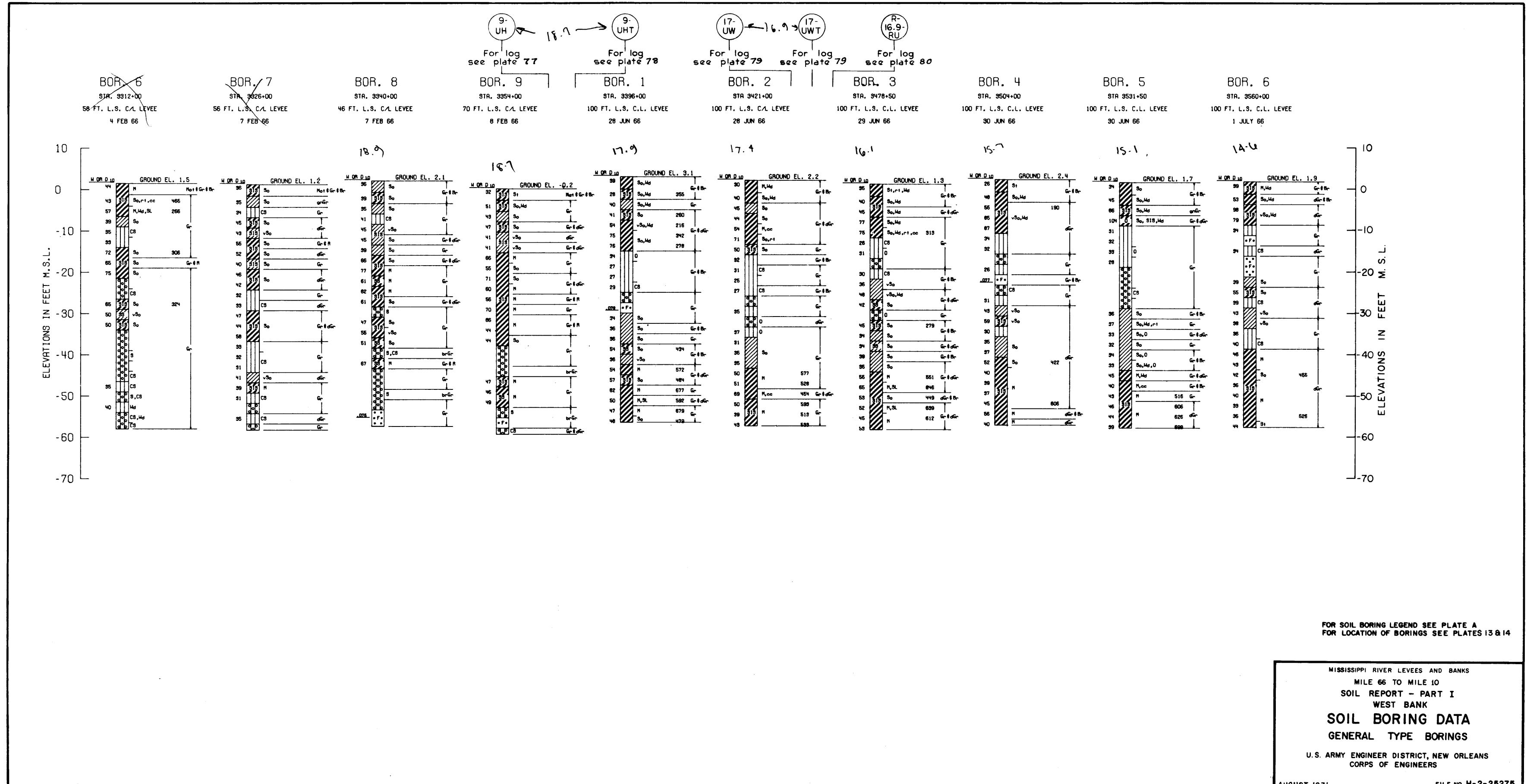
PLATE 31

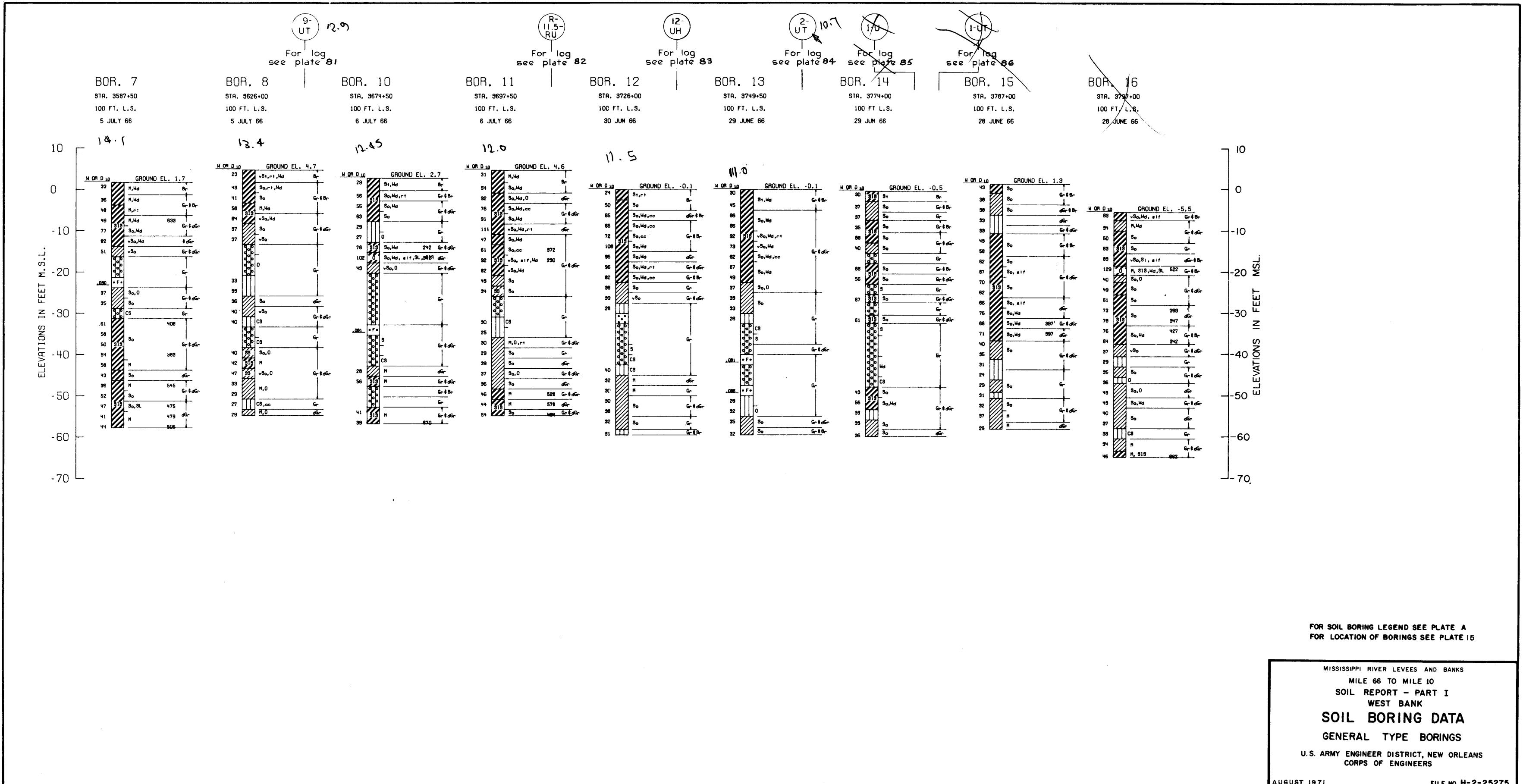


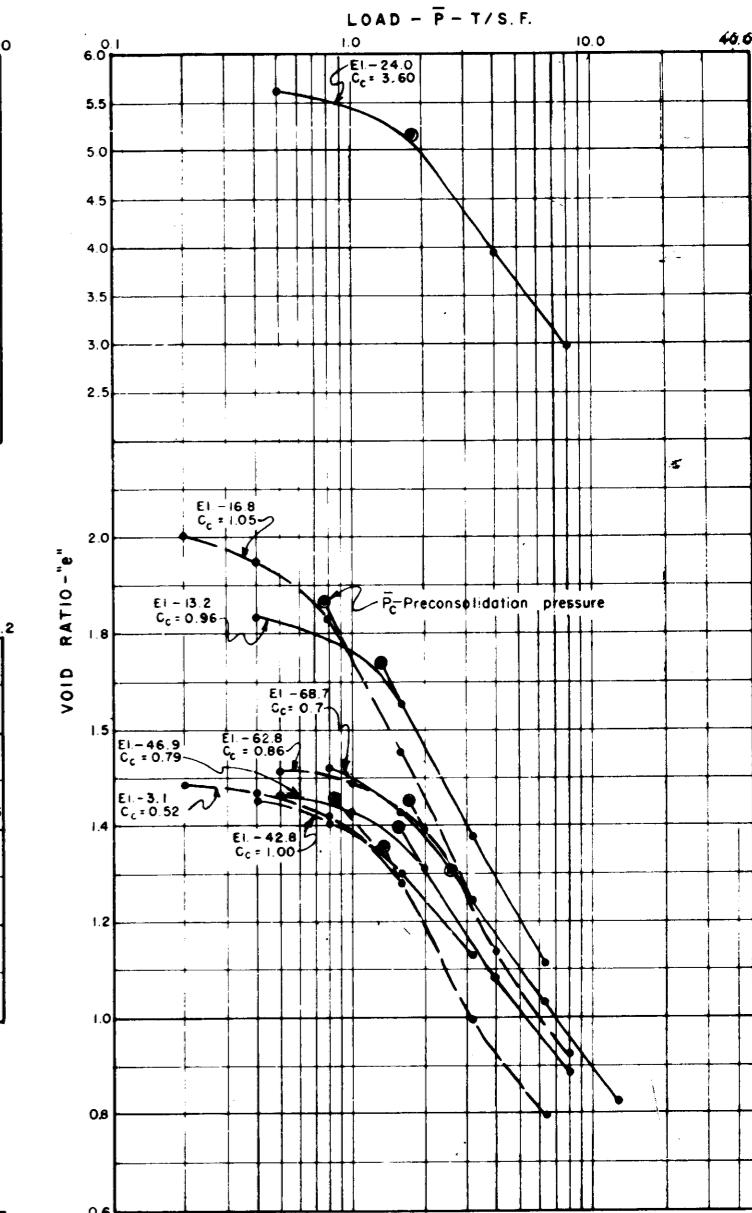
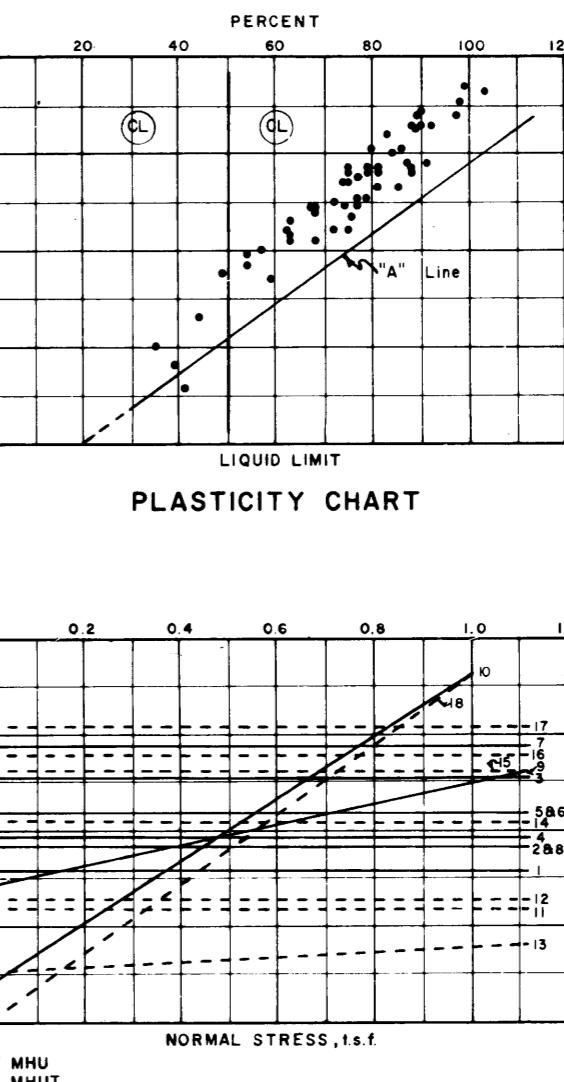
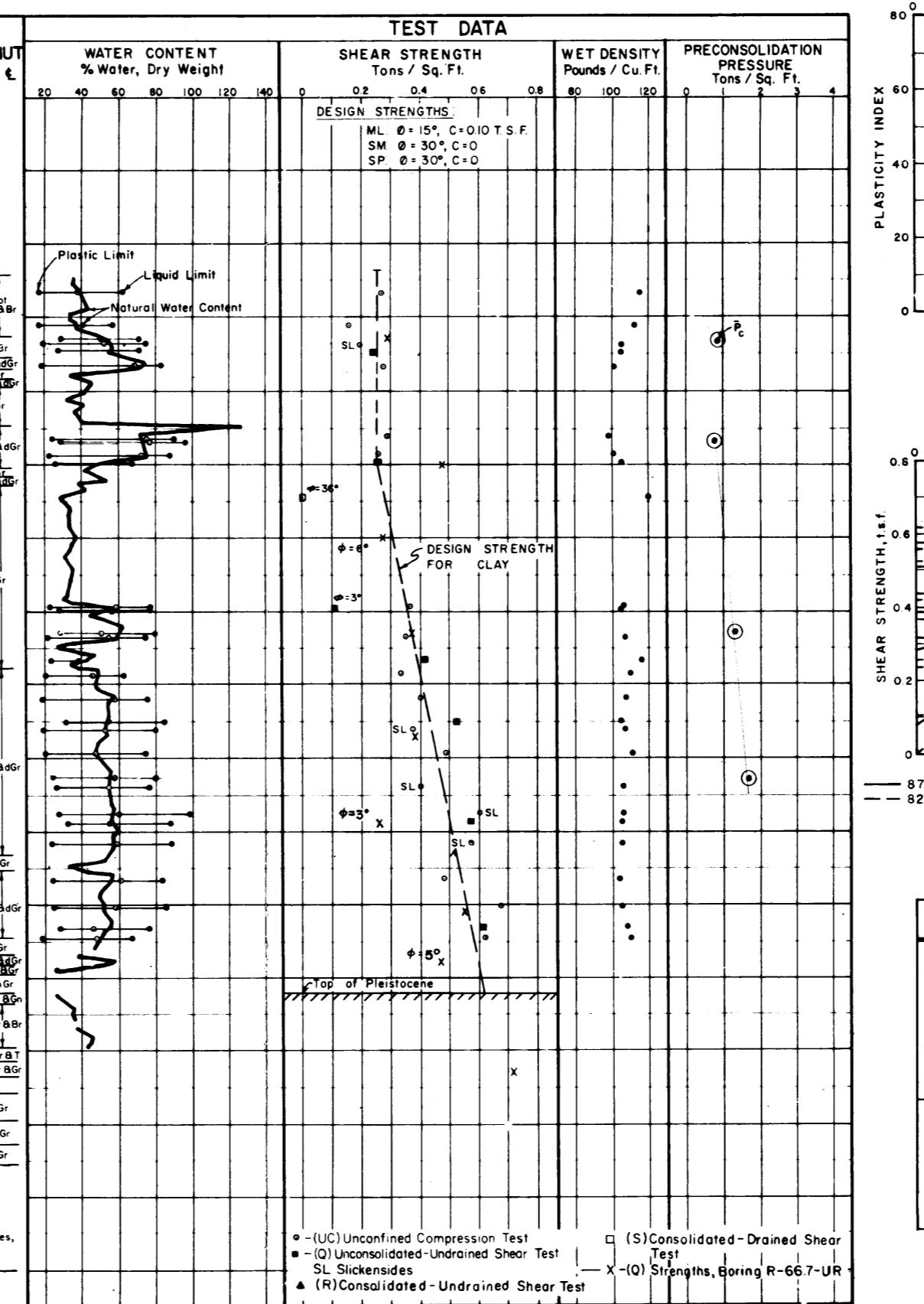
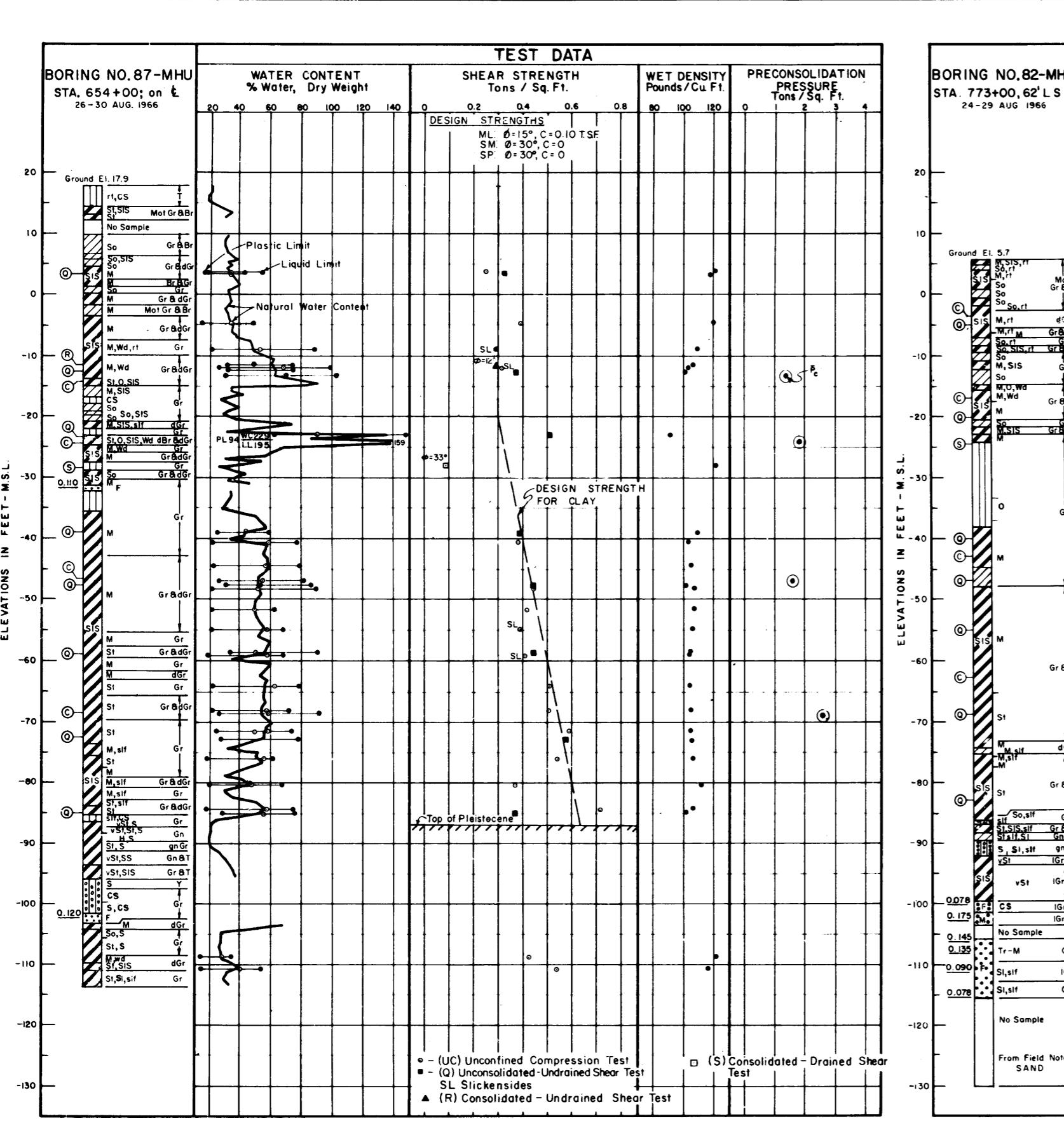




FOR SOIL BORING LEGEND SEE PLATE A
FOR LOCATION OF BORINGS SEE PLATE 13







CONSOLIDATION DATA

— Boring No.87-MHU
— Boring No.82-MHUT
For soil boring legend see plate A
For location of borings see plates 2 & 3

Borings were taken with a 5" diameter steel tube piston type sampler.

BORING NO.	ENVELOPE		TYPE	STRENGTH		CLASS
	NO	EL		ϕ°	(t. f.s.f.)	
87 MHU	1	- 3.4	Q	0	0.32	CL
	2	- 12.5		0	0.37	CH
	3	- 23.1		0	0.51	OH
	4	- 39.0		0	0.39	CH
	5	- 47.7		0	0.44	CH
	6	- 58.6		0	0.44	CH
	7	- 72.8		0	0.58	CH
	8	- 85.1		0	0.37	CH
	9	- 11.4	R	12	0.29	CH
	10	- 28.1	S	33	0.08	ML
82 MHUT	11	- 4.8	Q	0	0.24	CH
	12	- 19.7		0	0.26	CH
	13	- 39.8		3	0.11	CH
	14	- 46.7		0	0.42	CL
	15	- 55.0		0	0.53	CH
	16	- 68.7		0	0.56	CH
	17	- 82.9		0	0.62	CH
	18	- 24.5	S	36	0.00	ML

SHEAR STRENGTH DATA

protein-type complex

ANSWER *What is the name of the author of the book?*

MISSISSIPPI RIVER LEVEES AND BANKS

MILE 66 TO MILE 10

SOIL REPORT - PART I

WEST BANK

SOIL BORING DATA

87-MHU - STA. 654+0

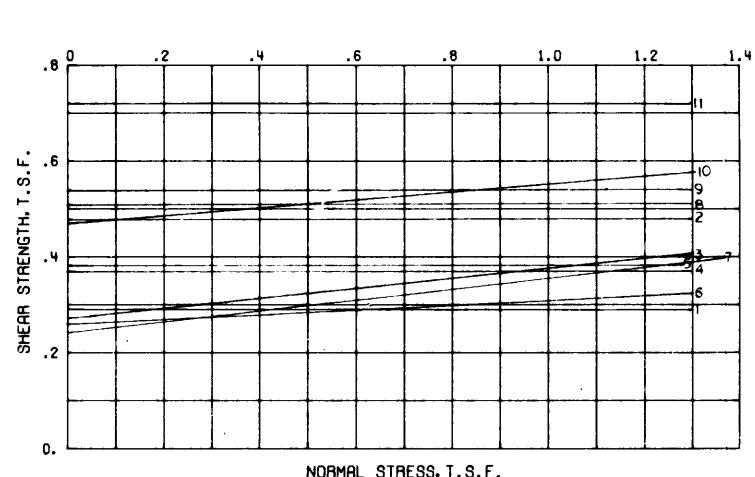
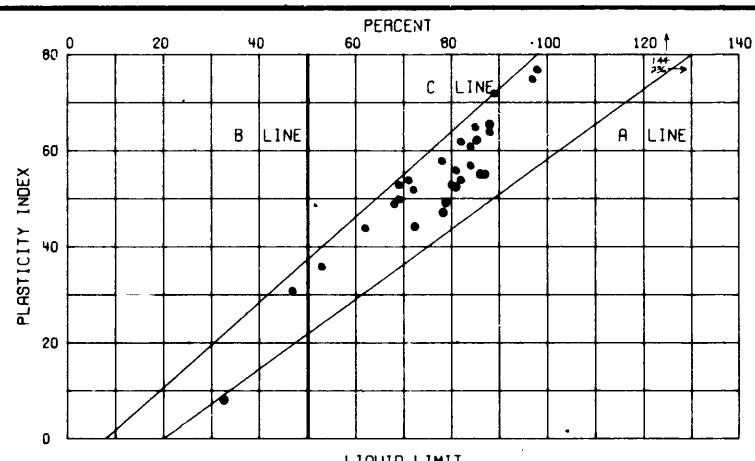
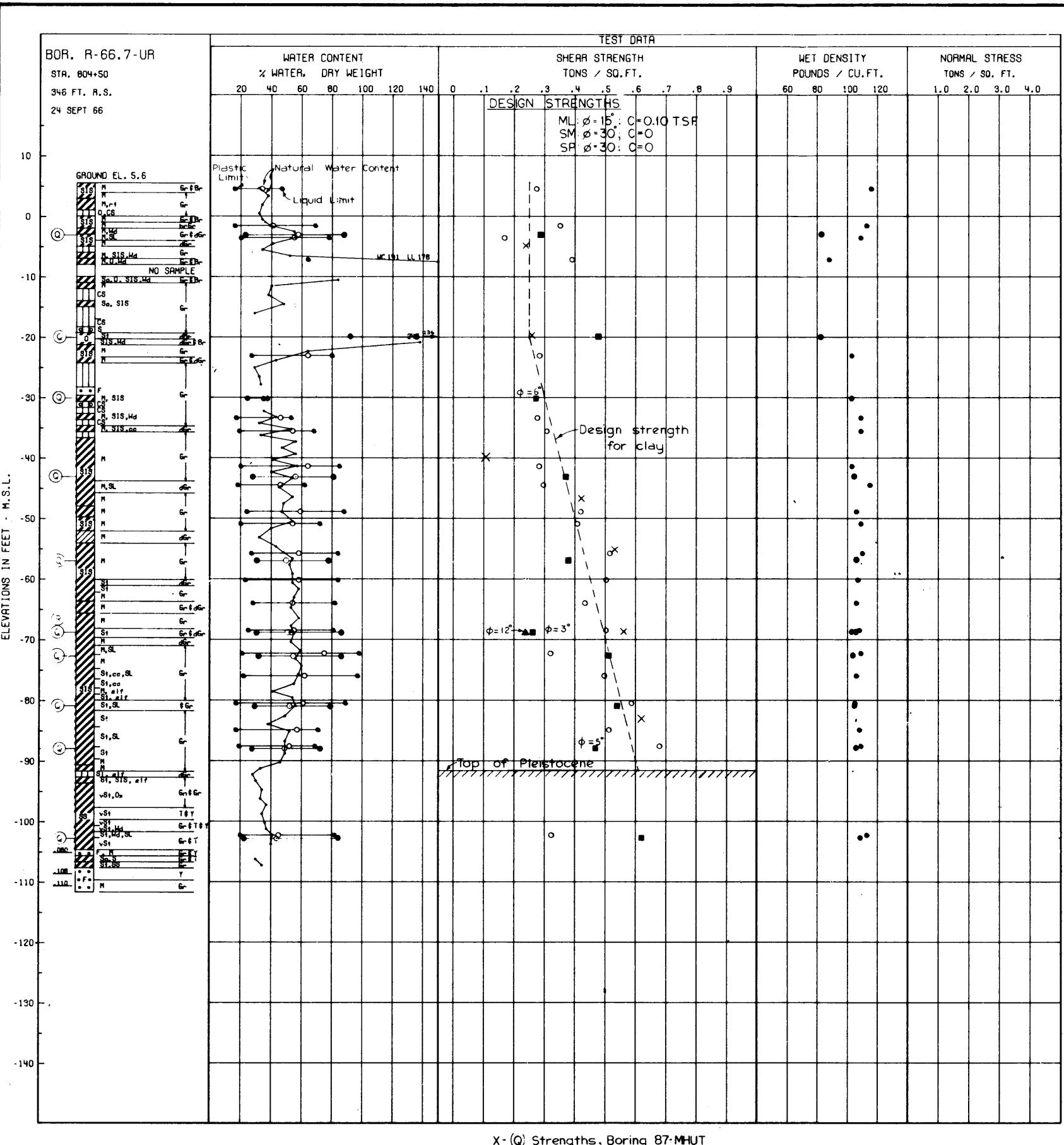
82-MHUT- STA. 773+0

U. S. ARMY ENGINEER DISTRICT, NEW ORLEANS

CORPS OF ENGINEERS

AUGUST 1971 FILE NO. H

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BORING NO.	ENVELOPE		TYPE	STRENGTH		CLASS
	NO.	EL.		Φ	C - TSF	
R-66.7-UR	1	- 3.0	Q	0°	0.29	CH
	2	-20.0	Q	U°	0.48	OH
	3	-30.1	Q	6°	0.27	ML
	4	-43.0	Q	0°	0.37	CH
	5	-56.9	Q	0°	0.38	CH
	6	-68.8	Q	3°	0.26	CH
	7	-68.8	R	12°	0.24	CH
	8	-72.6	Q	0°	0.51	CH
	9	-80.8	Q	0	0.54	CH
	10	-87.9	Q	5°	0.47	CH
	11	-102.9	Q	0°	0.72	CH

- - (UC) UNCONFINED COMPRESSION TEST
- - (Q) UNCONSOLIDATED - UNDRAINED SHEAR TEST
- ▲ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST
- (S) CONSOLIDATED - DRAINED SHEAR TEST

BORINGS WERE TAKEN WITH A 5 INCH DIAMETER
STEEL TUBE PISTON TYPE SAMPLER
FOR SOIL BORING LEGEND SEE PLATE A
FOR LOCATION OF BORINGS SEE PLATE 3

CONSOLIDATION DATA

- - (UC) UNCONFINED COMPRESSION TEST
- - (Q) UNCONSOLIDATED - UNDRAINED SHEAR TEST
- ▲ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST
- (S) CONSOLIDATED - DRAINED SHEAR TEST

BORINGS WERE TAKEN WITH A 5 INCH DIAMETER
STEEL TUBE PISTON TYPE SAMPLER
FOR SOIL BORING LEGEND SEE PLATE A
FOR LOCATION OF BORINGS SEE PLATE 3

MISSISSIPPI RIVER LEVEES AND BANKS

RIVER LEVELS AND BANKS
E 66 TO MILE 10

REPORT = PART I

**REPORT, PART 4
WEST BANK**

**WEST BANK
SOIL BORING DATA**

SITE BURNING DATA

R-66.7-UR

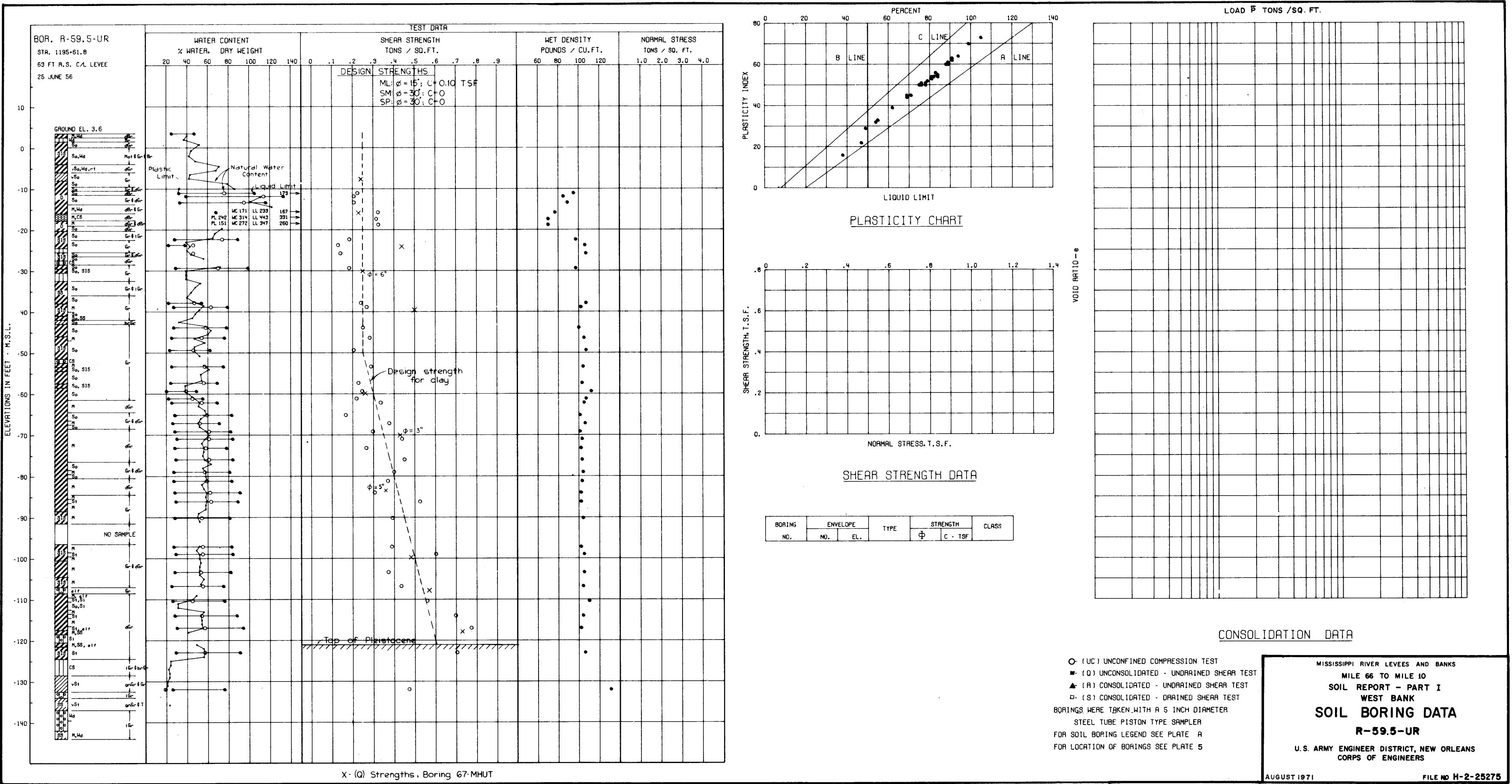
**ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS**

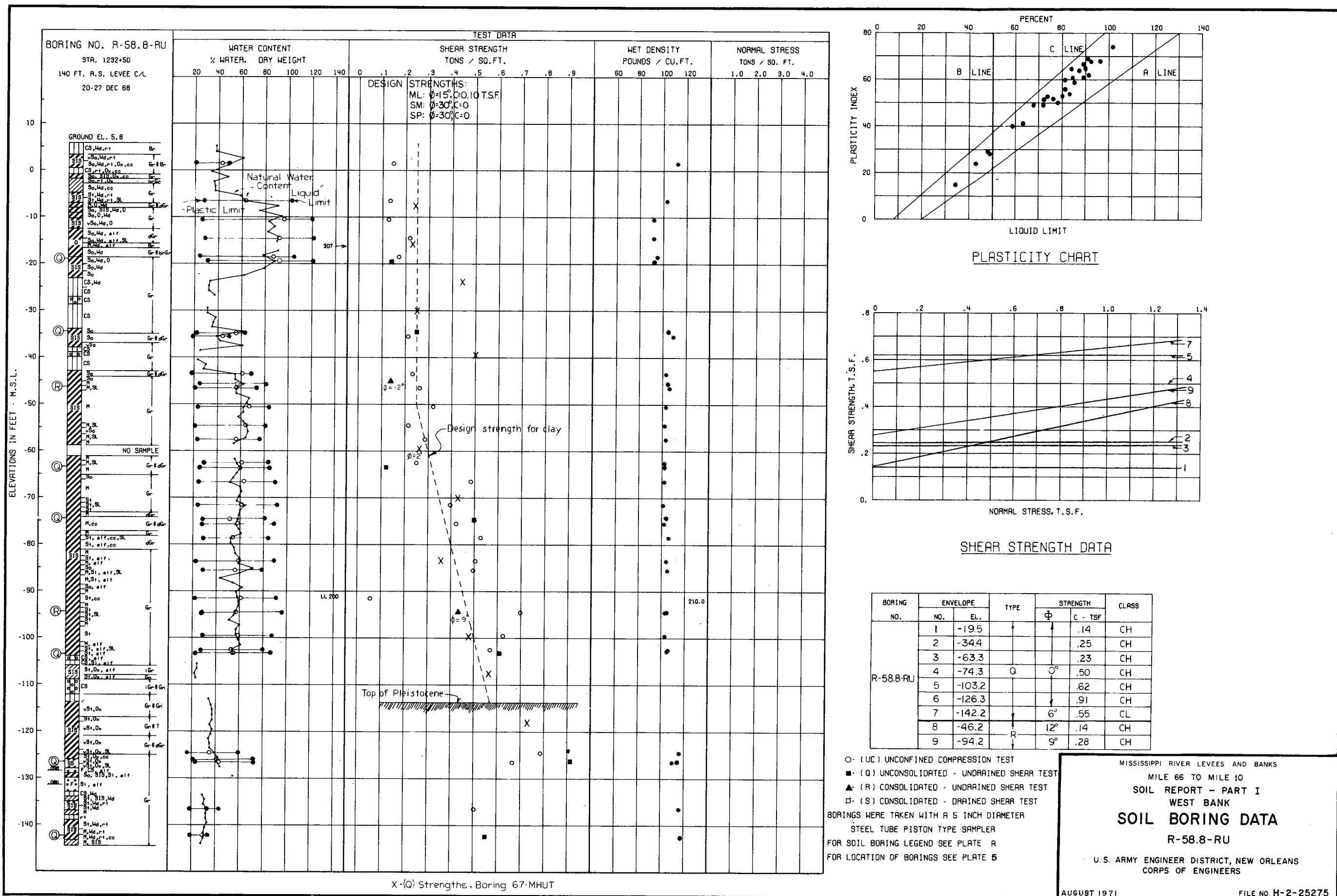
**U. S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS**

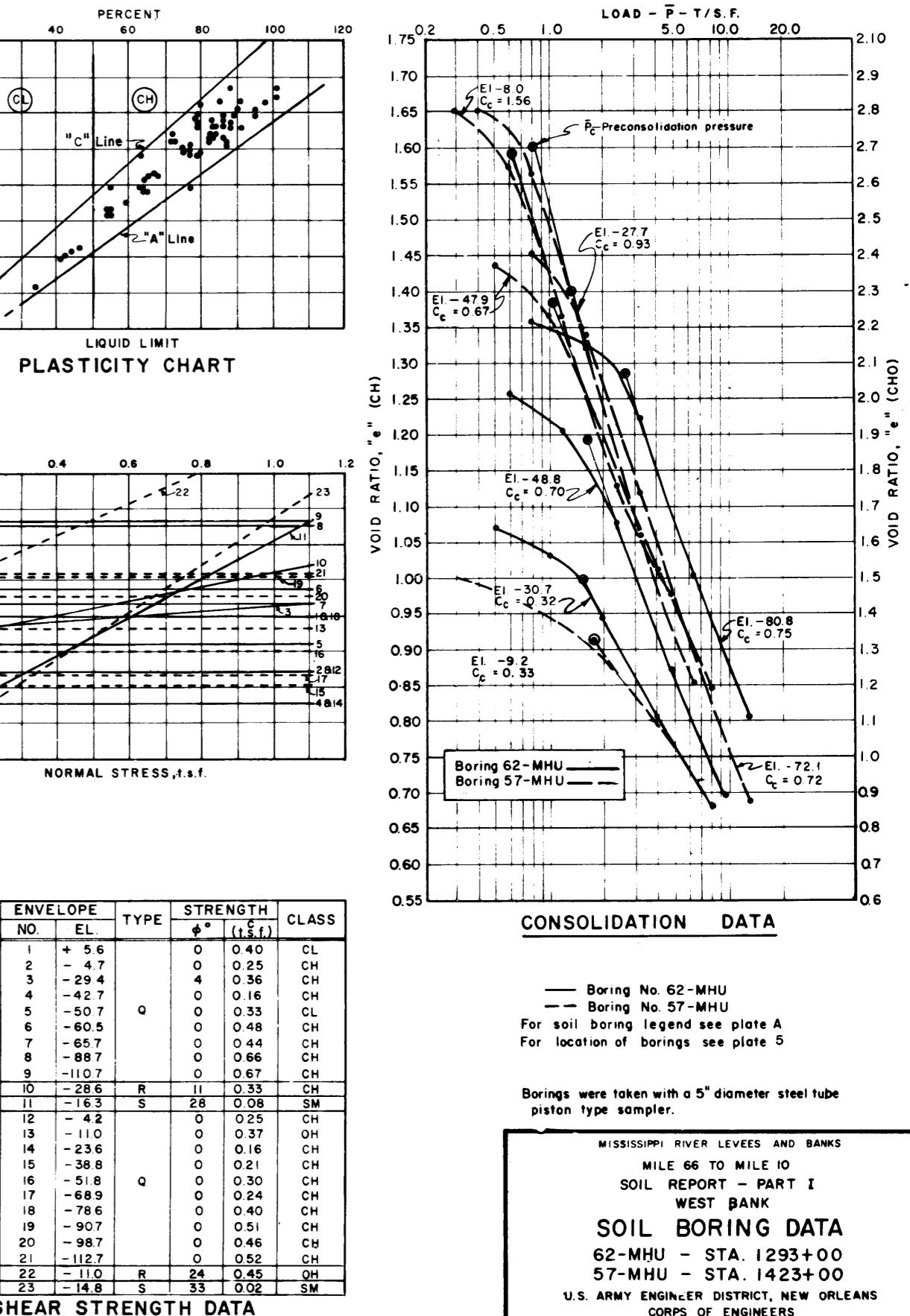
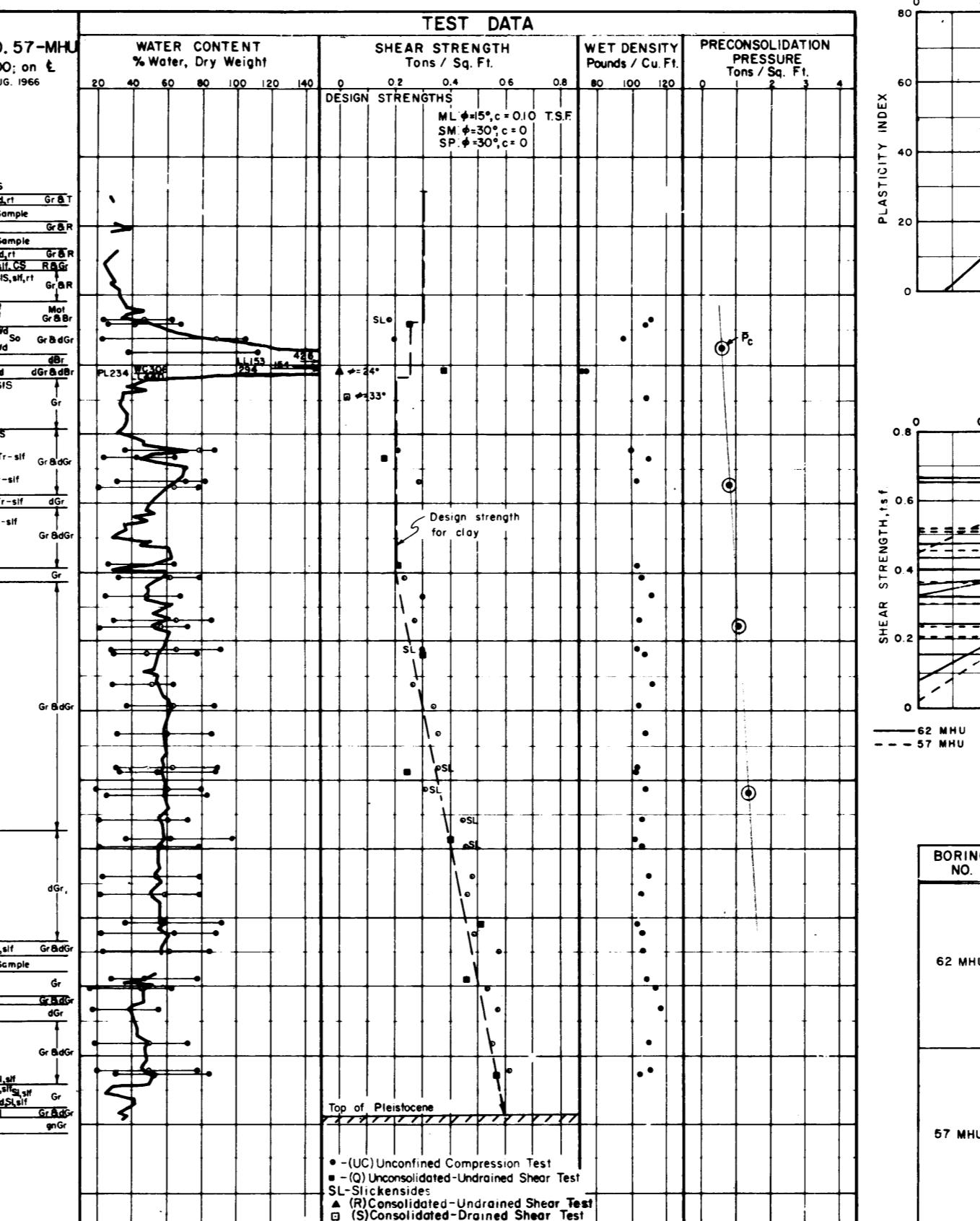
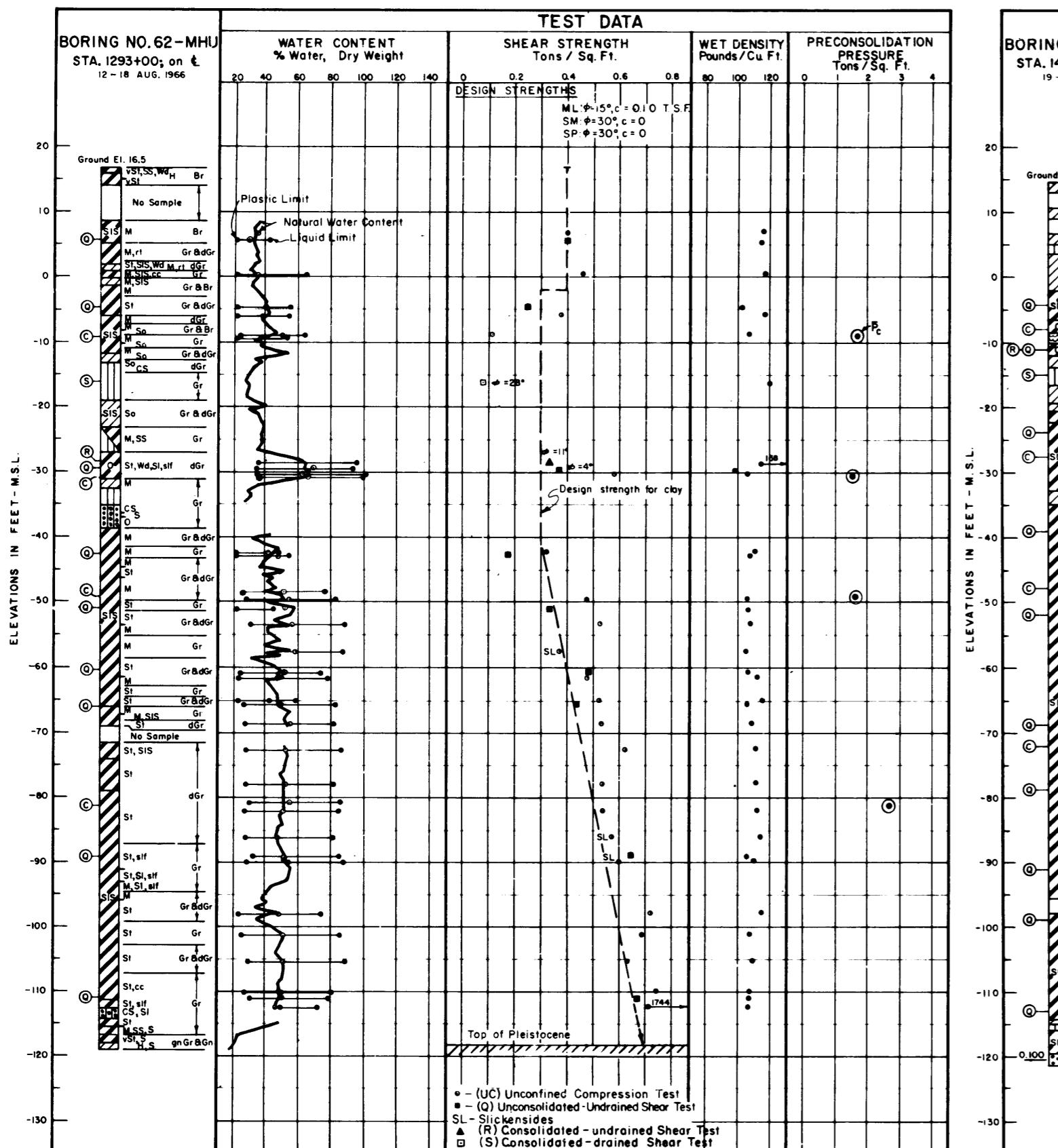
SOCIETY OF ENGINEERS

FILE NO. H-2-252

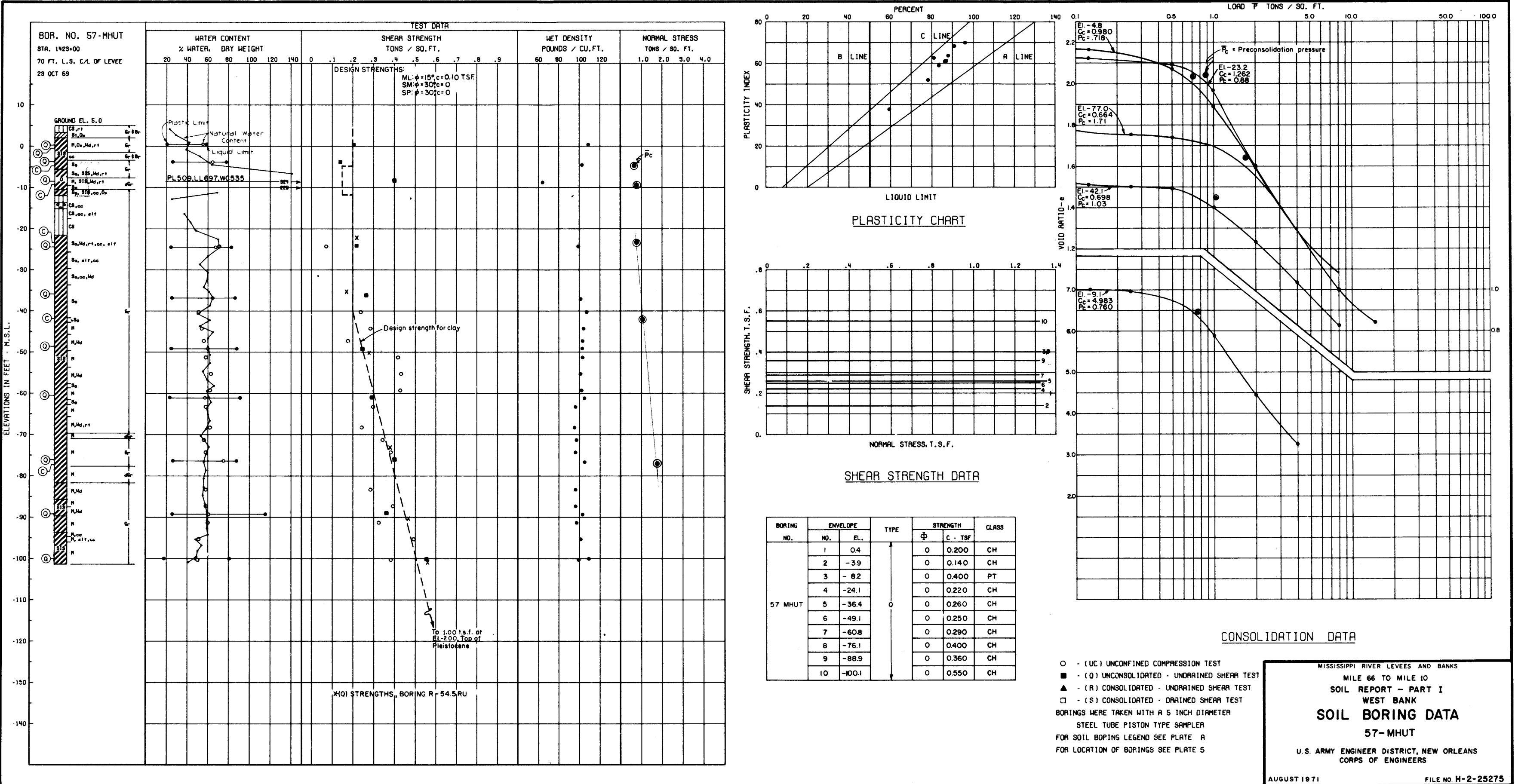
PLATÉ

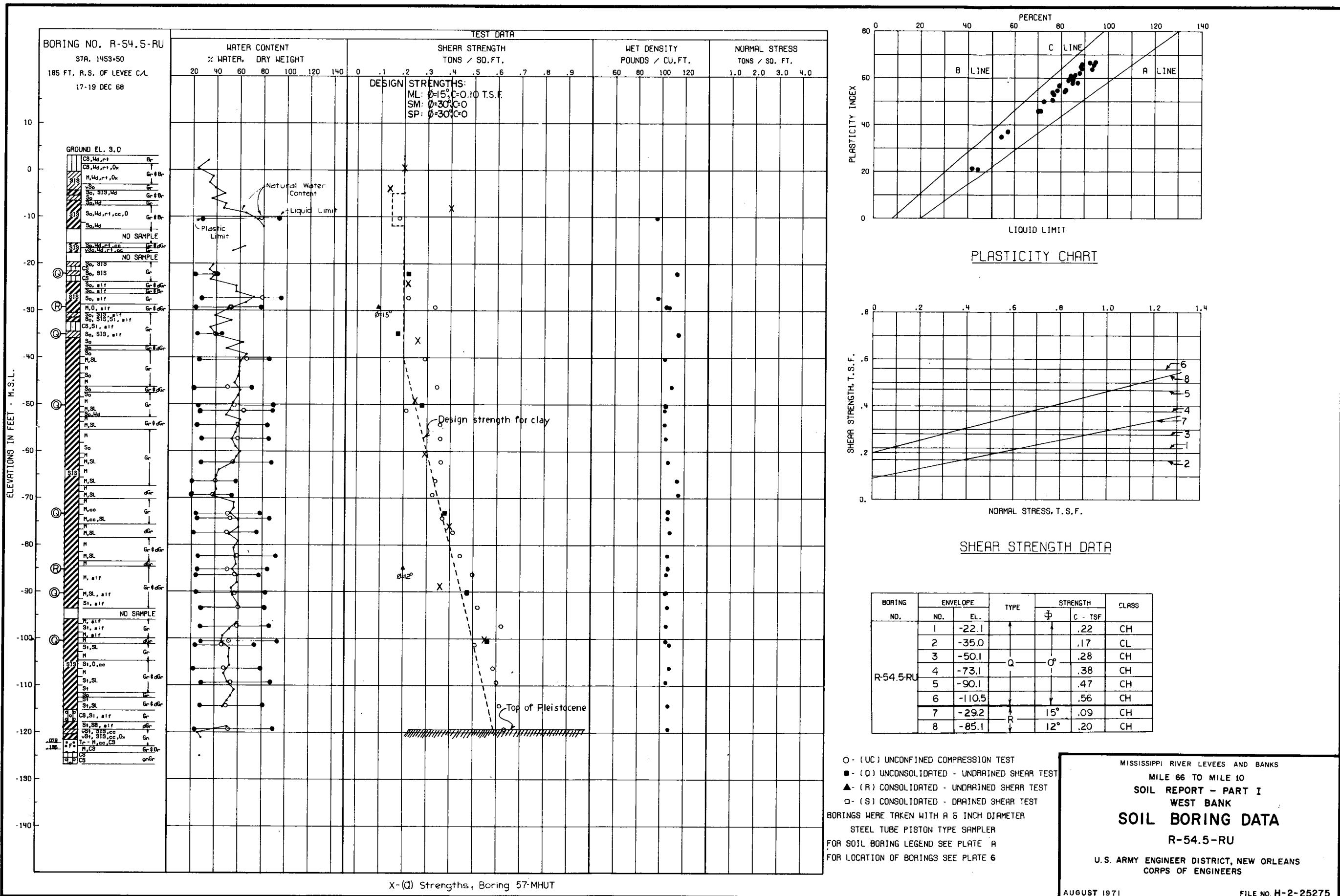


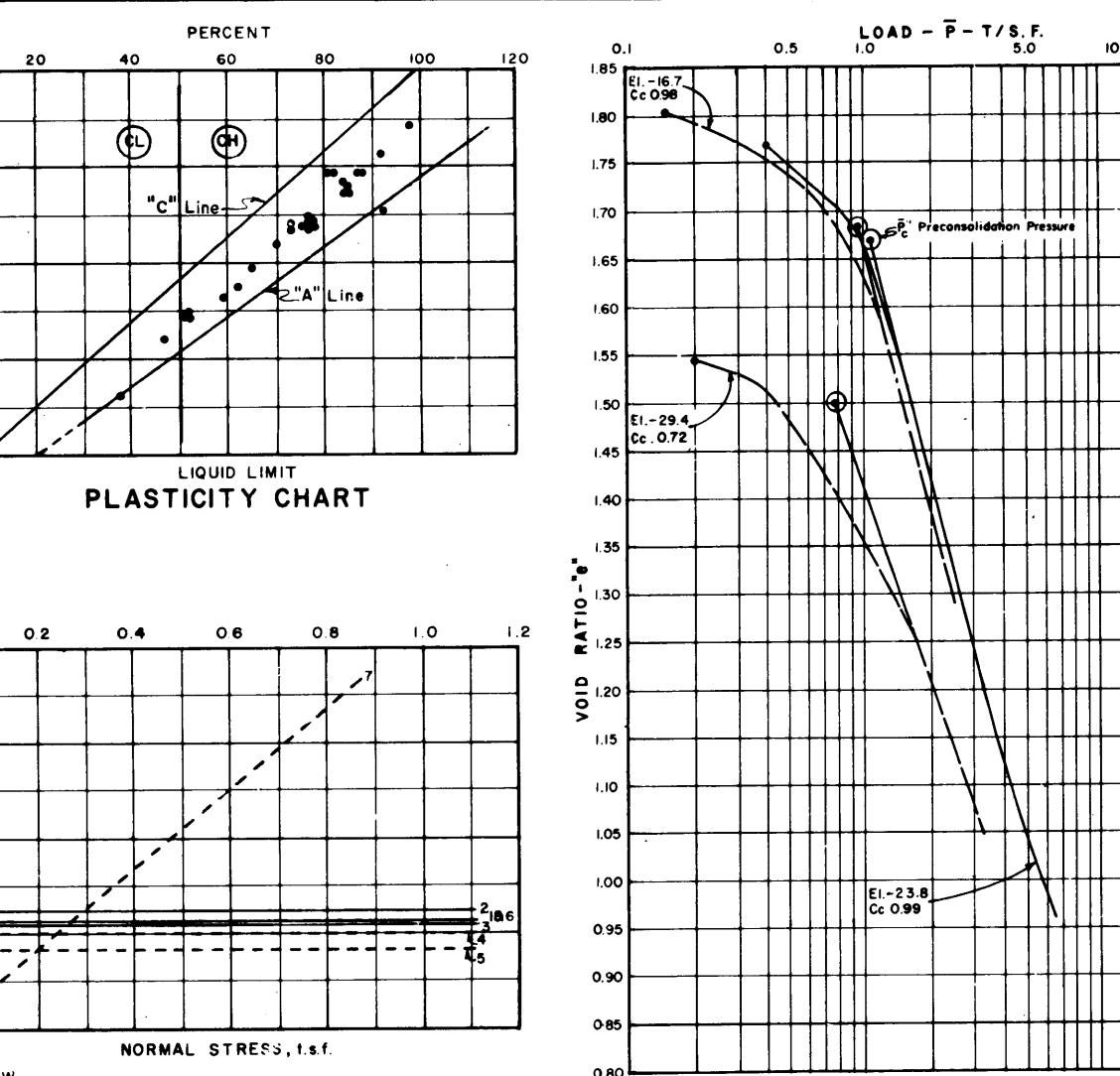
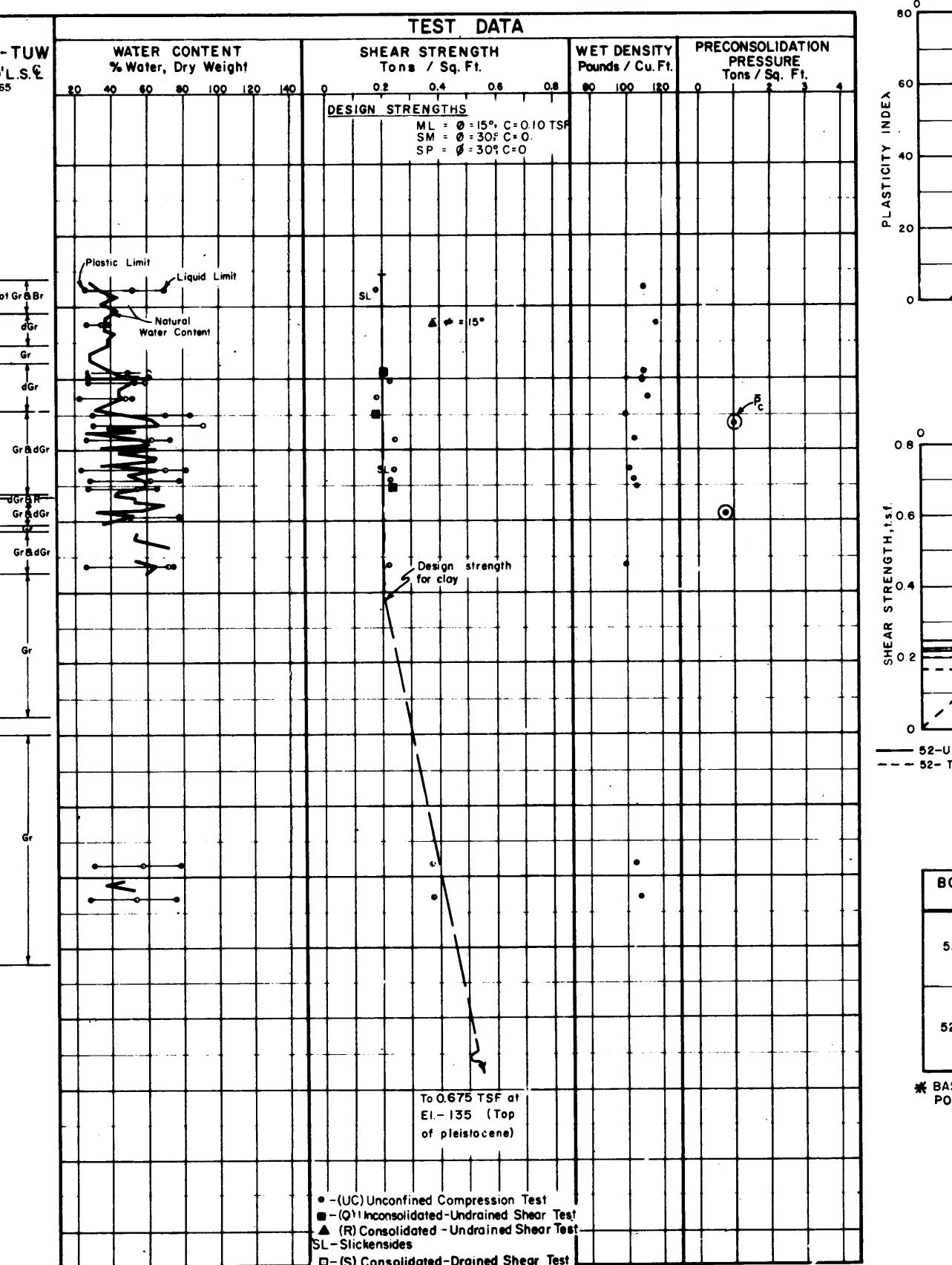
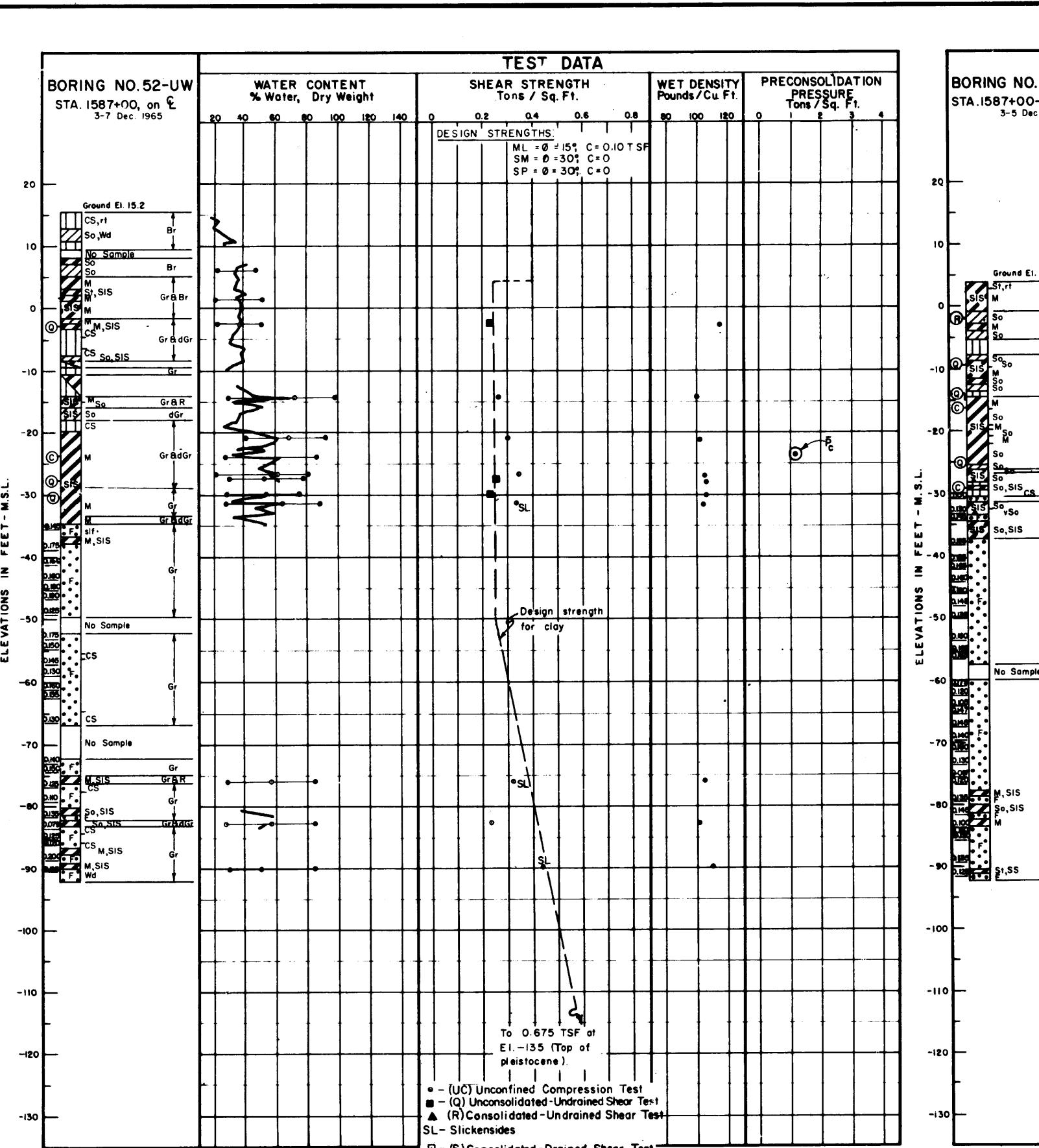




1







BORING NO.	ENVELOPE NO.	ENVELOPE EL.	TYPE	STRENGTH ϕ^* (I.S.F.)	CLASS
52-UW	1	- 2.5		0 0.23	CH
	2	- 27.8	Q	0 0.25	CH
	3	- 30.1	Q	0 0.22	CH
52-TUW	4	- 9.7		0 0.20	CH
	5	- 15.1	Q	0 0.17	CH
	6	- 25.5	Q	0 0.23	CH
	7	- 2.3	* R	40 0.00	CL

* BASED ON DEVIATOR STRESS AT MAXIMUM POSITIVE PORE PRESSURE: $\phi = 18.3^\circ, C = 0.06 \text{ TSF}$

SHEAR STRENGTH DATA

CONSOLIDATION DATA

— Boring No. 52-UW
--- Boring No. 52-TUW

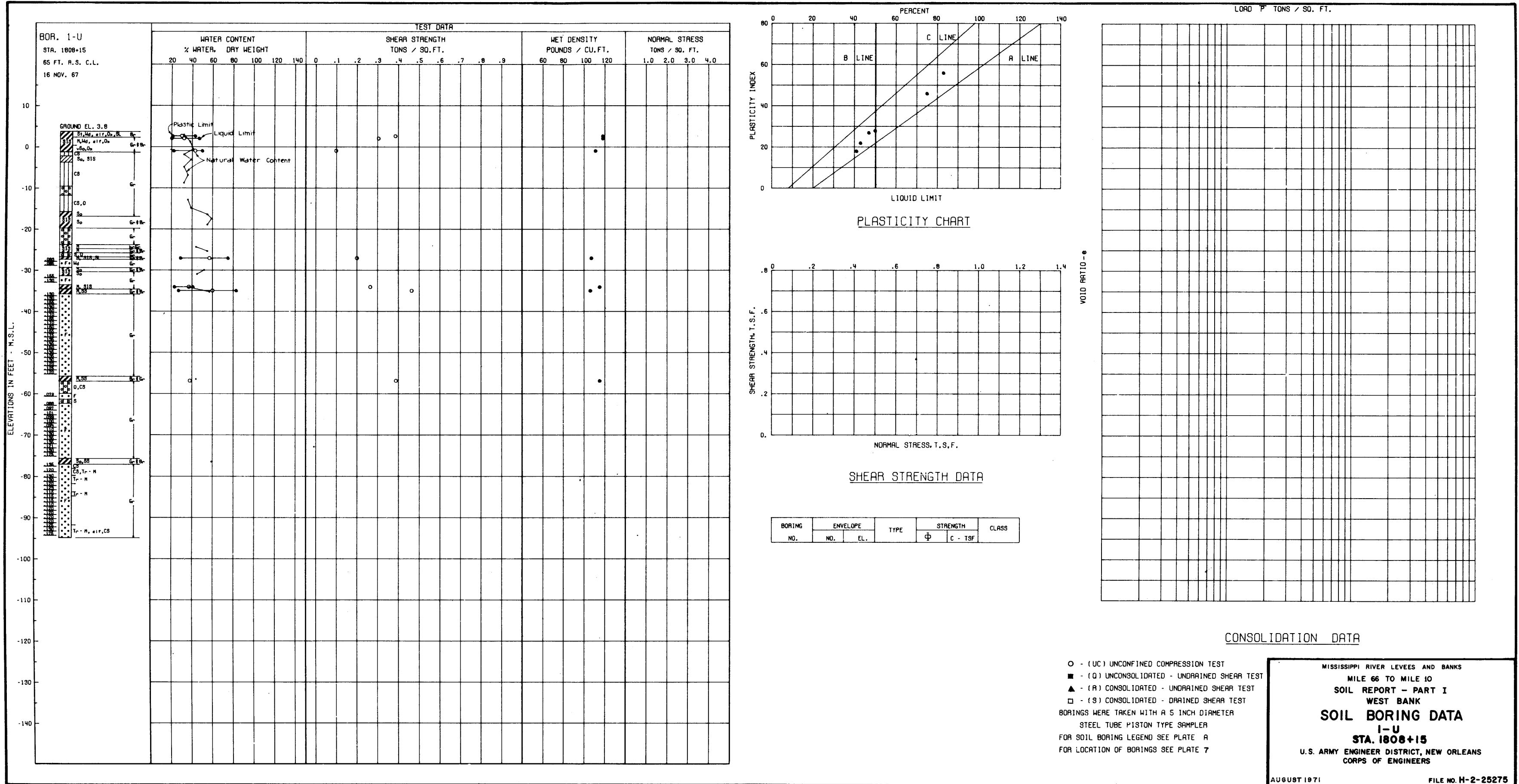
For soil boring legend see plate A
For location of borings see plate 6

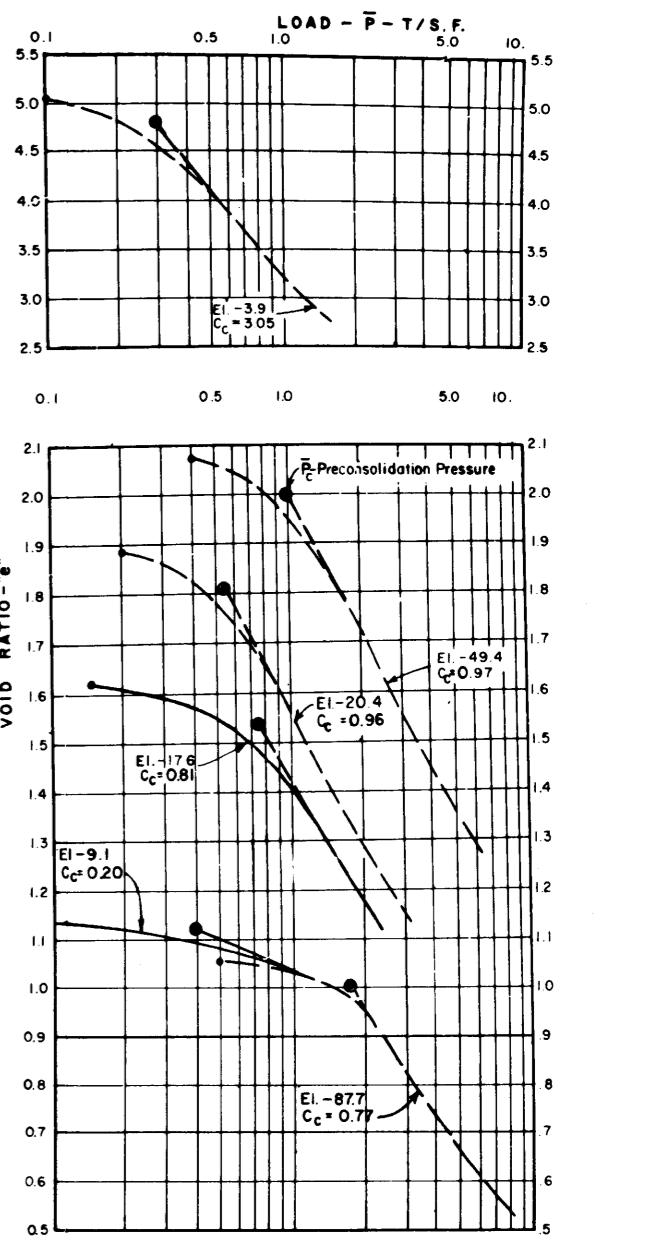
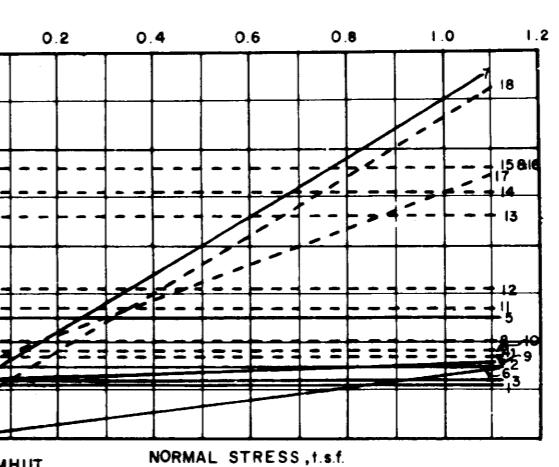
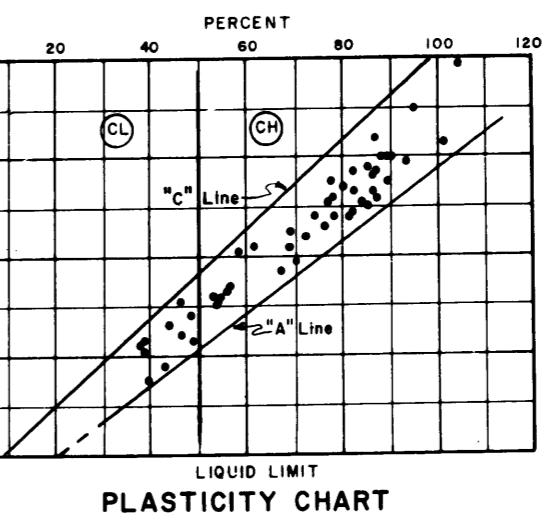
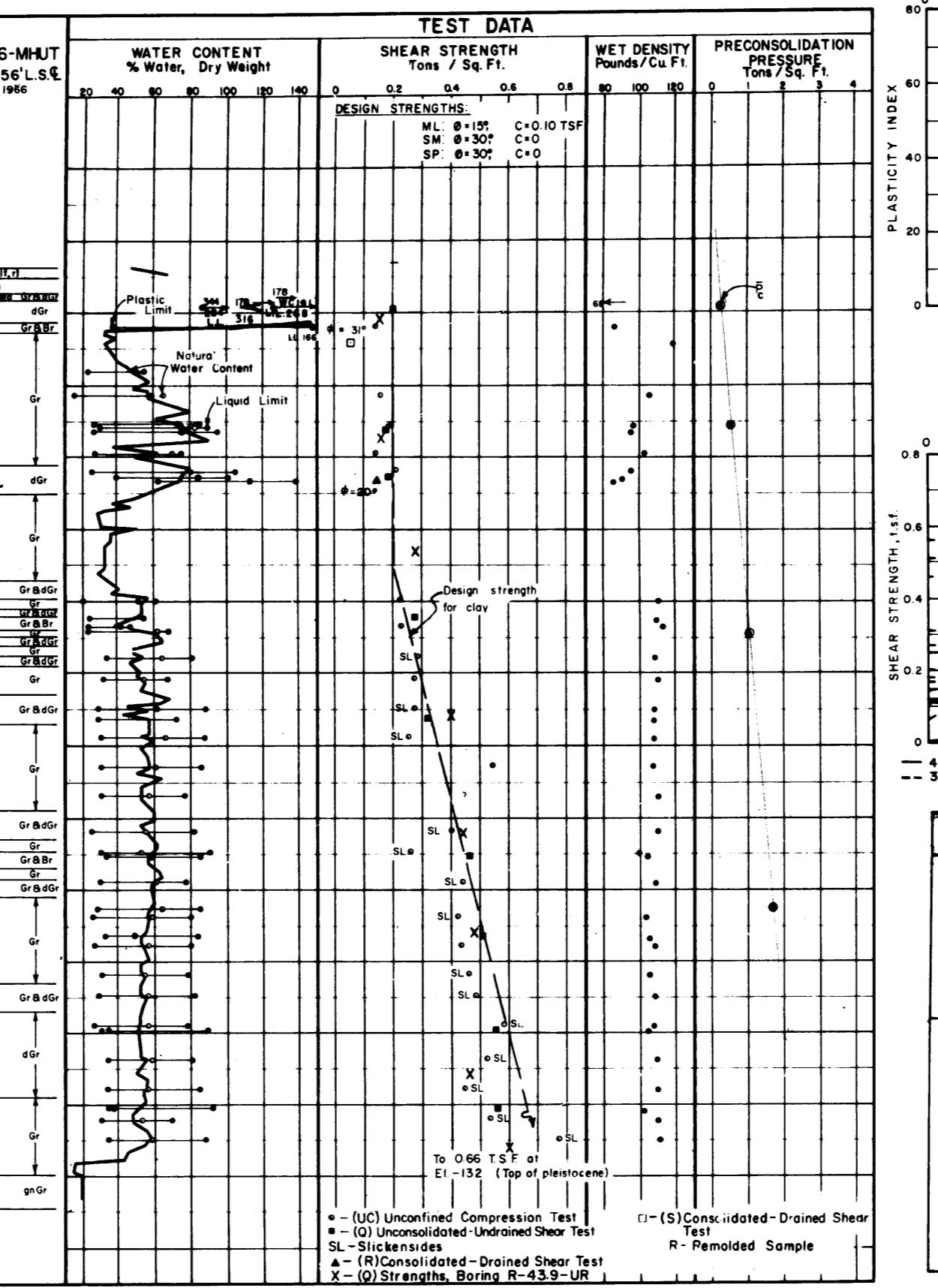
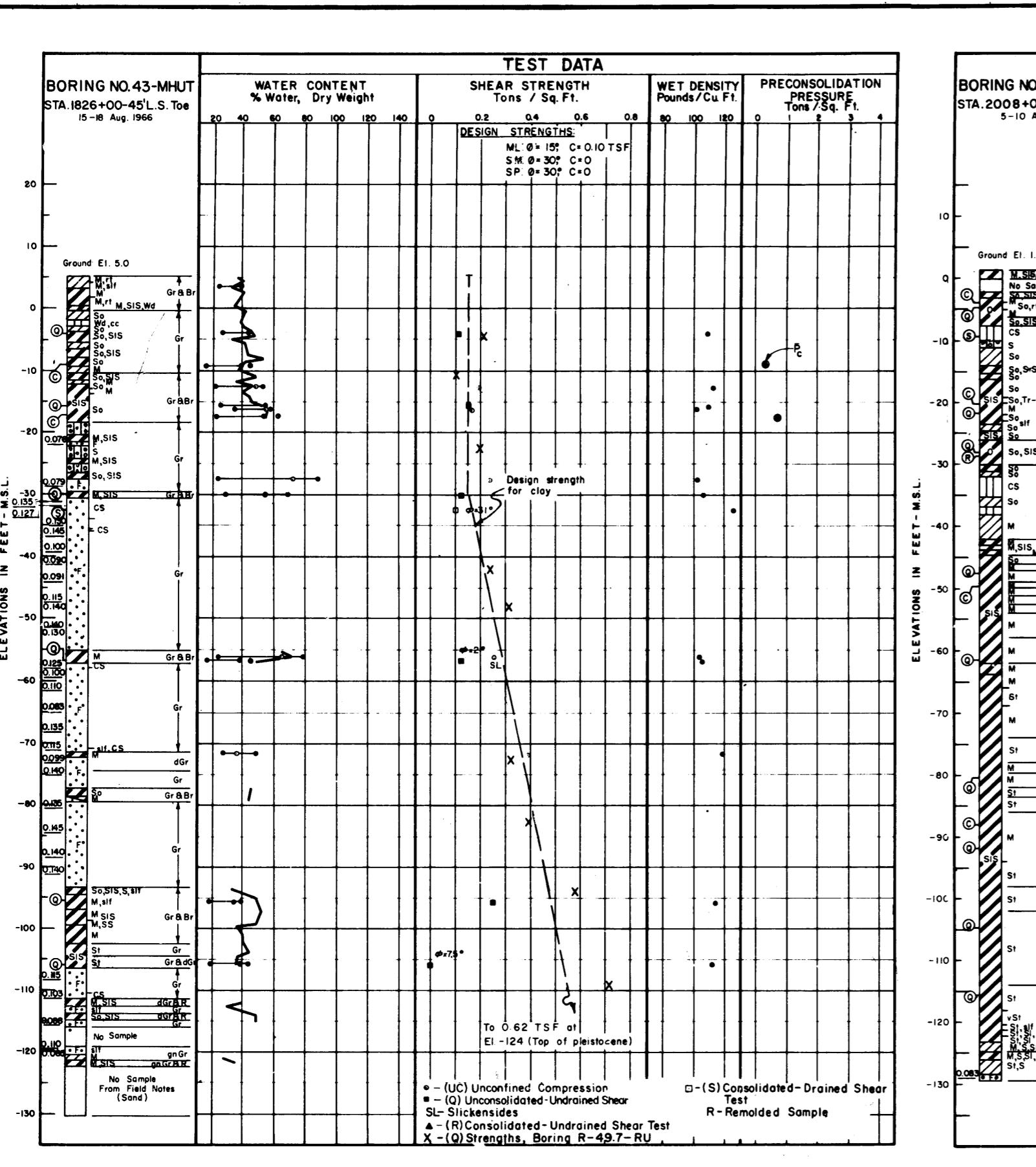
Borings were taken with a 5" diameter steel tube
piston type sampler.

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART T
WEST BANK
SOIL BORING DATA
52-UW AND 52-TUW
STA. 1587+00
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS

AUGUST 1971

FILE NO. H-Z-25275





— Boring No. 43-MHUT
 — Boring No. 36-MHUT
 For soil boring legend see plate A
 For location of borings see plates 7 & 8

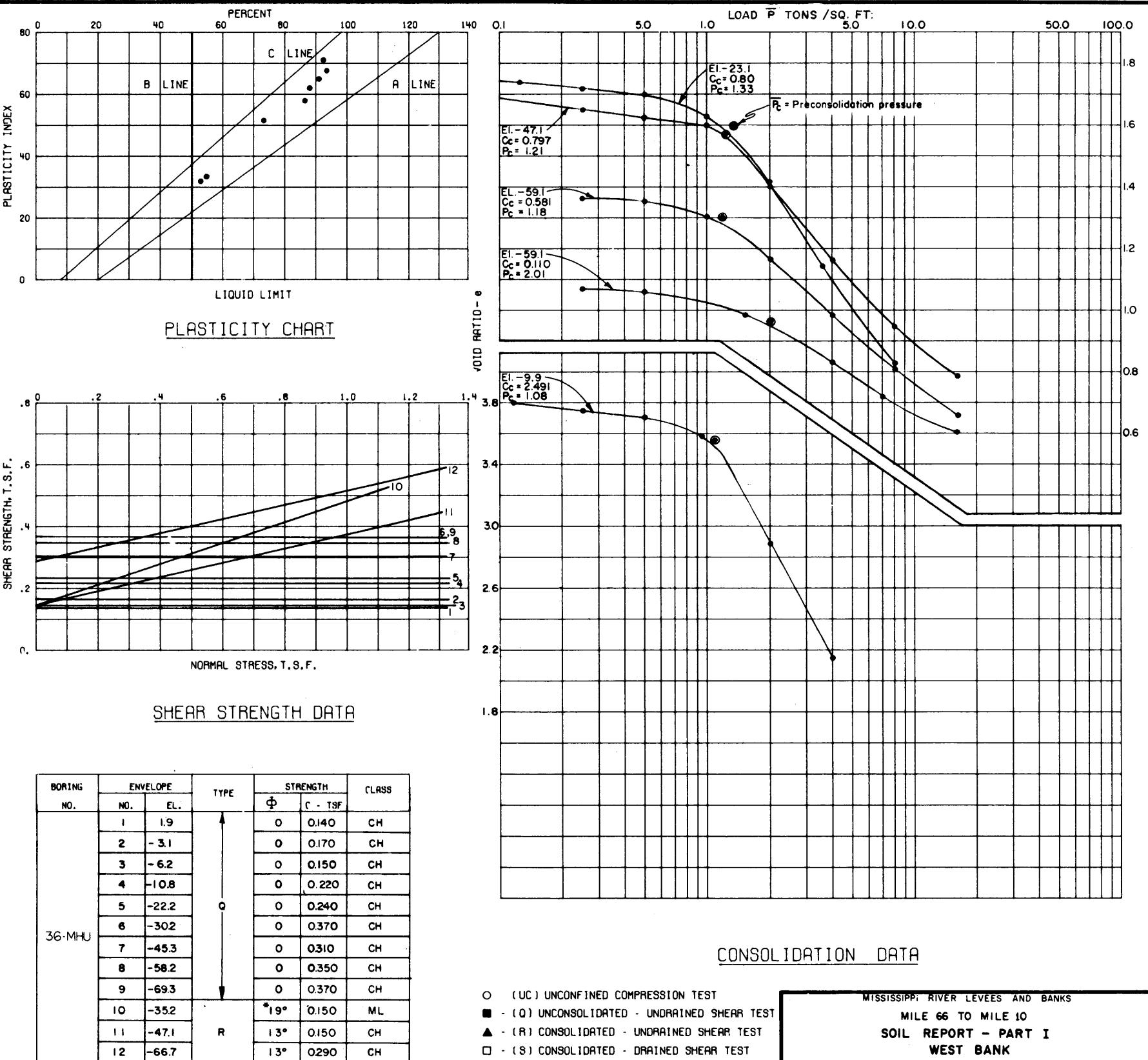
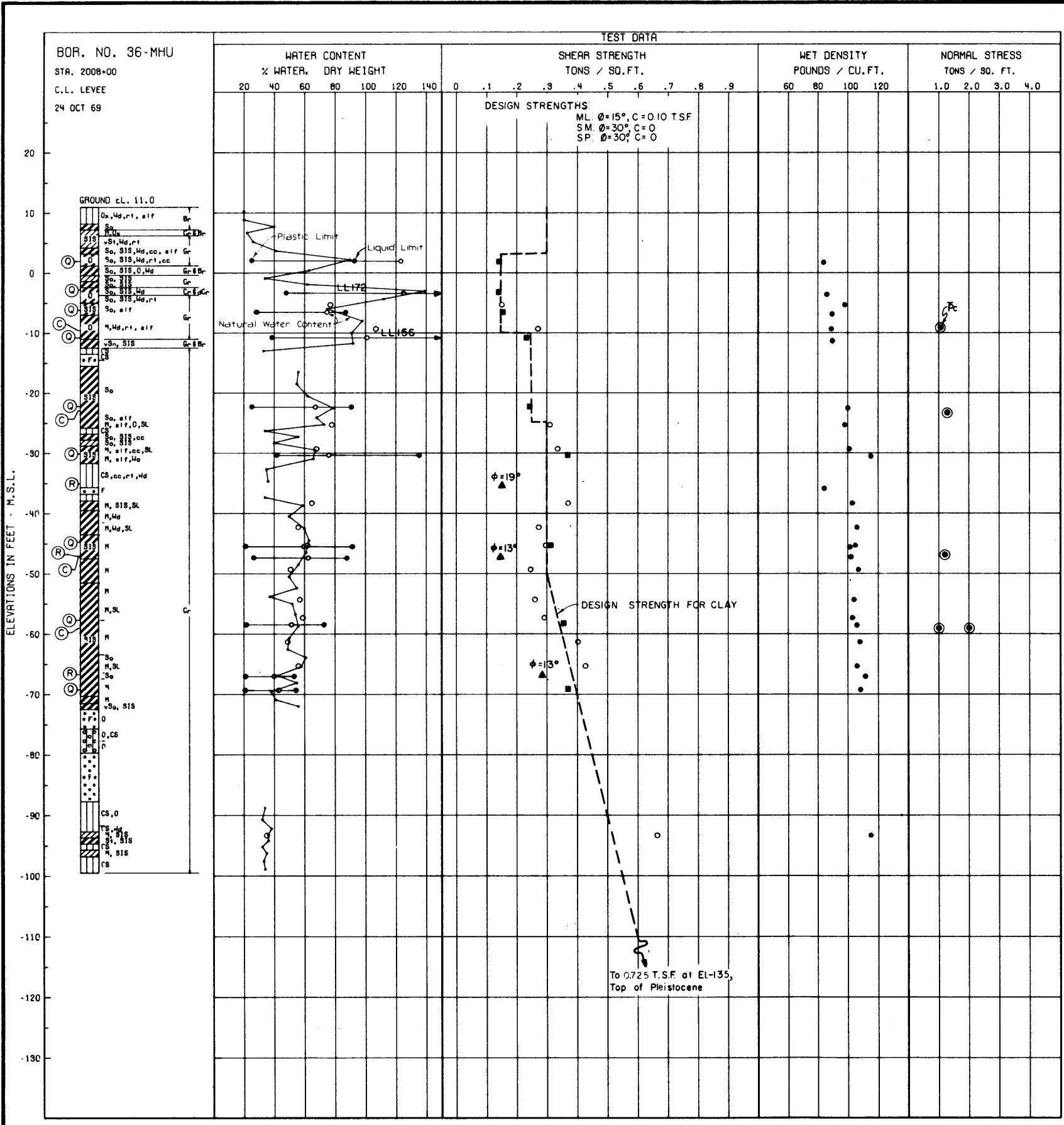
Borings were taken with a 5" diameter steel tube
 piston type sampler.

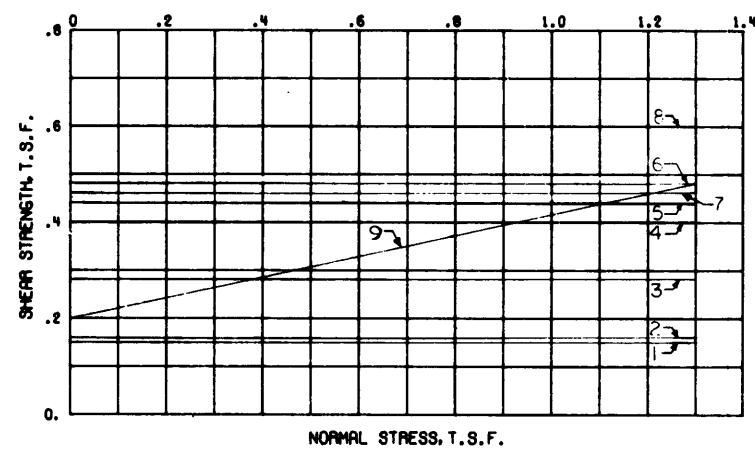
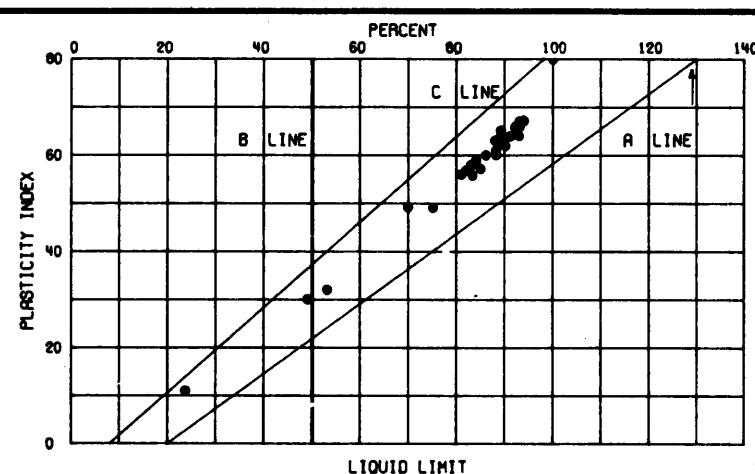
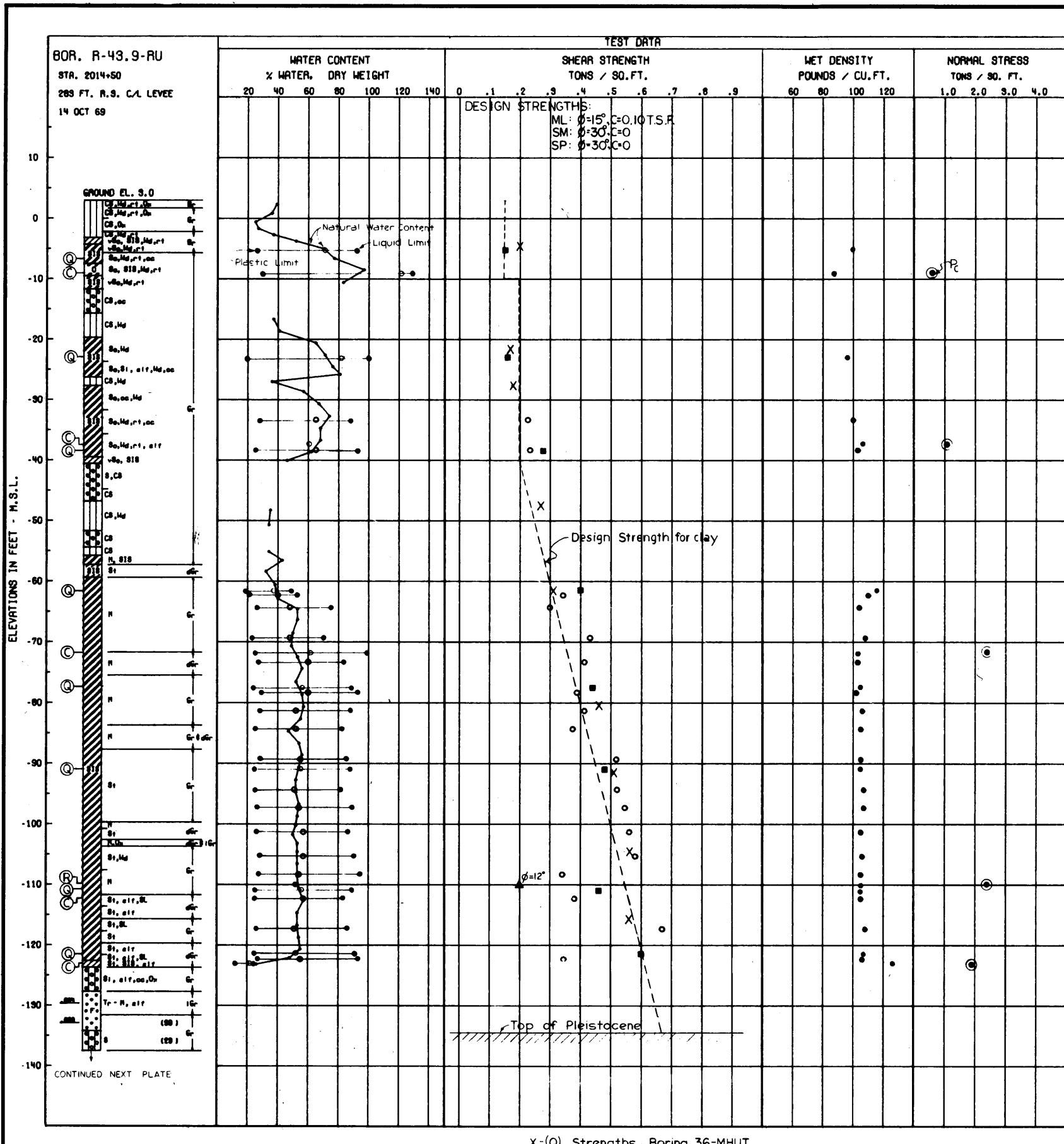
MISSISSIPPI RIVER LEVEES AND BANKS
 MILE 66 TO MILE 10
 SOIL REPORT - PART I
 WEST BANK
SOIL BORING DATA
 43-MHUT - STA.1826+00
 36-MHUT - STA.2008+00
 U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
 CORPS OF ENGINEERS

AUGUST 1971

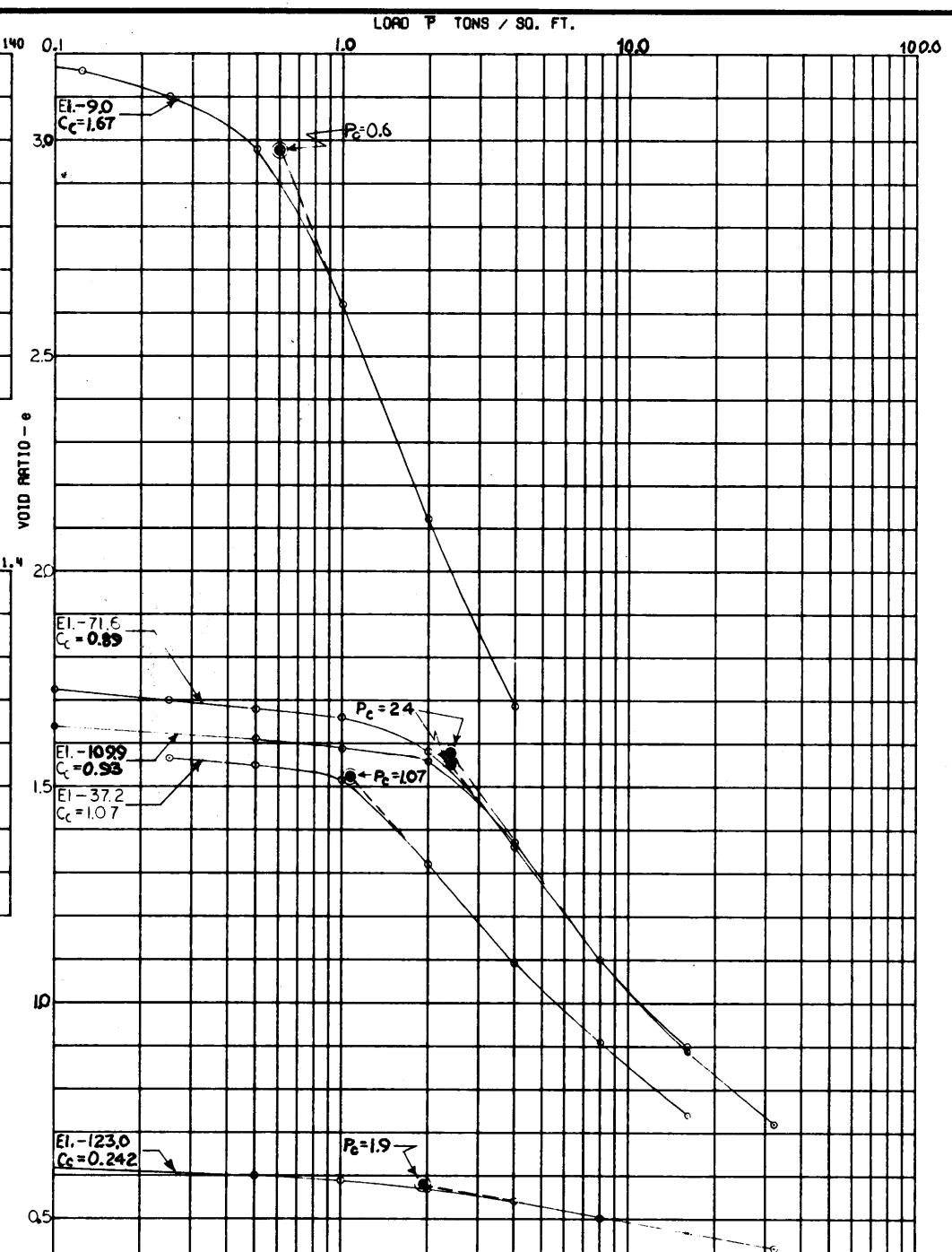
FILE NO. H-2-25275

PLATE 48



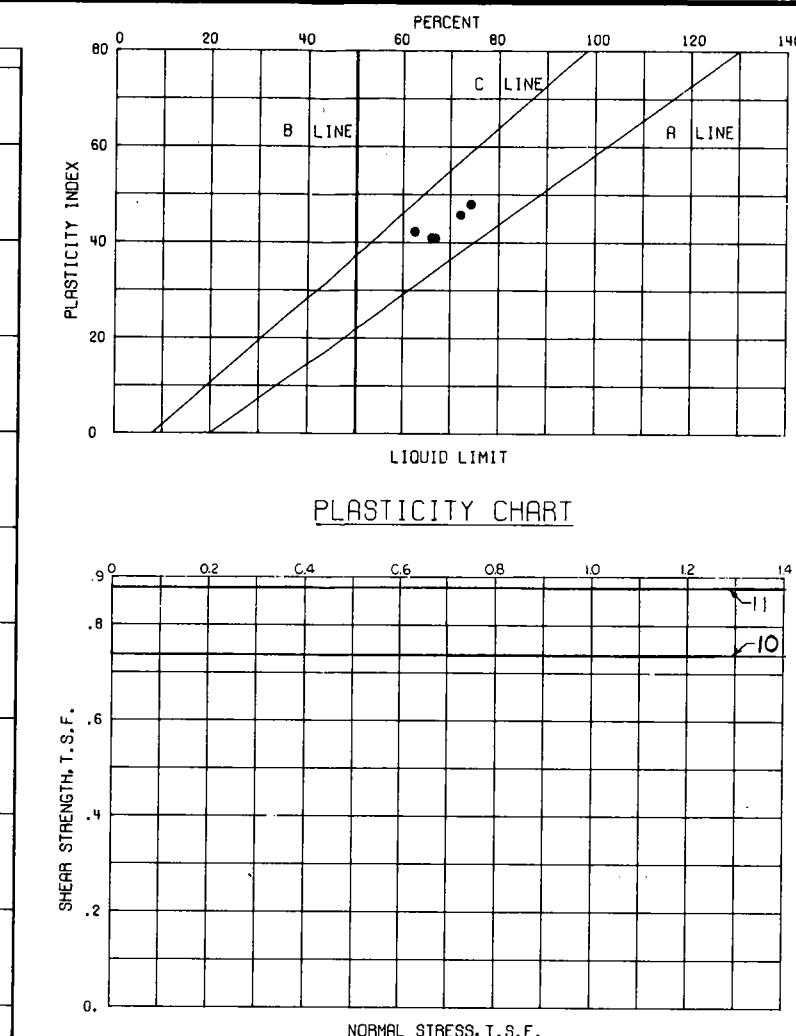
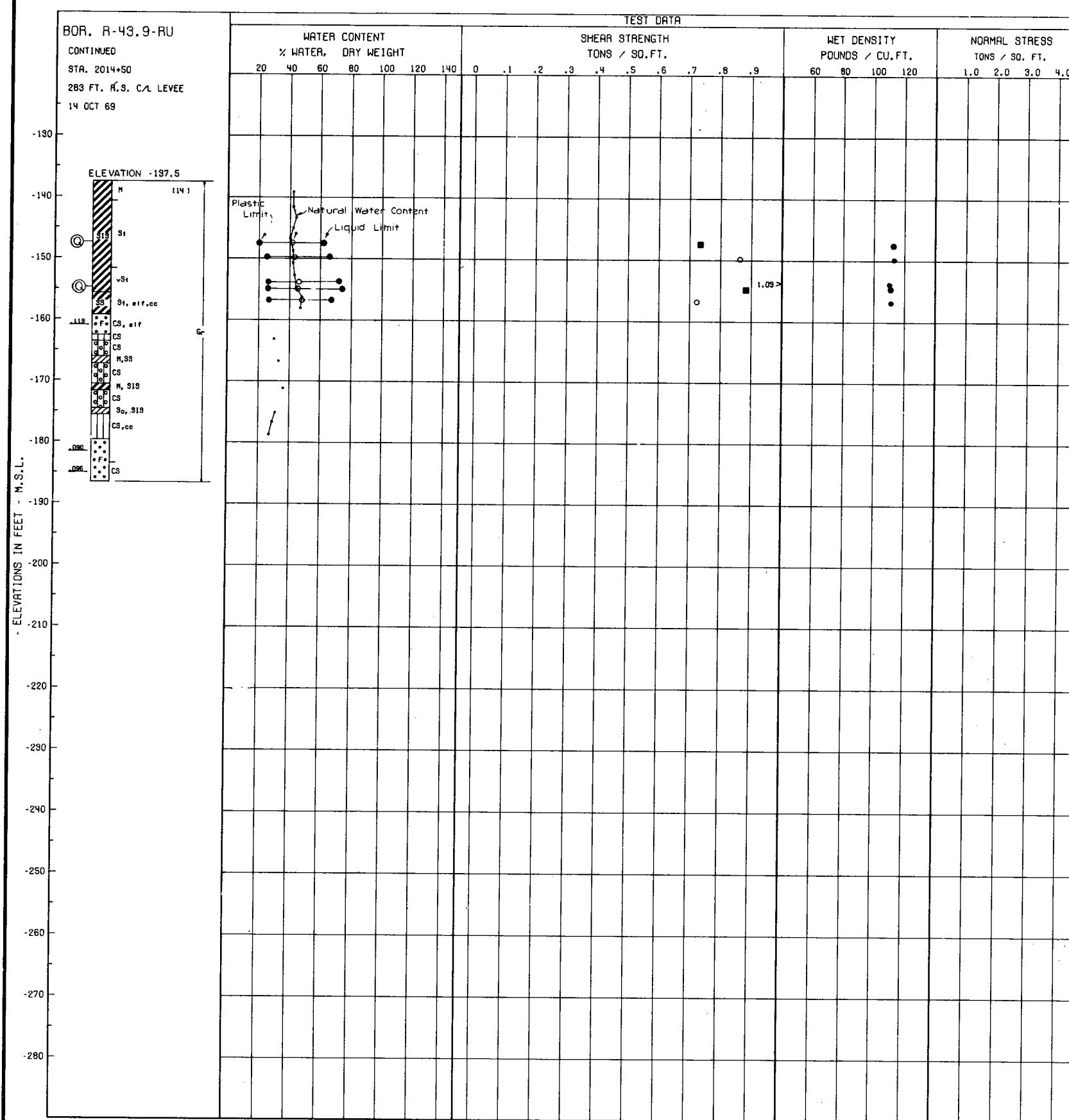


BORING NO.	ENVELOPE NO.	TYPE	STRENGTH		CLASS
			ϕ	C - T.S.F.	
R-43.9-RU 1	-64	Q	0	0.15	CH
2	-22.8	Q	0	0.16	
3	-38.1	Q	0	0.28	
4	-61.2	Q	0	0.40	
5	-77.1	Q	0	0.44	
6	-90.8	Q	0	0.48	
7	-110.8	Q	0	0.46	
8	-121.2	Q	0	0.60	
9	-109.9	R	12°	0.20	CH



○ - (UC) UNCONFINED COMPRESSION TEST
■ - (Q) UNCONSOLIDATED - UNDRAINED SHEAR TEST
△ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST
□ - (S) CONSOLIDATED - DRAINED SHEAR TEST
BORINGS WERE TAKEN WITH A 5 INCH DIAMETER
STEEL TUBE PISTON TYPE SAMPLER
FOR SOIL BORING LEGEND SEE PLATE A
FOR LOCATION OF BORINGS SEE PLATE B

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
WEST BANK
SOIL BORING DATA
R-43.9-RU
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS



SHEAR STRENGTH DATA

BORING NO.	ENVELOPE NO., EL.	TYPE	STRENGTH		CLASS
			ϕ	C - TSF	
R-43.9-RU	10 -147.1	Q	O	0.74	CH
	11 -154.8		O	0.88	CH

O - (UC) UNCONFINED COMPRESSION TEST

□ - (Q) UNCONSOLIDATED - UNDRAINED SHEAR TEST

▲ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST

□ - (S) CONSOLIDATED - DRAINED SHEAR TEST

BORINGS WERE TAKEN WITH A 5 INCH DIAMETER

STEEL TUBE PISTON-TYPE SAMPLER

FOR SOIL BORING LEGEND SEE PLATE A

FOR LOCATION OF BORINGS SEE PLATE B

MISSISSIPPI RIVER LEVEES AND BANKS

MILE 66 TO MILE 10

SOIL REPORT - PART I

WEST BANK

SOIL BORING DATA

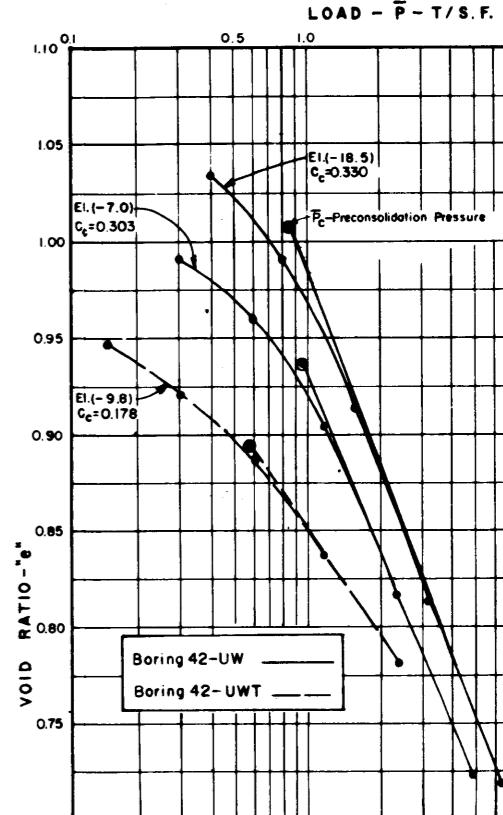
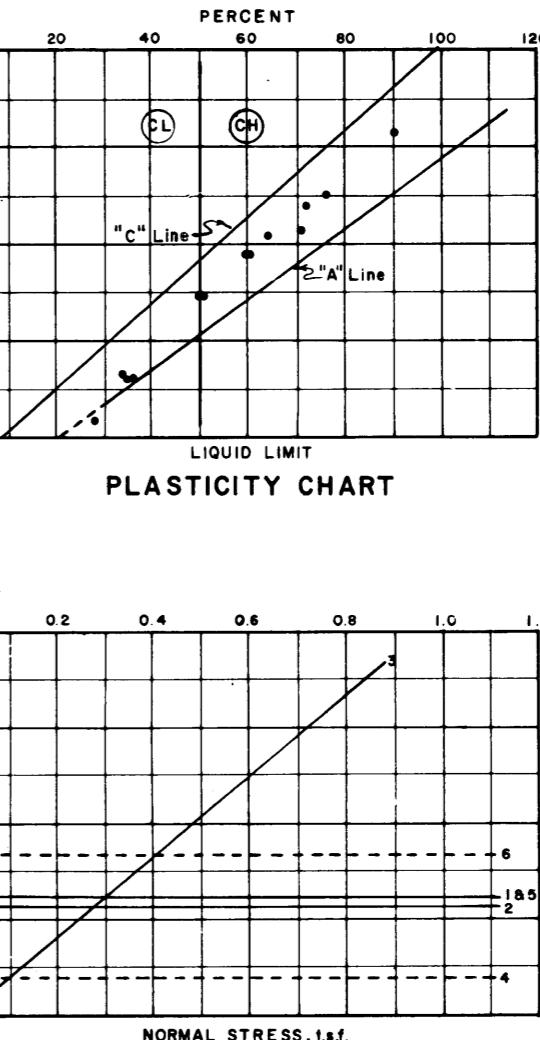
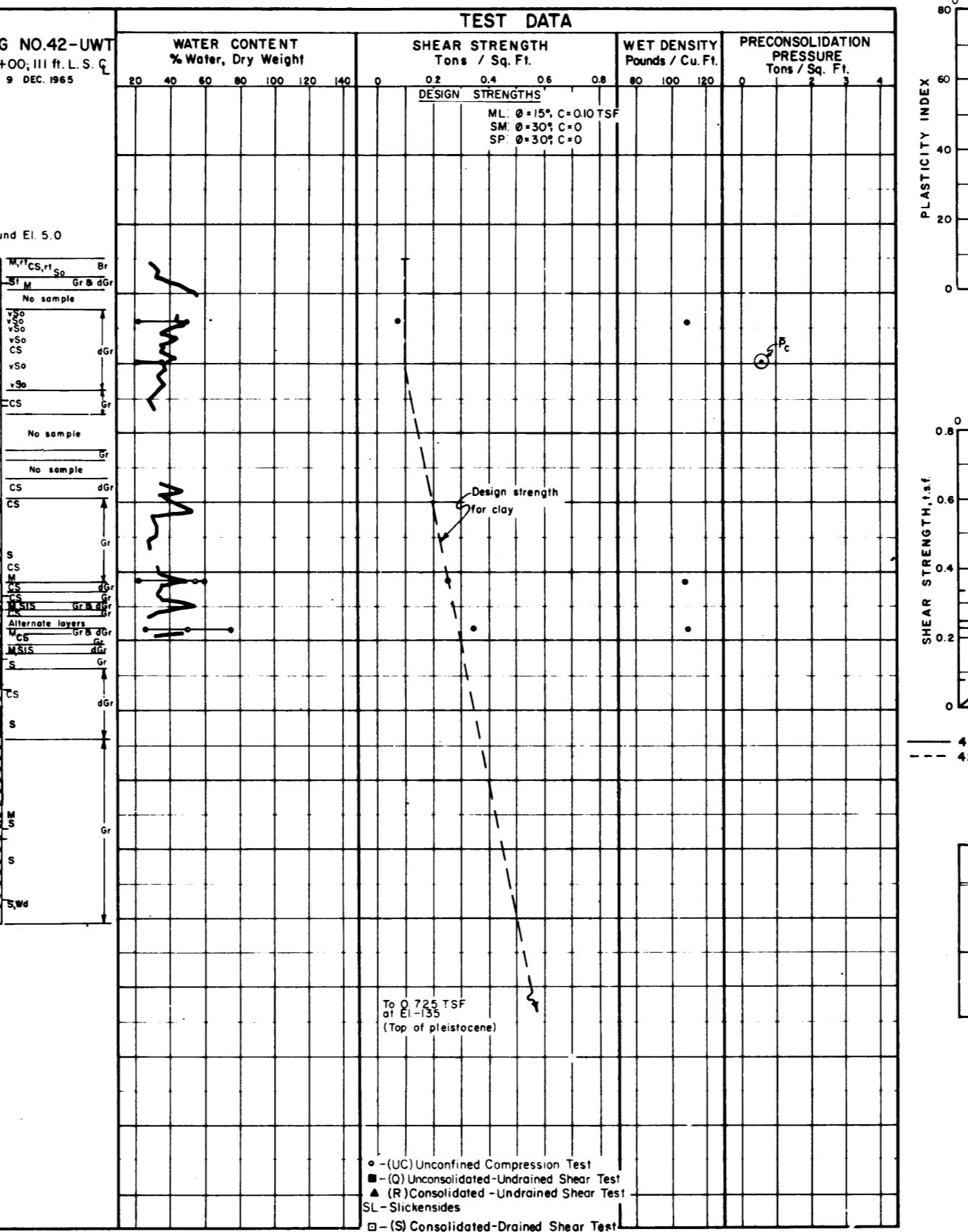
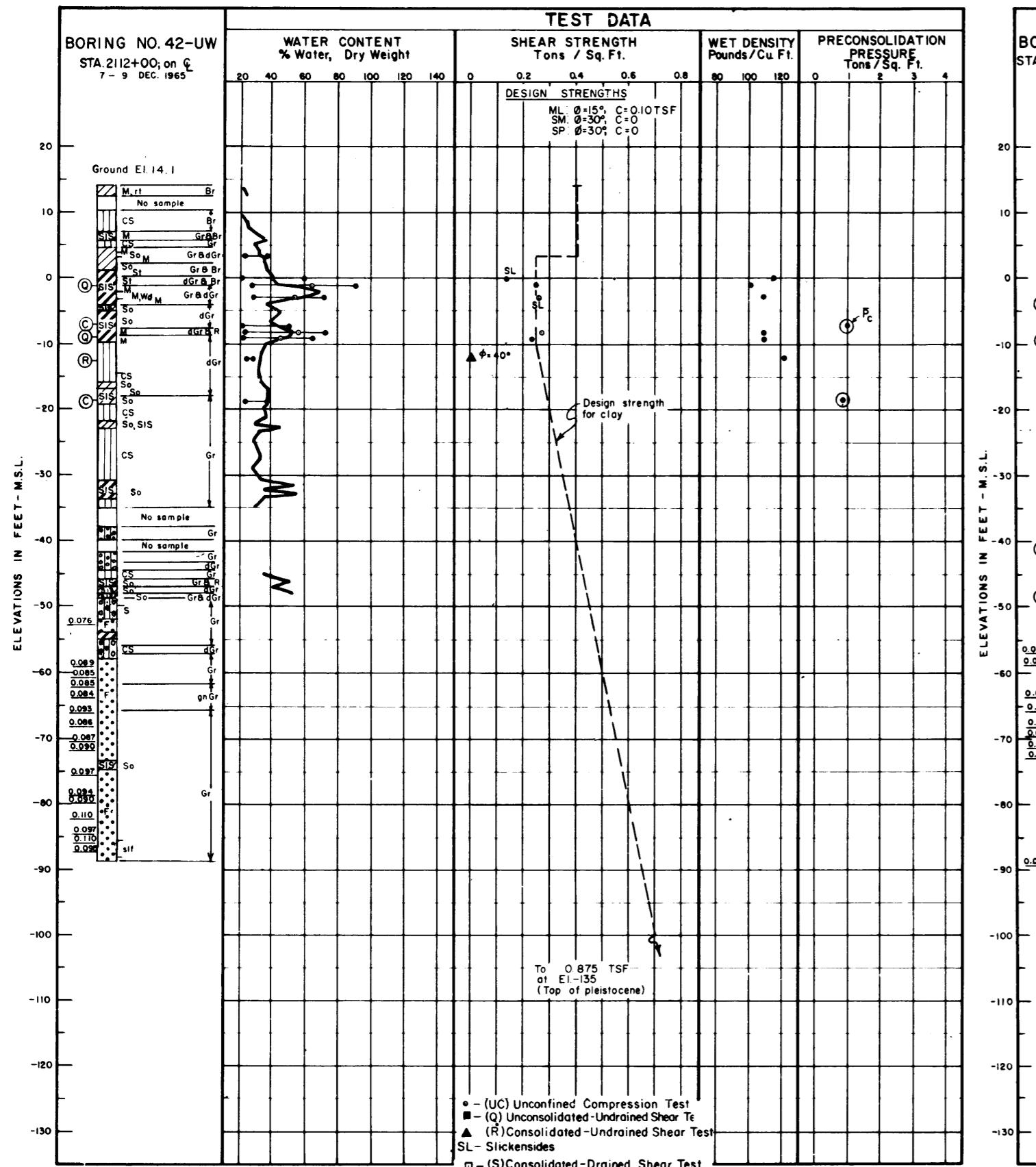
R-43.9-RU (CONT'D.)

U. S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS

AUGUST 1971

FILE NO. H-2-25275

PLATE 51



— Boring No. 42-UW
— Boring No. 42-UWT
For soil boring legend see pl.
For location of borings see pl.

Borings were taken with a 5" diameter steel piston type sampler.

PILOT STRESS AT MAXIMUM POSITIVE
GURE: $\theta = 18.3^\circ$, $C = 0.18$ TSF
AIR STRENGTH DATA

ING O.	ENVELOPE		TYPE	STRENGTH		CLASS
	NO.	EL.		φ°	C (1.s.f.)	
UW	1	- 0.9	Q	0	0.25	CH
	2	- 8.9		0	0.23	CH
	3	- 11.9	R*	40	0.00	ML
UWT	4	- 4.0		0	0.08	CH
	5	- 41.3	Q	0	0.25	CH
	6	- 48.2		0	0.34	CH

BASED ON DEVIATOR STRESS AT MAXIMUM POSITIVE
ORE PRESSURE: $\phi = 18.3^\circ$, $C = 0.18 \text{ TSF}$

EAR STRENGTH DATA

MISSISSIPPI RIVER LAWS AND

MISSISSIPPI RIVER LAWS AND

MISSISSIPPI RIVER LEVEES AND

MILE 66 TO MILE 10

SOIL REPORT - PART

STATE REPORT - WEST BANK

WEST BANK

SOIL BORING D

STATE BORROWING

43-11W AND 43-11V

42-UW AND 42-UV
STA 2112-1-02

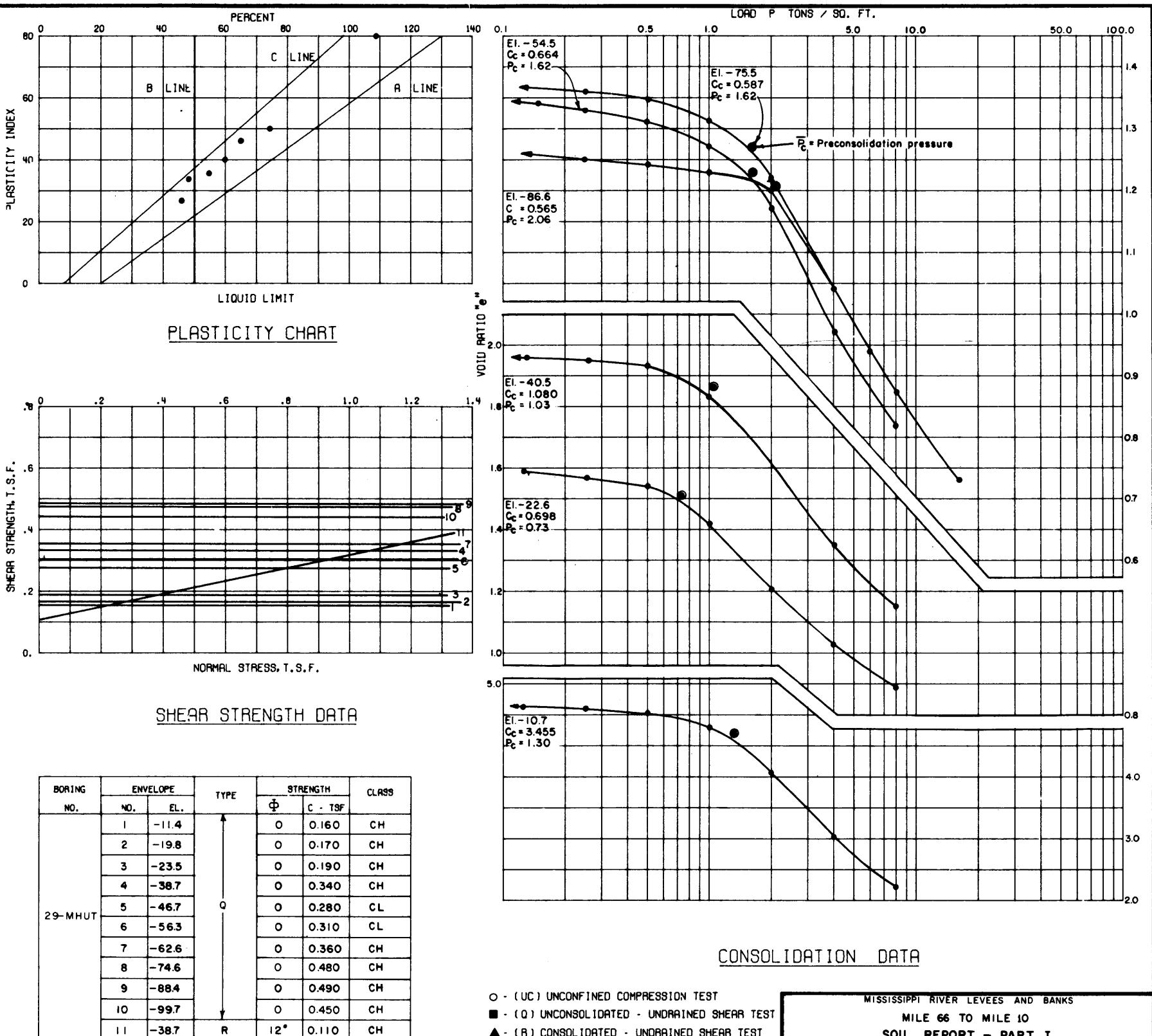
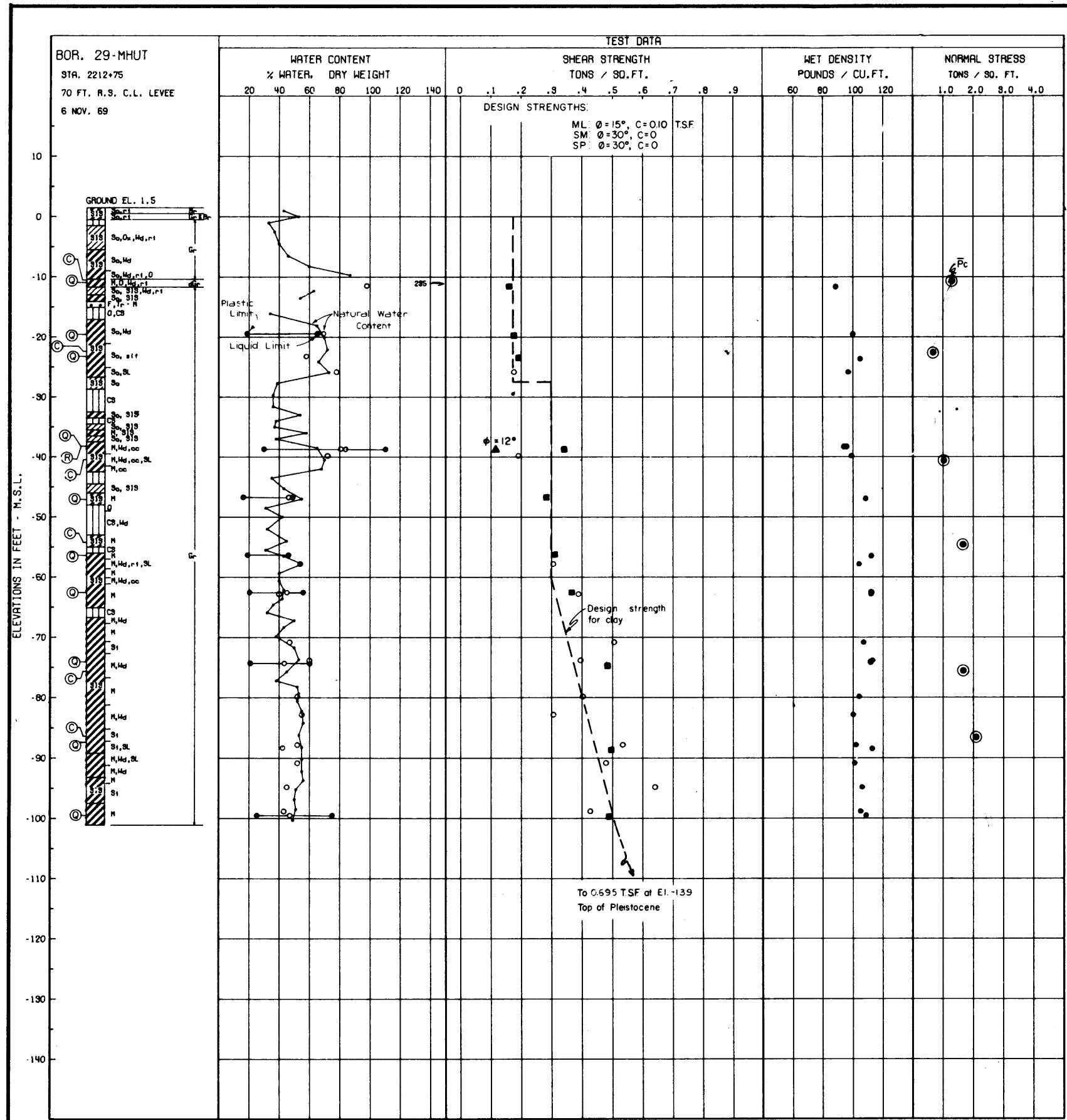
U.S. AIR MAIL ENCLUSED DISTRICT, NY

U.S. ARMY ENGINEER DISTRICT, NE

CORPS OF ENGINEERS

JULY 1971

AUGUST 1971



- - (UC) UNCONFINED COMPRESSION TEST
- - (Q) UNCONSOLIDATED - UNDRAINED SHEAR TEST
- ▲ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST
- - (S) CONSOLIDATED - DRAINED SHEAR TEST

RINGS WERE TAKEN WITH A 5 INCH DIAMETER
STEEL TUBE PISTON TYPE SAMPLER
R SOIL BORING LEGEND SEE PLATE A
R LOCATION OF BORINGS SEE PLATE 9

CONSOLIDATION DATA

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MISIPPI RIVER LEVEES AND BANKS

TO MILE 10

SOIL REPORT - PART I

WEST BANK

BORING DATA

REFERENCES

29-MHUT

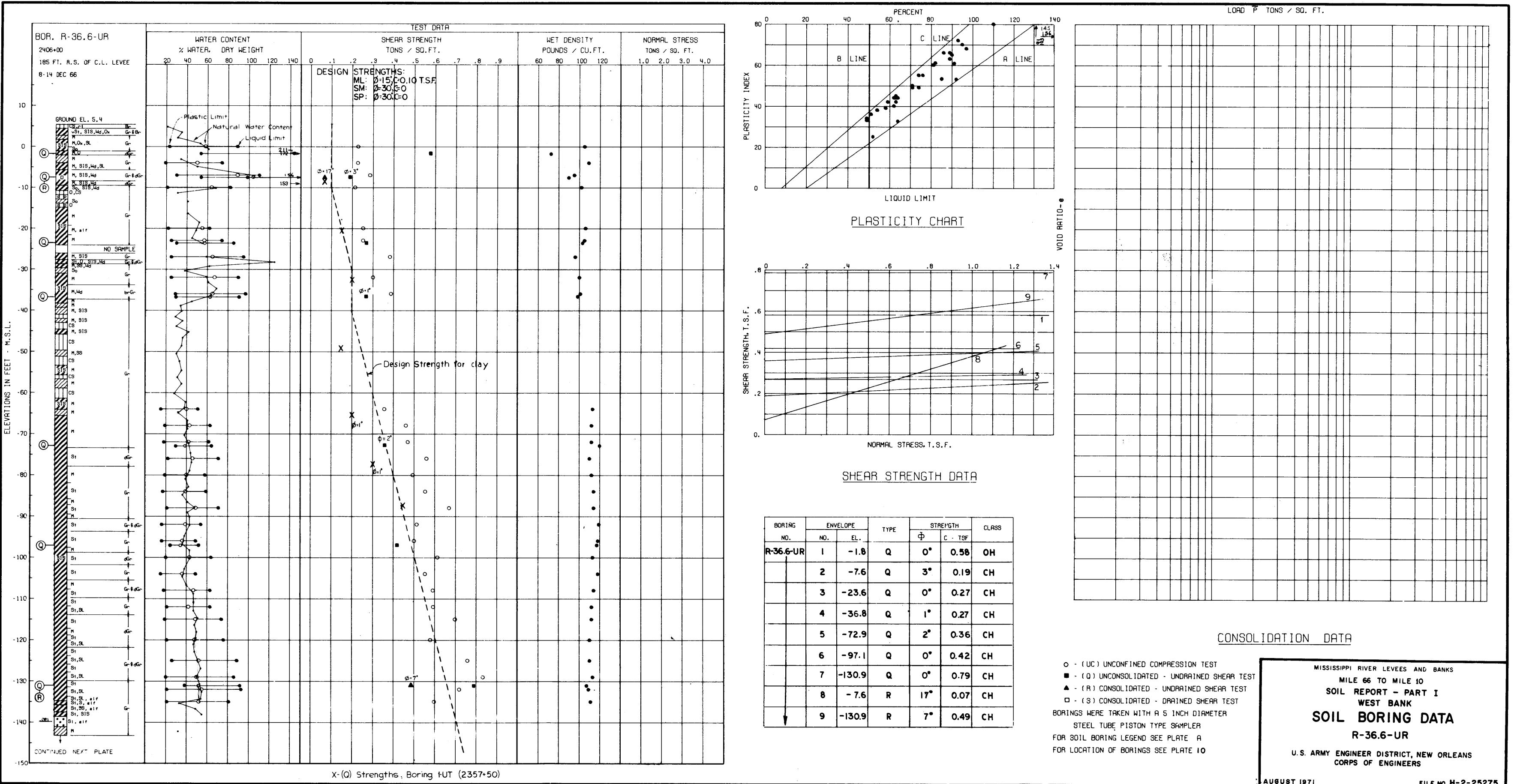
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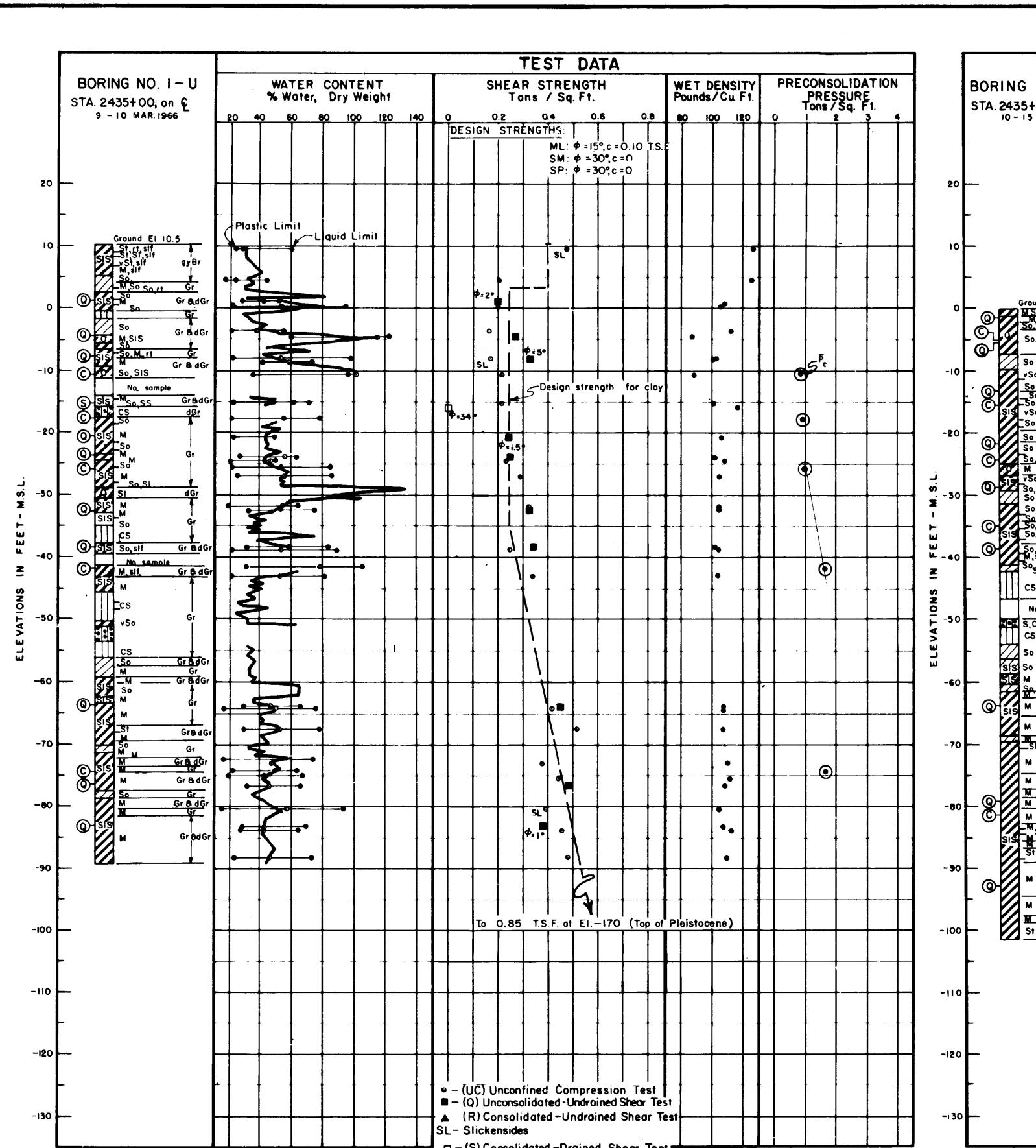
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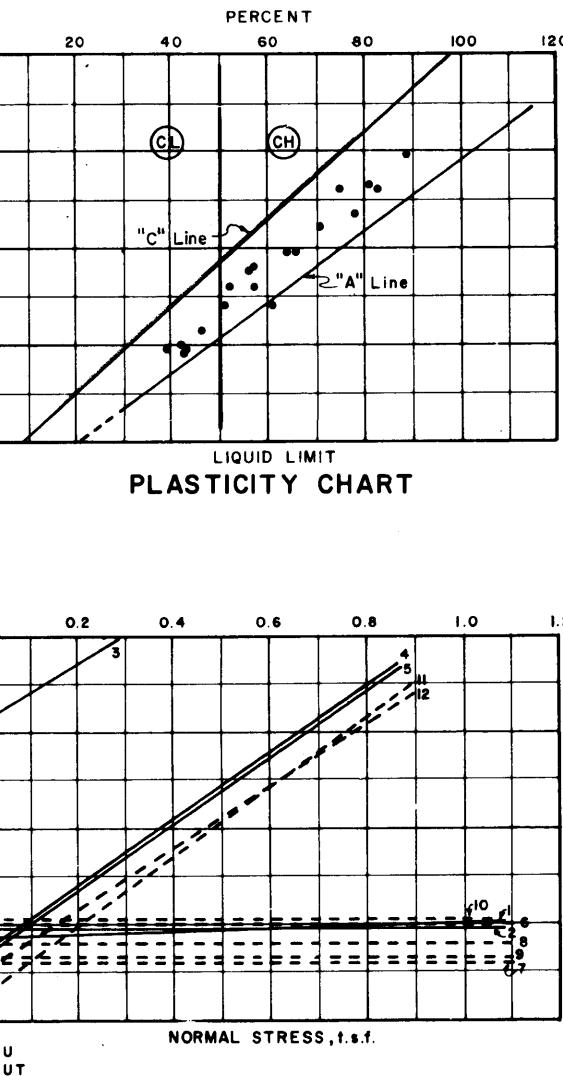
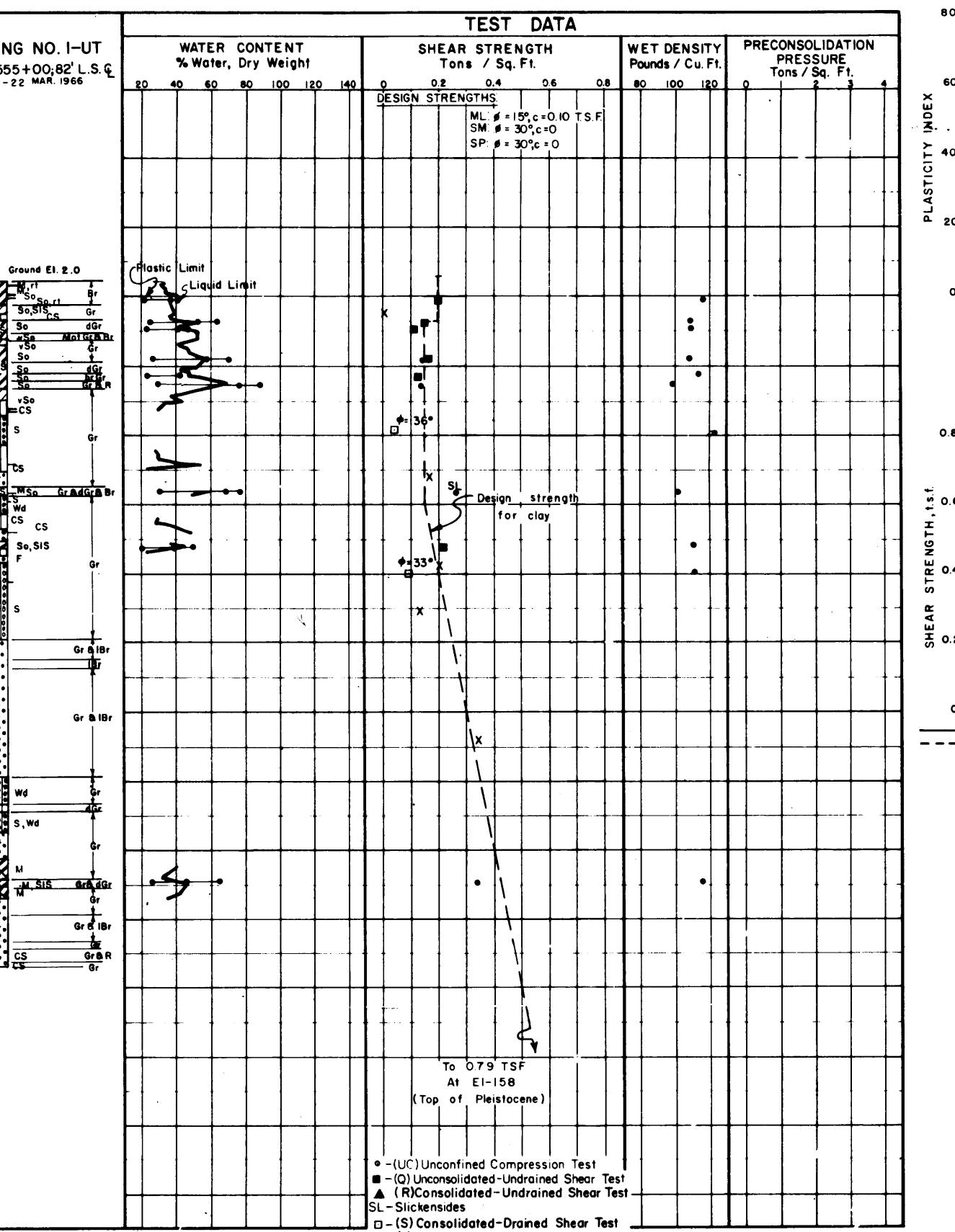
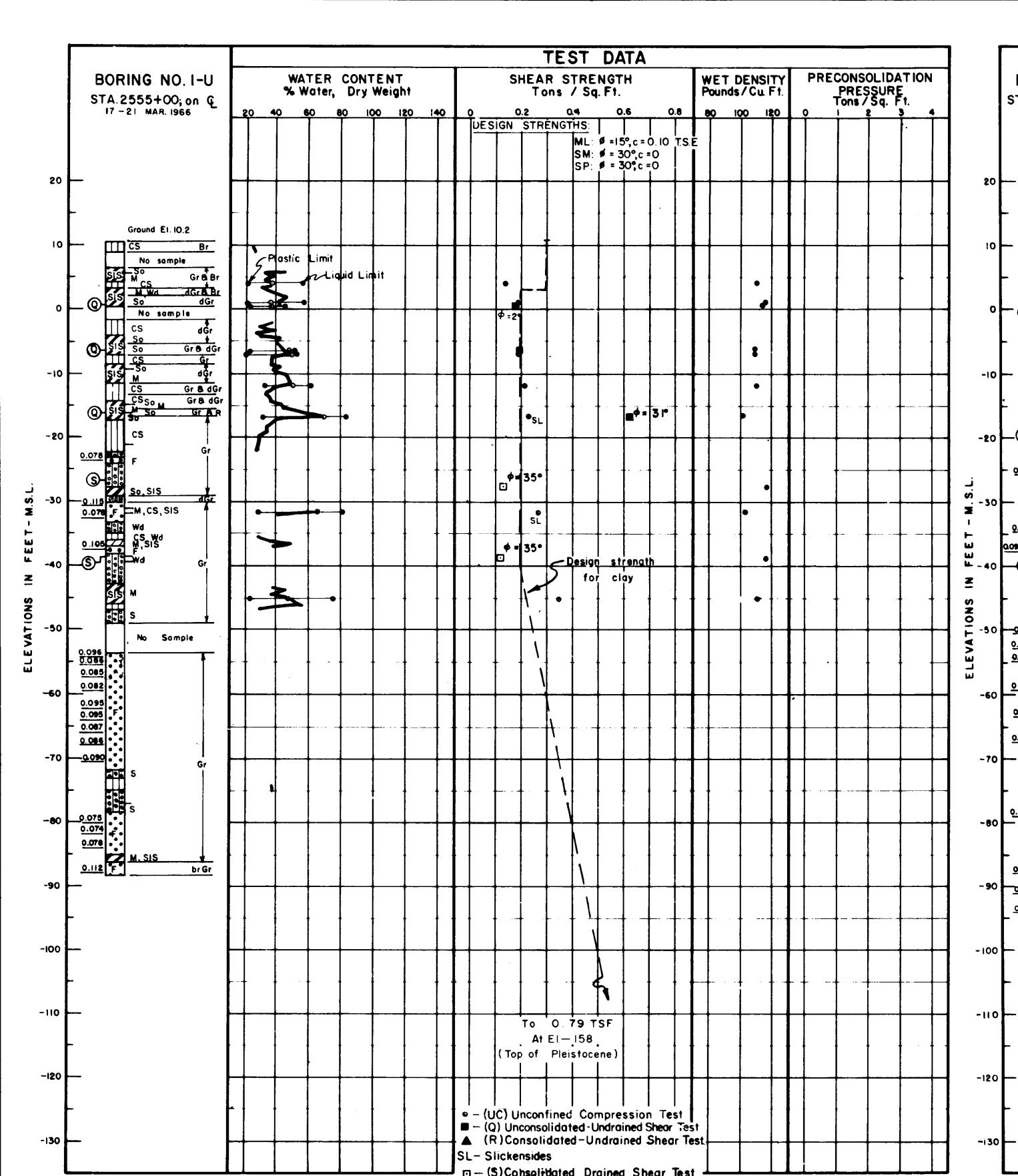
**U.S. ENGINEER STATION, NEW ORLEANS
CORPS OF ENGINEERS**

FILE NO. H-2-3

PLATE







BORING NO.	ENVELOPE		TYPE	STRENGTH		CLASS
	NO.	EL.		ϕ°	c (t.s.f.)	
I-U	1	+ 0.7	Q	2	0.17	CL
	2	- 6.2		0	0.19	CH
	3	- 16.0		31	0.62	ML
	4	- 27.2	S	35	0.14	SM
	5	- 59.0		35	0.13	SM
I-UT	6	- 0.5	Q	0	0.20	CL
	7	- 4.7		0	0.12	CL
	8	- 8.8		0	0.16	CH
	9	- 11.4		0	0.13	CL
	10	- 36.0		0	0.21	CL
	11	- 19.4	S	36	0.05	SP-SM
	12	- 40.2		33	0.10	SM

EAR STRENGTH DATA

CONSOLIDATION DATA

Borings were taken with a 5" diameter steel piston type sampler.

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MISSISSIPPI RIVER LEVEES AND

MILE 66 TO MILE 10

SOIL REPORT - PART

SOIL REPORT - PART

WEST BANK

www.BERLING.de

SOIL BORING DRILLING

I-II AND I-III

I-8 AND I-91
SEA 5555-1-62

STA. 2555 +00

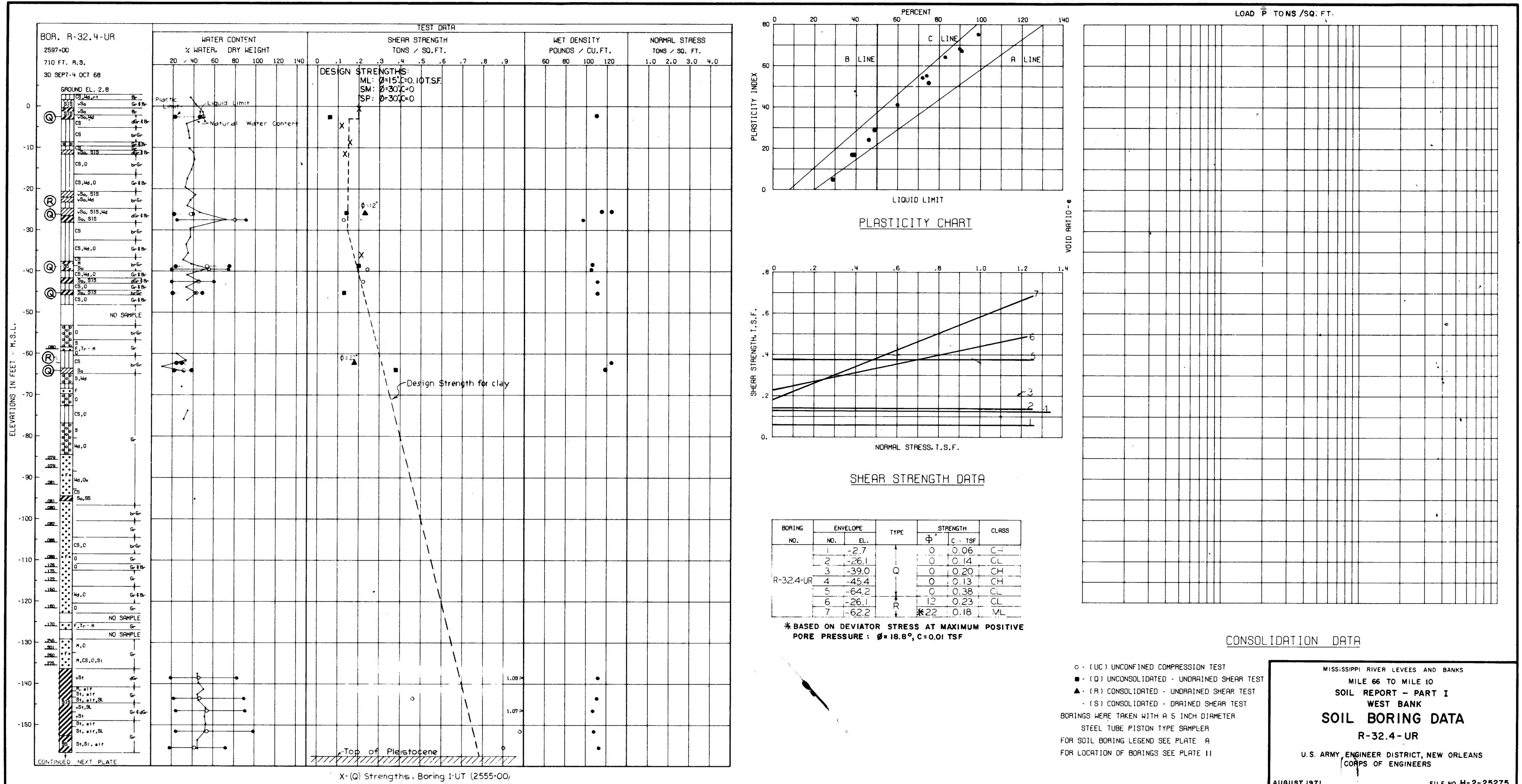
U.S. ARMY ENGINEER DISTRICT, NE

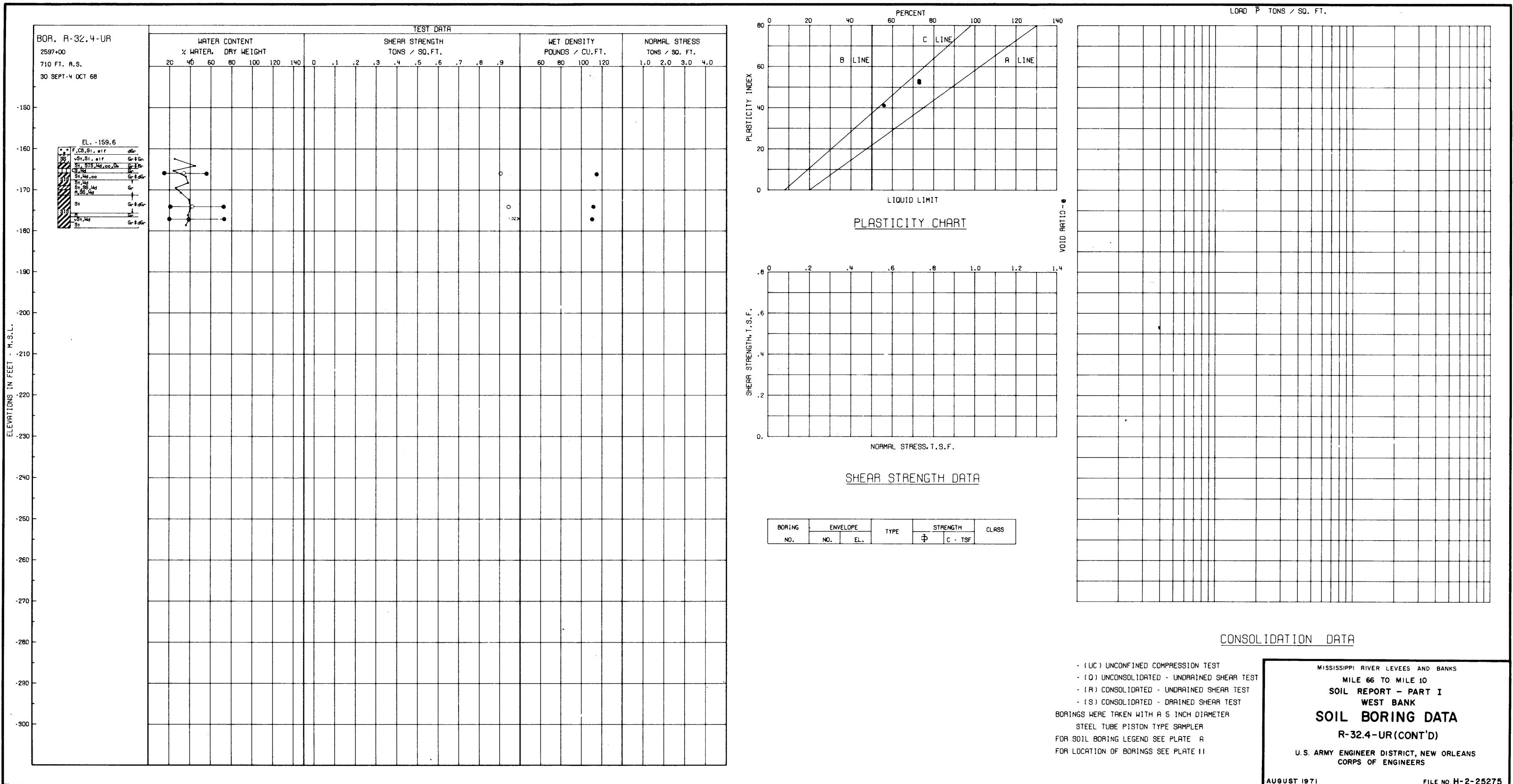
U.S. ARMY ENGINEER DIVISION, THE
CORPS OF ENGINEERS

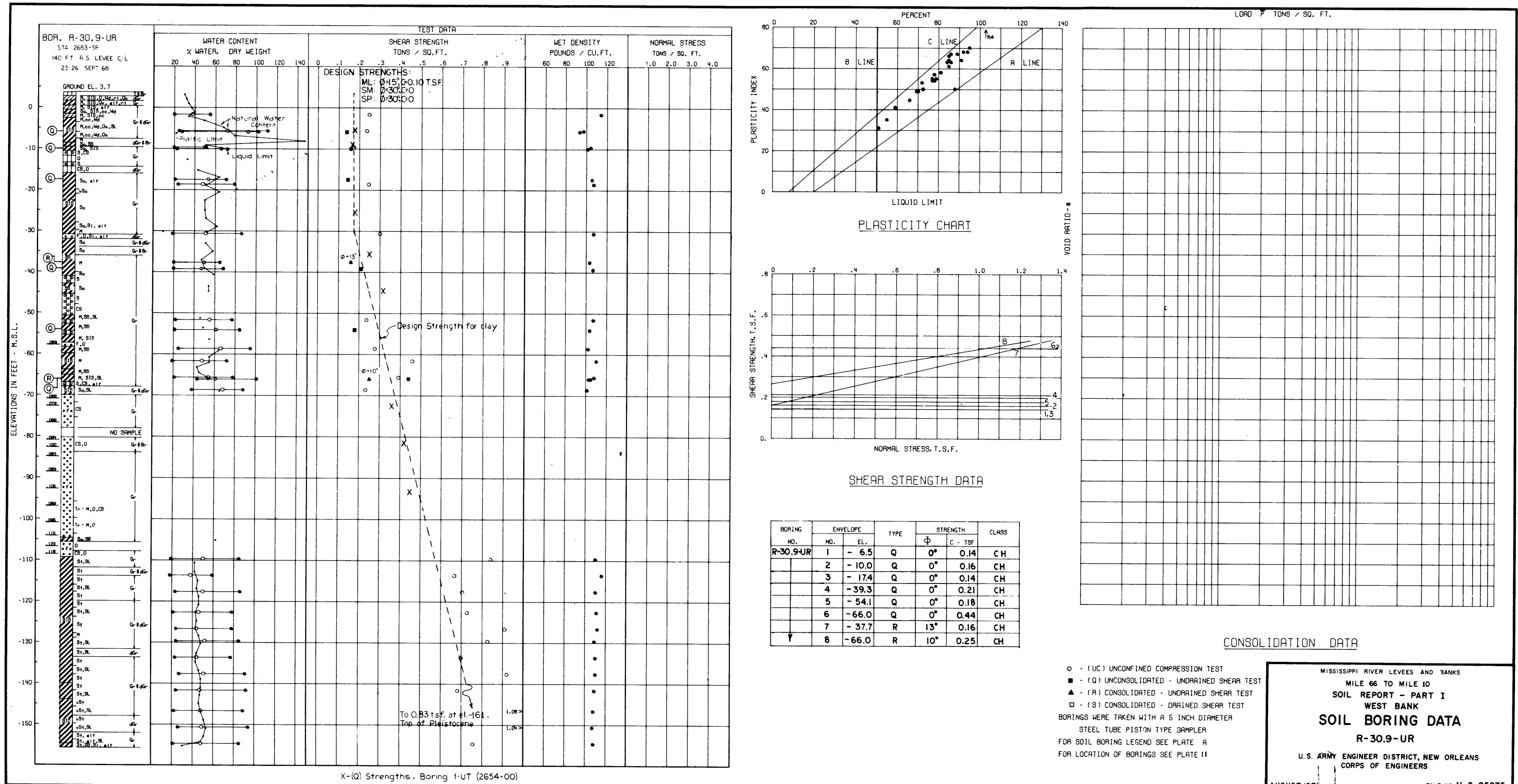
COLLEGE OF ENGINEERING

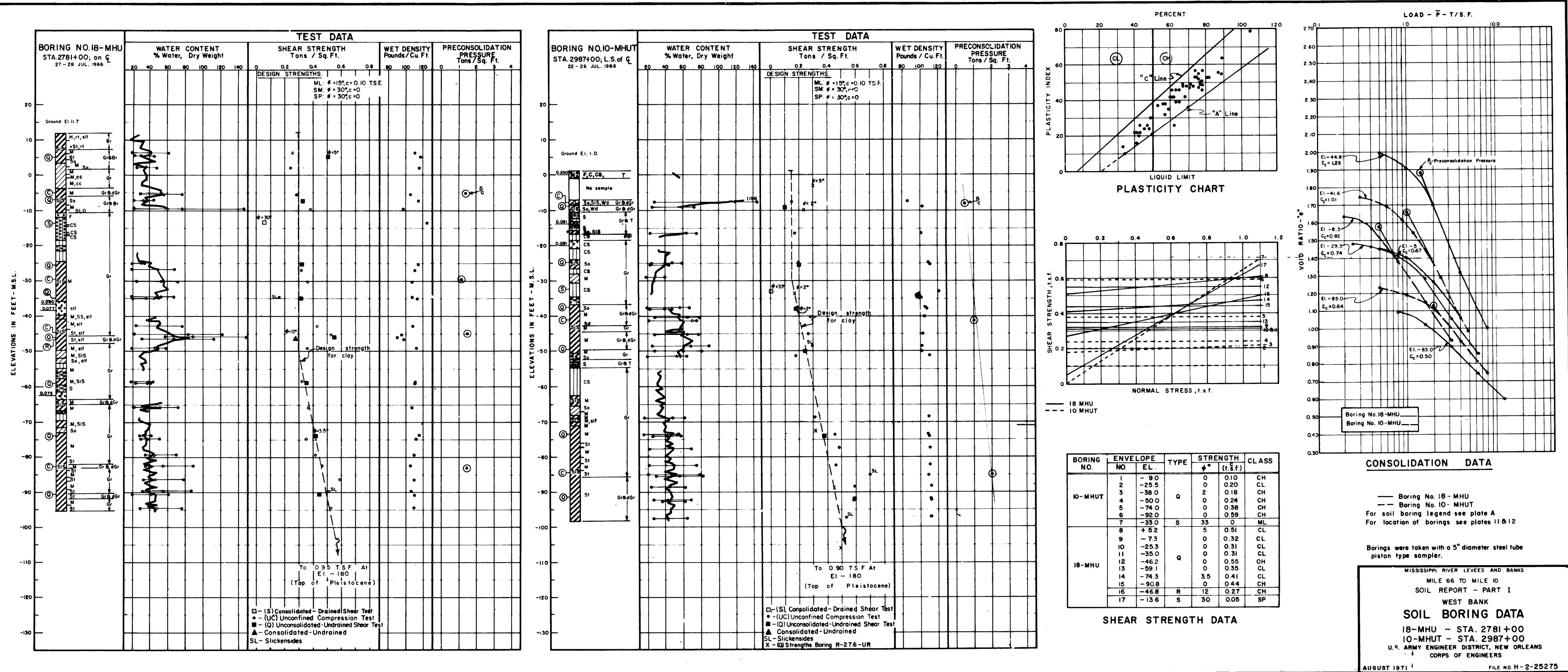
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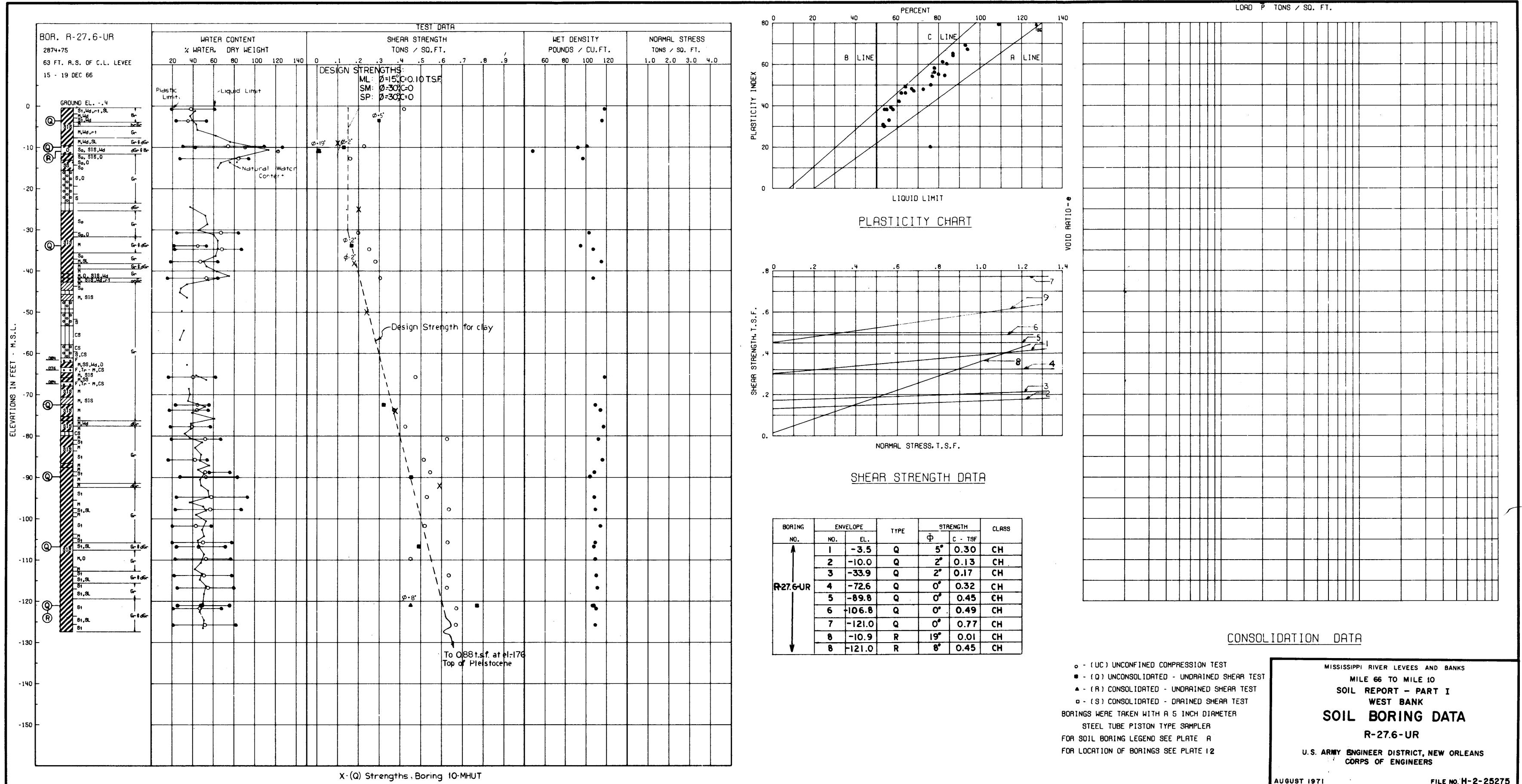
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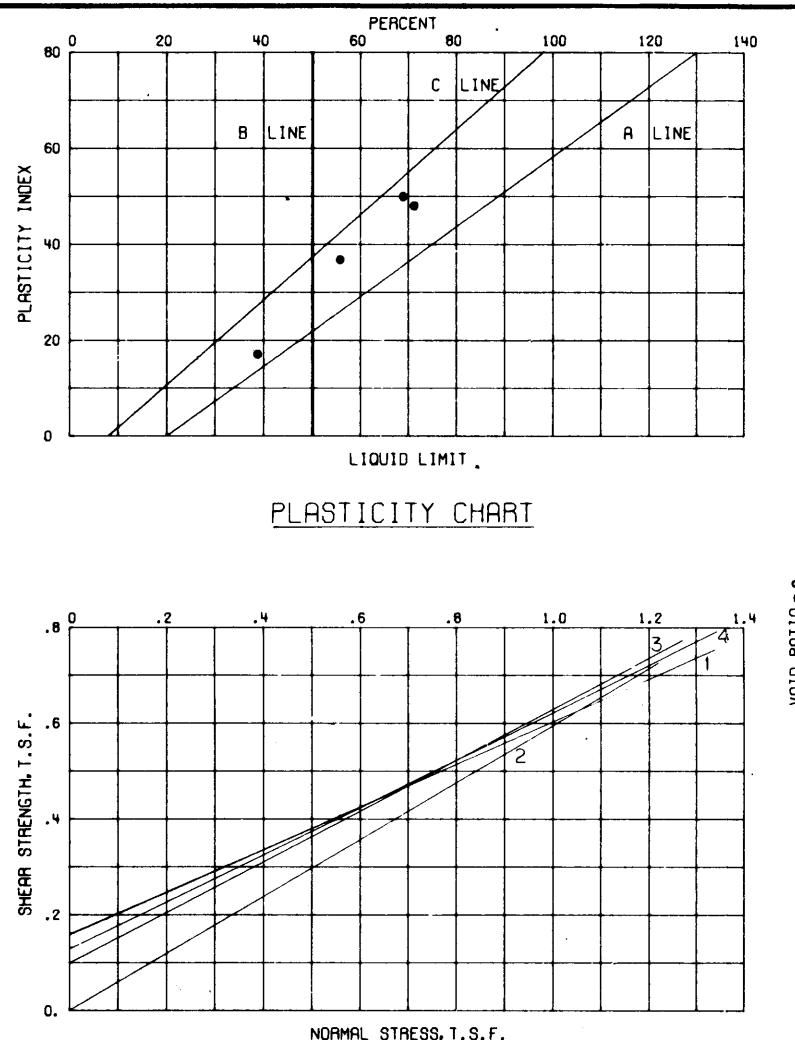
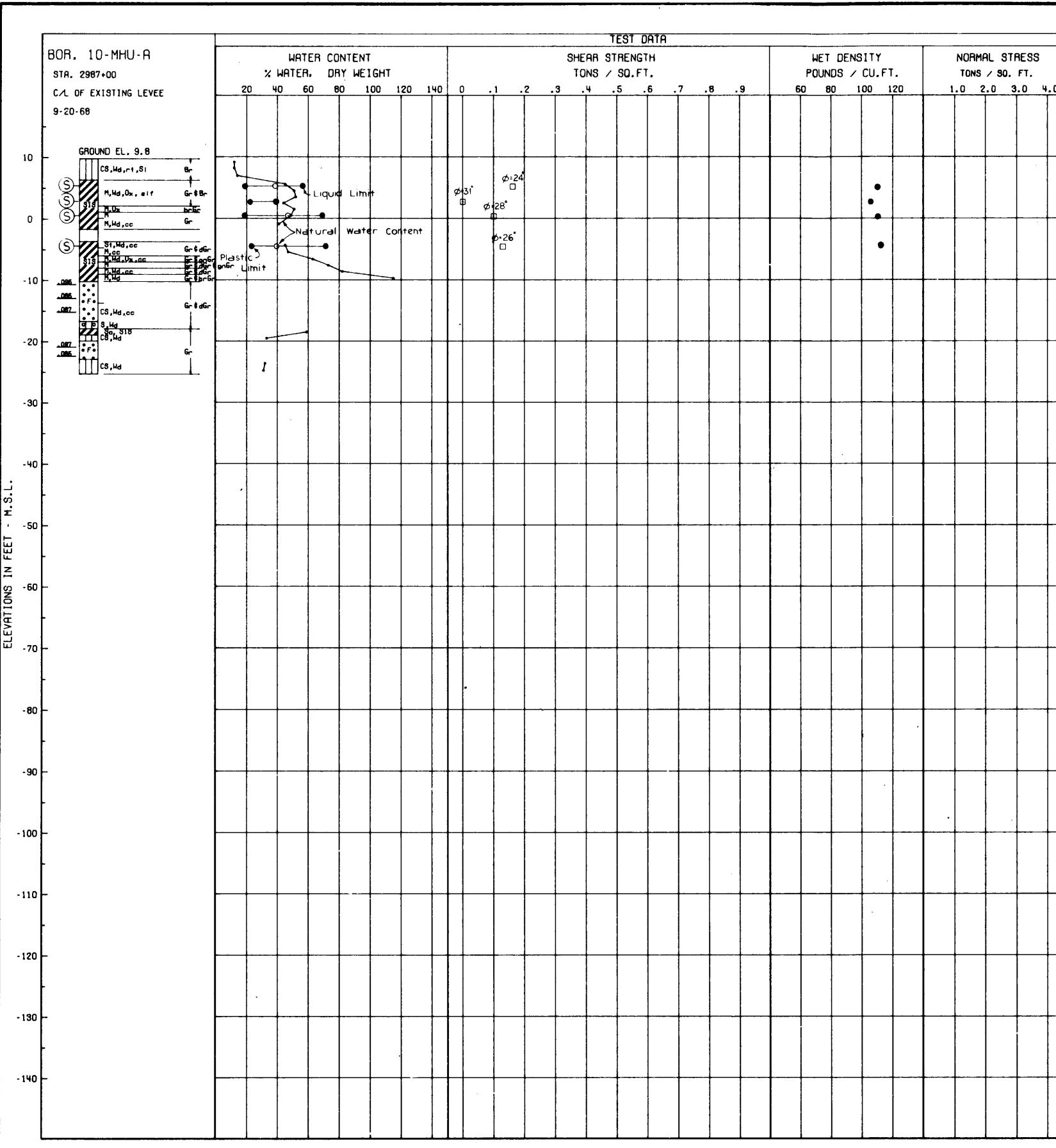












SHEAR STRENGTH DATA

BORING NO.	ENVELOPE		TYPE	STRENGTH		CLASS
	NO.	EL.		Φ	C - TSF	
10-MHU-A	1	+ 5.4	S	24°	0.16	CH
	2	+ 2.8		31°	0	CL
	3	+ 0.3		28°	0.10	CH
	4	- 4.4		26°	0.13	CH

CONSOLIDATION DATA

- - (UC) UNCONFINED COMPRESSION TEST
 - - (Q) UNCONSOLIDATED - UNDRAINED SHEAR TEST
 - ▲ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST
 - - (S) CONSOLIDATED - DRAINED SHEAR TEST

BORINGS WERE TAKEN WITH A 5 INCH DIAMETER
 STEEL TUBE PISTON TYPE SAMPLER
 OR SOIL BORING LEGEND SEE PLATE A
 OR LOCATION OF BORINGS SEE PLATE 12

B - (1) CONSOLIDATED - BRAINED SHEET IRON
PIECES WERE TAKEN WITH 2.5 INCH SIGNETTE

BORINGS WERE TAKEN WITH A 5 INCH DIAMETER

STEEL TUBE PISTON TYPE SAMPLER

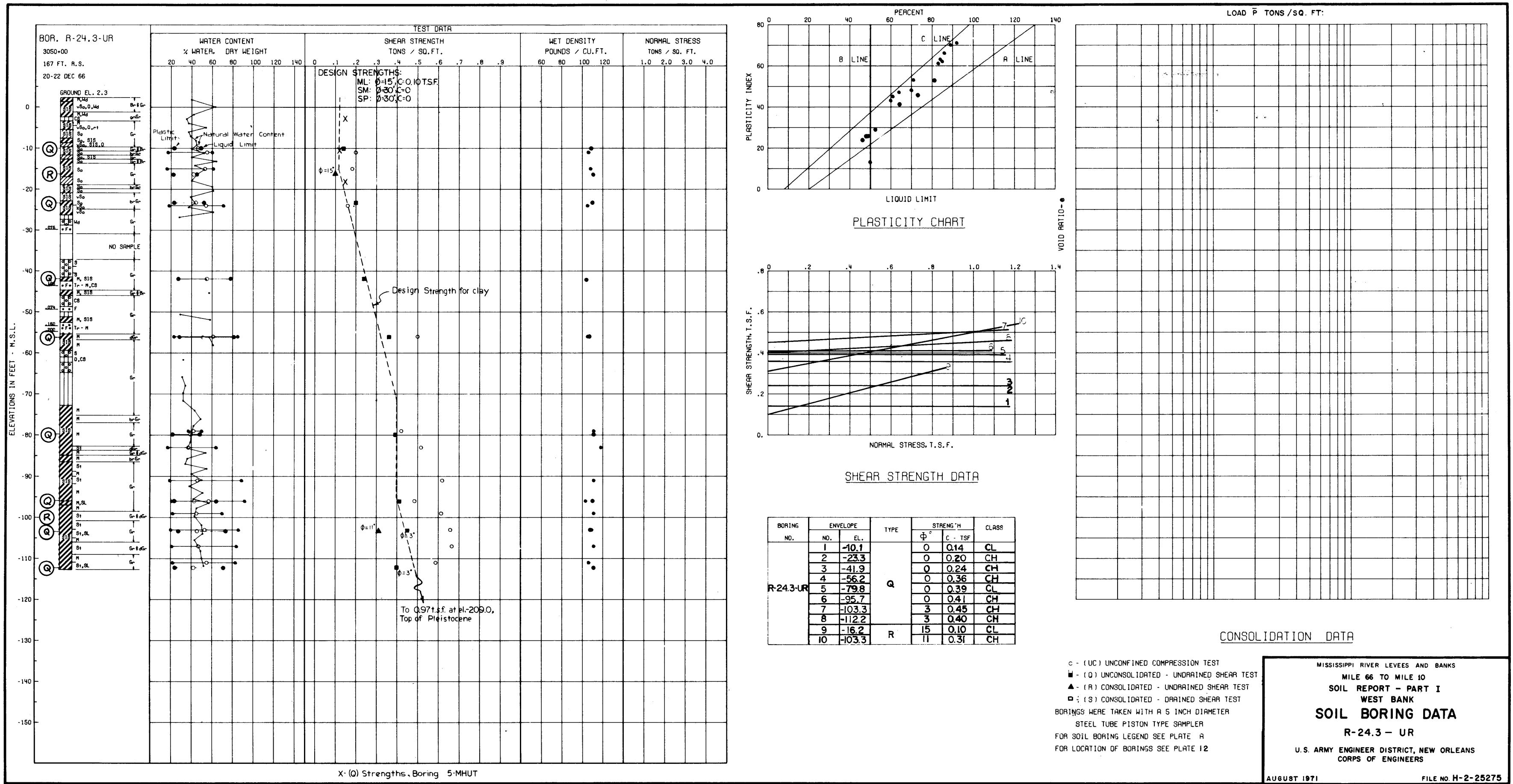
FOR SOIL BOILING LEGEND SEE PLATE A

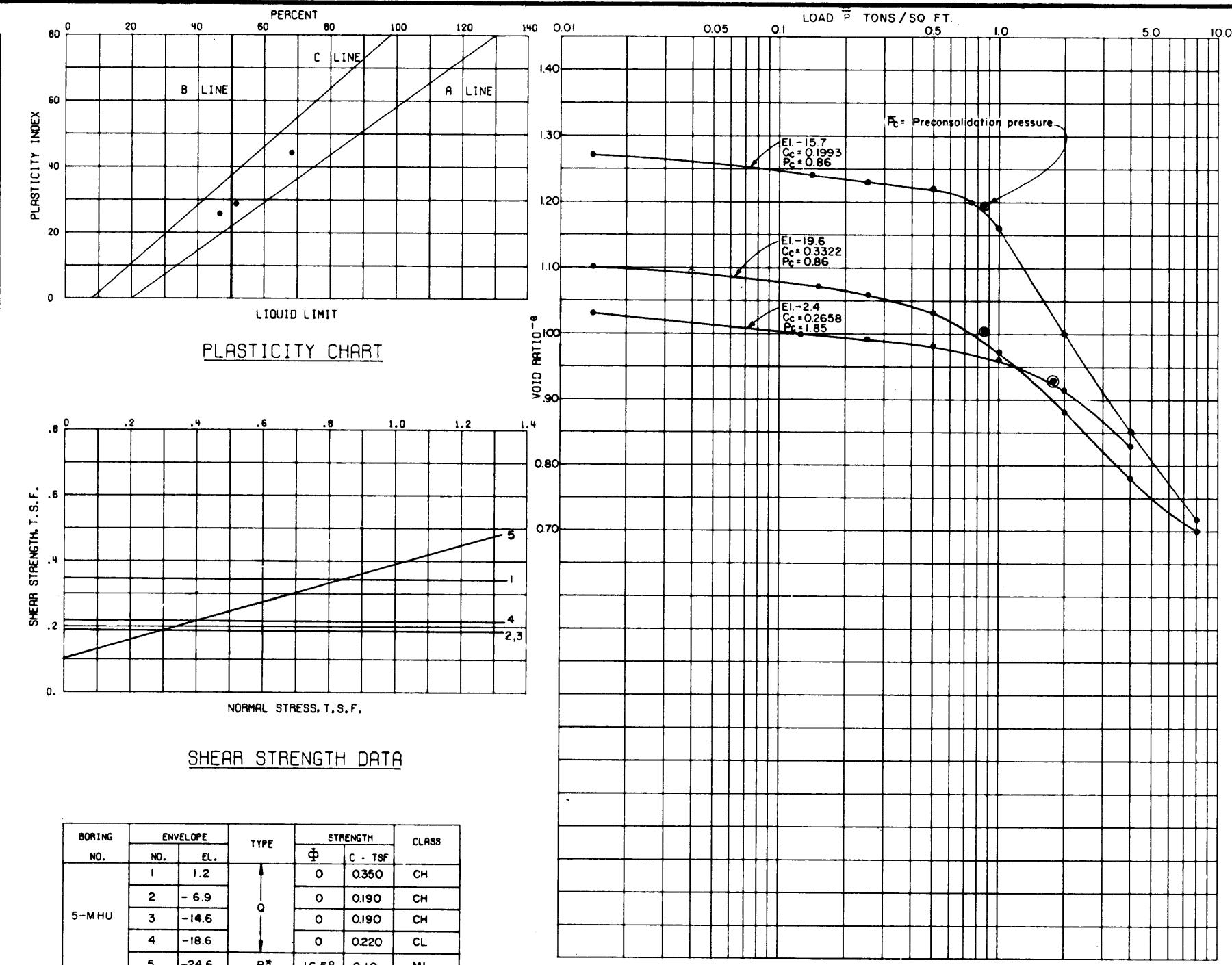
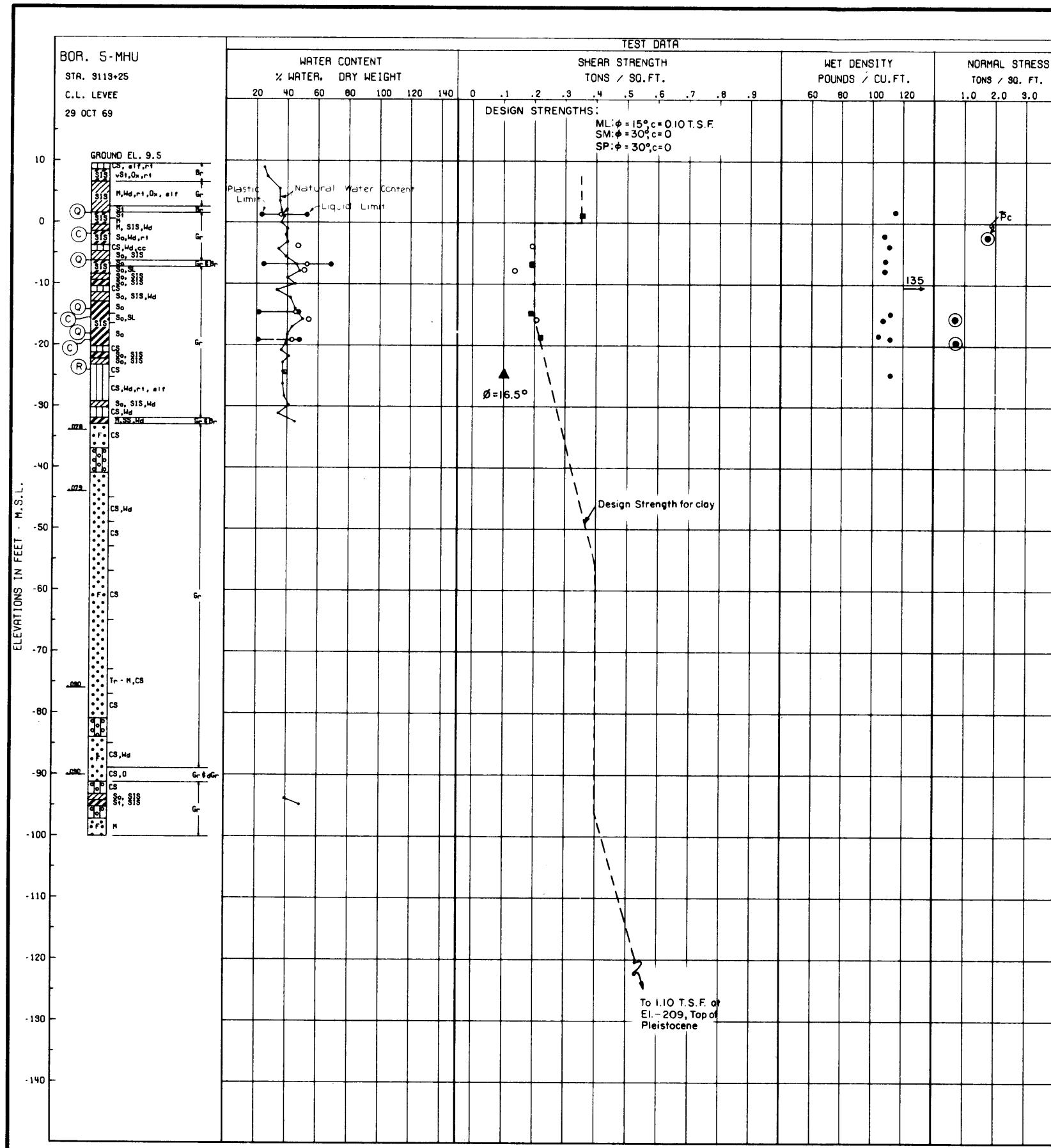
FOR SITE DOWNGE LEGEND SEE PAGE A
FOR ADDITIONAL INFORMATION SEE PAGE 15

FOR LOCATION OF BORINGS SEE PLATE 12

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MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
WEST BANK
SOIL BORING DATA
10-MHU-A





*BASED ON DEVIATOR STRESS AT MAXIMUM POSITIVE
PORE PRESSURE

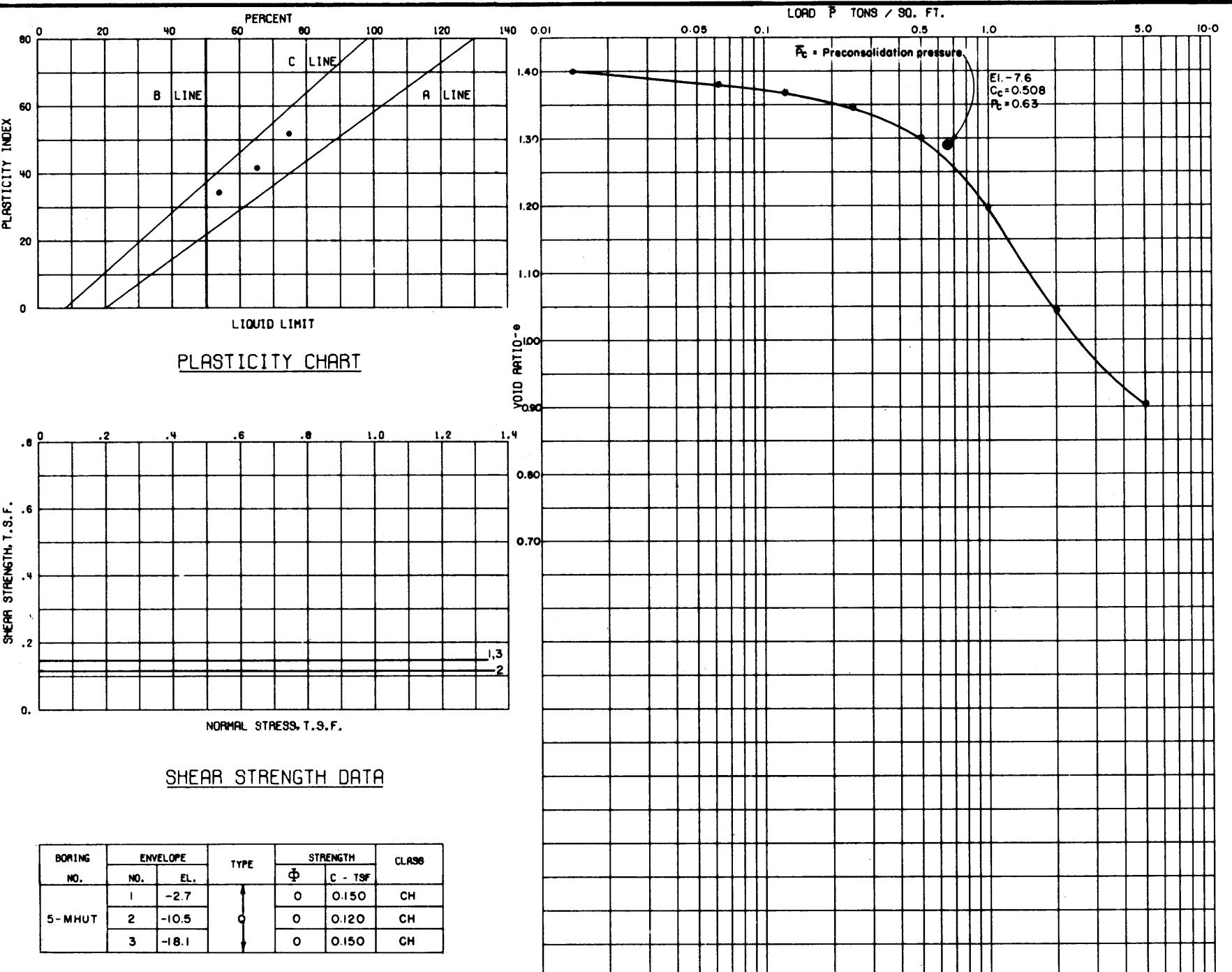
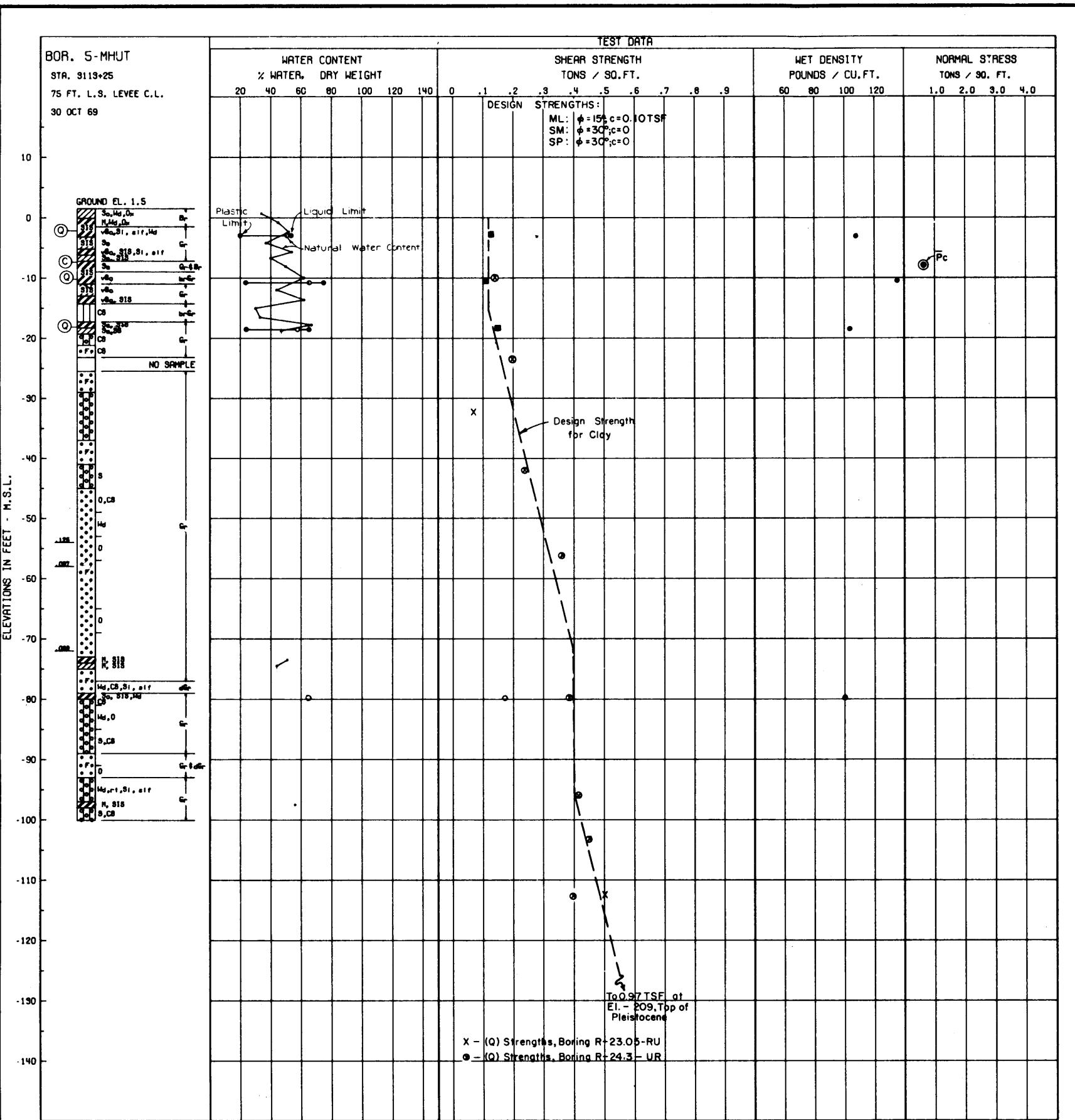
- - (UC) UNCONFINED COMPRESSION TEST
- - (D) UNCONSOLIDATED - UNDRAINED SHEAR TEST
- ▲ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST
- - (S) CONSOLIDATED - DRAINED SHEAR TEST

ROBINGS WERE TAKEN WITH A 5 INCH DIAMETER

STEEL TUBE PISTON TYPE SAMPLER
FOR SOIL BORING LEGEND SEE PLATE A
FOR LOCATION OF BORINGS SEE PLATE 13

CONSOLIDATION DATA

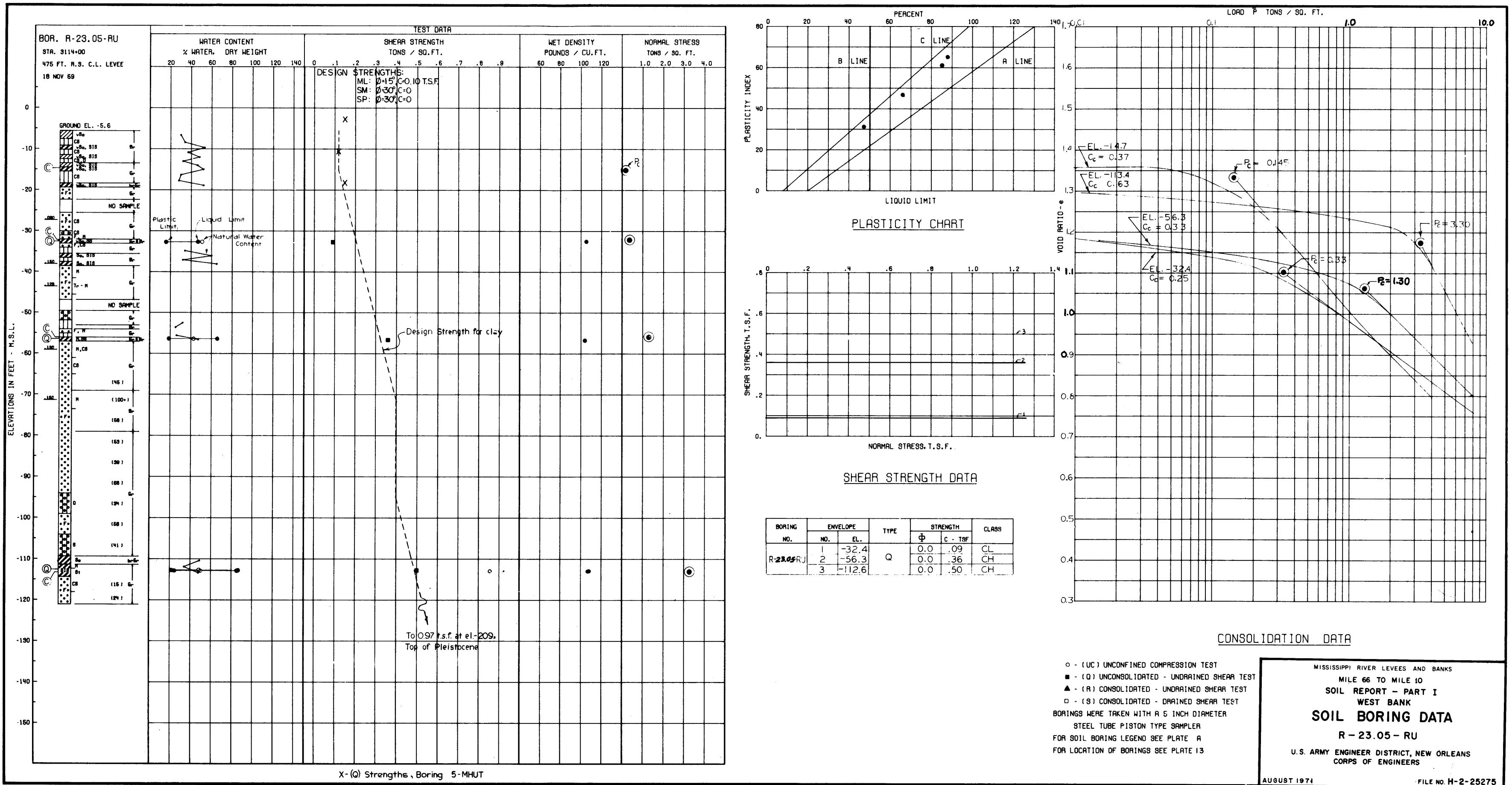
MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
WEST BANK
SOIL BORING DATA
5-MHU
U. S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS

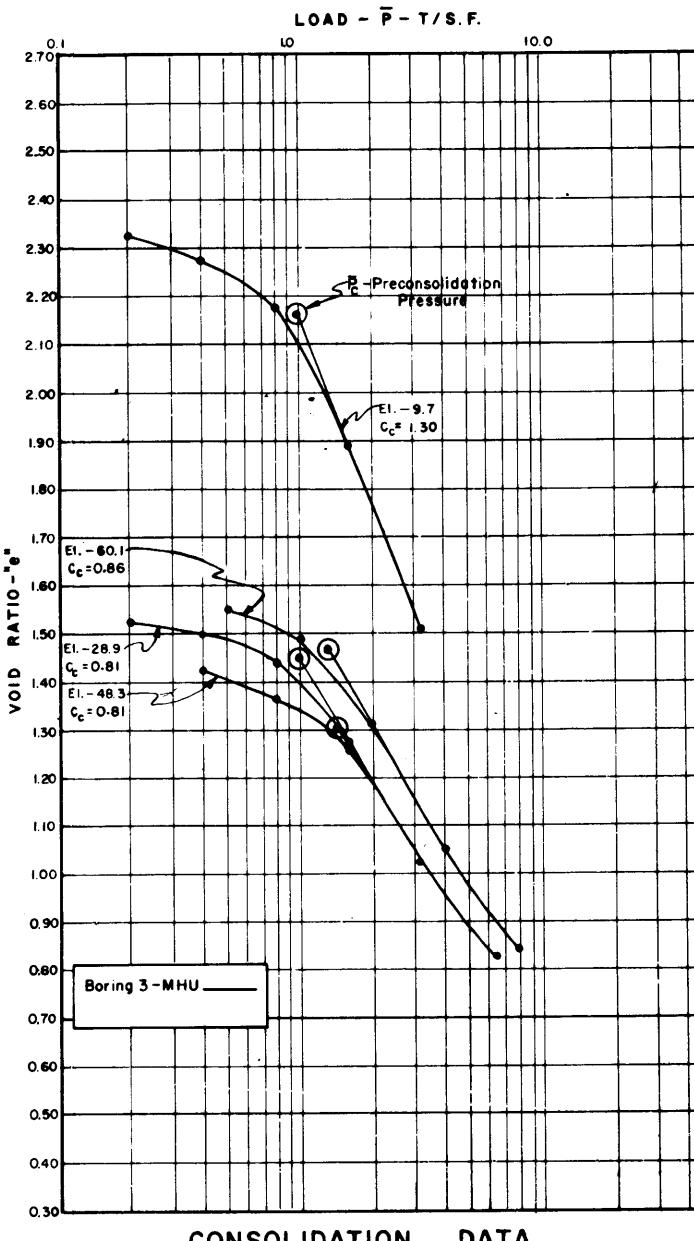
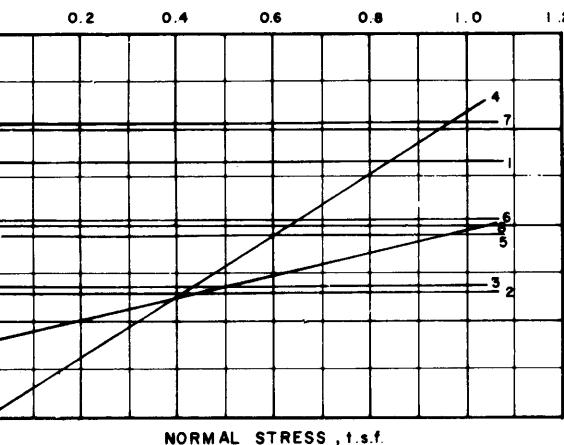
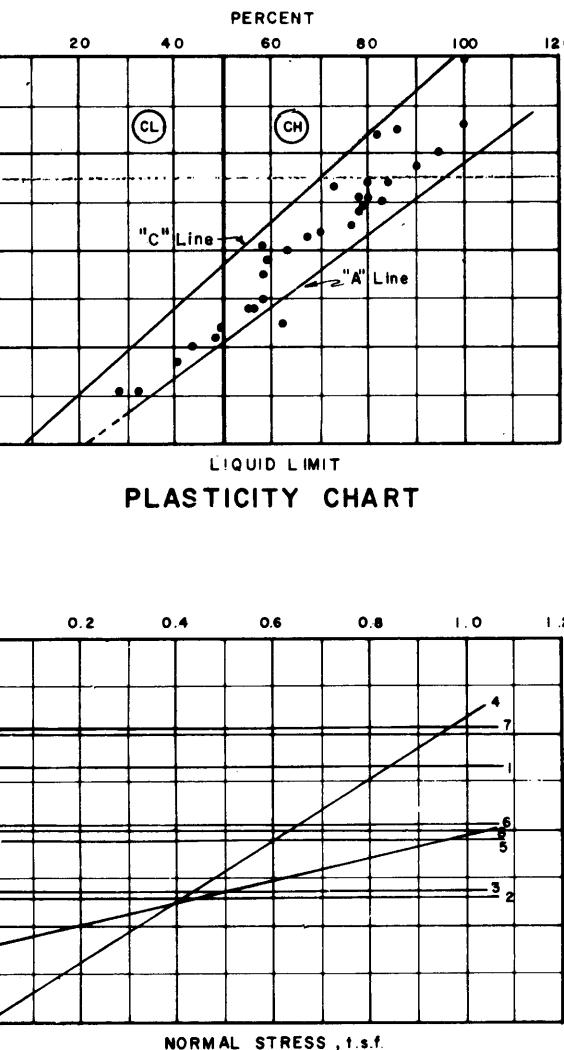
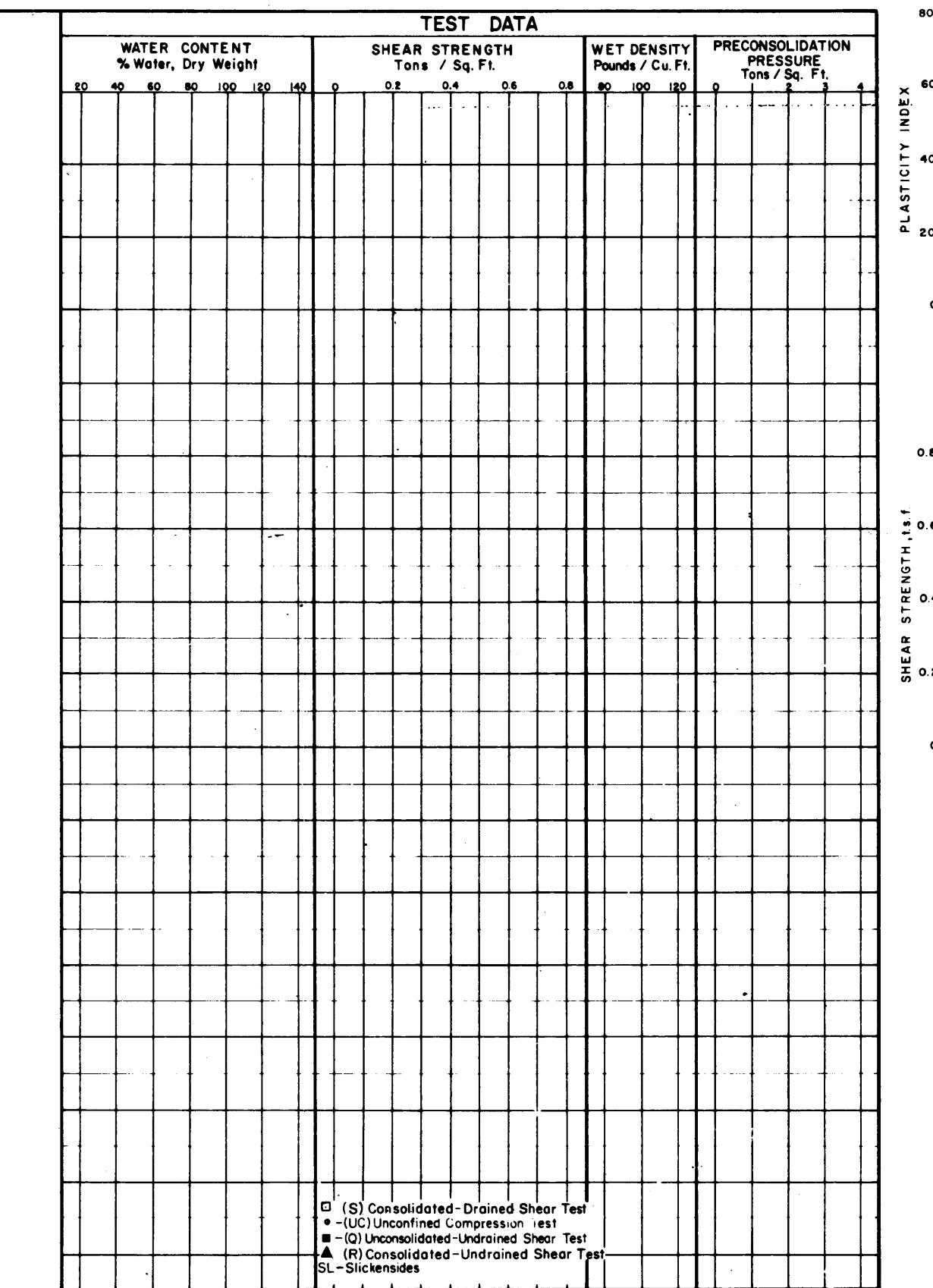
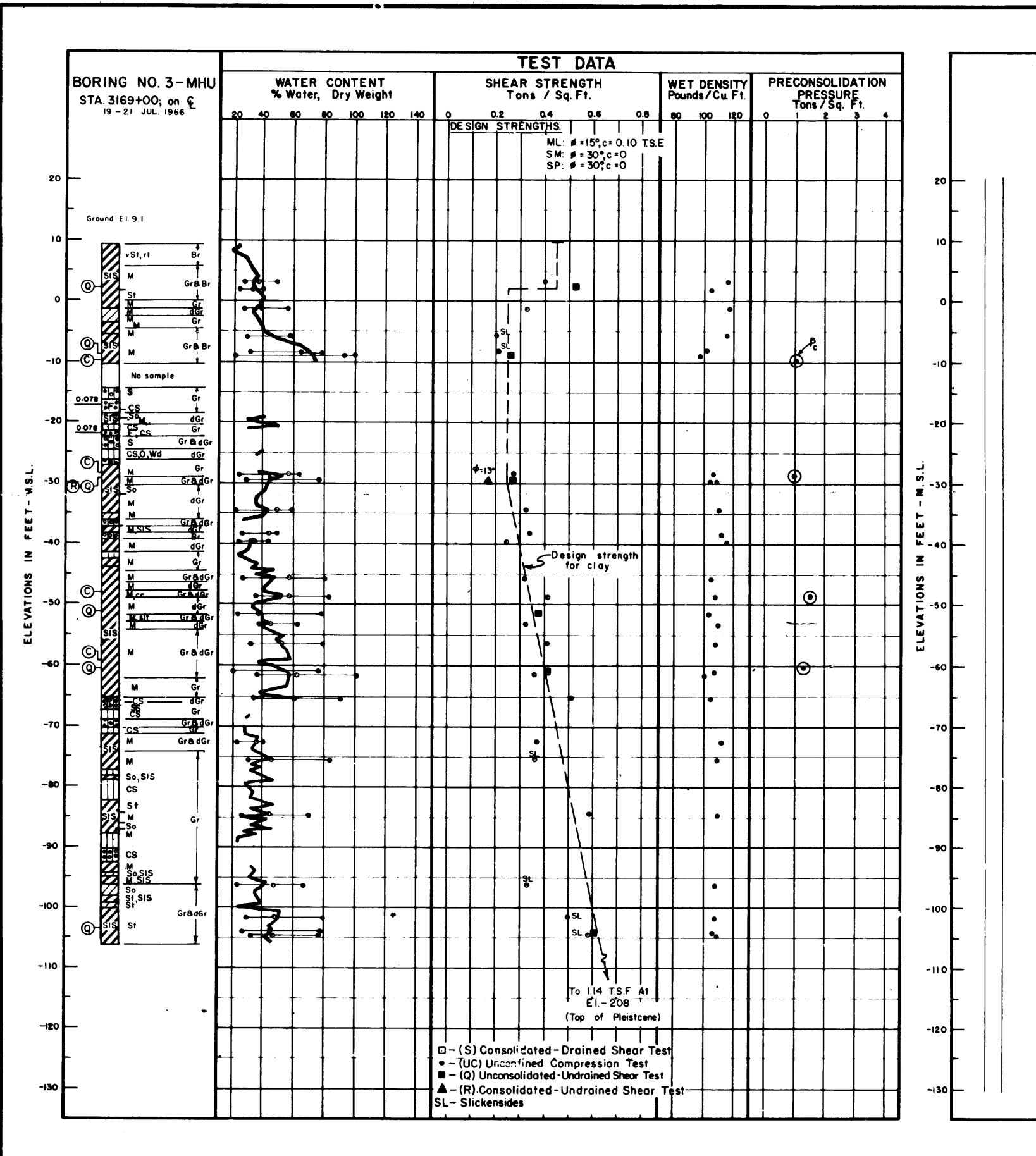


- - (UC) UNCONFINED COMPRESSION TEST
- - (O) UNCONSOLIDATED - UNDRAINED SHEAR TEST
- ▲ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST
- - (S) CONSOLIDATED - DRAINED SHEAR TEST

ORINGS WERE TAKEN WITH A 5 INCH DIAMETER
 STEEL TUBE PISTON TYPE SAMPLER
 OR SOIL BORING LEGEND SEE PLATE A
 OR LOCATION OF BORINGS SEE PLATE 13

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
WEST BANK
SOIL BORING DATA
5-MHUT
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS

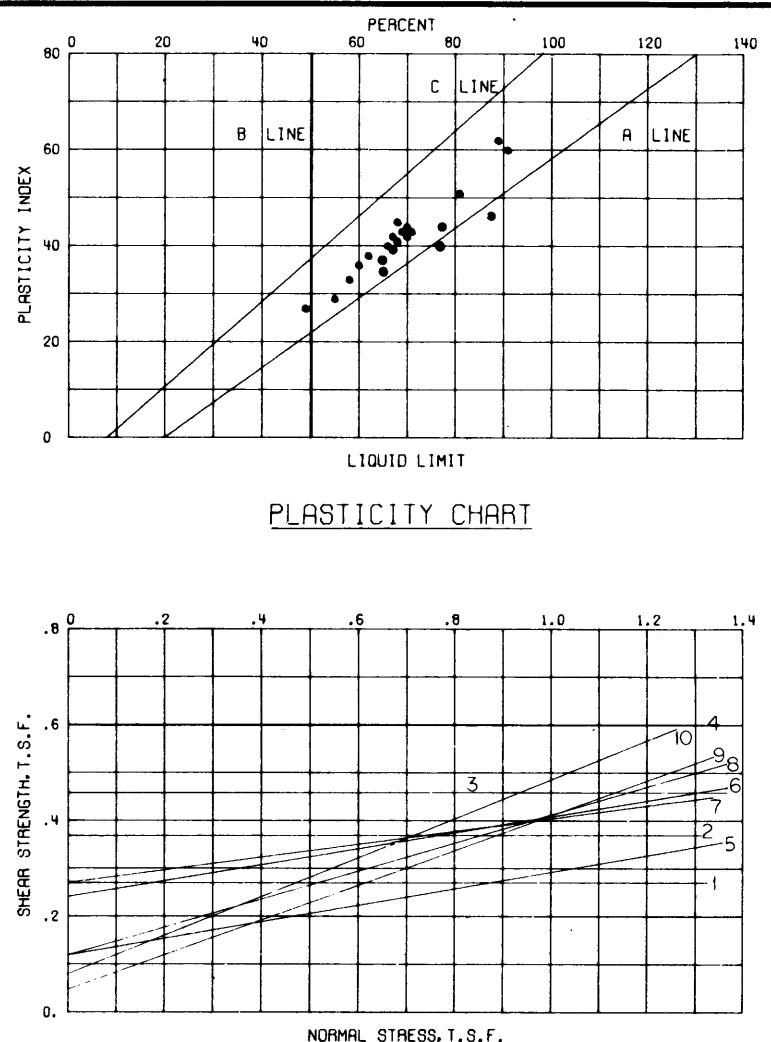
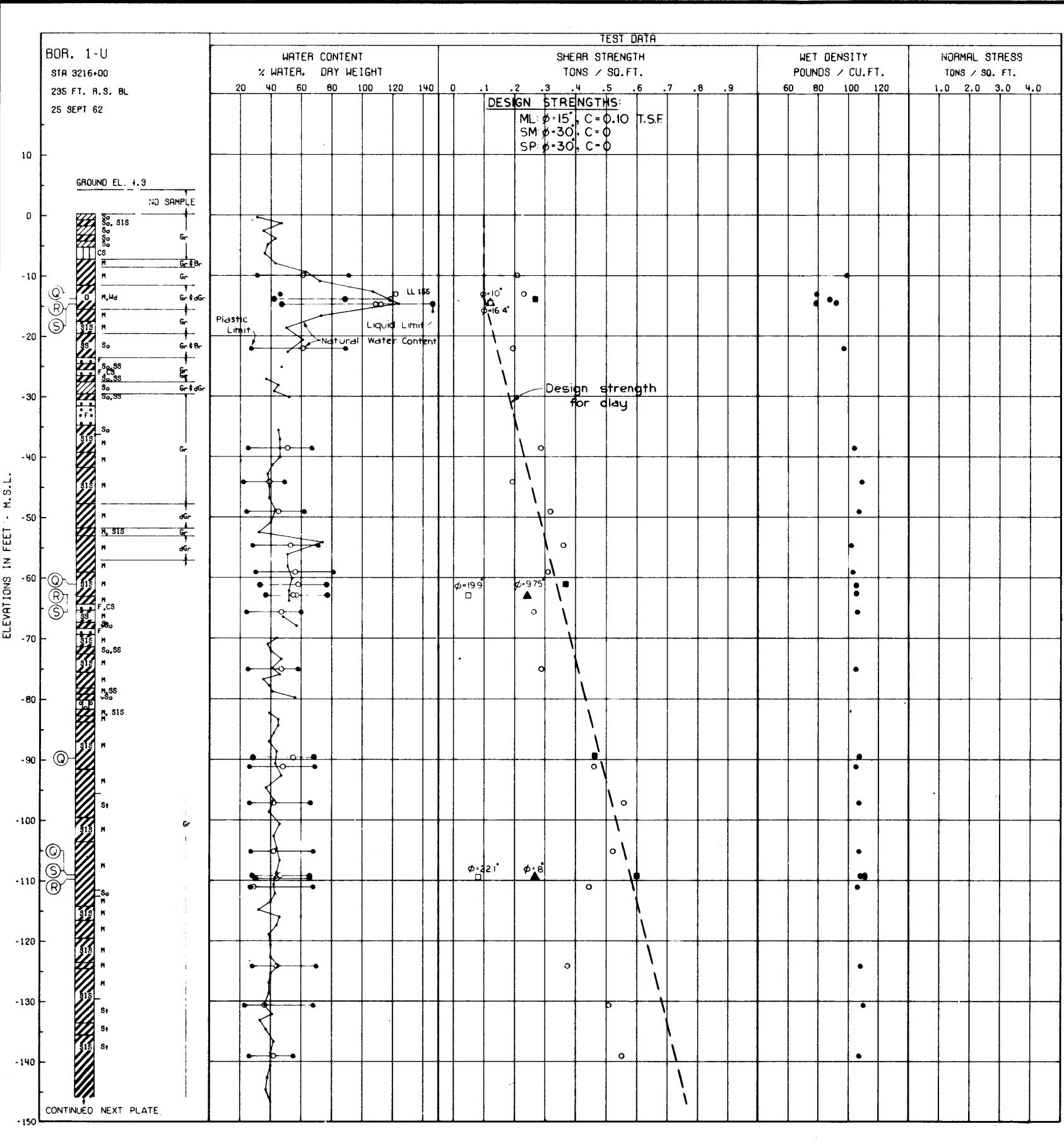




BORING NO.	ENVELOPE NO.	EL.	TYPE	STRENGTH ϕ^* (t.s.f.)	CLASS
3 - MUH	1	+ 2.1	Q	0	0.53 CL
	2	- 8.9		0	0.26 CH
	3	-29.4		0	0.27 CH
	4	-40.6		32.5 0	SP
	5	-51.6		0	0.38 CH
	6	-60.9		0	0.41 CH
	7	-103.9		0	0.61 CH
	8	-29.4	R	I3 0.16	CH

SHEAR STRENGTH DATA

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
WEST BANK
SOIL BORING DATA
3-MHU
STA. 3169 + 00
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS



BORING NO.	ENVELOPE		TYPE	STRENGTH		CLASS
	NO.	EL.		Φ	C - TSF	
1-U	1	-13.8	Q	O	0.27	OH
	2	-61.0		O	0.37	CH
	3	-89.8		O	0.46	CH
	4	-109.0		O	0.60	CH
	5	-14.7	I	10°	0.12	CH
	6	-62.7	R	9.75°	0.24	OH
	7	-109.8	I	8°	0.27	CH
	8	-14.7		16.4°	0.12	CH
	9	-62.7		19.9°	0.05	OH
	10	-109.0	S	22.1°	0.08	CH

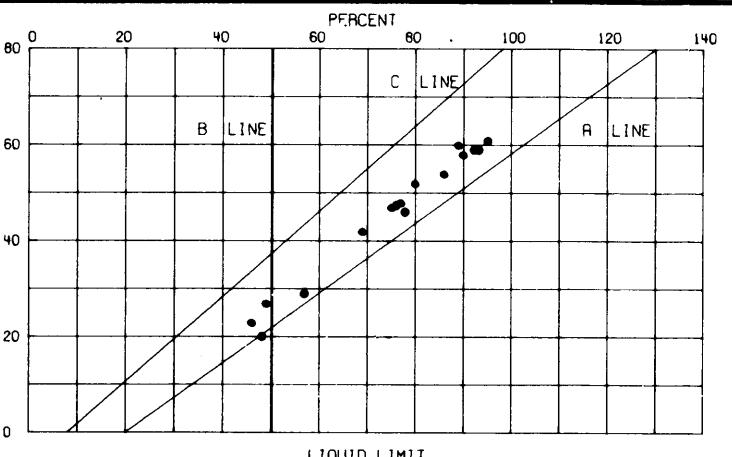
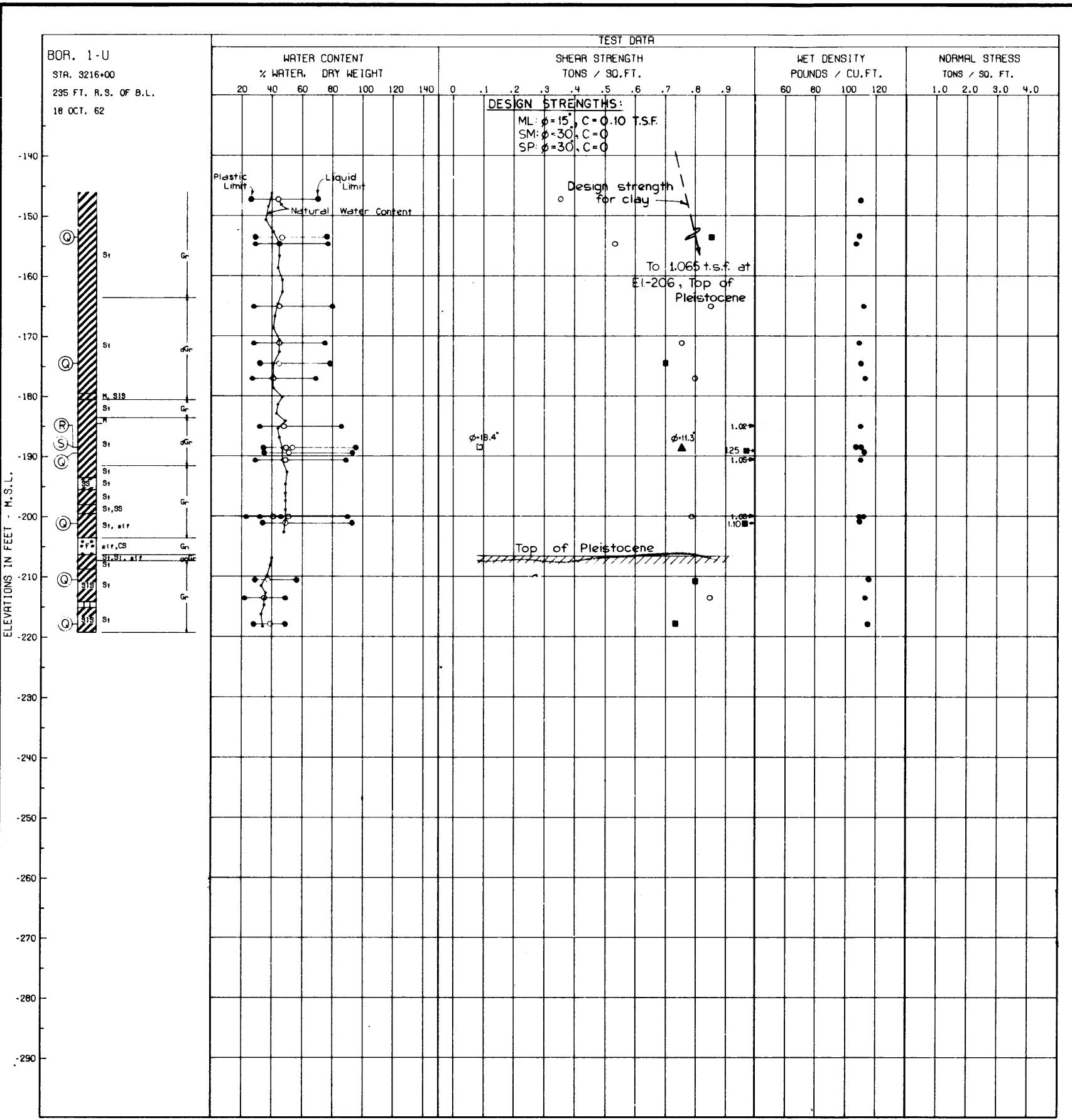
- - (UC) UNCONFINED COMPRESSION TEST
- - (Q) UNCONSOLIDATED - UNDRAINED SHEAR TEST
- ▲ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST
- - (S) CONSOLIDATED - DRAINED SHEAR TEST

BORINGS WERE TAKEN WITH A 5 INCH DIAMETER
STEEL TUBE PISTON TYPE SAMPLER
OR SOIL BORING LEGEND SEE PLATE A
OR LOCATION OF BORINGS SEE PLATE 13

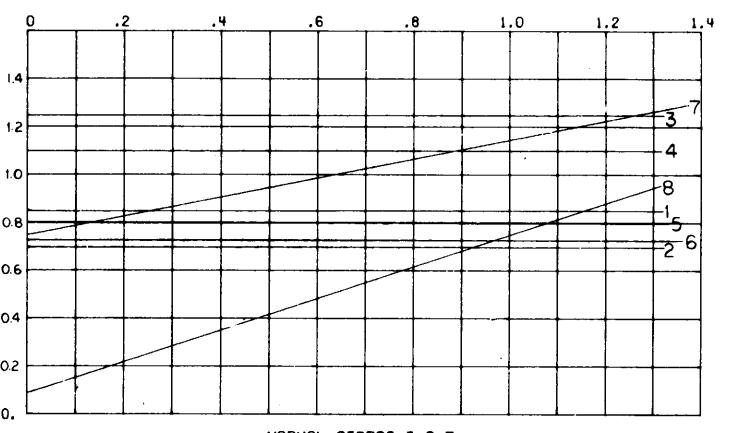
- - (UC) UNCONFINED COMPRESSION TEST
- - (Q) UNCONSOLIDATED - UNDRAINED SHEAR TEST
- ▲ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST
- - (S) CONSOLIDATED - DRAINED SHEAR TEST

BORINGS WERE TAKEN WITH A 5 INCH DIAMETER
 STEEL TUBE PISTON TYPE SAMPLER
 FOR SOIL BORING LEGEND SEE PLATE A
 FOR LOCATION OF BORINGS SEE PLATE 13

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
WEST BANK
SOIL BORING DATA
I-U
STA. 3216+00
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS



PLASTICITY CHART



SHEAR STRENGTH DATA

BORING NO.	ENVELOPE		TYPE	STRENGTH		CLASS
	NO.	EL.		Φ	C - TSF	
I-U	1	-153.6	P	O	0.85	CH
	2	-174.6		O	0.70	CH
	3	-189.5		O	1.25	CH
	4	-201.0		O	1.10	CH
	5	-210.5	Q	O	0.80	CH
	6	-217.9		O	0.73	OL
	7	-188.6		11.3°	0.75	CH
	8	-188.6	S	18.4°	0.09	CH

CONSOLIDATION DATA

- - (UC) UNCONFINED COMPRESSION TEST
- - (Q) UNCONSOLIDATED - UNDRAINED SHEAR TEST
- ▲ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST
- - (S) CONSOLIDATED - DRAINED SHEAR TEST

RINGS WERE TAKEN WITH A 5 INCH DIAMETER
STEEL TUBE PISTON TYPE SAMPLER
R SOIL BORING LEGEND SEE PLATE A
R LOCATION OF BORINGS SEE PLATE 13

RECORDS WERE TAKEN WITH 3.5 INCH DIAMETER

BURINGS WERE TAKEN WITH A 5 INCH DIAMETER

STEEL TUBE PISTON TYPE SAMPLER

FOR SOIL BORING LEGEND SEE PLATE 9

FOR SITE COUNTING LEGEND SEE PLATE A
FOR LOCATION OF ROBINS SEE PLATE 13

FOR LOCATION OF BORINGS SEE PLATE 13

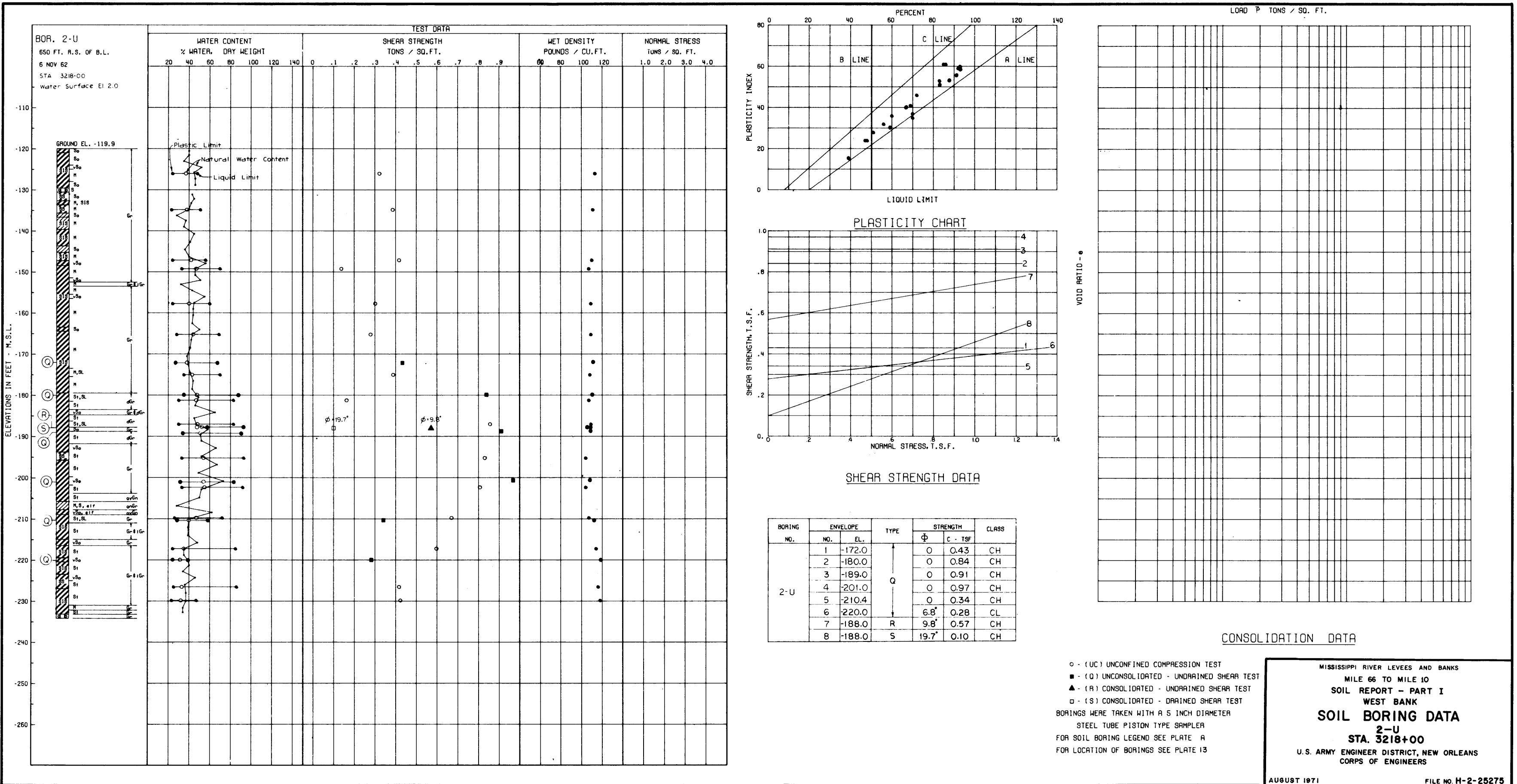
For more information about the study, please contact Dr. Michael J. Hwang at (319) 356-4530 or via email at mhwang@uiowa.edu.

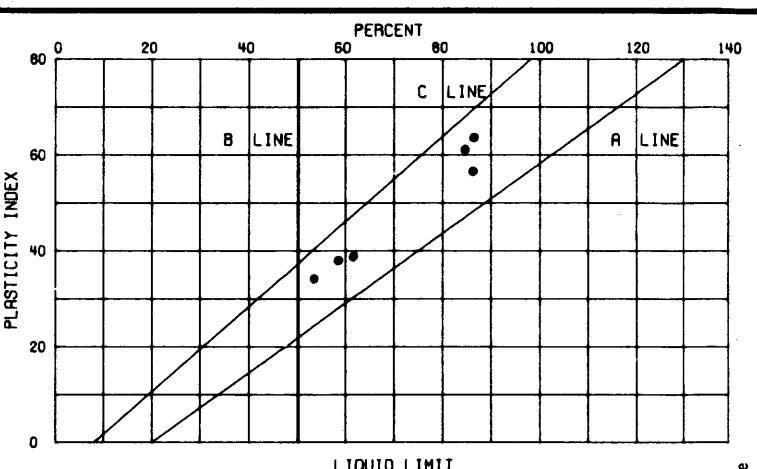
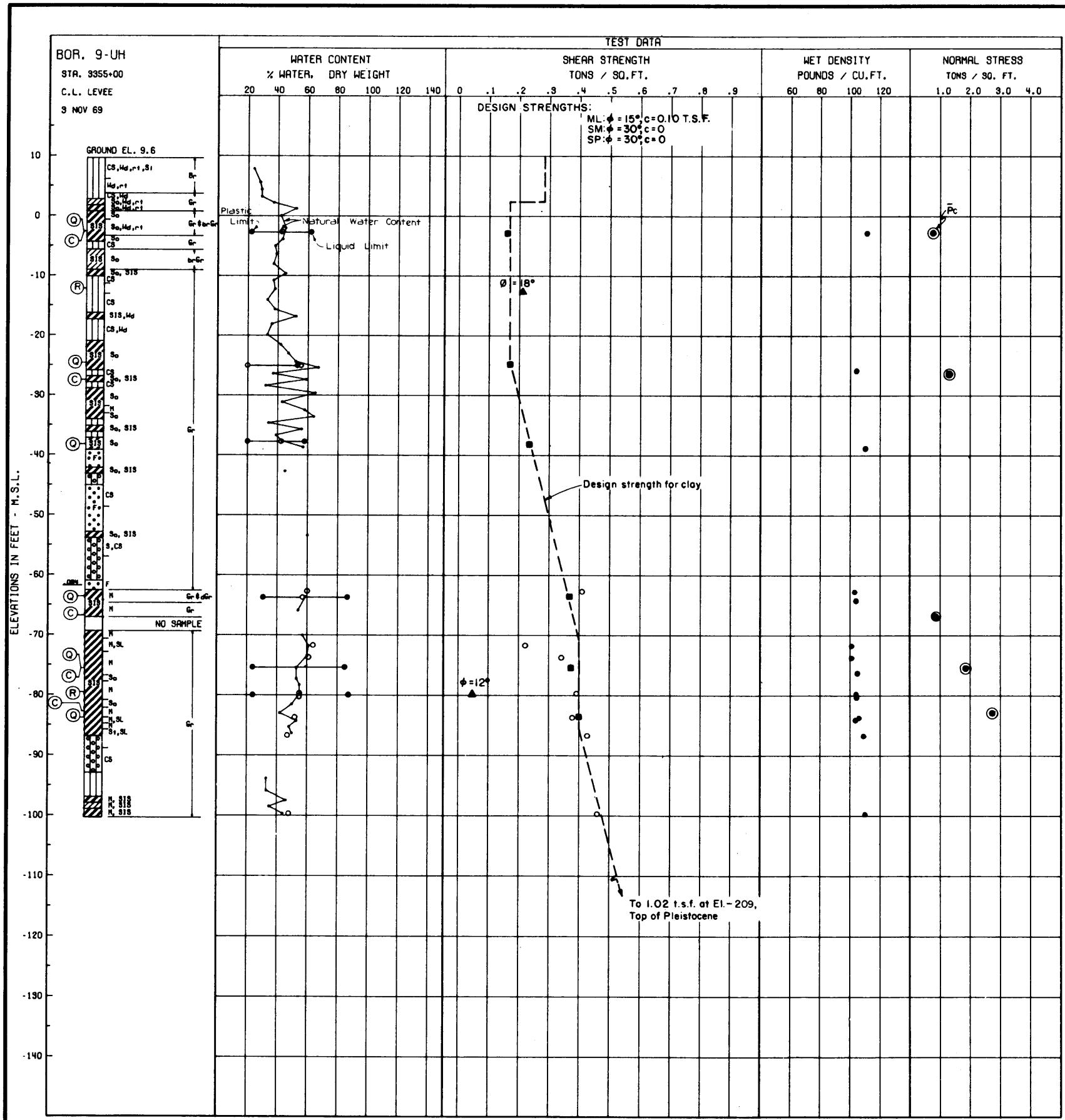
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For more information about the study, please contact Dr. Michael J. Koenig at (412) 248-1000 or via email at koenig@cmu.edu.

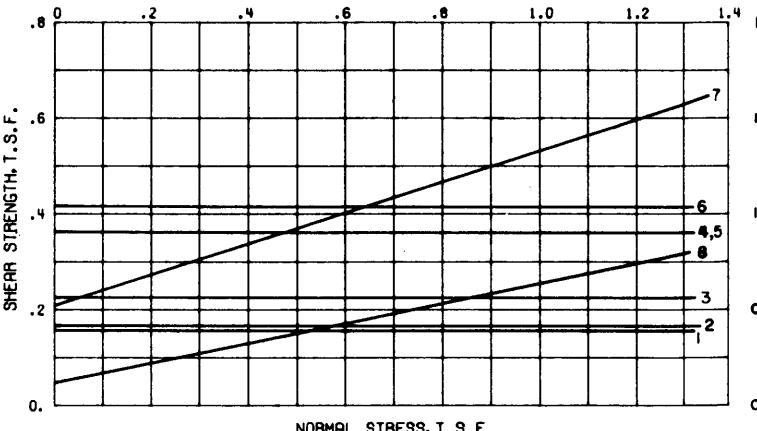
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MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
WEST BANK
SOIL BORING DATA
I-U (CONT.)
STA. 3216+00
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS





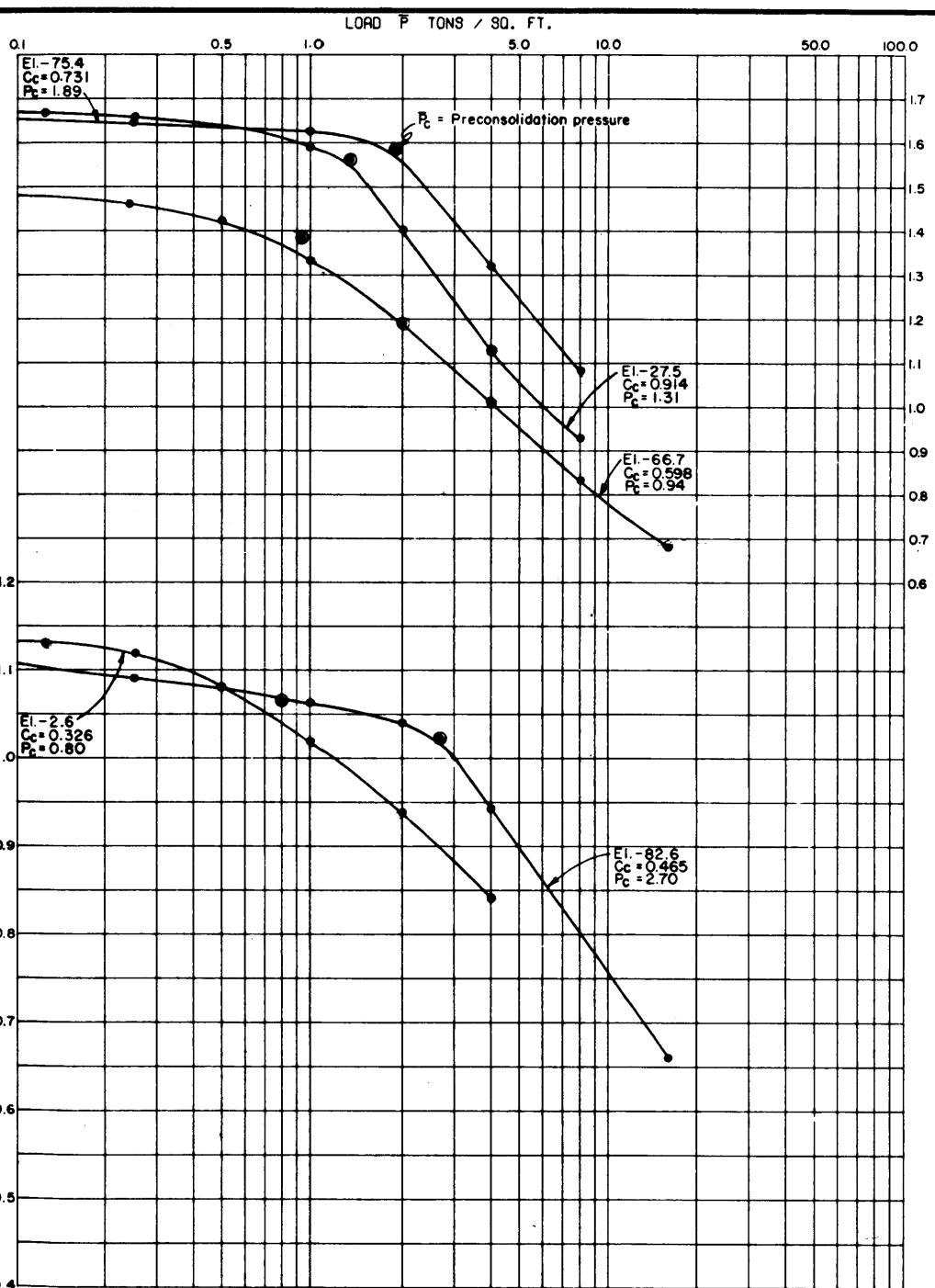
PLASTICITY CHART



HEAR STRENGTH DATA

BORING NO.	ENVELOPE		TYPE	STRENGTH		CLASS
	NO.	EL.		Φ	C - TSF	
9 - UH	1	- 2.6	Q	0	0.160	CH
	2	- 24.5		0	0.170	CH
	3	- 38.1		0	0.230	CH
	4	- 63.7		0	0.370	CH
	5	- 75.4		0	0.370	CH
	6	- 83.5		0	0.420	CH
	7	- 12.4		*18°	0.210	ML
	8	- 80.0	R	12°	0.050	CH

* BASED ON DEVIATOR STRESS AT MAXIMUM POSITIVE
PORE PRESSURE : $\phi = 17^\circ$, $C = 0.03$ TSF



CONSOLIDATION DATA

D - (UG) UNCONFINED COMPRESSION TEST

■ (C) UNCONFINED COMPRESSION TEST ■ (C) UNCONFINED COMPRESSION TEST

■ - (Q) UNCONSOLIDATED - UNDRAINED SHEAR TEST

▲ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST

□ - (S) CONSOLIDATED - DRAINED SHEAR TEST

INGS WERE TAKEN WITH A 5 INCH DIAMETER

STEEL TUBE PISTON TYPE SAMPLER

STEEL TUBE PISTON TYPE SAMPLER
801L BORING LEGEND SEE PLATE 2

SOIL BORING LEGEND SEE PLATE A

LOCATION OF BORINGS SEE PLATE 13

For more information about the study, please contact Dr. Michael J. Koenig at (314) 747-2146 or via email at koenig@dfci.harvard.edu.

ANSWER

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PI RIVER LEVEES AND BANKS

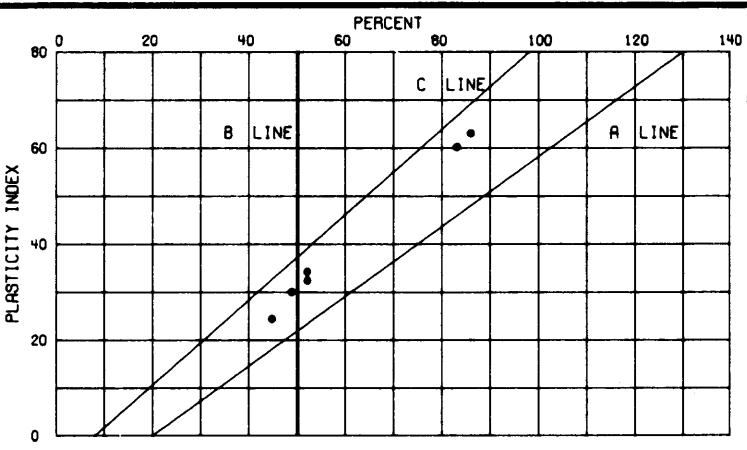
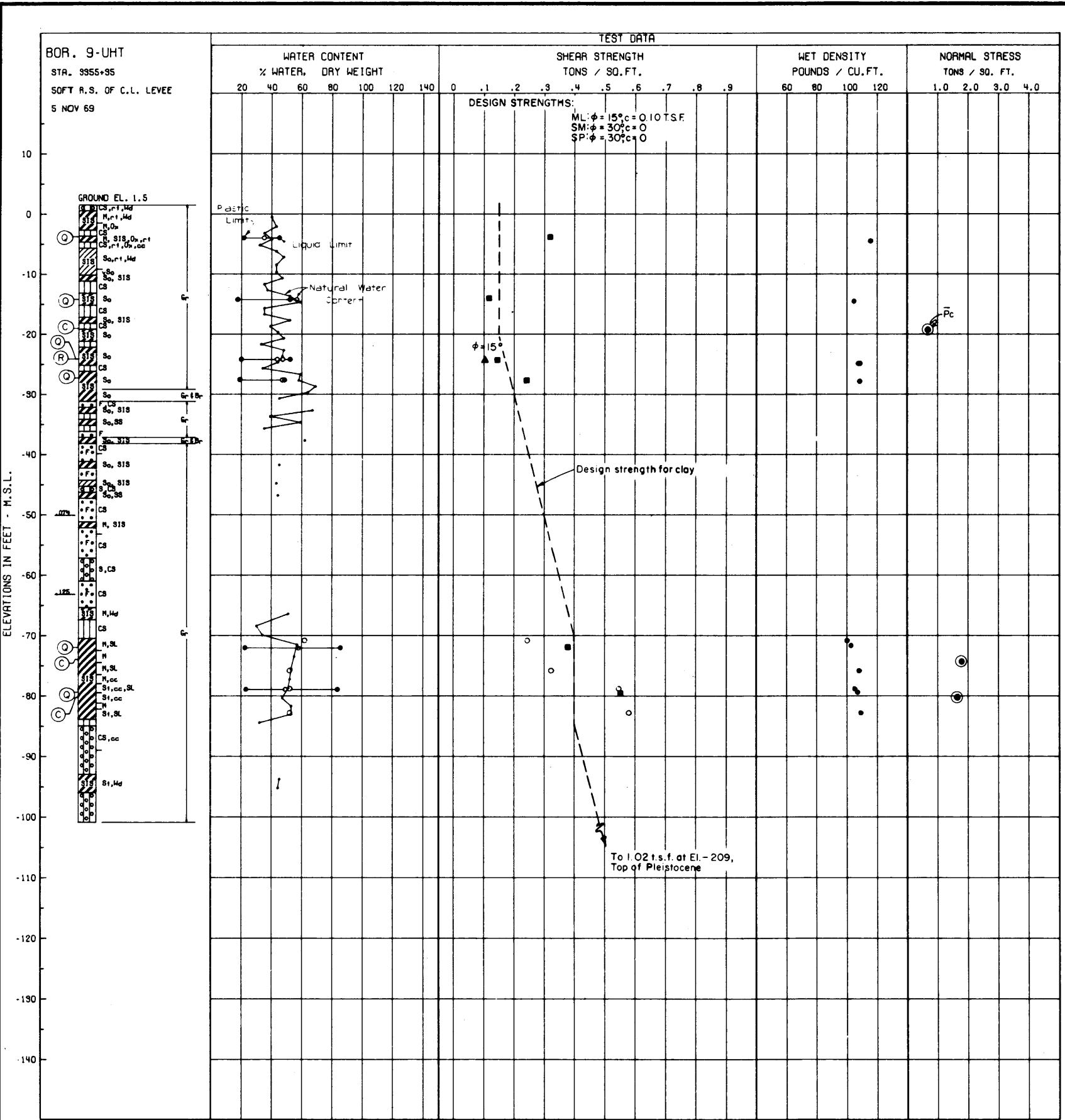
MILE 66 TO MILE 10

REPORT - PART I

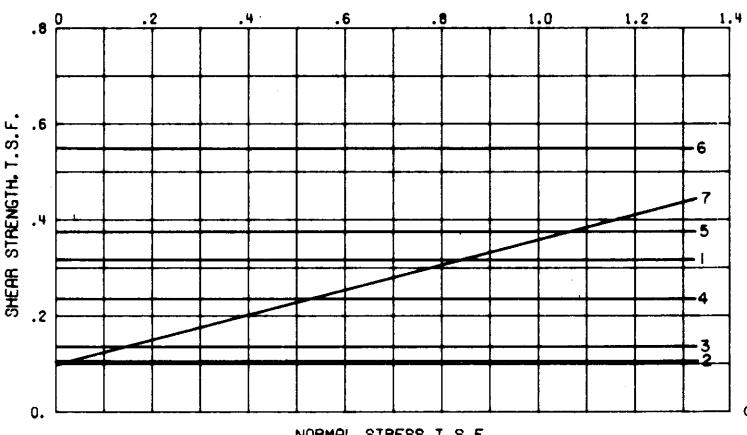
WEST BANK
SOIL BORING DATA

5 of 10

ARMY ENGINEER DISTRICT, NEW ORLEANS



PLASTICITY CHART



SHEAR STRENGTH DATA

BORING NO.	ENVELOPE		TYPE	STRENGTH		CLASS
	NO.	EL.		Φ	C - TSF	
9-UT	1	-4.0	Q	O	0.320	CH
	2	-14.5		O	0.110	CH
	3	-24.3		O	0.140	CH
	4	-27.5		O	0.240	CL
	5	-72.1		O	0.380	CH
	6	-79.3		O	0.550	CH
	7	-24.3	R	15°	0.100	CH

CONSOLIDATION DATA

- - (UC) UNCONFINED COMPRESSION TEST
 - - (Q) UNCONSOLIDATED - UNDRAINED SHEAR TEST
 - ▲ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST
 - - (S) CONSOLIDATED - DRAINED SHEAR TEST

BORINGS WERE TAKEN WITH A 5 INCH DIAMETER
 STEEL TUBE PISTON TYPE SAMPLER
 FOR SOIL BORING LEGEND SEE PLATE A
 FOR LOCATION OF BORINGS SEE PLATE 13

B-13 CONSOLIDATED - DRILLED SHEAR TEST
BOREHOLE HOLE TAKEN WITH 3.5 INCH DIAMETER

BORINGS WERE TAKEN WITH A 5 INCH DIAMETER

STEEL TUBE PISTON TYPE SAMPLER

FOR SOIL BORING LEGEND SEE PLATE A

FOR SITE BORING LEGEND SEE PLATE 8
FOR LOCATION OF BORINGS SEE PLATE 13

FOR LOCATION OF BORINGS SEE PLATE 13

For more information about the study, please contact Dr. Michael J. Hwang at (319) 356-4530 or via email at mhwang@uiowa.edu.

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MISSISSIPPI RIVER LEVEES AND FLOODS

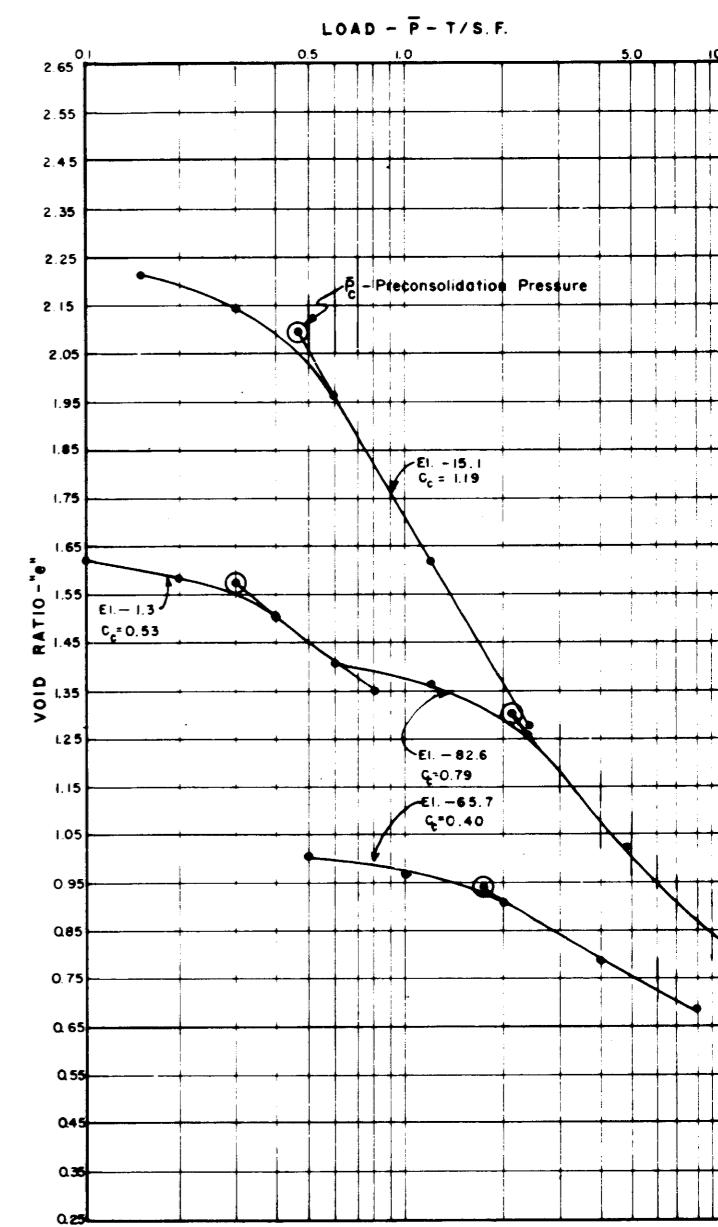
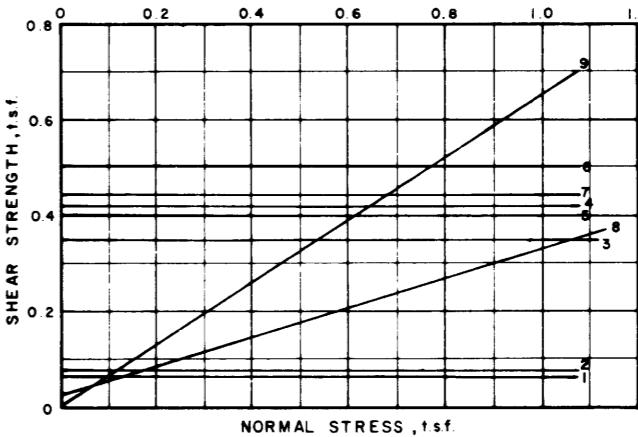
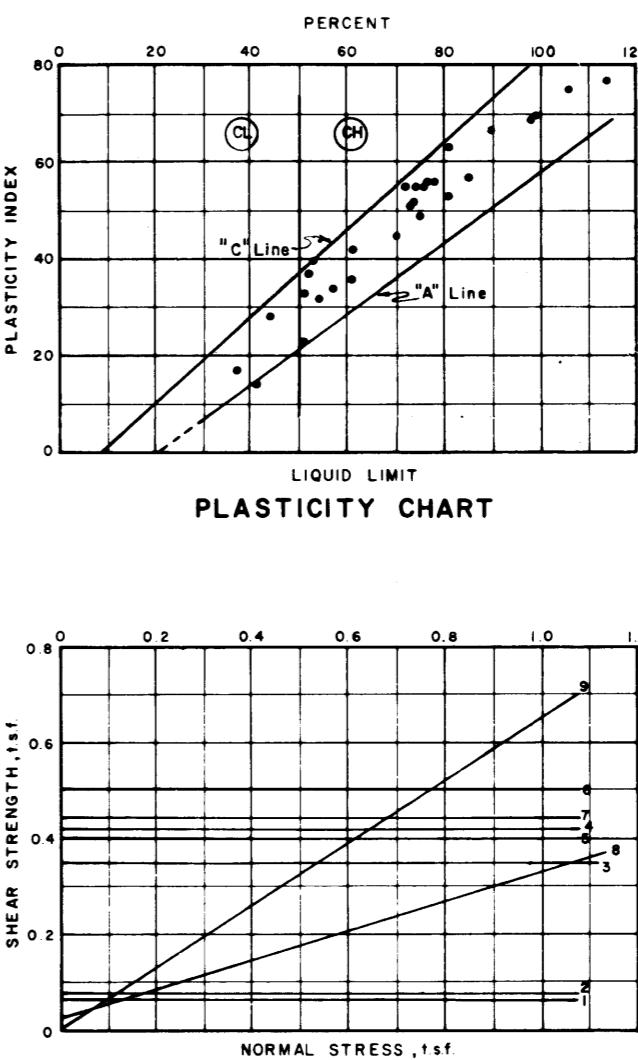
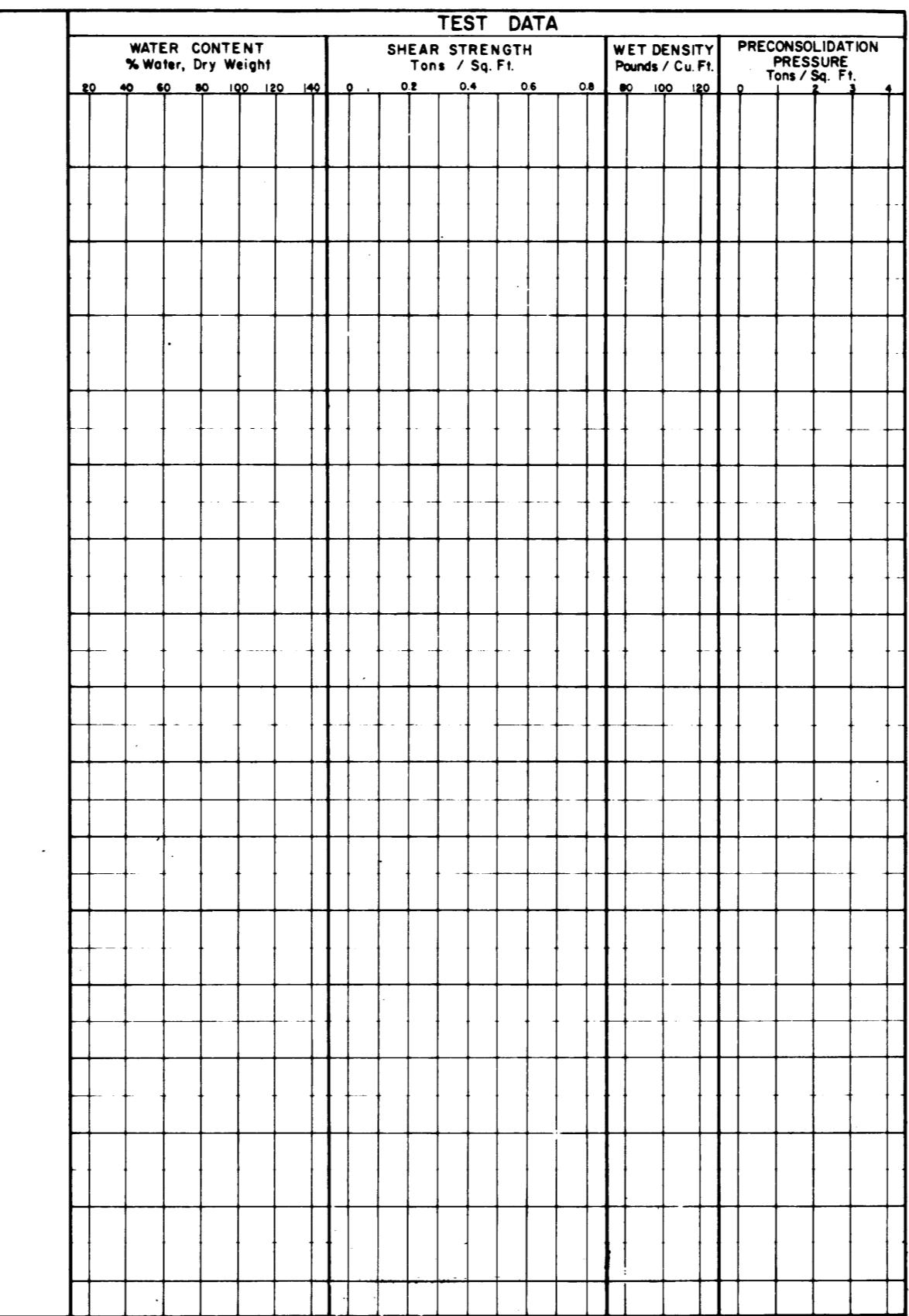
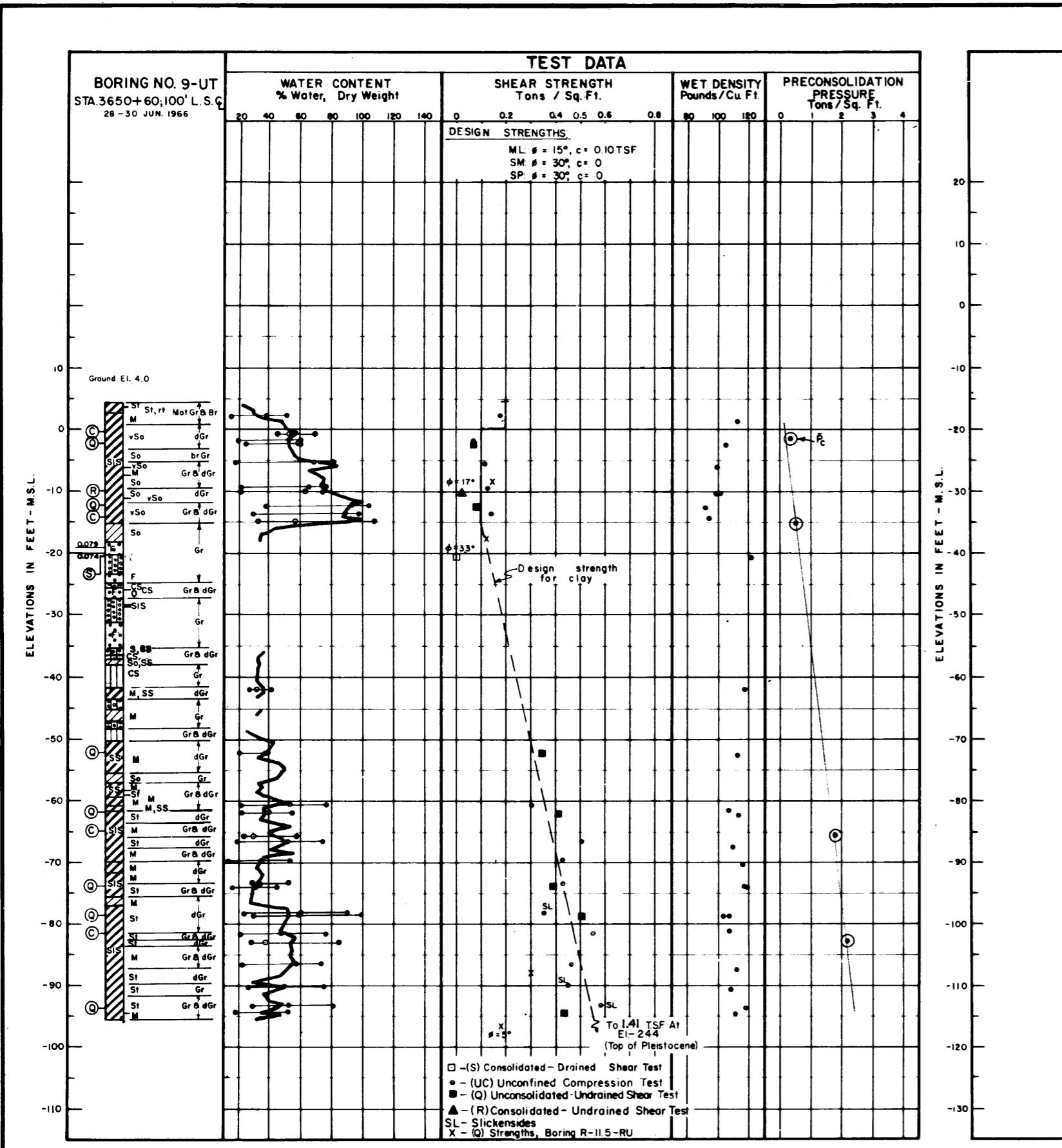
MILE 66 TO MILE 10

OIL REPORT - PART I

WEST BANK

BORINGS

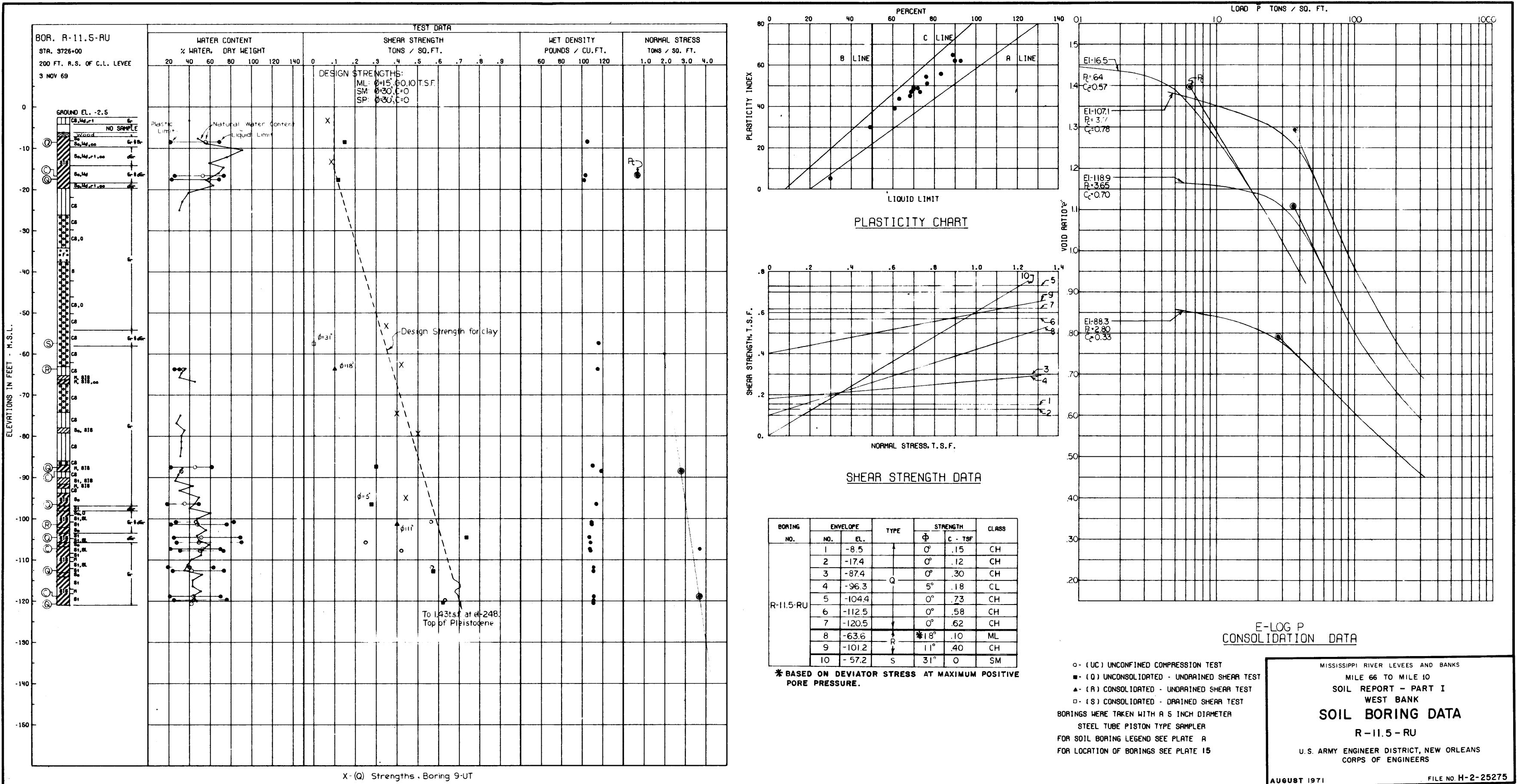
S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS

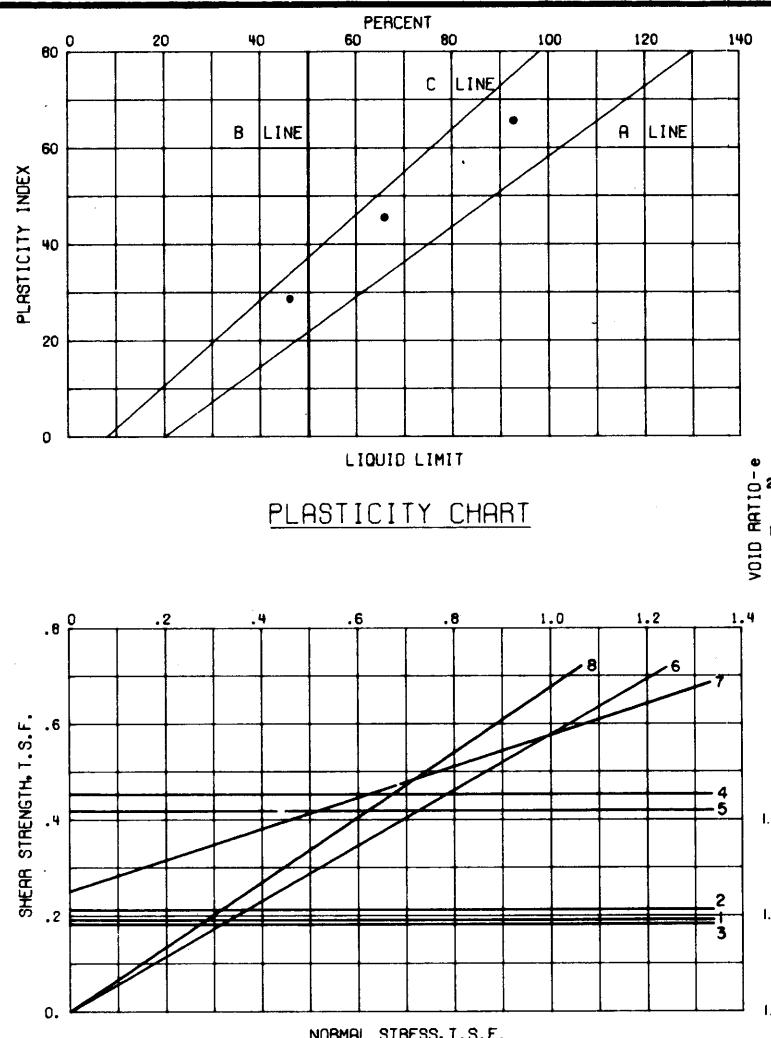
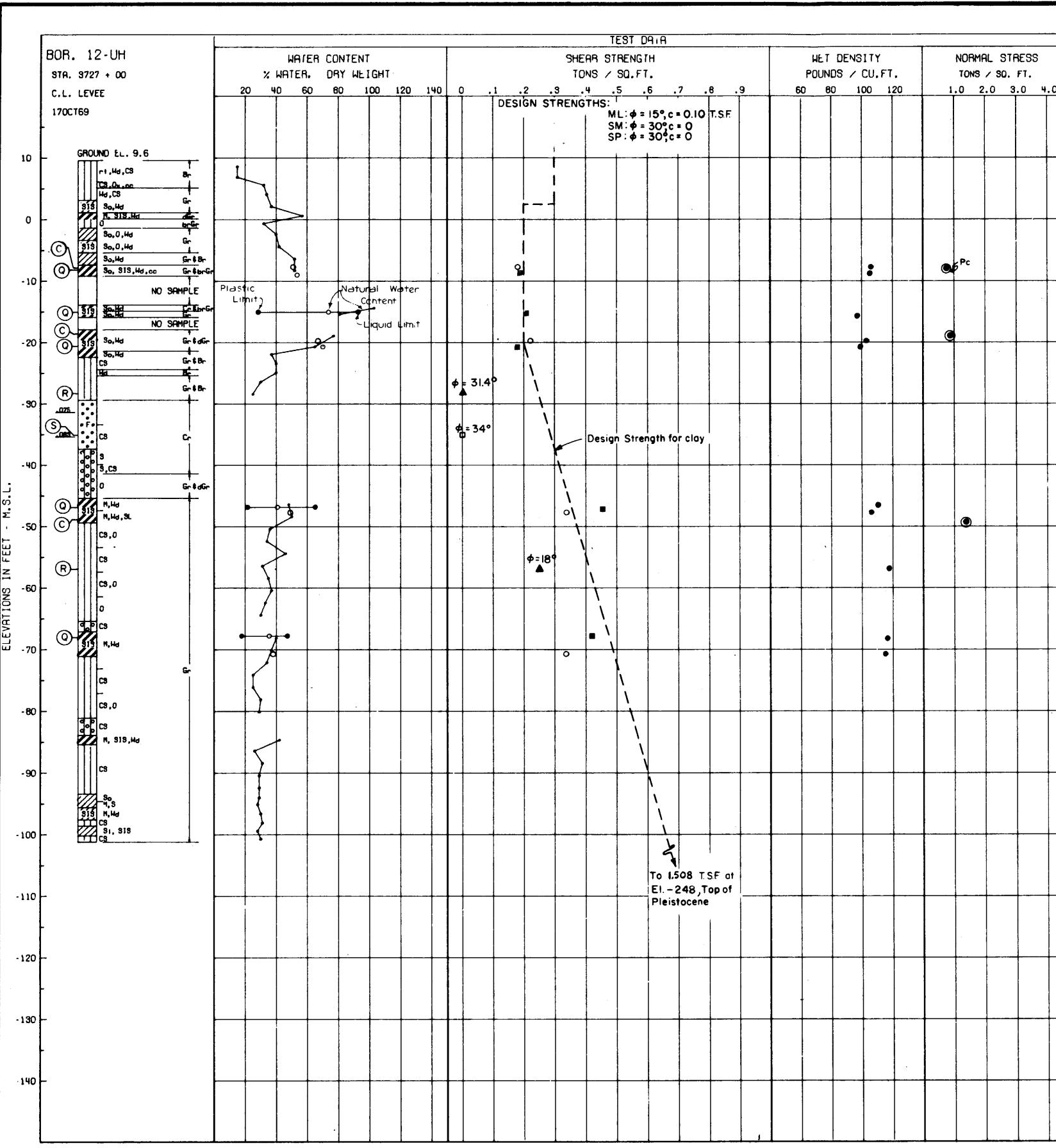


BORING NO.	ENVELOPE NO.	EL.	TYPE	STRENGTH ϕ^* (t.s.f.)	CLASS
9-UT	1	- 3.1		0 0.07	CH
	2	- 13.2		0 0.08	CH
	3	- 53.1		0 0.35	CL
	4	- 62.7	Q	0 0.42	CH
	5	- 74.6		0 0.40	CL
	6	- 79.3		0 0.50	CH
	7	- 94.8		0 0.44	CH
	8	- 10.8	R	17 0.02	CH
	9	- 21.3	S	33 0.00	SM

SHEAR STRENGTH DATA

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT - PART I
WEST BANK
SOIL BORING DATA
9-UT
STA. 3650+60
U. S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS





SHEAR STRENGTH DATA

BORING NO.	ENVELOPE		TYPE	STRENGTH		CLASS
	NO.	EL.		Φ	C - TSF	
12- UH	1	-8.7	Q	0	0.190	CH
	2	-15.1		0	0.210	CH
	3	-20.7		0	0.180	CH
	4	-47.1		0	0.450	CH
	5	-67.8		0	0.420	CL
	6	-28.1	R*	31.4°	0.0	SM
	7	-56.9		18°	0.250	ML
	8	-35.1	S	34°	0.0	SP

*BASED ON DEVIATOR STRESS AT MAXIMUM
POSITIVE PORE PRESSURE

- (UC) UNCONFINED COMPRESSION TEST
 - (Q) UNCONSOLIDATED - UNDRAINED SHEAR TEST
 - (B) CONSOLIDATED - UNDRAINED SHEAR TEST

□ - (S) CONSOLIDATED - DRILLED SHEAR TEST
BORINGS WERE TAKEN WITH A 5 INCH DIAMETER
STEEL TUBE PISTON TYPE SAMPLER
FOR SOIL BORING LEGEND SEE PLATE A
FOR LOCATION OF BORINGS SEE PLATE 15

CONSOLIDATION DATA

MISSISSIPPI RIVER, LEMEES, AND BANKS

MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10

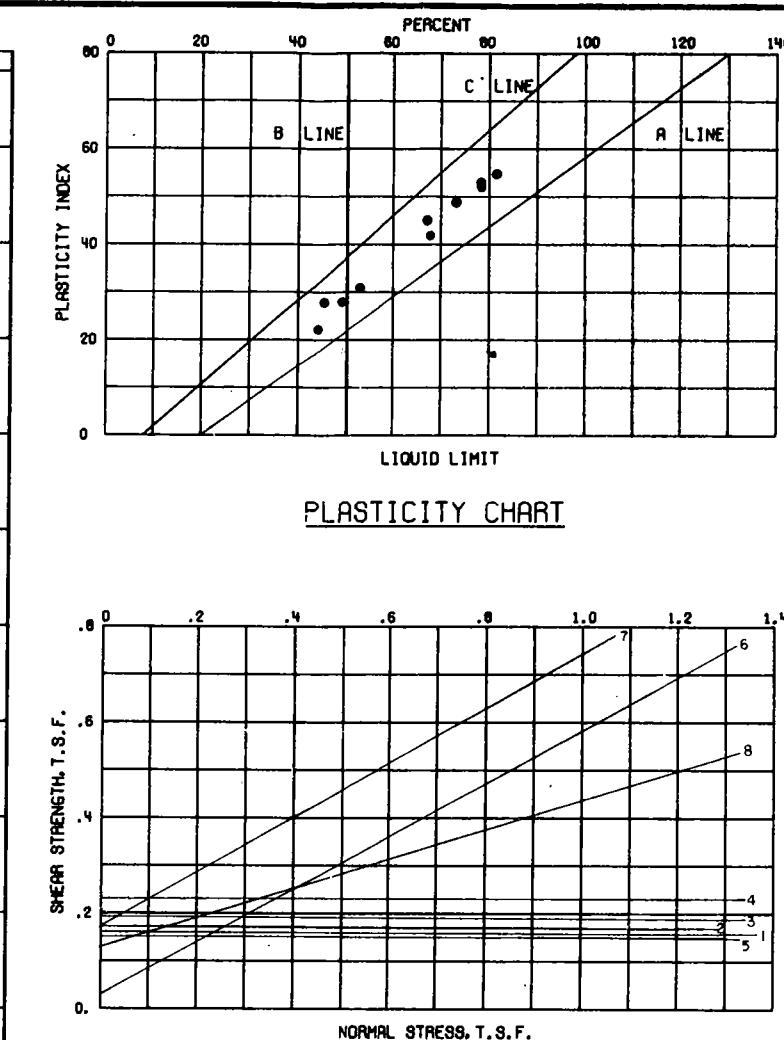
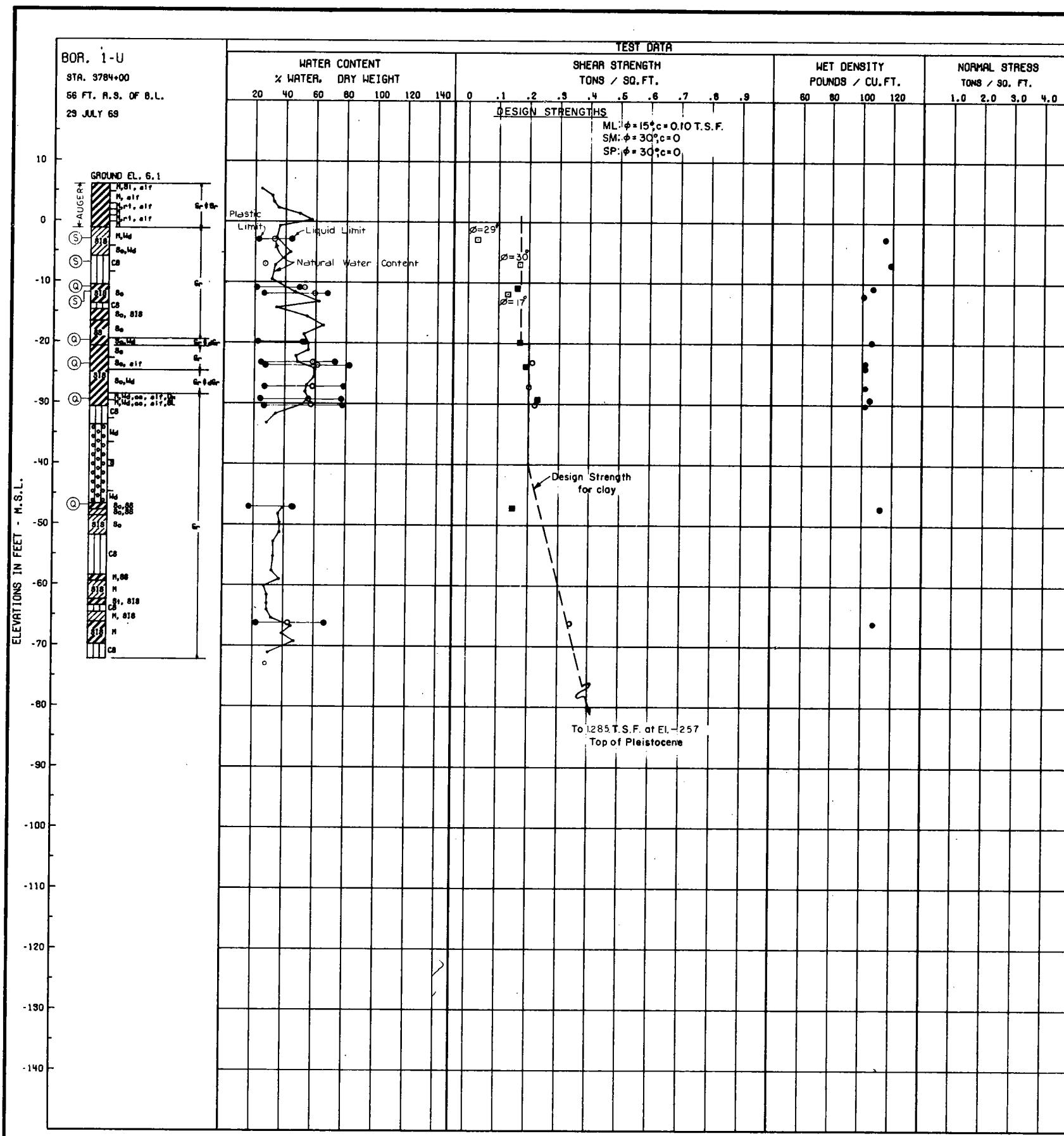
10 MILE 10
ORT - BART

WEST BANK

240

**U. S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS**

AUGUST 1971



BORING NO.	ENVELOPE		TYPE	STRENGTH		CLASS
	NO.	EL.		Φ°	C - TSF	
I-U	1	-11.0	Q	0	0.16	CL
	2	-19.9		0	0.17	CH
	3	-23.8		0	0.19	CH
	4	-29.2		0	0.23	CH
	5	-47.0		0	0.15	CL
	6	-3.0	S	29°	0.03	CL
	7	-6.7		30°	0.17	ML
	8	-11.9		17°	0.13	CH

- - (UC) UNCONFINED COMPRESSION TEST
- - (Q) UNCONSOLIDATED - UNDRAINED SHEAR TEST
- ▲ - (R) CONSOLIDATED - UNDRAINED SHEAR TEST
- - (S) CONSOLIDATED - DRAINED SHEAR TEST

BORINGS WERE TAKEN WITH A 5 INCH DIAMETER

STEEL TUBE PISTON TYPE COMBINE

STEEL TUBE PISTON TYPE SAMPLER

FOR SOIL BORING LEGEND SEE PLATE A

FOR LOCATION OF BORINGS SEE PLATE 15

MISSISSIPPI RIVER LEVEES AND BANKS

MILE 66 TO MILE 10

SOIL REPORT - PART I

BANK

SOIL BORING DATA

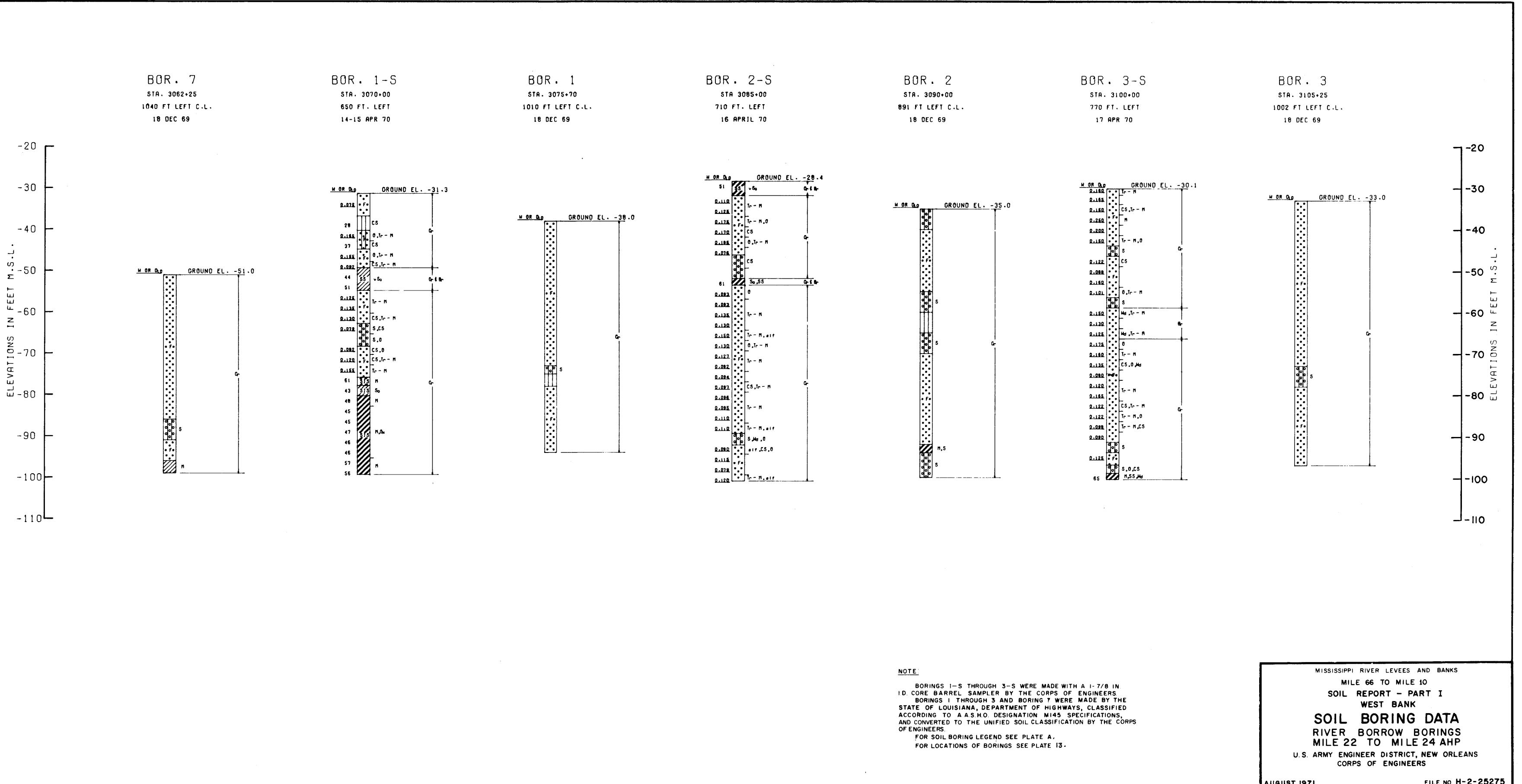
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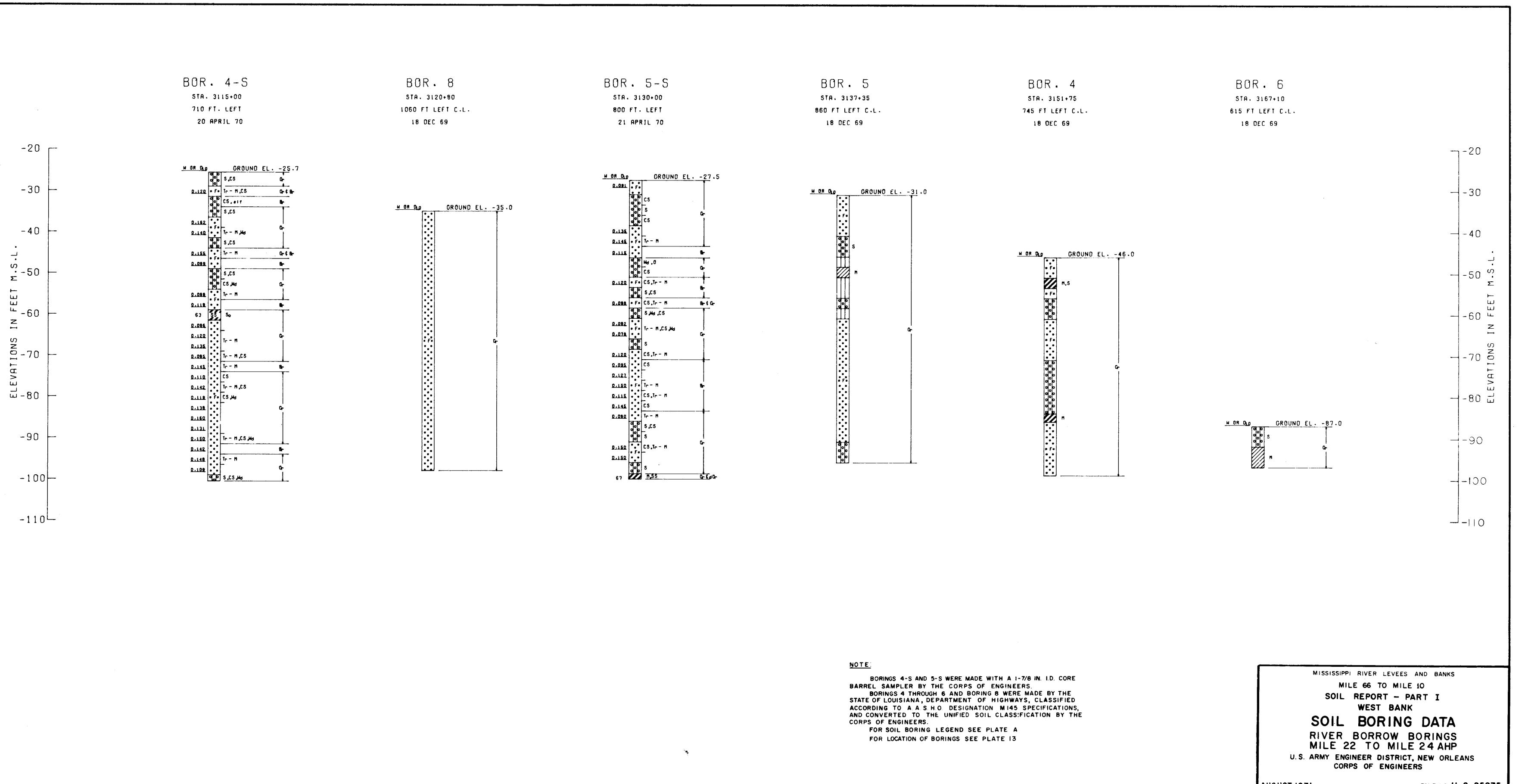
STA. 3784+00

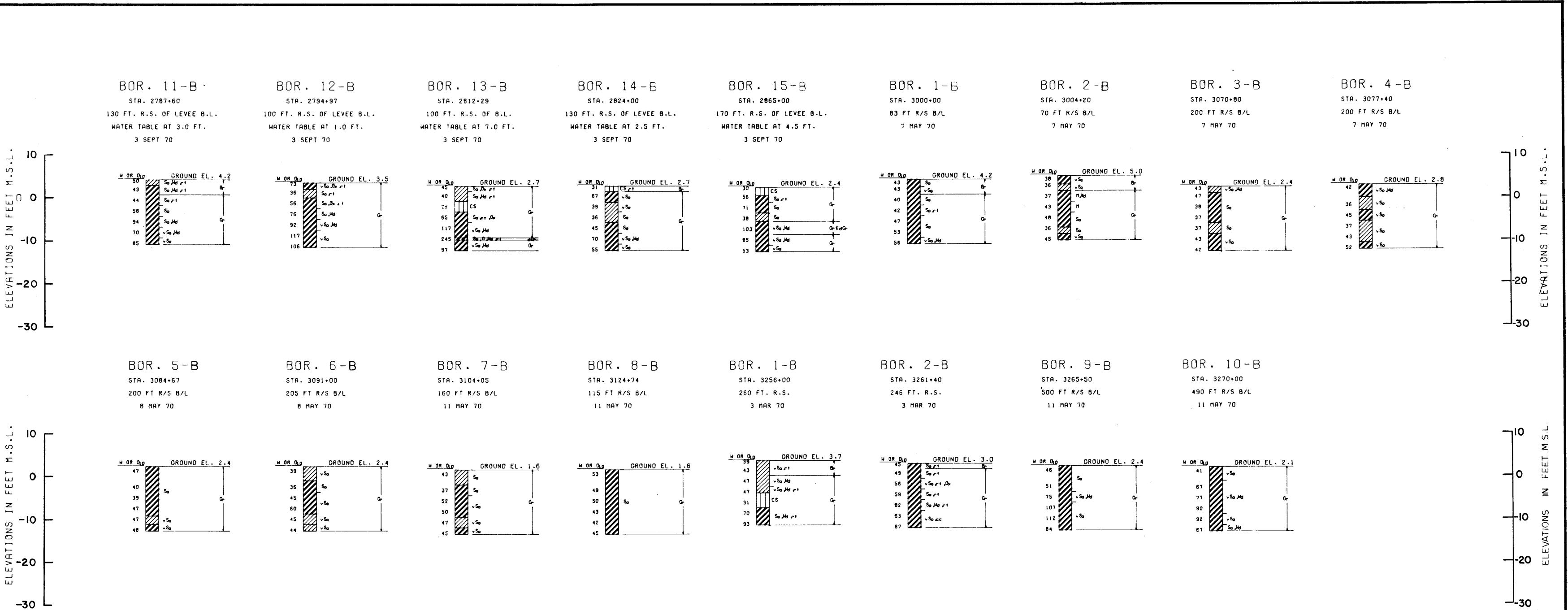
ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS

AUGUST 1971

FILE NO. H-2-25275







BORINGS SHOWN ON THIS PLATE WERE MADE
WITH A 4-IN. DIAMETER POST HOLE AUGER.

SOIL BORING LEGEND SEE PLATE A
LOCATION OF BORINGS SEE PLATES II THRU 13

MISSISSIPPI RIVER LEVEES AND BANKS

MILE 66 TO MILE 10

BORT - BART

WEST BANK

THE BURING DATA

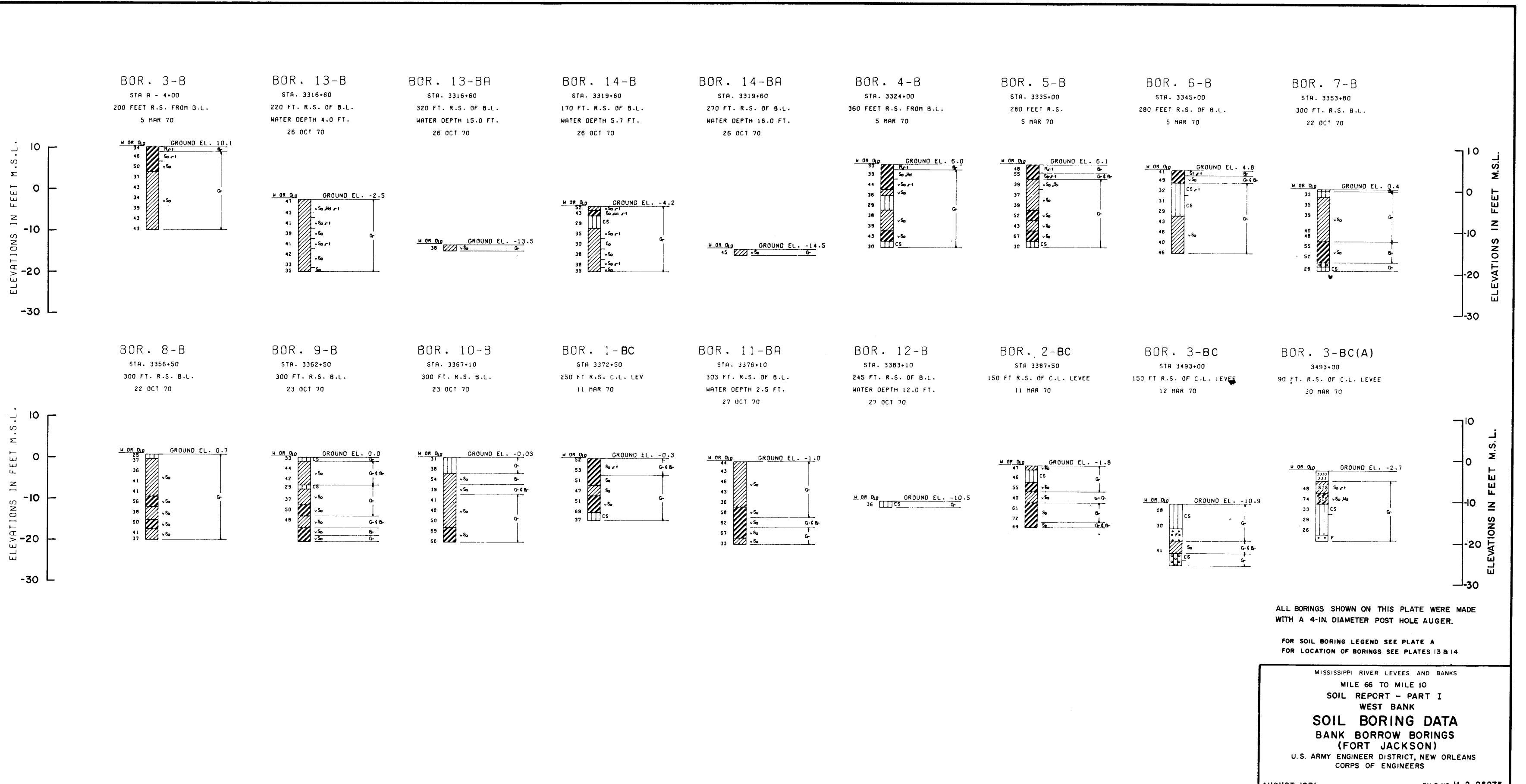
ANK BORROW BORINGS

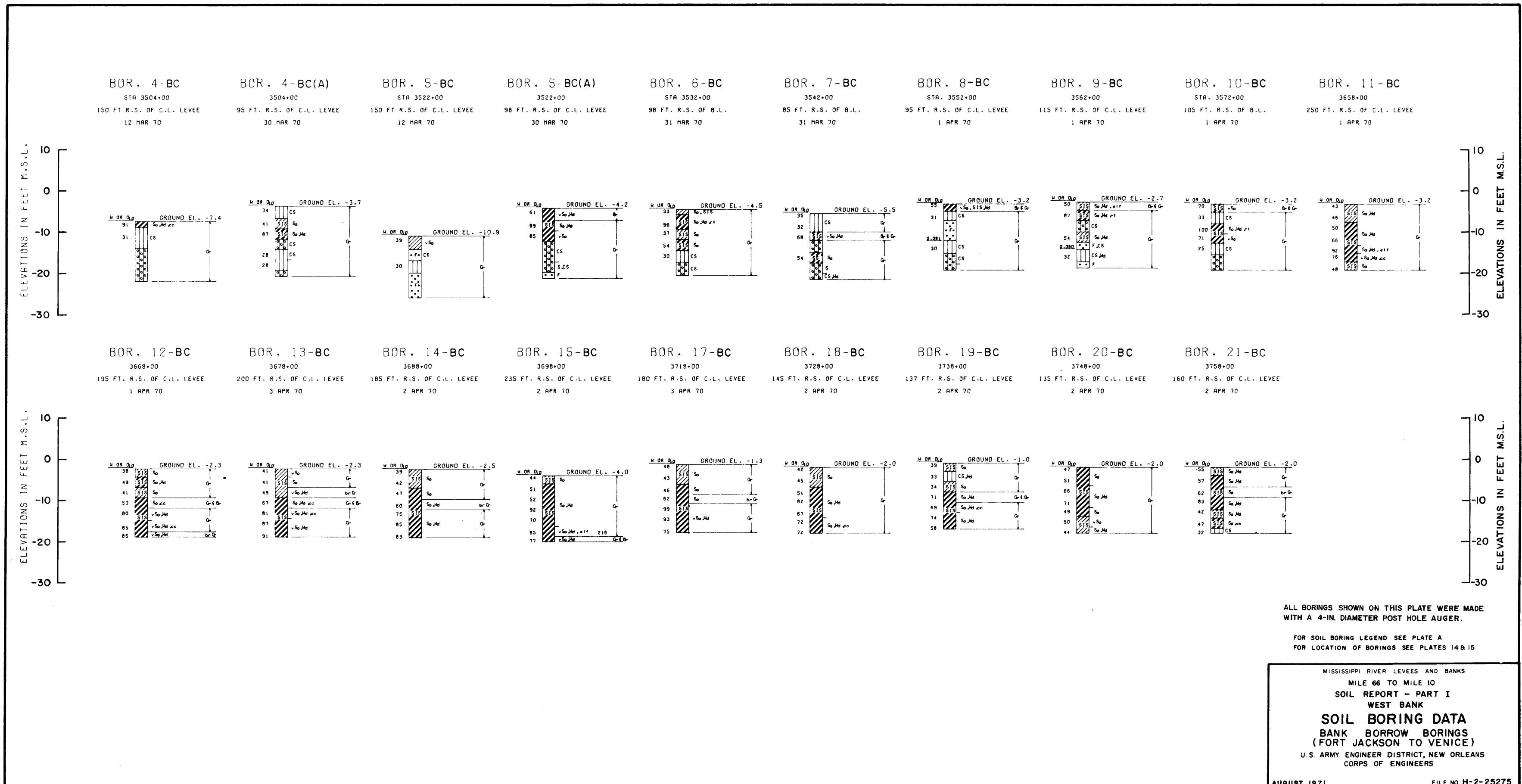
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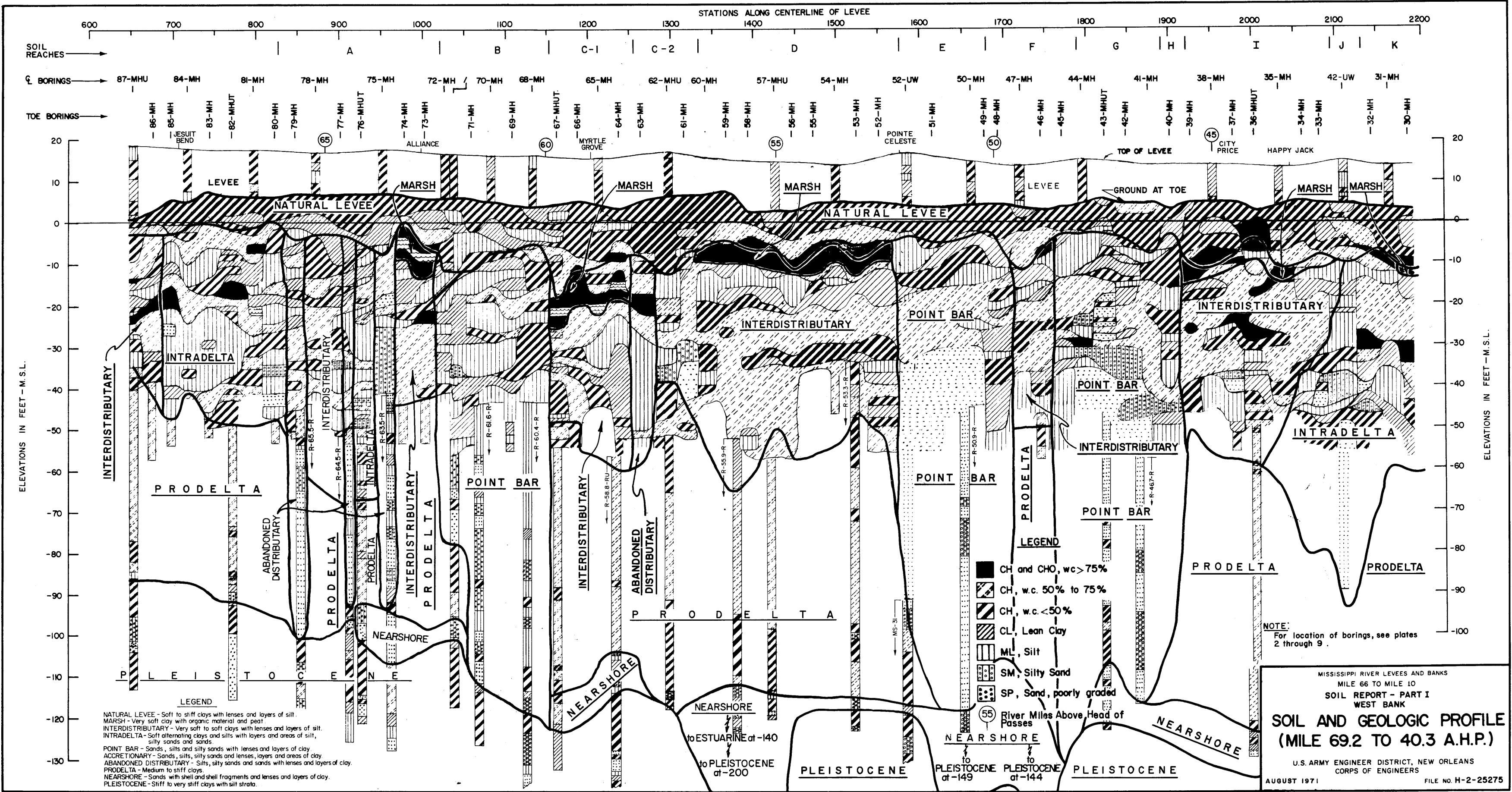
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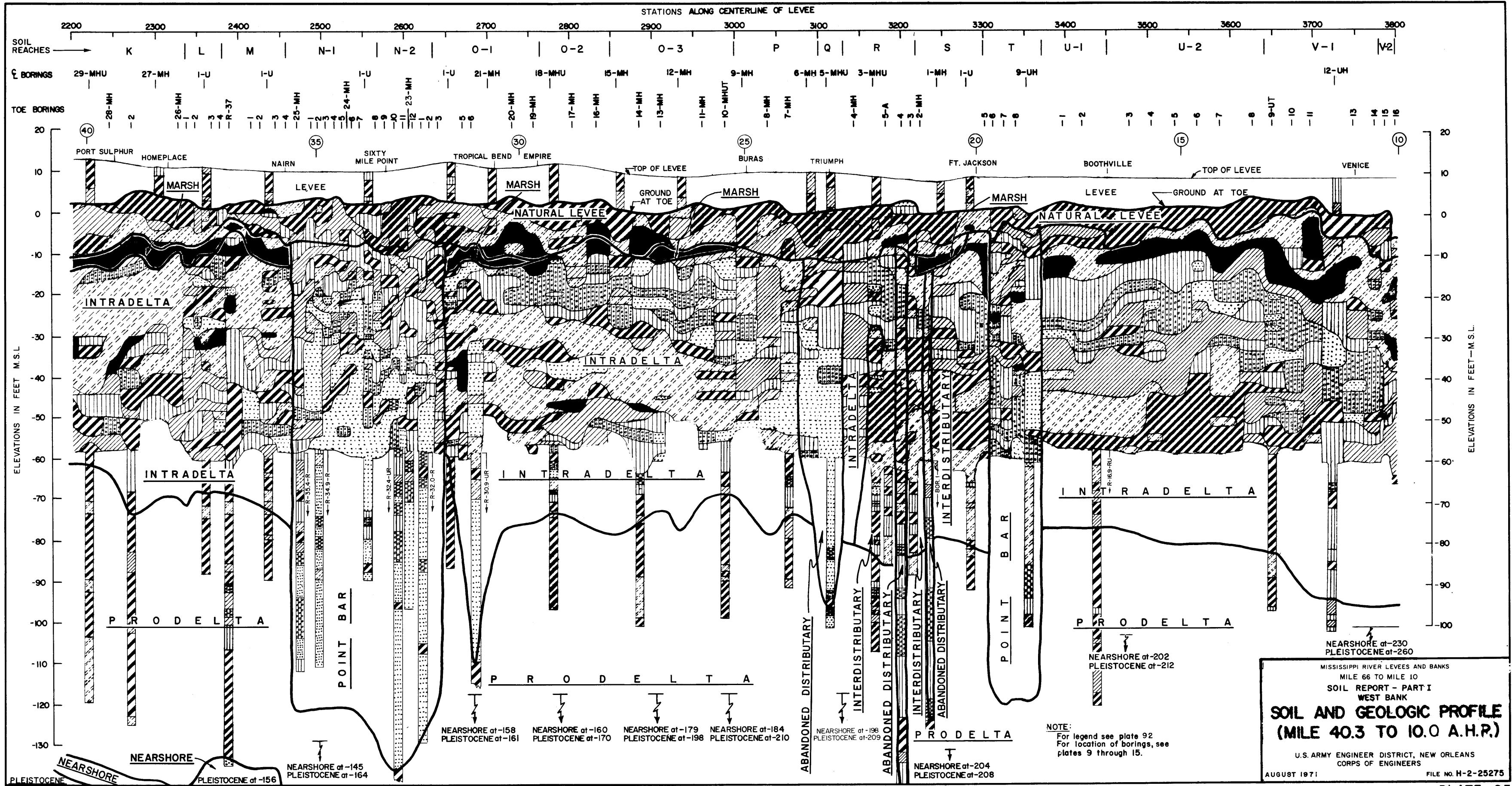
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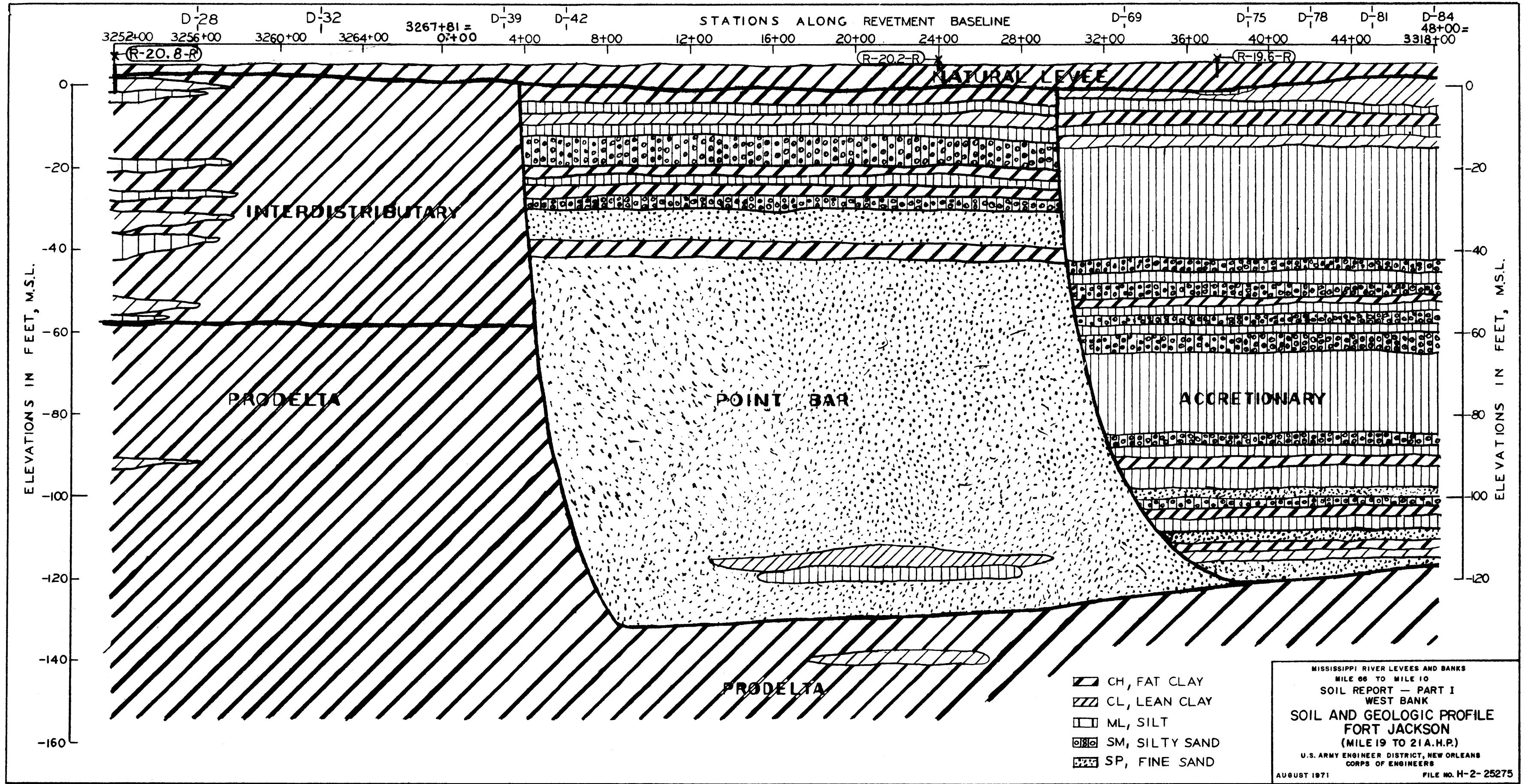
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MISSISSIPPI RIVER LEVEES AND BANKS
MILE 66 TO MILE 10
SOIL REPORT — PART I
WEST BANK
IL AND GEOLOGIC PROFILE
FORT JACKSON
(MILE 19 TO 21 A.H.P.)

**U. S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS**

W E U S T 1971

2-25275

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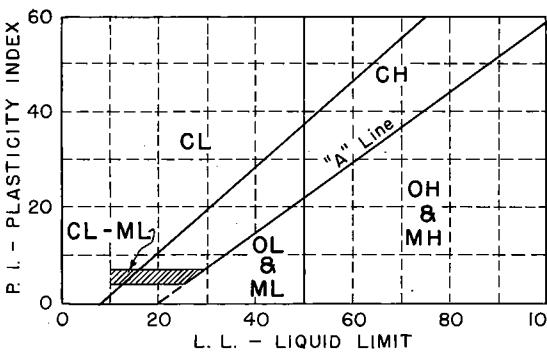
UNIFIED SOIL CLASSIFICATION

MAJOR DIVISION	TYPE	LETTER SYMBOL	TYPICAL NAMES
COARSE - GRAINED SOILS More than half of material is larger than No. 200 sieve size.	CLEAN GRAVEL (Little or No Fines)	GW	GRAVEL, Well Graded, gravel-sand mixtures, little or no fines
	GRAVELS More than half of coarse fraction is larger than No. 4 sieve size.	GP	GRAVEL, Poorly Graded, gravel-sand mixtures, little or no fines
	GRAVEL WITH FINES (Appreciable Amount of Fines)	GM	SILTY GRAVEL, gravel-sand-silt mixtures
	CLEAN SAND (Little or No Fines)	GC	CLAYEY GRAVEL, gravel-sand-clay mixtures
	SANDS More than half of coarse fraction is smaller than No. 4 sieve size.	SW	SAND, Well-Graded, gravelly sands
	SANDS More than half the material is smaller than No. 200 sieve size.	SP	SAND, Poorly-Graded, gravelly sands
	SANDS WITH FINES (Appreciable Amount of Fines)	SM	SILTY SAND, sand-silt mixtures
		SC	CLAYEY SAND, sand-clay mixtures
	SILTS AND CLAYS (Liquid Limit < 50)	ML	SILT & very fine sand, silty or clayey fine sand or clayey silt with slight plasticity
	SILTS AND CLAYS (Liquid Limit > 50)	CL	LEAN CLAY, Sandy Clay, Silty Clay, of low to medium plasticity
FINE - GRAINED SOILS More than half the material is smaller than No. 200 sieve size.	OL	OL	ORGANIC SILTS and organic silty clays of low plasticity
	MH	MH	SILT, fine sandy or silty soil with high plasticity
	CH	CH	FAT CLAY, inorganic clay of high plasticity
	OH	OH	ORGANIC CLAYS of medium to high plasticity, organic silts
	Pt	Pt	PEAT, and other highly organic soil
	WOOD	Wd	WOOD
	SHELLS	SI	SHELLS
	NO SAMPLE		

NOTE: Soils possessing characteristics of two groups are designated by combinations of group symbols

DESCRIPTIVE SYMBOLS

COLOR	
COLOR	SYMBOL
TAN	T
YELLOW	Y
RED	R
BLACK	BK
GRAY	Gr
LIGHT GRAY	lGr
DARK GRAY	dGr
BROWN	Br
LIGHT BROWN	IBr
DARK BROWN	dBri
BROWNISH-GRAY	br Gr
GRAYISH-BROWN	gy Br
GREENISH-GRAY	gn Gr
GRAYISH-GREEN	gy Gn
GREEN	Gn
BLUE	Bl
BLUE-GREEN	Bi Gn
WHITE	Wh
MOTTLED	Mot



CONSISTENCY FOR COHESIVE SOILS		MODIFICATIONS
CONSISTENCY	COHESION IN LBS./SQ.FT. FROM UNCONFINED COMPRESSION TEST	MODIFICATION
VERY SOFT	< 250	vSo
SOFT	250 - 500	So
MEDIUM	500 - 1000	M
STIFF	1000 - 2000	St
VERY STIFF	2000 - 4000	vSt
HARD	> 4000	H

NOTES:	
FIGURES TO LEFT OF BORING UNDER COLUMN "W OR D ₁₀ "	
Are natural water contents in percent dry weight	
When underlined denotes D ₁₀ size in mm *	
FIGURES TO LEFT OF BORING UNDER COLUMNS "LL" AND "PL"	
Are liquid and plastic limits, respectively	
SYMBOLS TO LEFT OF BORING	
<u>V</u> Ground-water surface and date observed	
(C) Denotes location of consolidation test **	
(S) Denotes location of consolidated-drained direct shear test **	
(R) Denotes location of consolidated-undrained triaxial compression test **	
(Q) Denotes location of unconsolidated-undrained triaxial compression test **	
(T) Denotes location of sample subjected to consolidation test and each of the above three types of shear tests **	
FW Denotes free water encountered in boring or sample	
FIGURES TO RIGHT OF BORING	
Are values of cohesion in lbs./sq.ft. from unconfined compression tests	
In parenthesis are driving resistances in blows per foot determined with a standard split spoon sampler (1 1/8" I.D., 2" O.D.) and a 140 lb. driving hammer with a 30" drop	
Where underlined with a solid line denotes laboratory permeability in centimeters per second of undisturbed sample	
Where underlined with a dashed line denotes laboratory permeability in centimeters per second of sample remoulded to the estimated natural void ratio	

* The D₁₀ size of a soil is the grain diameter in millimeters of which 10% of the soil is finer, and 90% coarser than size D₁₀.

** Results of these tests are available for inspection in the U.S. Army Engineer District Office, if these symbols appear beside the boring logs on the drawings.

GENERAL NOTES:

While the borings are representative of subsurface conditions at their respective locations and for their respective vertical reaches, local variations characteristic of the subsurface materials of the region are anticipated and, if encountered, such variations will not be considered as differing materially within the purview of clause 4 of the contract.

Ground-water elevations shown on the boring logs represent ground-water surfaces encountered on the dates shown. Absence of water surface data on certain borings implies that no ground-water data is available, but does not necessarily mean that ground water will not be encountered at the locations or within the vertical reaches of these borings.

Consistency of cohesive soils shown on the boring logs is based on driller's log and visual examination and is approximate, except within those vertical reaches of the borings where shear strengths from unconfined compression tests are shown.

SOIL BORING LEGEND

3	5-3-71	ADDED UPPER LIMIT LINE (P.I.=0.9(LL-8)) ON PLASTICITY CHART	LMVED-G LETTER D'D 29 APRIL 1971
2	6-8-64	SYMBOL FW, NOTE REVISED	ORAL FROM LMV.G. 5 JUNE 1964
1	9-17-63	1ST. PAR. OF GENERAL NOTES REVISED	L.M.V.D. MULTIPLE LETTER, DATED 5 SEPT., 1963

U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS
CORPS OF ENGINEERS
FILE NO. H-2-21800