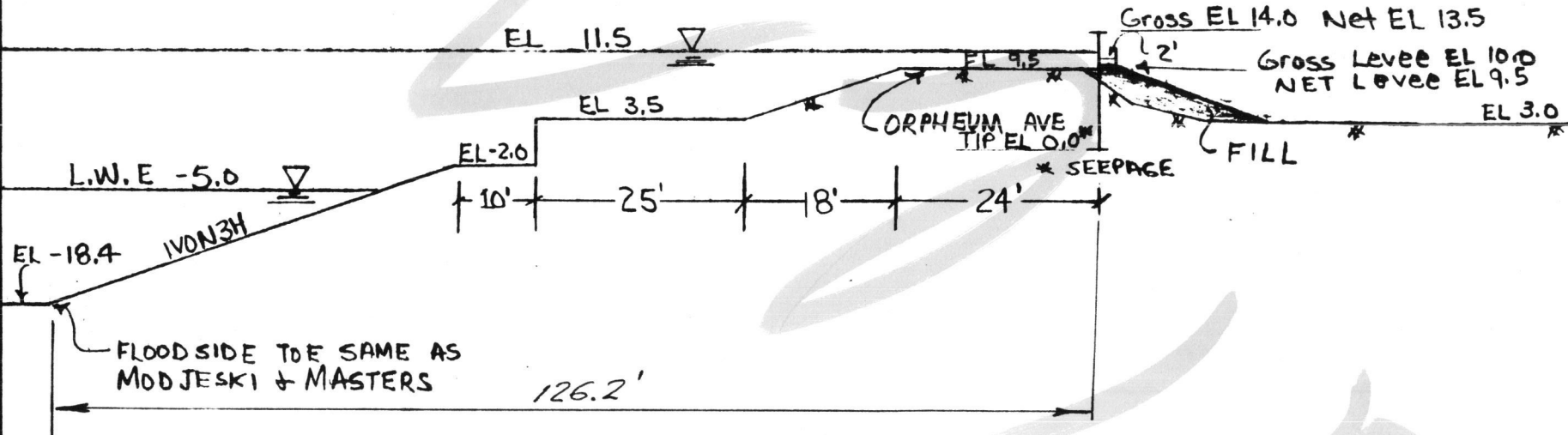


STA 549+22 TO STA 552+70
JEFFERSON SIDE



SCALE : 1" = 20'

ELEVATION IN FEET NGVD

Q FILES	FS	S.W.L	CASE
Q549JA	1.0	11.5	S
Q549JB	1.5	11.5	S
Q549JD	1.0	13.5	Q

Incls 1-8 M+M toe is as shown in 1987 plans

PROJECT 17th St Offfall Canal	PAGE	OF	COMPUTED BY	DATE
SUBJECT STA 549+22 TO STA 552+70 Jefferson			EV	June 87
	CHECKED BY			DATE

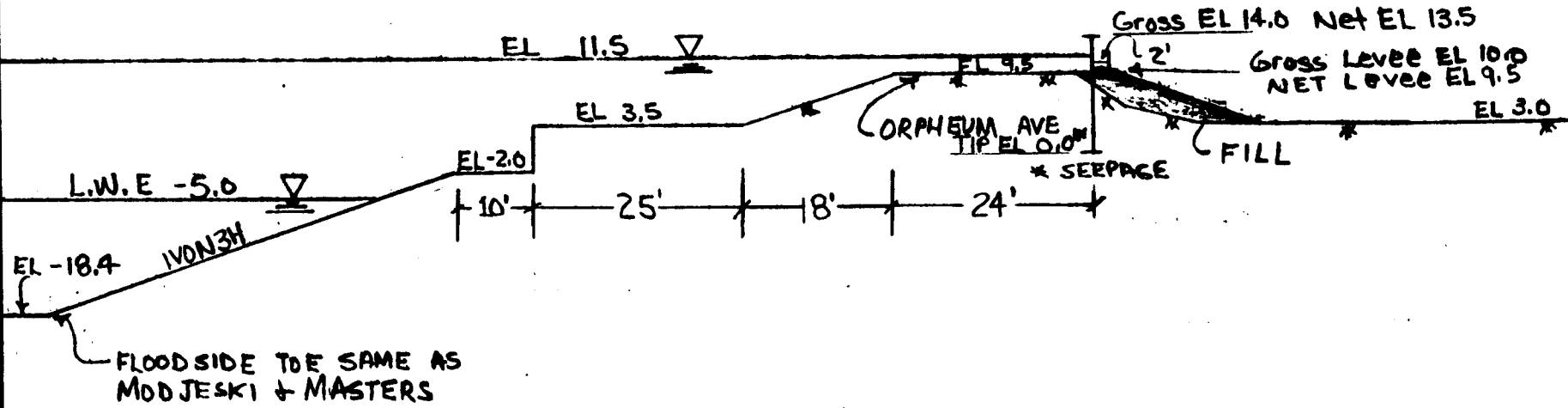
COMPUTATION SHEET

(A0000671)

REVISED 4/88

Encl :

STA 549+22 TO STA 552+70
JEFFERSON SIDE



SCALE : 1" = 20'

ELEVATION IN FEET NGVD

Q549J0

Q FILES	FS	S.W.L	CASE
Q549JA	1.0	11.5	S
Q549JB	1.5	11.5	S
Q549JD	1.0	13.5	Q

ADVANCE

SUBJECT TO CORRECTION

DRW 22C

PROJECT 17th St Offfall Canal	PAGE	OF	COMPUTED BY EJV	DATE June 87
SUBJECT STA 549+22 TO STA 552+70	CHECKED BY			

REVISED - 4/88

Q549Ji

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH STR-Q549JA-SWL=11.5-S CASE-FS=1.0
=1.

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.00 INCHES AND OCCURS AT MEMBER COORDINATE
13.50 FT.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE(FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
4.94	POINT LD	0.00 LBF
4.94	COUPLE	-28.44 LBF-FT

FRUD 13
OK

INPUTTED LOADS

DISTANCE FROM REFERENCE(FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
11.50	CONTN LD	0.00 LBF/SQ FT
10.50	CONTN LD	62.50 LBF/SQ FT
9.50	CONTN LD	125.00 LBF/SQ FT
9.50	CONTN LD	125.00 LBF/SQ FT
8.75	CONTN LD	0.00 LBF/SQ FT
8.50	CONTN LD	-42.78 LBF/SQ FT
7.50	CONTN LD	-153.69 LBF/SQ FT
6.50	CONTN LD	-237.37 LBF/SQ FT
6.06	CONTN LD	-273.97 LBF/SQ FT
5.75	CONTN LD	0.00 LBF/SQ FT
4.94	CONTN LD	698.71 LBF/SQ FT
4.94	CONTN LD	0.00 LBF/SQ FT

Z-22 PROPERTIES ARE AS FOLLOWS.

MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
CROSS SECTIONAL AREA= 1.84 SQ IN.
ELASTIC MODULUS= 29000000. LBF/SQ IN.
DEFLECTION REFERENCE IS AT 0.000

58
 59
 60 THE MAXIMUM BENDING MOMENT IS 383.00 LBF-FT AND OCCURS AT 7.13
 61 WHICH HAS THE SHEAR FORCE OF 5.15 LBF.
 62

65	DEFLECTION FROM TANG. THRU DEFLE REFERENCE				
66	DISTANCE (FEET)	SHEAR FOR (LBF)	SHEAR STR (LBF/SQIN)	BENDING MOM (LBF-FT)	(INCHES)
68	13.500	0.0	0.0	0.0	0.0050
69	13.499	0.0	0.0	0.0	0.0050
70	13.000	0.0	0.0	0.0	0.0046
71	12.000	0.0	0.0	0.0	0.0038
72	11.000	7.8	4.2	1.3	0.0029
73	10.000	70.3	38.2	35.2	0.0021
74	9.000	166.5	90.5	158.0	0.0013
75	8.755	171.6	93.2	199.6	0.0012
76	8.000	130.9	71.1	318.3	0.0007
77	7.129	5.1	2.8	383.0	0.0003
78	7.000	-19.4	-10.6	382.1	0.0002
79	6.000	-254.9	-138.6	251.0	0.0000
80	5.746	-283.0	-153.8	181.5	0.0000
81	5.744	-283.0	-153.8	181.0	0.0000
82	5.000	-43.5	-23.7	29.9	0.0000
83	4.936	-0.7	-0.4	28.4	0.0000
84	4.934	0.0	0.0	0.0	0.0000
85	4.000	0.0	0.0	0.0	0.0000
86	3.000	0.0	0.0	0.0	0.0000
87	2.000	0.0	0.0	0.0	0.0000
88	1.000	0.0	0.0	0.0	0.0000
89	0.001	0.0	0.0	0.0	0.0000
90	0.000	0.0	0.0	0.0	0.0000

91
 92
 93
 94 *RUN COMPLETED*
 95

EOT..
 LI Q549J2

1
 2 BEAMS (SHEAR, MOMENT, DEFLECTION)
 3

4
 5 17TH STR -Q549JB-SWL=11.5-S CASE-FS=1.5
 6 =1.
 7

8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
 9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
 10 AS COUNTERCLOCKWISE.
 11

12
 13 THE MAXIMUM DEFLECTION IS 0.02 INCHES AND OCCURS AT MEMBER COORDINATE
 14 13.50 FT.
 15

16
 17
 18 Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19
 20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.
 21
 22

23 CALCULATED EXTERNAL LOADS

24
 25

DISTANCE FROM REFERENCE(FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
2.38	POINT LD	0.00 LBF
2.38	COUPLE	-11.82 LBF-FT

26
 27
 28
 29
 30

31
 32 INPUTTED LOADS

33
 34

DISTANCE FROM REFERENCE(FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
11.50	CONTN LD	0.00 LBF/SQ FT
10.50	CONTN LD	62.50 LBF/SQ FT
9.50	CONTN LD	125.00 LBF/SQ FT
9.50	CONTN LD	125.00 LBF/SQ FT
8.50	CONTN LD	22.35 LBF/SQ FT
8.20	CONTN LD	0.00 LBF/SQ FT
7.50	CONTN LD	-51.62 LBF/SQ FT
6.50	CONTN LD	-95.44 LBF/SQ FT
5.50	CONTN LD	-139.26 LBF/SQ FT
4.50	CONTN LD	-183.07 LBF/SQ FT
3.63	CONTN LD	-221.00 LBF/SQ FT
3.36	CONTN LD	0.00 LBF/SQ FT
2.38	CONTN LD	769.61 LBF/SQ FT
2.38	CONTN LD	0.00 LBF/SQ FT

35
 36
 37
 38
 39
 40
 41
 42
 43
 44
 45
 46
 47
 48
 49
 50
 51

52
 53 Z-22 PROPERTIES ARE AS FOLLOWS.

54
 55
 56 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
 57 CROSS SECTIONAL AREA= 1.84 SQ IN.
 58 ELASTIC MODULUS= 29000000. LBF/SQ IN.
 59 DEFLECTION REFERENCE IS AT 0.000
 60

61
 62 THE MAXIMUM BENDING MOMENT IS 657.72 LBF-FT AND OCCURS AT 5.59
 63 WHICH HAS THE SHEAR FORCE OF 5.31 LBF.
 64

65
 66
 67

DISTANCE (FEET)	SHEAR FOR (LBF)	SHEAR STR (LBF/SQIN)	BENDING MOM (LBF-FT)	DEFLECTION FROM TANG. THRU DEFLE REFERENCE (INCHES)
70 13.500	0.0	0.0	0.0	0.0156
71 13.499	0.0	0.0	0.0	0.0156
72 13.000	0.0	0.0	0.0	0.0146
73 12.000	0.0	0.0	0.0	0.0125
74 11.000	7.8	4.2	1.3	0.0104
75 10.000	70.3	38.2	35.2	0.0083

76	9.000	174.7	94.9	159.3	0.0063
77	8.198	202.1	109.8	314.4	0.0047
78	8.000	200.6	109.0	354.3	0.0044
79	7.000	152.7	83.0	535.9	0.0027
80	6.000	57.3	31.1	644.6	0.0014
81	5.588	5.3	2.9	657.7	0.0010
82	5.000	-81.9	-44.5	635.9	0.0005
83	4.000	-265.0	-144.0	466.1	0.0001
84	3.356	-373.7	-203.1	254.2	0.0000
85	3.354	-373.7	-203.1	253.4	0.0000
86	3.000	-323.6	-175.9	126.9	0.0000
87	2.385	-0.8	-0.4	11.8	0.0000
88	2.383	0.0	0.0	0.0	0.0000
89	2.000	0.0	0.0	0.0	0.0000
90	1.000	0.0	0.0	0.0	0.0000
91	0.001	0.0	0.0	0.0	0.0000
92	0.000	0.0	0.0	0.0	0.0000

93
94
95
96

RUN COMPLETED

97
EOT..

LI Q549J3

1

2 BEAMS (SHEAR, MOMENT, DEFLECTION)

3

4

5 17TH STR.-Q549JD-SWL=13.5-Q CASE-FS=1.0

6 =1.

7

8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
10 AS COUNTERCLOCKWISE.

11

12

13 THE MAXIMUM DEFLECTION IS 0.01 INCHES AND OCCURS AT MEMBER COORDINATE
14 13.50 FT.

15

16

17

18 Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19

20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21

22

23 CALCULATED EXTERNAL LOADS

24

25 DISTANCE FROM TYPE OF MAGNITUDE OF
26 REFERENCE(FT) LOAD LOAD

27

28 6.52 POINT LD 53.40 LBF

29 6.52 COUPLE 1.87 LBF-FT

30

31

32 INPUTTED LOADS

33

34 DISTANCE FROM TYPE OF MAGNITUDE OF

35	REFERENCE (FT)	LOAD	LOAD
36			
37	13.50	CONTN LD	0.00 LBF/SQ FT
38	12.50	CONTN LD	62.50 LBF/SQ FT
39	11.50	CONTN LD	125.00 LBF/SQ FT
40	10.50	CONTN LD	187.50 LBF/SQ FT
41	9.50	CONTN LD	250.00 LBF/SQ FT
42	9.50	CONTN LD	0.00 LBF/SQ FT
43	9.50	CONTN LD	-550.00 LBF/SQ FT
44	8.50	CONTN LD	-597.50 LBF/SQ FT
45	7.66	CONTN LD	-637.29 LBF/SQ FT
46	7.33	CONTN LD	0.00 LBF/SQ FT
47	6.52	CONTN LD	1577.89 LBF/SQ FT
48	6.52	CONTN LD	0.00 LBF/SQ FT

51 Z-22 PROPERTIES ARE AS FOLLOWS.

54 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
 55 CROSS SECTIONAL AREA= 1.84 SQ IN.
 56 ELASTIC MODULUS= 29000000. LBF/SQ IN.
 57 DEFLECTION REFERENCE IS AT 0.000

60 THE MAXIMUM BENDING MOMENT IS 888.29 LBF-FT AND OCCURS AT 8.63
 61 WHICH HAS THE SHEAR FORCE OF 5.52 LBF.

60	DISTANCE (FEET)	SHEAR FOR (LBF)	SHEAR STR (LBF/SQIN)	BENDING MOM (LBF-FT)	DEFLECTION FROM TANG. THRU DEFLE REFERENCE (INCHES)
67	13.500	0.0	0.0	0.0	0.0079
68	13.499	0.0	0.0	0.0	0.0079
70	13.000	7.8	4.2	1.3	0.0070
71	12.000	70.3	38.2	35.2	0.0053
72	11.000	195.3	106.1	162.8	0.0036
73	10.000	382.8	208.1	446.6	0.0020
74	9.500	500.0	271.7	666.7	0.0014
75	9.000	219.1	119.1	846.9	0.0008
76	8.633	5.5	3.0	888.3	0.0005
77	8.000	-378.4	-205.7	771.2	0.0002
78	7.334	-695.8	-378.1	391.0	0.0000
79	7.332	-695.8	-378.1	389.6	0.0000
80	7.000	-588.1	-319.6	170.3	0.0000
81	6.520	-55.0	-29.9	-1.8	0.0000
82	6.518	0.0	0.0	0.0	0.0000
83	6.000	0.0	0.0	0.0	0.0000
84	5.000	0.0	0.0	0.0	0.0000
85	4.000	0.0	0.0	0.0	0.0000
86	3.000	0.0	0.0	0.0	0.0000
87	2.000	0.0	0.0	0.0	0.0000
88	1.000	0.0	0.0	0.0	0.0000
89	0.001	0.0	0.0	0.0	0.0000
90	0.000	0.0	0.0	0.0	0.0000

91
92

93
94 *RUN COMPLETED*

95
EOT..

DRW22C

1 100 1 13.5 0.0 1 0.0 0 -1

2 200 PZ-22

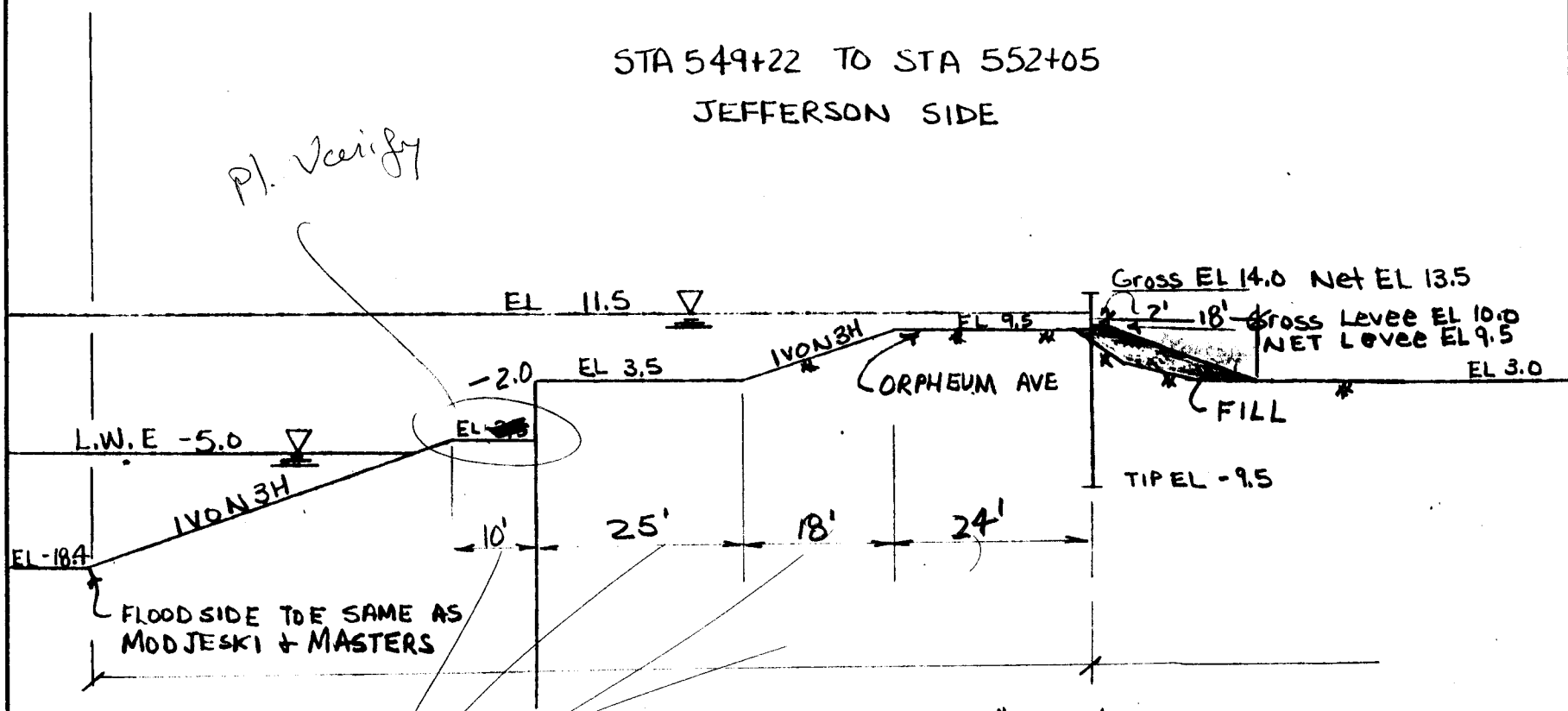
3 300 29000000 1.84 84.38

EOF..
EOT..

0+00 w/L
to 2+43.10 w/L

STA 549+22 TO STA 552+05
JEFFERSON SIDE

Pl. Verify



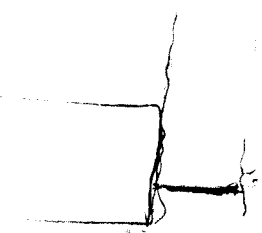
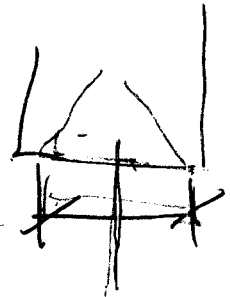
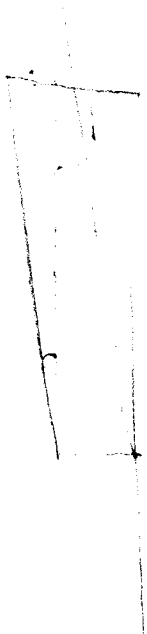
SCALE : 1" = 20'
Q FILE : Q 549 I J F.S. = 1.5
ELEVATION IN FEET NGVD

please give these dimensions

WAD

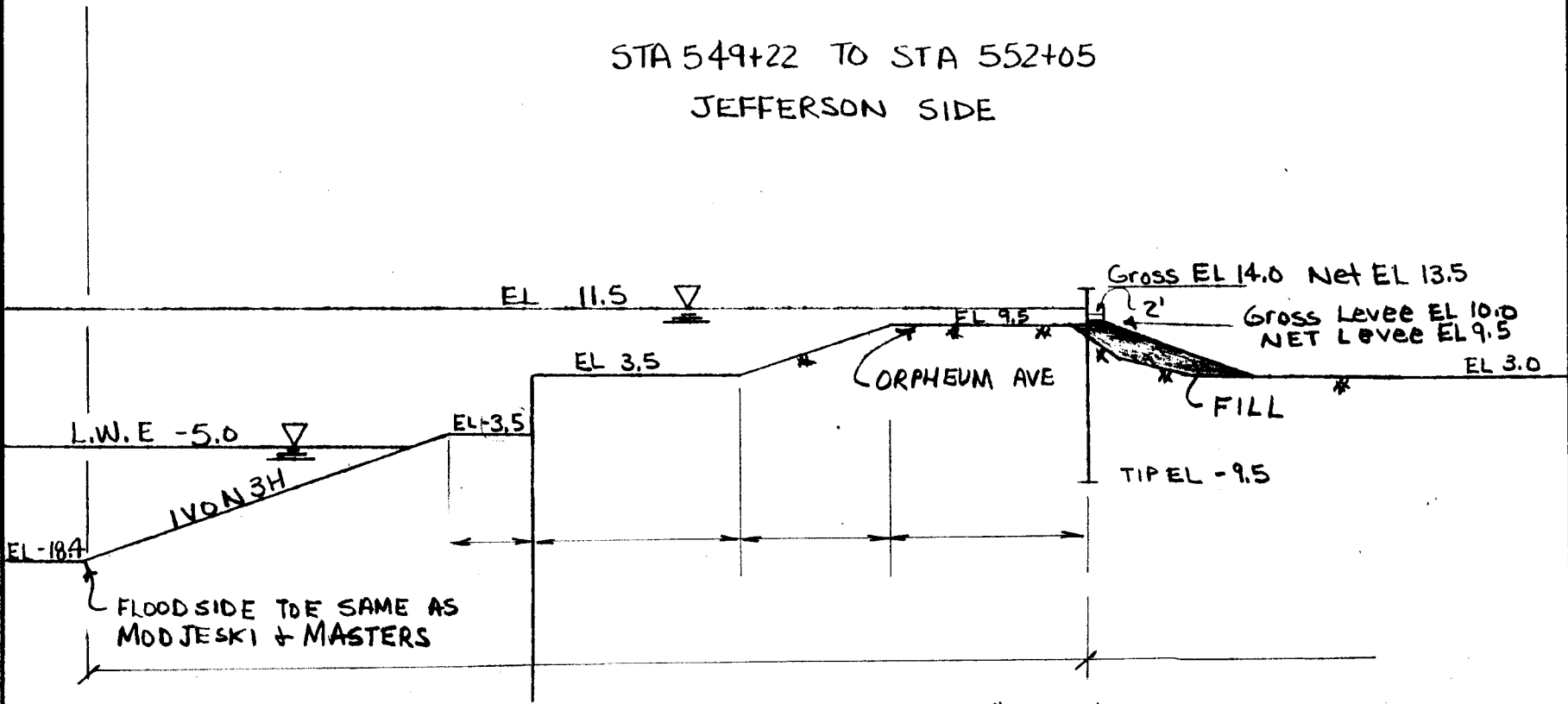
PROJECT	17th St Cutoff Canal	PAGE	OF	COMPUTED BY	DATE
SUBJECT	STA 549+22 TO STA 552+05	CHECKED BY			June 87

COMPUTATION SHEET



PROJECT	17th St Outfall Canal	PAGE	OF	COMPUTED BY	DATE
SUBJECT	STA 549+22 TO STA 552+05	CHECKED BY			June 87

STA 549+22 TO STA 552+05
JEFFERSON SIDE



SCALE : 1" = 20'
Q FILE : Q 549.IJ F.S. = 1.5
ELEVATION IN FEET NGVD

Q549IJ

JOBCTRL ER 117 : INVALID JCL OR PROGRAM NAME.

LIST Q549IJ

1 10001 17TH STREET CANAL HLP FROM STA. 549+22 TO STA. 552+05
10002 I-WALL TOP EL. 13.5 BOTT. EL. -9.5 F.S. =1.5

3	10003	3	0.13500000E+02	0.00000000E+01
4	10004	3	0.12500000E+02	0.62500000E+02
5	10005	3	0.11500000E+02	0.12500000E+03
6	10006	3	0.10500000E+02	0.18750000E+03
7	10007	3	0.95000000E+01	0.25000000E+03
8	10008	3	0.95000000E+01	0.25000000E+03
9	10009	3	0.85000000E+01	0.14734995E+03
10	10010	3	0.75000000E+01	0.73377086E+02
11	10011	3	0.65000000E+01	0.29560611E+02
12	10012	3	0.58253540E+01	0.00000000E+01
13	10013	3	0.55000000E+01	-0.14255865E+02
14	10014	3	0.45000000E+01	-0.58072340E+02
15	10015	3	0.35000000E+01	-0.10188882E+03
16	10016	3	0.25000000E+01	-0.14570529E+03
17	10017	3	0.15000000E+01	-0.18952177E+03
18	10018	3	0.50000000E+00	-0.23333824E+03
19	10019	3	0.00000000E+01	-0.25524648E+03
20	10020	3	0.00000000E+01	-0.25524648E+03
21	10021	3	-0.10000000E+01	-0.24052724E+03
22	10022	3	-0.20000000E+01	-0.22580801E+03
23	10023	3	-0.30000000E+01	-0.21108877E+03
24	10024	3	-0.40000000E+01	-0.19636954E+03
25	10025	3	-0.50000000E+01	-0.18165030E+03
26	10026	3	-0.60000000E+01	-0.16693106E+03
	10027	3	-0.70000000E+01	-0.15221183E+03
28	10028	3	-0.75712067E+01	-0.14380410E+03
29	10029	3	-0.77208040E+01	0.00000000E+01
30	10030	3	-0.94240649E+01	0.16373009E+04
31	10031	4	-0.94240649E+01	0.00000000E+01
32	10032	0	-0.94240649E+01	0.00000000E+01
33	10033	-0.94240649E+01	-0.29802322E-07	0.45638112E+01

EOT..

LIST Q549IJO

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH STREET CANAL HLP FROM STA. 549+22 TO STA. 552+05

WAL

THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
AS COUNTERCLOCKWISE.

THE MAXIMUM DEFLECTION IS 0.90 INCHES AND OCCURS AT MEMBER COORDINATE
15.50 FT.

Z-22 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

CALCULATED EXTERNAL LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
-9.42	POINT LD	0.00 LBF
-9.42	COUPLE	-4.56 LBF-FT

INPUTTED LOADS

DISTANCE FROM REFERENCE (FT)	TYPE OF LOAD	MAGNITUDE OF LOAD
13.50	CONTN LD	0.00 LBF/SQ FT
12.50	CONTN LD	62.50 LBF/SQ FT
11.50	CONTN LD	125.00 LBF/SQ FT
10.50	CONTN LD	187.50 LBF/SQ FT
9.50	CONTN LD	250.00 LBF/SQ FT
9.50	CONTN LD	250.00 LBF/SQ FT
8.50	CONTN LD	147.35 LBF/SQ FT
7.50	CONTN LD	73.38 LBF/SQ FT
6.50	CONTN LD	29.56 LBF/SQ FT
5.83	CONTN LD	0.00 LBF/SQ FT
5.50	CONTN LD	-14.26 LBF/SQ FT
4.50	CONTN LD	-58.07 LBF/SQ FT
3.50	CONTN LD	-101.89 LBF/SQ FT
2.50	CONTN LD	-145.71 LBF/SQ FT
1.50	CONTN LD	-189.52 LBF/SQ FT
0.50	CONTN LD	-233.34 LBF/SQ FT
0.00	CONTN LD	-255.25 LBF/SQ FT
0.00	CONTN LD	-255.25 LBF/SQ FT
-1.00	CONTN LD	-240.53 LBF/SQ FT
-2.00	CONTN LD	-225.81 LBF/SQ FT
-3.00	CONTN LD	-211.09 LBF/SQ FT

58	-4.00	CONTN LD	-196.37	LBF/SQ FT
59	-5.00	CONTN LD	-181.65	LBF/SQ FT
60	-6.00	CONTN LD	-166.93	LBF/SQ FT
61	-7.00	CONTN LD	-152.21	LBF/SQ FT
62	-7.57	CONTN LD	-143.80	LBF/SQ FT
63	-7.72	CONTN LD	0.00	LBF/SQ FT
64	-9.42	CONTN LD	1637.30	LBF/SQ FT
65	-9.42	CONTN LD	0.00	LBF/SQ FT

Z-22 PROPERTIES ARE AS FOLLOWS.

70
71 MOMENT OF INERTIA= 84.38 IN. TO THE 4TH PER FOOT OF WALL
72 CROSS SECTIONAL AREA= 6.47 SQ IN.
73 ELASTIC MODULUS= 29000000. LBF/SQ IN.
74 DEFLECTION REFERENCE IS AT -9.500

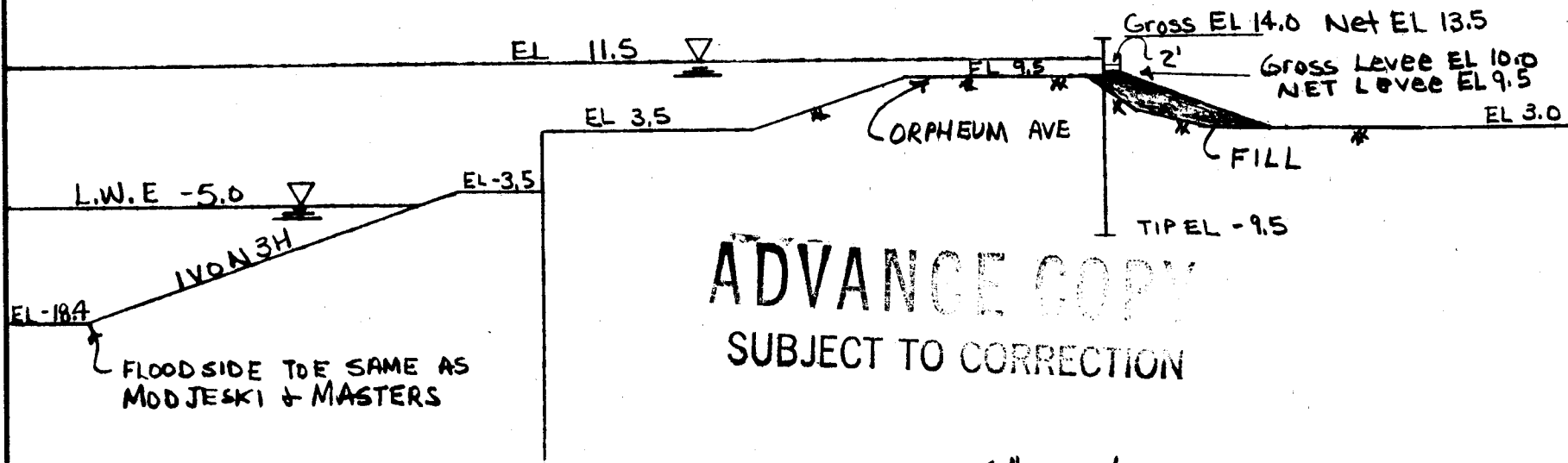
75
76
77 THE MAXIMUM BENDING MOMENT IS 7117.16 LBF-FT AND OCCURS AT -0.51
78 WHICH HAS THE SHEAR FORCE OF -1.84 LBF.

80	DEFLECTION				
81	FROM TANG.				
82	THRU DEFLE				
83	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	DEFLECTION
84	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	REFERENCE
85					(INCHES)
85	15.500	0.0	0.0	0.0	0.9021
86	15.499	0.0	0.0	0.0	0.9021
87	15.000	0.0	0.0	0.0	0.8730
88	14.000	0.0	0.0	0.0	0.8145
89	13.000	7.8	1.2	1.3	0.7561
90	12.000	70.3	10.9	35.2	0.6977
91	11.000	195.3	30.2	162.8	0.6393
92	10.000	382.8	59.2	446.6	0.5810
93	9.000	612.2	94.6	945.8	0.5231
94	8.000	763.1	118.0	1640.8	0.4659
95	7.000	840.2	129.9	2447.4	0.4098
96	6.000	869.8	134.5	3306.0	0.3555
97	5.825	870.5	134.6	3458.0	0.3463
98	5.000	855.6	132.3	4172.4	0.3035
99	4.000	797.5	123.3	5002.5	0.2545
100	3.000	695.6	107.5	5752.7	0.2090
101	2.000	549.9	85.0	6379.1	0.1675
102	1.000	360.4	55.7	6837.9	0.1305
103	0.000	127.0	19.6	7085.3	0.0983
104	-0.512	-1.8	-0.3	7117.2	0.0838
105	-1.000	-120.9	-18.7	7087.1	0.0711
106	-2.000	-354.0	-54.7	6848.4	0.0489
107	-3.000	-572.5	-88.5	6384.0	0.0315
108	-4.000	-776.2	-120.0	5708.4	0.0186
109	-5.000	-965.2	-149.2	4836.5	0.0097
110	-6.000	-1139.5	-176.1	3782.9	0.0041
111	-7.000	-1299.1	-200.8	2562.4	0.0013
112	-7.720	-1394.4	-215.5	1589.3	0.0004
113	-7.722	-1394.4	-215.5	1586.5	0.0004
114	-8.000	-1356.9	-209.8	1202.1	0.0002
115	-9.000	-607.9	-94.0	139.6	0.0000

116	-9.423	-1.6	-0.3	4.6	0.0000
117	-9.425	0.0	0.0	0.0	0.0000
118	-9.499	0.0	0.0	0.0	0.0000
119	-9.500	0.0	0.0	0.0	0.0000

120
121
122
123 *RUN COMPLETED*
124
EOT..

STA 549+22 TO STA 552+05
JEFFERSON SIDE



ADVANCE COPY
SUBJECT TO CORRECTION

SCALE : 1" = 20'
Q FILE : Q 549 IJ F.S. = 1.5
ELEVATION IN FEET NGVD

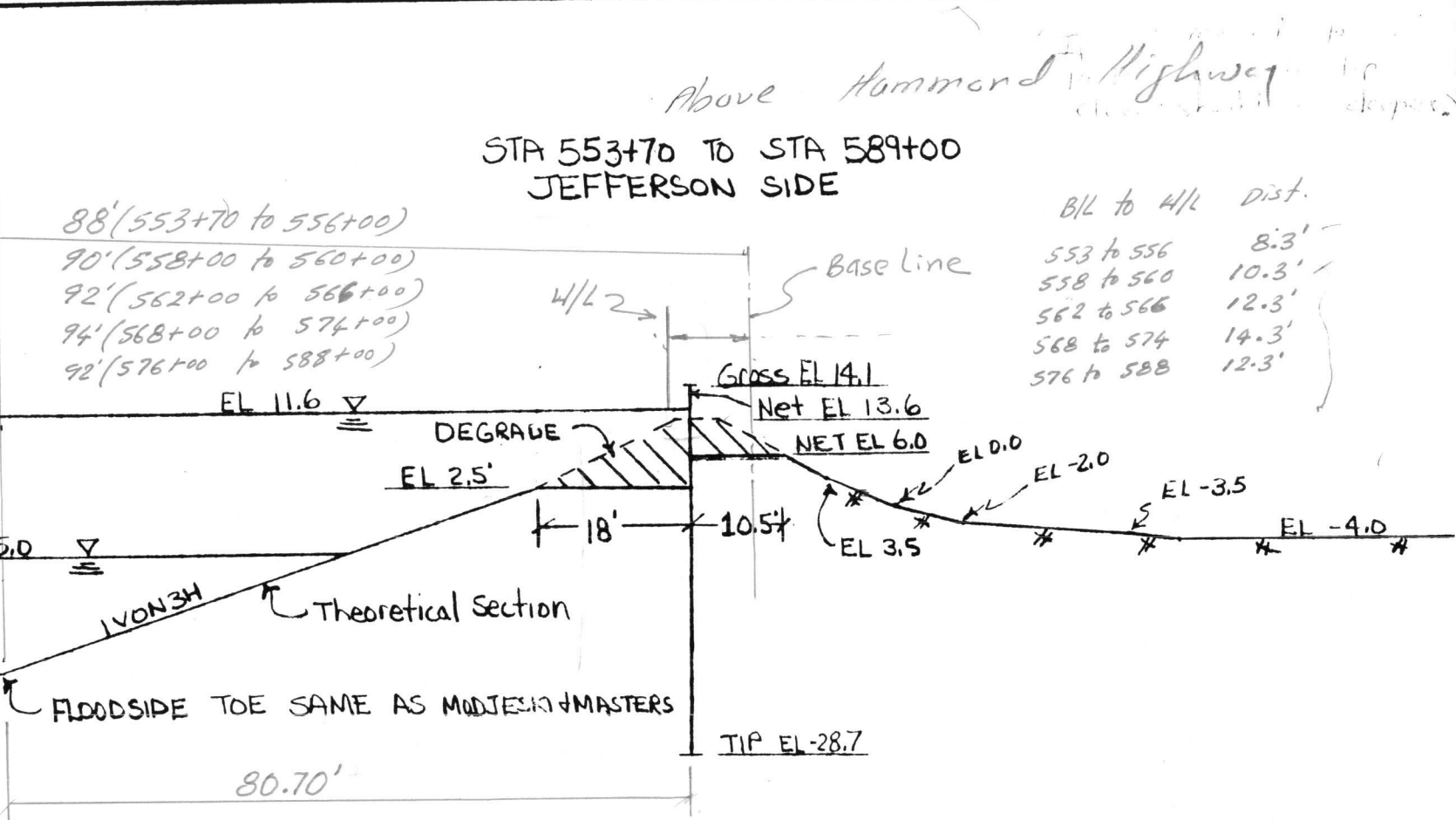
PROJECT	17th St Offfall Canal	PAGE	OF	COMPUTED BY	DATE
SUBJECT	STA 549+22 TO STA 552+05			EJV	June 87
		CHECKED BY			DATE

COMPUTATION SHEET

Received
6/25

PROJECT 17th St Outfall Canal
 SUBJECT Sta 553+70 To Sta 589+00 Jefferson

DATE 7/24/87
 DATE
 COMPUTED BY
 CHECKED BY



Above Hammond Highway
 STA 553+70 TO STA 589+00
 JEFFERSON SIDE

SCALE 1" = 20'
 Elevation in Feet N.G.V.D

Q File 8 Q 589IJ (F.S.=15)
 Q 589IJA (F.S.=1.0)

7/2/87

LIST Q589IJA

1 10001 17TH ST CANAL HLP STA 553+70 TO STA 589+00
2 10002 I-WALL TOP EL. 14.1 BOTT. EL. -28.7 F.S.=1.0
3 10003 3 0.136000000E+02 0.000000000E+01
A 10004 3 0.126000000E+02 0.625000000E+02
10005 3 0.116000000E+02 0.125000000E+03
6 10006 3 0.106000000E+02 0.187500000E+03
7 10007 3 0.960000000E+01 0.250000000E+03
8 10008 3 0.860000000E+01 0.312500000E+03
9 10009 3 0.760000000E+01 0.375000000E+03
10 10010 3 0.660000000E+01 0.437500000E+03
11 10011 3 0.600000000E+01 0.475000000E+03
12 10012 3 0.600000000E+01 0.475000000E+03
13 10013 3 0.500000000E+01 0.26358524E+03
14 10014 3 0.400000000E+01 0.52170487E+02
15 10015 3 0.37532316E+01 0.000000000E+01
16 10016 3 0.350000000E+01 -0.53536891E+02
17 10017 3 0.350000000E+01 -0.53536891E+02
18 10018 3 0.250000000E+01 -0.22614706E+03
19 10019 3 0.250000000E+01 -0.22614706E+03
20 10020 3 0.150000000E+01 -0.38101448E+03
21 10021 3 0.500000000E+00 -0.53588190E+03
22 10022 3 0.000000000E+01 -0.61331561E+03
23 10023 3 0.000000000E+01 -0.61331561E+03

24 10024 3 -0.100000000E+01 -0.68319974E+03
25 10025 3 -0.200000000E+01 -0.66190325E+03
26 10026 3 -0.300000000E+01 -0.64060677E+03
27 10027 3 -0.400000000E+01 -0.61931028E+03
28 10028 3 -0.500000000E+01 -0.59801379E+03
W 10029 3 -0.59797585E+01 -0.57714838E+03
J 10030 3 -0.73795785E+01 0.000000000E+01
31 10031 3 -0.11224531E+02 0.15852812E+04
32 10032 4 -0.11224531E+02 0.000000000E+01
33 10033 0 -0.11224531E+02 0.000000000E+01
34 10034 -0.11224531E+02 0.000000000E+01 0.29113019E+02
EOT..

LIST Q589IJAO

1
2
3

BEAMS (SHEAR, MOMENT, DEFLECTION)

17TH ST CANAL HLP STA 553+70 TO STA 589+00
WAL

6
7

8 THE REFERENCE SYSTEM SELECTED DEFINES POSITIVE FORCES AS TO THE LEFT
9 INCREASING MEMBER COORDINATES AS UPWARD, AND POSITIVE MOMENTS
10 AS COUNTERCLOCKWISE.

11
12

13 THE MAXIMUM DEFLECTION IS 0.97 INCHES AND OCCURS AT MEMBER COORDINATE
14 14.10 FT.

15
16

17
18 Z-27 HAS BEEN GIVEN TO SUPPORT THE LOAD SYSTEM.

19
20

20 THE WEIGHT OF THIS VERTICAL MEMBER HAS BEEN NEGLECTED.

21
22

23 CALCULATED EXTERNAL LOADS

24
25

25	DISTANCE FROM	TYPE OF	MAGNITUDE OF
26	REFERENCE(FT)	LOAD	LOAD
28	-11.22	POINT LD	0.00 LBF
29	-11.22	COUPLE	-29.12 LBF-FT

30
31

32 INPUTTED LOADS

33
34

34	DISTANCE FROM	TYPE OF	MAGNITUDE OF
35	REFERENCE(FT)	LOAD	LOAD
37	13.60	CONTN LD	0.00 LBF/SQ FT
38	12.60	CONTN LD	62.50 LBF/SQ FT
39	11.60	CONTN LD	125.00 LBF/SQ FT
40	10.60	CONTN LD	187.50 LBF/SQ FT
41	9.60	CONTN LD	250.00 LBF/SQ FT
42	8.60	CONTN LD	312.50 LBF/SQ FT
43	7.60	CONTN LD	375.00 LBF/SQ FT
44	6.60	CONTN LD	437.50 LBF/SQ FT
45	6.00	CONTN LD	475.00 LBF/SQ FT
46	6.00	CONTN LD	475.00 LBF/SQ FT
47	5.00	CONTN LD	263.59 LBF/SQ FT
48	4.00	CONTN LD	52.17 LBF/SQ FT
49	3.75	CONTN LD	0.00 LBF/SQ FT
50	3.50	CONTN LD	-53.54 LBF/SQ FT
51	3.50	CONTN LD	-53.54 LBF/SQ FT
52	2.50	CONTN LD	-226.15 LBF/SQ FT
53	2.50	CONTN LD	-226.15 LBF/SQ FT
54	1.50	CONTN LD	-381.01 LBF/SQ FT
55	0.50	CONTN LD	-535.88 LBF/SQ FT
56	0.00	CONTN LD	-613.32 LBF/SQ FT
57	0.00	CONTN LD	-613.32 LBF/SQ FT

58	-1.00	CONTN LD	-683.20	LBF/SQ FT
59	-2.00	CONTN LD	-661.90	LBF/SQ FT
60	-3.00	CONTN LD	-640.61	LBF/SQ FT
61	-4.00	CONTN LD	-619.31	LBF/SQ FT
62	-5.00	CONTN LD	-598.01	LBF/SQ FT
63	-5.98	CONTN LD	-577.15	LBF/SQ FT
64	-7.38	CONTN LD	0.00	LBF/SQ FT
65	-11.22	CONTN LD	1585.28	LBF/SQ FT
66	-11.22	CONTN LD	0.00	LBF/SQ FT

Z-27 PROPERTIES ARE AS FOLLOWS.

70
71
72 MOMENT OF INERTIA= 184.20 IN. TO THE 4TH PER FOOT OF WALL
73 CROSS SECTIONAL AREA= 7.94 SQ IN.
74 ELASTIC MODULUS= 29000000. LBF/SQ IN.
75 DEFLECTION REFERENCE IS AT -28.700

78 THE MAXIMUM BENDING MOMENT IS 17669.84 LBF-FT AND OCCURS AT -1.74
79 WHICH HAS THE SHEAR FORCE OF -2.07 LBF.

84	DISTANCE	SHEAR FOR	SHEAR STR	BENDING MOM	DEFLECTION
85	(FEET)	(LBF)	(LBF/SQIN)	(LBF-FT)	FROM TANG.
86					THRU DEFLE
87					REFERENCE
88					(INCHES)
86	14.100	0.0	0.0	0.0	0.9739
87	14.099	0.0	0.0	0.0	0.9739
88	14.000	0.0	0.0	0.0	0.9676
89	13.000	11.2	1.4	2.2	0.9040
90	12.000	80.0	10.1	42.7	0.8404
91	11.000	211.2	26.6	183.1	0.7768
92	10.000	405.0	51.0	486.0	0.7133
93	9.000	661.2	83.3	1013.9	0.6499
94	8.000	980.0	123.4	1829.3	0.5869
95	7.000	1361.2	171.4	2994.7	0.5245
96	6.000	1805.0	227.3	4572.7	0.4631
97	5.000	2174.3	273.8	6579.9	0.4032
98	4.000	2332.2	293.7	8850.8	0.3454
99	3.753	2338.6	294.5	9427.3	0.3315
100	3.000	2283.5	287.6	11174.6	0.2905
101	2.000	2059.6	259.4	13359.8	0.2392
102	1.000	1678.5	211.4	15241.7	0.1922
103	0.000	1142.7	143.9	16665.2	0.1501
104	-1.000	494.4	62.3	17489.6	0.1134
105	-1.735	-2.1	-0.3	17669.8	0.0900
106	-2.000	-178.2	-22.4	17645.9	0.0823
107	-3.000	-829.4	-104.5	17140.4	0.0569
108	-4.000	-1459.4	-183.8	15994.2	0.0369
109	-5.000	-2068.0	-260.5	14228.8	0.0222
110	-6.000	-2655.3	-334.4	11865.3	0.0120
111	-7.000	-3018.0	-380.1	8994.3	0.0056
112	-7.379	-3047.7	-383.8	7844.3	0.0040
113	-7.381	-3047.7	-383.8	7838.2	0.0040
114	-8.000	-2968.3	-373.8	5966.8	0.0021
115	-9.000	-2506.4	-315.7	3195.1	0.0006

116	-10.000	-1632.1	-205.6	1091.5	0.0001
117	-11.000	-345.6	-43.5	68.3	0.0000
118	-11.224	-1.6	-0.2	29.1	0.0000
119	-11.226	0.0	0.0	0.0	0.0000
120	-12.000	0.0	0.0	0.0	0.0000
121	-13.000	0.0	0.0	0.0	0.0000
122	-14.000	0.0	0.0	0.0	0.0000
123	-15.000	0.0	0.0	0.0	0.0000
124	-16.000	0.0	0.0	0.0	0.0000
125	-17.000	0.0	0.0	0.0	0.0000
126	-18.000	0.0	0.0	0.0	0.0000
127	-19.000	0.0	0.0	0.0	0.0000
128	-20.000	0.0	0.0	0.0	0.0000
129	-21.000	0.0	0.0	0.0	0.0000
130	-22.000	0.0	0.0	0.0	0.0000
131	-23.000	0.0	0.0	0.0	0.0000
132	-24.000	0.0	0.0	0.0	0.0000
133	-25.000	0.0	0.0	0.0	0.0000
134	-26.000	0.0	0.0	0.0	0.0000
135	-27.000	0.0	0.0	0.0	0.0000
136	-28.000	0.0	0.0	0.0	0.0000
137	-28.699	0.0	0.0	0.0	0.0000
138	-28.700	0.0	0.0	0.0	0.0000

139

140

141

142 *RUN COMPLETED*

143

DT..