MEMORANDUM THRU Area Engineer, NOAO PB C/Const Div, ATTN: Contr Admin Br

FOR C/Engr Div

STBJECT: Narrative Completion Report for Contract No. DLCW29-93-C-0071, Lake Pontchartrain, Louisiana & Vicinity, High Level Plan, New Orleans Lakefront Levee West of IHNC, Orleans Avenue Canal Flood Protection Improvement Phase II-B, (West Side: Sta. 64+51.53 B/L to Sta. 90+26.91 B/L), Orleans Parish, Louisiana

- 1. The subject contract dated 8 Jun 93 was awarded to Johnson Brothers Corporation of Louisiana, 2002 20th Street, Suite B101, Kenner, Louisiana 70062. The Notice to Proceed was issued on 25 Jun 93 with construction to start no later than 5 Jul 93. The original completion date was set for 17 Dec 94, with the estimated amount of the contract being \$5,078,484.
- 2. Required work under this contract included the construction of a pile supported, reinforced concrete, T-type floodwall; clearing, grubbing, and degrading of existing levee for structure construction as well as structural backfill; furnishing and driving 86,724 linear feet of prestressed concrete piles, approximately 86,361 square feet of permanent steel sheet piling, and steel sheet piling for cofferdam; placement of cofferdam backfill; dewatering; demolition of existing wall concrete cap and sheet piling; fertilizing, seeding, and mulching; cleaning and coating of existing retaining wall; placing erosion control silt fence; performing miscellaneous paving; and all other restoration.
- 3. The Preconstruction Conference was held at the New Orleans Area Office on 25 Jun 93. Detailed minutes of this meeting are located in the contract file.
- 4. The Notice to Proceed was signed by the Contractor on 25 Jun 93. The contractor began mobilizing equipment, preparing staging area T-4, and performing preliminary survey work on 28 Jun 93. The project trailer and the T-4 yard area including security fence were completed by 12 Jul 93. Work on staging area T-5 began on 27 Jul 93. Driving of cofferdam sheets began on 18 Aug 93. A P&H 670 crane, Ice 612 vibratory hammer with power pack, and a 17-foot motorized flatboat was mobilized for this task.

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- 5. This contract provided for 14 major construction phases: (1) Clearing and Grubbing, (2) Fertilizing, Seeding, and Mulching (3) Structural Excavation and Backfill, (4) Cofferdam Backfill, (5) Steel Sheet Piling, Type PSA-23, (6) Steel Sheet piling, PZ-27, (7) Steel Sheet Piling, PZ-22, (8) Steel Sheet Piling, PZ-27 and PSA-23, Cofferdam, (9) Piling, Concrete Precast, Prestressed, (14-inch x 14-inch), (10) Reinforced Concrete Floodwall, (11) Miscellaneous Metalwork and Specialty Items, (12) Sidewalk Concrete, (13) Selective Demolition, (14) Clean and Coat Existing Retaining Wall. Some of the work phases were of a routine nature; however, there were some phases that required a certain amount of skill to obtain satisfactory results such as the reinforced concrete floodwall and driving the precast concrete piles.
- 6. The driving of cofferdam sheets began on 18 Aug 95 with the closure at approximate Wall-Line Sta. 35+87. The 58-foot-long CZ-114 sheets were driven to a tip elevation of -47.5 feet using stationary barges as a template. The cofferdam was composed of 8 cells with additional closures at Wall-Line Sta's. 33+29, 31+29, 28+92, 25+40, 19+97, 16+44, 12+82, and 9+88. Work primary to each cell was completed before the next cell was started, with the sheets being advanced from one cell to the next. Equipment used in this phase of work was one 4000 Manitowac crane, two 30-by 120-foot barges, one Ice 612 vibratory hammer, one P&H 670 crane (initially), and one 17-foot motorized flatboat. The last of the cofferdam sheets were extracted on 21 Jul 95.
- 7. On 23 Nov 93, the driving of 85,602 square feet of CZ-114, 493.3 square feet of PSA-23, and 266 square feet of PZ-22 permanent steel sheet piling began. A cold rolled sheet piling substitute (CZ-114) was used instead of the PZ-27, PZ-22, and most of the PSA-23 sheets; two CZ101 sheets were substituted for PSA-23 sheets as well. Pilings were driven from wall-line stations 9+95.17 to 35+80.21. The sheets were aligned using secured mats for a template, and all were driven to tip elevation of -38.5 feet. Cathodic protection was installed concurrent with the installation of the structural reinforcement. Equipment used to drive the piling was one 4000 Manitowoc crane, two 30- by 120-foot barges, one Ice 612 vibratory hammer with power pack, and one 17-foot motorized flatboat. This phase of work was completed on 6 Jun 95.
- 8. The driving of the first 14- by 14-inch precast, prestressed concrete piles began on 10 Dec 93. A total of 396 77-foot 3-inch piles and 726 79-foot 3-inch piles were driven on a 2V to 1H

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batter. A Manitowoc 4000 barge-mounted crane was used to drive the concrete piles. During driving operations, an extensive guide template was used to assure correct alignment. Quality assurance was maintained during the driving by using a transit, marking the pile footage, and recording blow counts for the 86,724 linear feet of 14-inch-square, prestressed, precast piles. All concrete piles were driven to grade. Equipment used included one 4000 Manitowoc crane, two 30- by 120-foot barges, one Vulcan 512 air hammer, and one 17-foot motorized flatboat. Concrete pile driving was completed on 31 May 95.

- The contractor commenced construction of the reinforced 9. The work consisted of concrete floodwall on 11 Jan 94. constructing 66 T-type concrete monoliths. The contractor used a forming system designed and supplied by Karlson Forming Specialties, Inc. A gang form construction technique was used employing laminated sheeting and wooden structural members secured with she-bolts and braced with screwjacks attached to the footing and/or concrete deadmen. The floodwall was constructed atop its footing in 2 placements throughout the first 16 monoliths, W/L Sta's 35+75-29+51. Following the deletion by modification P00013 of the construction joint at elevation 4.00 feet, they were placed monolithically. The concrete was delivered to the job site by concrete mixer trucks and was placed into the forms by use of elephant trunks (tremees) of various lengths. One and three yard concrete buckets were used. addition, the following equipment/material was used: one Manitowoc 4000 crane (occasionally); one 599 American crane; two concrete dump buckets; two concrete vibrators; form oil; curing compound; miscellaneous hand tools; two electric generators; and electrically driven power tools. Quality control was maintained daily through use of 1246 check-out list. Air content tests, slump tests, and molding of test cylinders were performed by Delta Testing. All concrete cylinders were tested at the soils lab at the New Orleans District. This phase of work was completed on 23 Jun 95.
- 10. On 26 Aug 93, the contractor began excavating the first cell. Structural backfill and excavation were performed on an intermittent basis depending on cell requirements and weather conditions. Backfill material was also placed as cofferdam backfill until deleted in compliance with modification P0002, which raised the bottom of the footing to elevation -6.33 feet. Thereafter, excavated fill was stored and processed at the west levee storage area. In addition to the on-site material, 11,276 truck-yards of spillway material was incorporated into the structural backfill. Because of the sequential nature of the

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work necessitated by the use of multiple cells, the backfill operation became a critical choke point in the steady progression of the work. As a result of untimely rains, which adversely effected the backfill operations, work over the entire project was sometimes halted. Equipment used for this phase of work included at one time or another the following: one Mikasa vibratory roller; one plate compactor; one Cat D-8 dozer; one Cat D-5 dozer; one Cat D-4 dozer; one Cat 235 backhoe; four dump trucks; one 166 Bantam backhoe; and occasionally the 4000 Manitowoc with clam bucket. Structural Excavation of the final cell ended on 2 May 95; Structural Backfill was completed on 24 Jul 95.

- 11. Selective Demolition began on 15 Sep 93 and concluded on 29 Apr 95 with the removal of approximately 2,600 feet of concrete cap and sheet piles. Concrete cap was crushed with a La Bounty hydraulic concrete crusher either mounted on a 235 Cat backhoe or the 4000 Manitowoc crane. The existing sheet piles were extracted with the Ice 612 vibratory hammer. Equipment used consisted of one La Bounty concrete crusher, one 235 Cat backhoe, one 4000 Manitowoc crane, one Ice vibratory hammer, and one dump truck.
- 12. Cementitious paining of the floodwall began on 27 Apr 95 and continued intermittently through 11 Aug 95. The cleaning and coating of the existing retaining wall began on 11 Aug 95 and was completed on 12 Aug 95. The subcontractor presented several test panels before one was found acceptable. The cementitious coat was applied in a two-coat system following the manufacturer's directions, followed by two coats of Acrylic Emulsion. The cementitious coat was applied with rollers, while the Acrylic was both rolled and applied by hairless sprayer. The retaining wall received one coat of Acrylic Emulsion, as well as one coat of clear sealer. Both the floodwall and retaining wall were cleaned with a 2,500 psi pressure washer. Equipment used included the pressure washer, one 8- by 24-foot barge, one airless sprayer, and miscellaneous hand tools.
- 13. The subcontractor, Economy Grassing, Inc., performed the fertilizing, seeding, and mulching phases of work. Work began on 23 Aug 95 with the protected side of the west embankment, which was completed on 24 Aug 95. The east levee embankment, storage sites, and yard area were started on 2 Oct 95 and completed on 3 Oct 95. After the fertilizer was broadcast, it was cut-in to a depth of 2 inches with a harrow. Bermuda grass seed was broadcast and then sealed with a pass of a culti-packer. Finally, mulch was applied with a Hydro-mulcher. A total of 4.93

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acres was prepared. The contractor used one farm tractor, one spike-tooth harrow, one Hydro-mulch sprayer, one culti-packer, one tractor-mounted rotary broadcaster, and one farm tractor.

- 14. There were 19 modifications on the contract, and a summary of each follows.
- a. P00001, dated 14 Sep 93, modified the contract by adding a remittance address to Block 16 on SF-1442.
- b. P00002, dated 28 Oct 93, modified the contract to raise the T-wall base elevation to -6.33.
- c. P00003, dated 12 Nov 93, extended the required completion date of the contract 2 calendar days as a result of unusually severe weather.
- d. P00004, dated 19 Jan 94, modified the contract to provide funds for payment in accordance with Special Clause H-28, an increase of 1,427,000.
- e. P00005, dated 26 Jan 94, modified the contract to exclude the raised garland, column patterns, chamfer, dummy joints, and urns from receiving a cementitious finish. The contract price was increased by \$4,158.09 and the contract time remained unchanged.
- f. P00006, dated 28 Jan 94, defined the parameters of UCO-01, Mod P00002. Bid Item 0016, Adjustment of T-wall Elevation, was added, which increased the contract price by \$230,019.80 and increased the contract time by 95 calendar days.
- g. P00007, dated 7 Feb 94, extended the required completion date of the contract 8 calendar days as a result of unusually severe weather.
- h. P00008, dated 26 Apr 94, modified the contract to provide funds for payment in accordance with Special Clause H-28, an increase of \$1,989,484.
- i. P00009, dated 4 May 94, extended the required completion date of the contract 8 calendar days as a result of unusually severe weather.
  - j. P00010, dated 6 Jun 94, modified the contract to drive

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- 6,151.25 square feet of PZ-27 steel sheet piling 2-feet 0-inches from the protected side face of the existing floodwall from B/L Sta. 65+79 to Sta. 67+64. Note Mod P00011.
- k. P00011, dated 20 Jul 94, modified the contract by adding Bid Item 0017, Lump Sum Adjustment for driving steel sheet piling between Sta's. 65+79 and 67+64. The contract price was increased by \$18,403.74 and the contract time was increased by 2 calendar days.
- 1. P00012, dated 2 Aug 94, extended the required completion date of the contract 44 calendar days as a result of unusually severe weather.
- m. P00013, dated 15 Sep 94, modified the contract to remove the construction joints on the floodwall via a Value Engineering change proposal. Bid Item 0018, Removal of Construction Joints, and Bid Item 0019, VECP Payment Lump Sum for Removal of Construction Joints, were added. The contract price was decreased by \$10,800 with no change in contract time.
- n. P00014, dated 18 Oct 94, modified the contract to tie-in the open areas at each end of the emergency sheet piling. Bid Item 0020, Emergency Flooding Protection (CIN-05). The contract price was increased by \$905 with no change in contract time.
- o. P00015, dated 9 Nov 94, extended the required completion completion date of the contract 18 calendar days as a result of unusually severe weather.
- p. P00016, dated 13 Feb 95, extended the required completion date of the contract 11 calendar days as a result of unusually severe weather.
- q. POOO17, dated 8 May 95, extended the required completion date of the contract 50 calendar days as a result of unusually severe weather.
- r. P00018, dated 15 Aug 95, extended the required completion date of the contract 29 calendar days as a result of unusually severe weather.
- s. A0001, dated 8 Nov 95, extended the required completion date of the contract 38 calendar days as a result of unusually severe weather.

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- 15. Following is a list of major suppliers on the subject contract:
  - a. Concrete Carlo Ditta, Inc., Harvey, LA
  - b. Reinforcing Steel Lulich Steel Corp., Slidell, LA
- c. Concrete Piling Gulf Coast Pre-Stress, Inc., Pass Christian, MS.
  - d. Waterstop Building Specialties, Inc., New Orleans, LA.
  - e. Steel Sheet Pilings Casteel Group, Inc., Covington, La.
- f. Cathodic Protection Cathodic Protection Services Co., Metairie, LA.
- g. Cementitious Paint Construction Materials, Inc., Baton Rouge, LA.
  - h. Forms Karlson Forming Specialties, Inc., Amery, WI.
  - Spillway Material Hamps Trucking Co., New Orleans, LA.
- 16. Subcontractors performing work on this project along with subcontractor amounts and contract responsibilities are as follows:
- a. McMillion Dozer Service, Inc., 11204 Jefferson Highway, River Ridge, LA 70123. This subcontract payment was over \$100,000. with payment made under 04 Structural Excavation and Backfill.
- b. Scott Derr Painting Co., 1904 San Joaquin Parkway, Friedship, TX 77546. This subcontract payment was for under \$50,000. with payment made under Items 11 and 15 Reinforced Concrete Floodwall and Clean and Coat Existing Retaining Wall, respectively.
- c. Economy Grassing Inc., 7054 W. T. Hall Rd., Ethel, LA 70002. This subcontract payment was for under \$25,000. with payment made under Item 3 Fertilizing, Seeding, and Mulching.
- 17. The contractor submitted and enforced an adequate Accident Prevention Program. Weekly tool box safety meetings were held. The contractor was cooperative in the performance of the work, and there were no lost time accidents.

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- 18. The contractor was efficient and professional in the performance of the work and the extra work required to complete this project. The equipment used was generally in excellent condition and capable of performing the work. The contractor maintained an adequate quality control organization and system which helped him complete the project on schedule.
- 19. Included herewith is a comparison of contract quantities versus actual quantities. A copy of the as-built drawings and surveys of the reference marker elevations are also enclosed.

Ite: No	•	Qty & Unit	Unit Price	Est. Amt.	Qty	Earnings to Date
1	Mob & Demob	LS	LS \$	150,000	100%	\$ 150,000
2	Clear & Grubbing	LS	LS \$	29,000	100%	\$ 29,000
3	Fert, Seed, and Mulch.	LS	LS \$	5,000	100%	\$ 5,000
4	Struc Excavation & Backfill	LS	LS \$	350,000	100%	\$ 350,000
5	Cofferdam Backfill	L LS	LS \$	20,000	100%	\$ 20,000
6	Piling, Steel Sheet, Type PSA-27	7 465 S	F 30. \$	13,950	493.3	\$ 14,799
7	Piling, Steel Sheet, Type PZ-27	85,441	SF 9. \$	768,969	85,602	\$ 770,418
8	Piling, Steel Sheet, Type PZ-22	244 S	F 9.\$	2,196	266	\$ 2,394
9	Piling, Steel Shee Type, PZ-27 & PSA- Cofferdam		LS \$	850,000	100%	\$ 850,000
10	Piling, Concrete I Prestressed, (14-1 14-Inch) 86	Inch	12.25 \$	1,062,369	86,724	\$1,062,369
11	Reinforce Concrete Floodwall		LS \$1,	5 <b>04,158.</b> 09	100% \$3	1,504,158.09

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12	Misc. Metalwork							
	& Specialty Items	LS	LS	\$	3,000	100%	\$	3,000
13	Sidewalk Concrete	LS	LS	\$	4,000	100%	\$	4,000
14	Selective Demolition	LS	LS	\$	300,000	100%	\$	300,000
15	Clean and Coat Existing Retaining Wall	LS	LS	\$	20,000	100%	\$	20,000
16	Change in Elevation of T-wall, Base Sla (P00006)		LS \$	230	,019.80 1	00% \$	3 2	30,019.80
17	Lump Sum Adj. for Driving Sheet Pilin between Sta. 65+79 to Sta. 67+64		LS	\$1	8,403.74	100%	\$	18,403.74
18	Removal of Const. Joint	LS	LS	(\$	24,000)	100%	(\$	24,000)
19	VECP Payment for Removal of Const. Joints	LS	LS	\$	13,200	100%	\$	13,200
20	Emergency Flood- ing Protection	LS	LS	\$	905	100%	\$	905

20. The contract was completed in accordance with the contract plans and specifications with final acceptance on 12 Dec 95.

Encl

CHRIS WAGNER

Project Engineer

New Orleans Area Office

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CF:
CELMN-CD-Q
CELMN-PA
CELMN-CT
CELMN-ED-C
CELMN-CD-B
CELMN-PP
CELMN-CD-CS
CELMN-OD-ON
Proj Engr (Wagner)
Proj Insp (B. Brogna)
Ofc Engr w/as-built

Safety is a Part of Your Contract

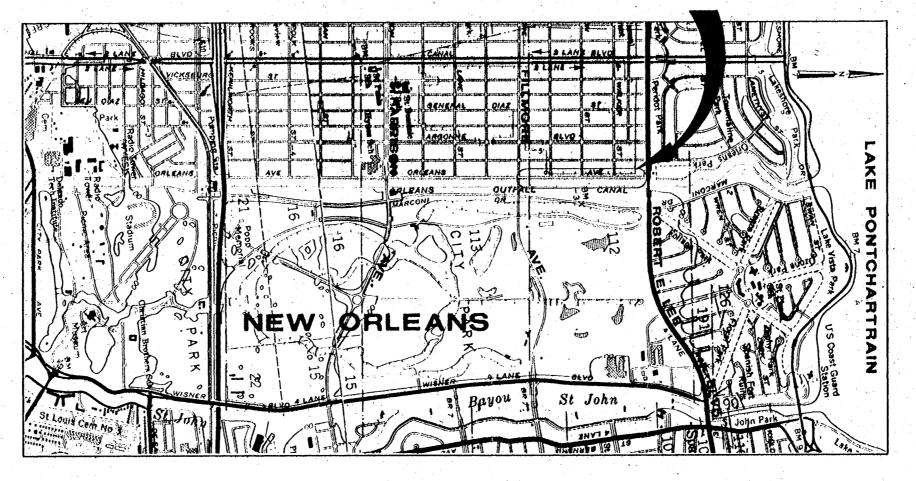
# ORLEANS AVENUE CANAL

"As built"

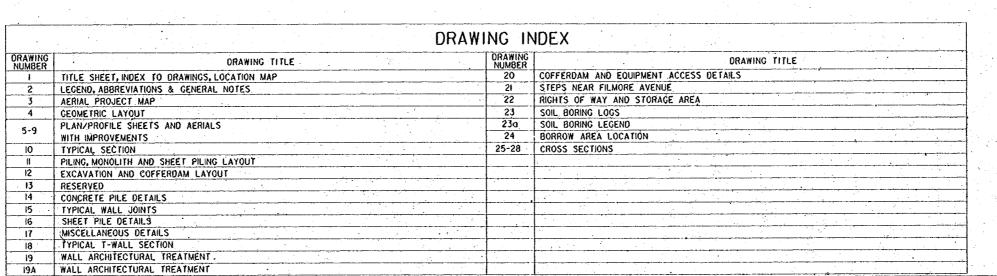
## FLOOD PROTECTION IMPROVEMENT PROJECT

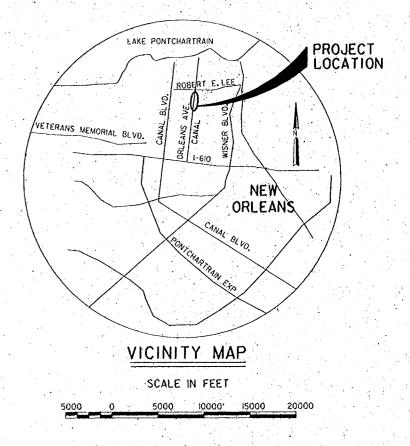
ORLEANS PARISH, LOUISIANA

PHASE 11-B LOCATION OF WORK

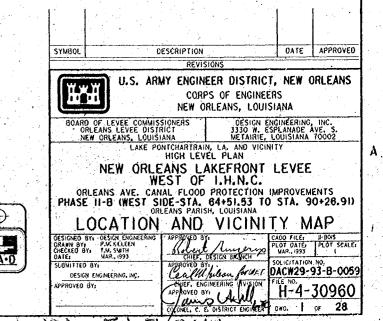


SCALE IN FEET 1000 0 1000 2000 3000 4000





PRAWINGS IN THIS FOLIO OF HAVE BEEN REDUCED ONE OF HALF THE ORIGINAL SCALE OF HALF THE ORIGINAL SCALE OF THE O





2 Not in CAD file JDJC Feb. 7, 5000

#### PROJECT BENCH MARK (M.S.L. EL = NGVD EL.)

EL. 7.11 M.S.L. (1983 DATUM). C. & G.S. REFERENCE MARK DISK "CHRYSLER RM". AT NEW ORLEANS, ABOUT 0.45 MILES EAST ALONG LAKESHORE DRIVE FROM JUNCTION OF CANAL BOULEVARD, 125 YARDS NORTHWEST OF THE NORTHWEST CORNER OF LAKESHORE DRIVE BRIDGE OVER ORLEANS CANAL, SET IN THE TOP OF THE CONCRETE SEA WALL ALONG SHORE OF LAKE PONTCHARTRAIN, 33 FEET NORTHEAST OF THE NORTH ONE OF A GROUP OF PALM TREES, 66 FEET WEST OF CHRYSLER STATION DESCRIBED, 270 FEET NORTH OF THE CENTER LINE OF LAKESHORE DRIVE AND ABOUT 2 FEET ABOVE THE LEVEL OF THE

- 1) Values in the table shall apply for bars spaced laterally not less than 3db and when bars have concrete cover which complies with
- 2) When bar spacing laterally is greater than 5db and when bars have 2.5db or more inches of concrete cover, the values in the table may be reduced by a factor of 0.8.
- 3) Embedment and splice lengths shall be a minimum of 12 inches.
- shall be spliced with mechanical butt splices only.
- 5) Top bars are horizontal bars and bars inclined less than 45 degress with respect to a horizontal plane which are placed such that

#### **ABBREVIATIONS**

BP

ВМ BENCH MARK BRACE POLE СВ CATCH BASIN CMP CORRUGATED METAL PIPE CN CONCRETE NAIL

CONC CONCRETE CP CONCRETE PIPE CSP CORRUGATED STEEL PIPE

CY CUBIC YARD DI DROP INLET

DIA DIAMETER D/W DRIVEWAY EL ELEVATION **EXIST EXISTING** 

FND **FOUND** FT FOOT HORIZ HORIZONTAL HWY HIGHWAY

ID INTERNAL DIAMETER

**INVERT** INV IR IRON ROD LF LINEAR FEET LG LONG

LT LEFT MAX MAXIMUM МН MANHOLE MIN MINIMUM

MSL. MEAN SEA LEVEL OD OUTSIDE DIAMETER OLB ORLEANS LEVEE BOARD PI POINT OF INTERSECTION

POT POINT ON TRAVERSE PROJ **PROJECT** 

PVC POLYVINYL CHLORIDE PIPE RCP REINFORCED CONCRETE PIPE

RD RET RETAINING REINE REINFORCING REQ'D REQUIRED

ROW OR R/W RIGHT-OF-WAY RR RAILROAD RRS RAILROAD SPIKE

RT RIGHT SECT SECTION

SMH SEWER MANHOLE ST STREET

STA BASELINE STATION TOP OF CURB

TC TEL TELEPHONE TYP TYPICAL UG **UNDERGROUND** 

USCGS UNITED STATES COAST & GEODETIC SURVEY VERT VERTICAL

WT WEIGHT WS WATER SURFACE

C/L CENTER LINE LEVEE/WALL WEST BASELINE

BE OR W.B WALL LINE W/L GALVANIZED IRON PIPE GIP

NIC NOT IN CONTRACT

NATIONAL GEODETIC VERTICAL DATUM NGVD

#### GENERAL NOTES

- I. All azimuths are true south azimuths turned in a clockwise direction from 0 degrees (due south).
- 2. All elevations are expressed in feet and refer to National Geodetic Vertical Datum (N.G.V.D.).
- All R/W and construction easement lines are parallel with or perpendicular to the baseline, unless otherwise indicated.
- 4. For log of borings, see DWG 23.
- 5. For existing utilities location and approximate location of relocated utilities see "Plan and Profile" drawings.

#### STEEL NOTES (STRUCTURAL STEEL)

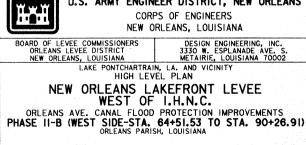
- I. All structural steel shall be ASTM A36, unless otherwise
- 2. To prevent corrosion by moisture between steel surfaces in contact, all such contacts shall be sealed watertight by running a continuous  $\frac{1}{8}$  fillet weld along all edges of the contact, unless otherwise noted.
- 3. All welding shall be electric welding. Workmanship and technique where applicable shall conform to the American Welding Society specifications AWS DI-83 Structural Welding
- 4. Welding symbols shown are those adopted by the American Welding Society and indicate only size and type of welds required. Detailed information shall be shown on the shop drawings and submitted by the contractor for approval.
- 5. Items marked C.R.S. shall be corrosion resistant steel (Stainless Steel). See specifications.

#### CONCRETE NOTES (CAST-IN-PLACE CONCRETE)

I. f'c = 3000 PSI, fy = 60,000 PSI.

C·A·D

- 2. All unformed surfaces shall be given a float finish, unless otherwise specified.
- 3. All exterior formed surfaces not covered by backfill shall be class "A" finish and surfaces covered by backfill shall be class "D" finish, unless otherwise noted.
- 4. Construction joints shall be provided where shown.
- 5. Unless otherwise noted, provide  $\frac{3}{4}$  chamfer at all exposed joints, edges, external corners, vertical expansion joints and horizontal construction joints.
- 6. All primary reinforcement shall have a minimum cover of 3' unless otherwise noted. The cover for secondary reinforcement may be reduced from the above by the diameter
- 7. All bends of reinforcement and all bar spacers and supports shall be in accordance with the American Concrete Institute Manual of Standard Practice for Detailing Reinforced Concrete Structures' in effect at the time of fabrication.
- 8. Reinforcing bar designation numbers conform to the current numbering system of the Concrete Reinforcing Steel Institute.
- 9. All reinforcing shall be lapped or embedded according to the tables on this drawing, unless otherwise noted.



DESCRIPTION

DATE APPROVE

### GENERAL NOTES AND LEGEND

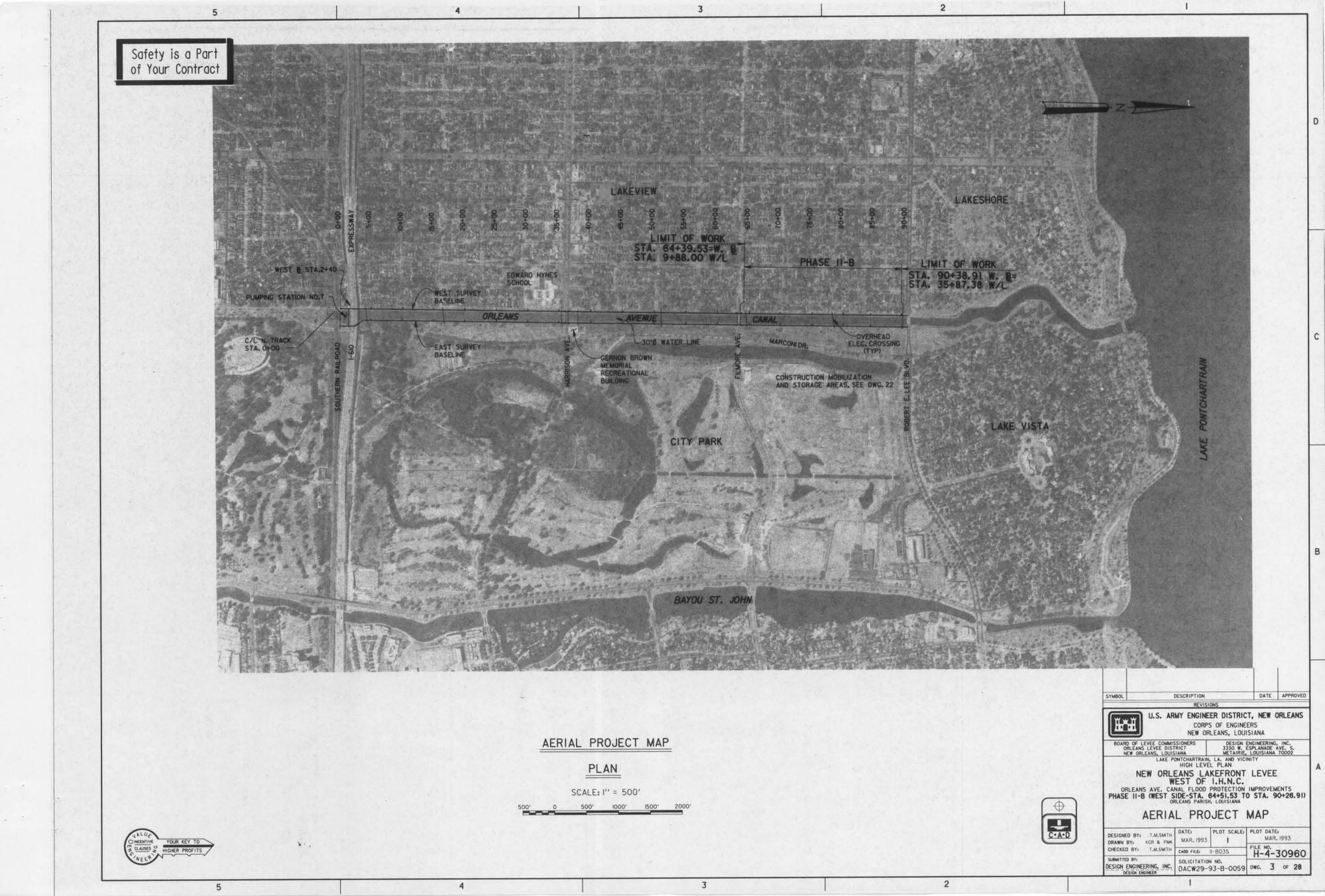
DESIGNED BY: M.B.SHUKLA	DATE: MAR. 1993	PLOT SCALE:	PLOT DATE: MAR. 1993		
CHECKED BY: P.M.KILLEEN CHECKED BY: T.M.SMITH		I-B02S	FILE NO. H-4-30960		
SUBMITTED BY: DESIGN ENGINEERING, INC. DESIGN ENGINEER	SOLICITATION DACW29-9		DWG. 2 OF 28		

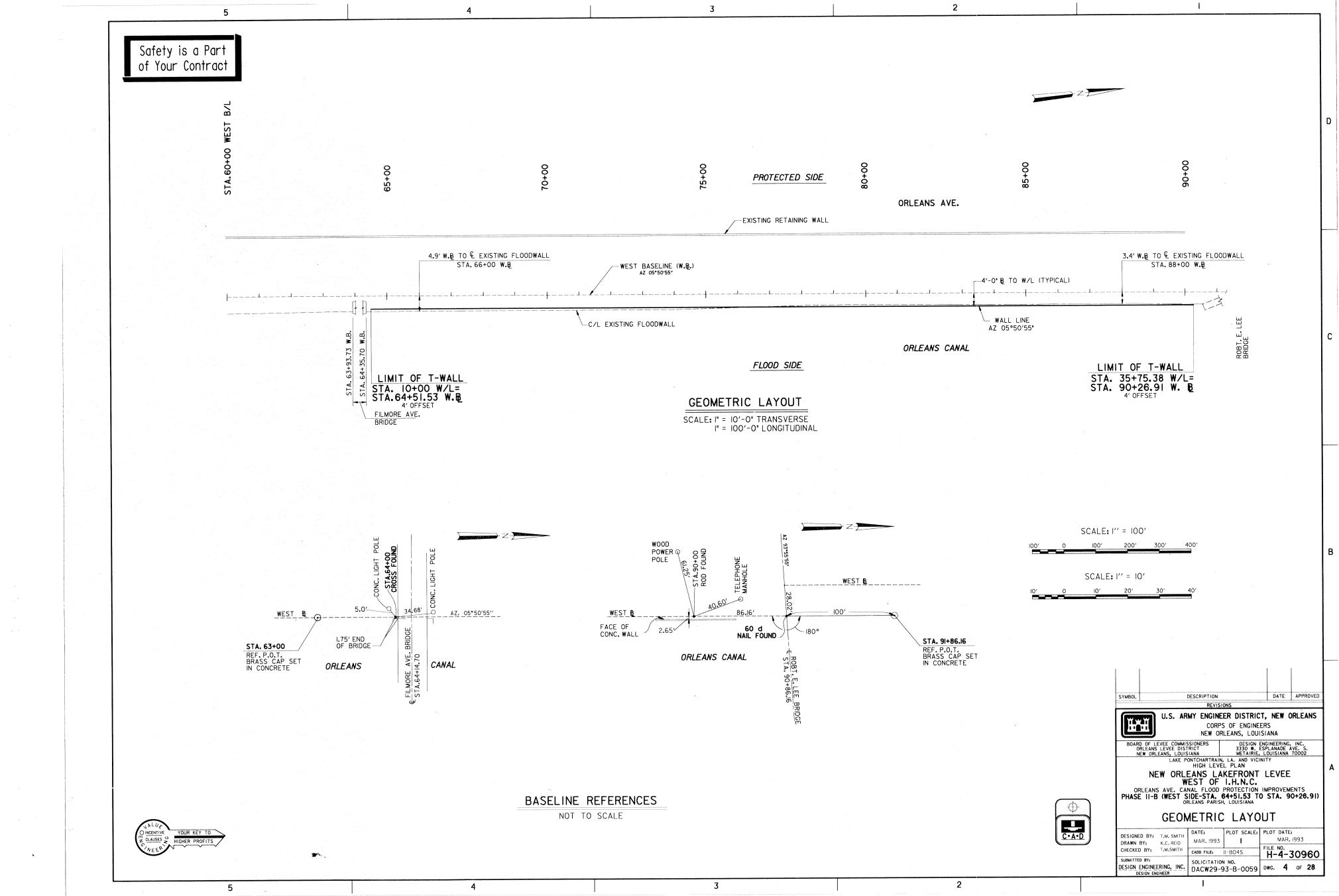
U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS

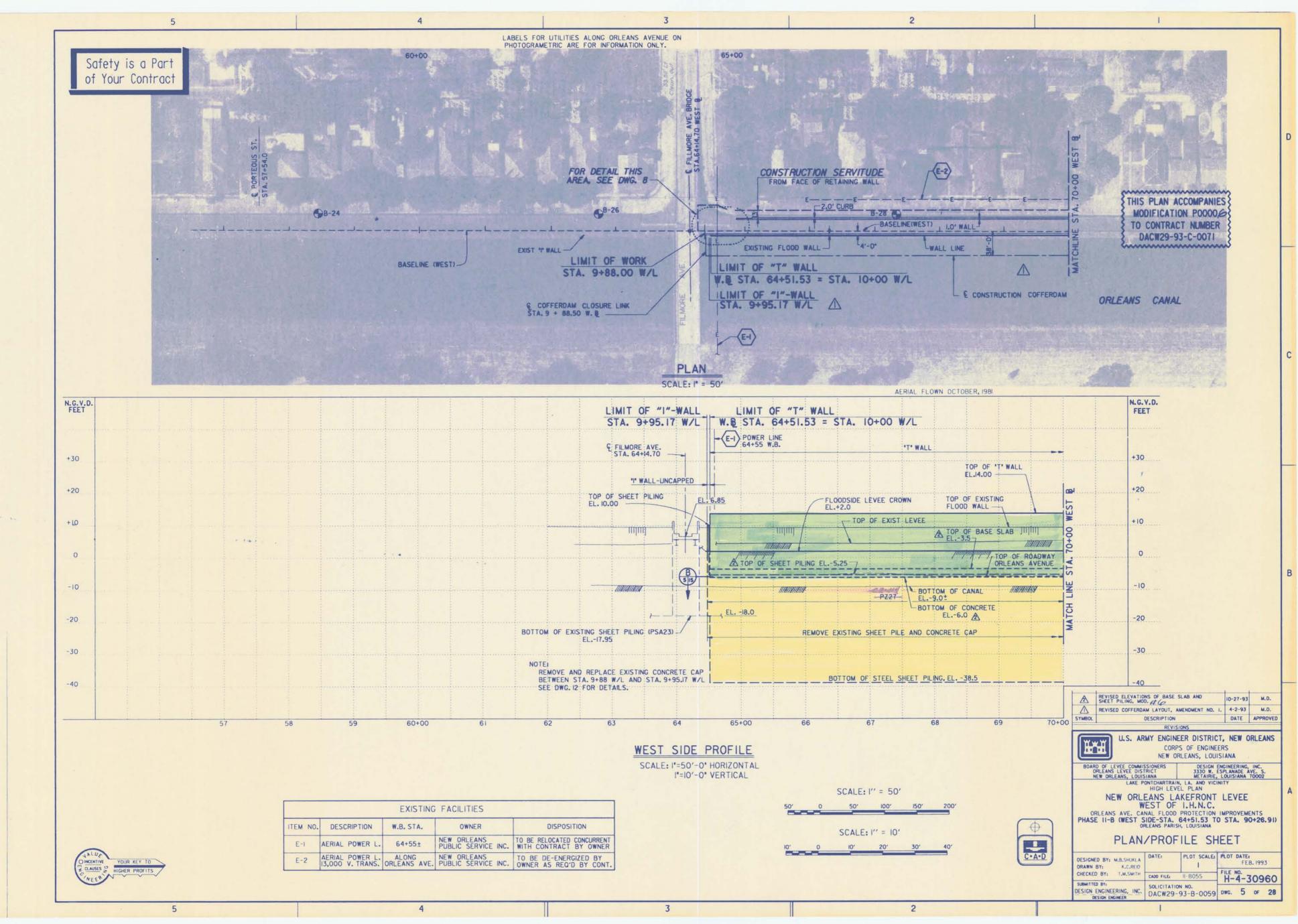
4) \*Number 14 and 18 bars shall not be lap spliced. These bars

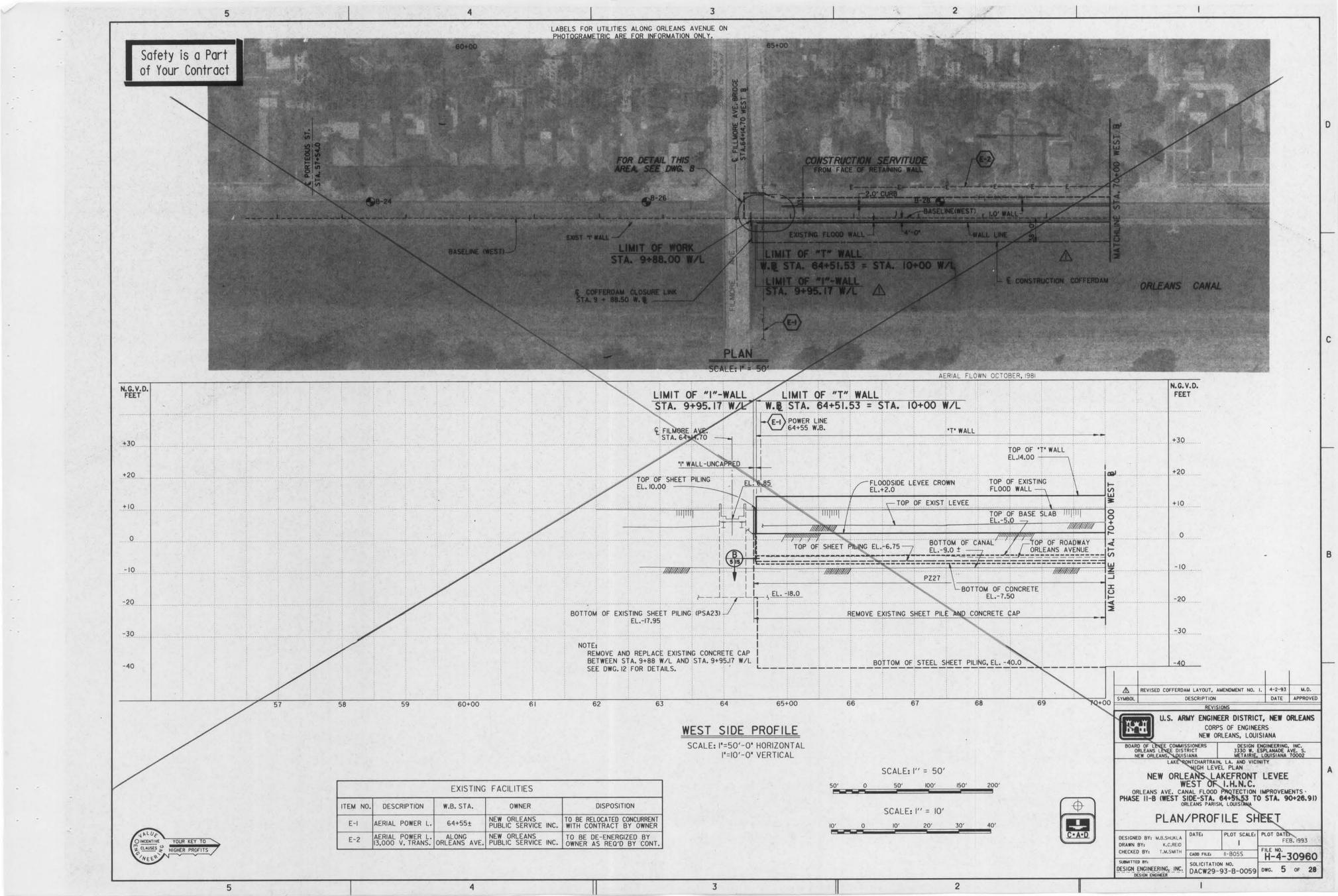
more than 12 inches of concrete is cast in the member below the bar.

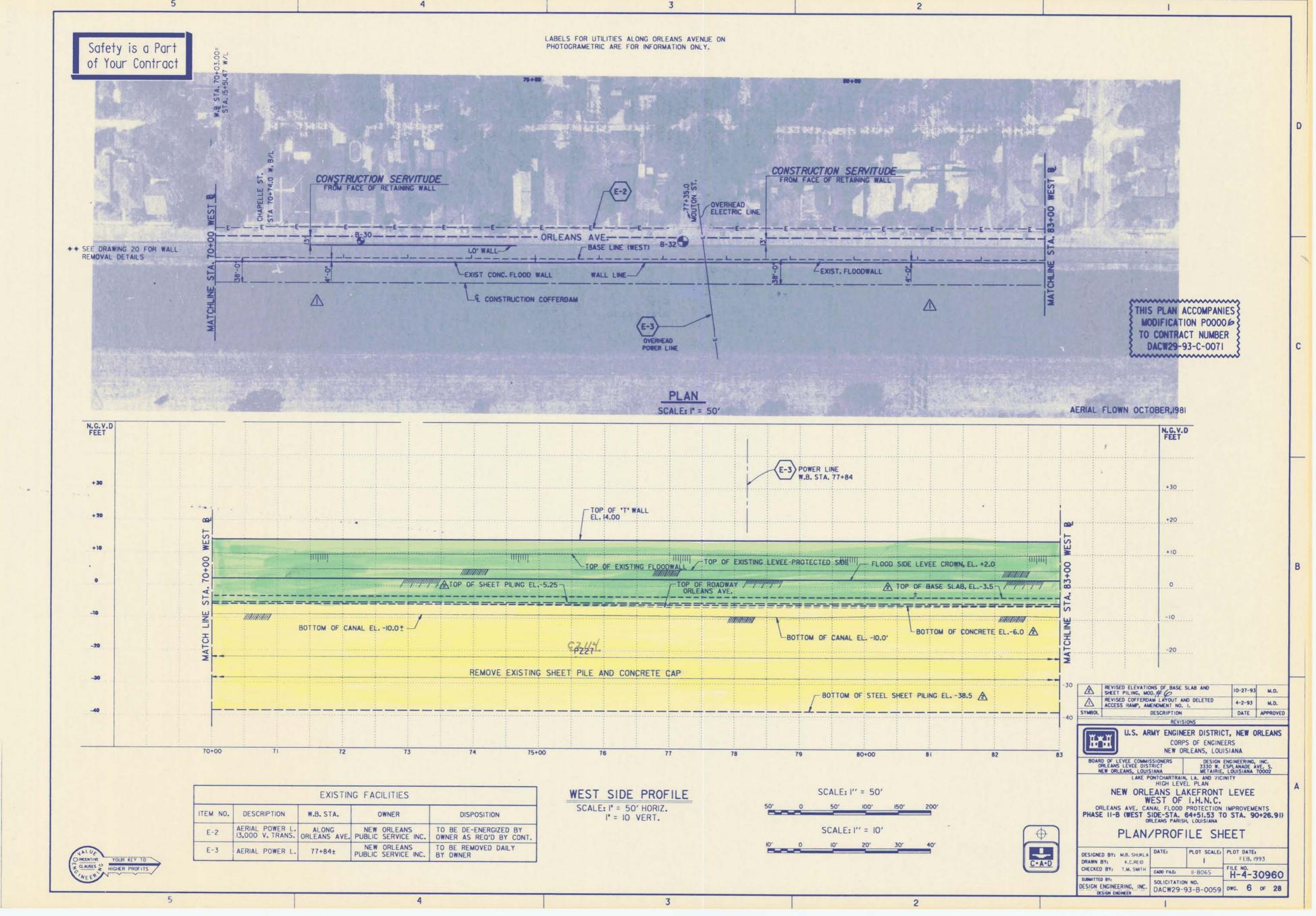
2

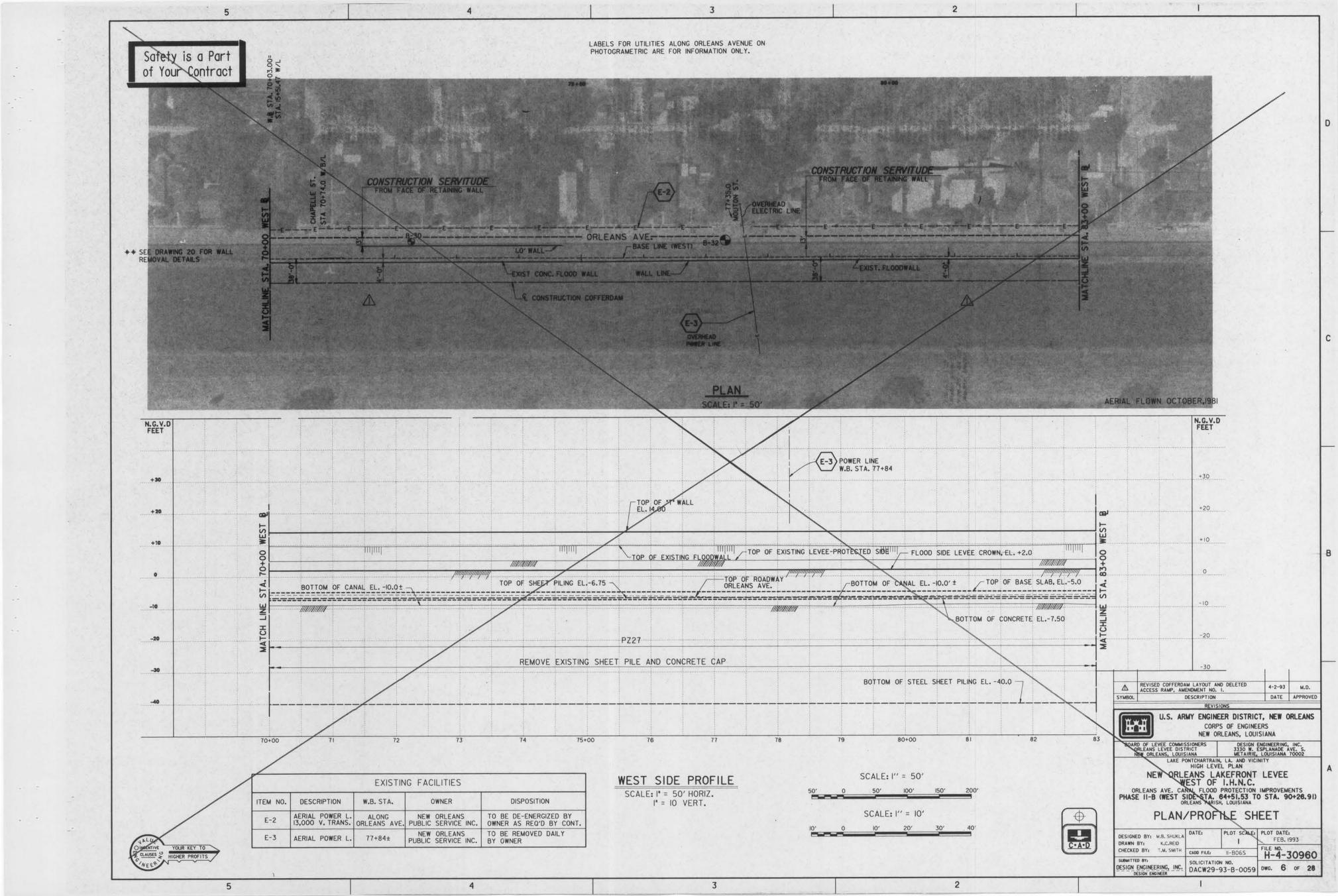


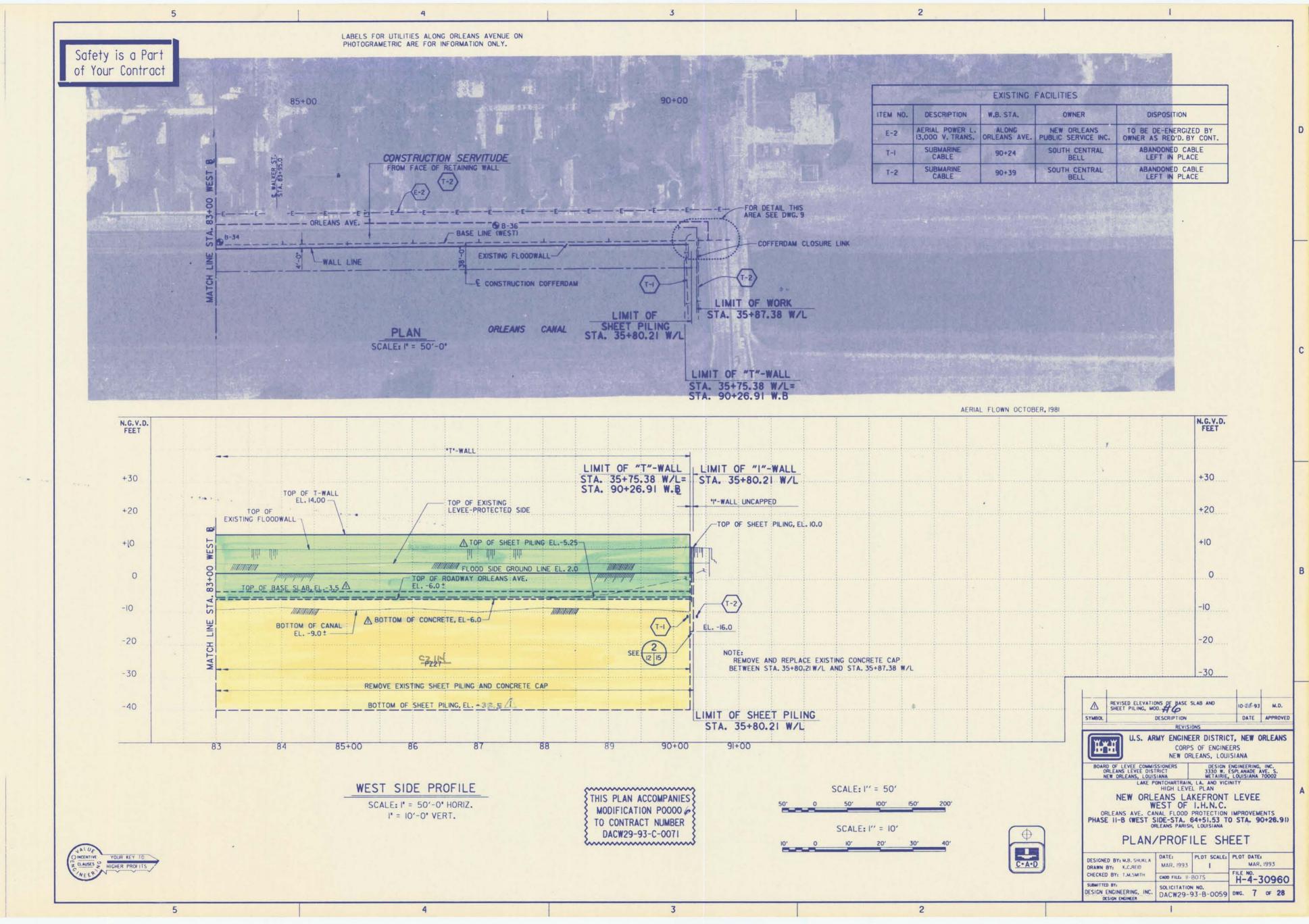


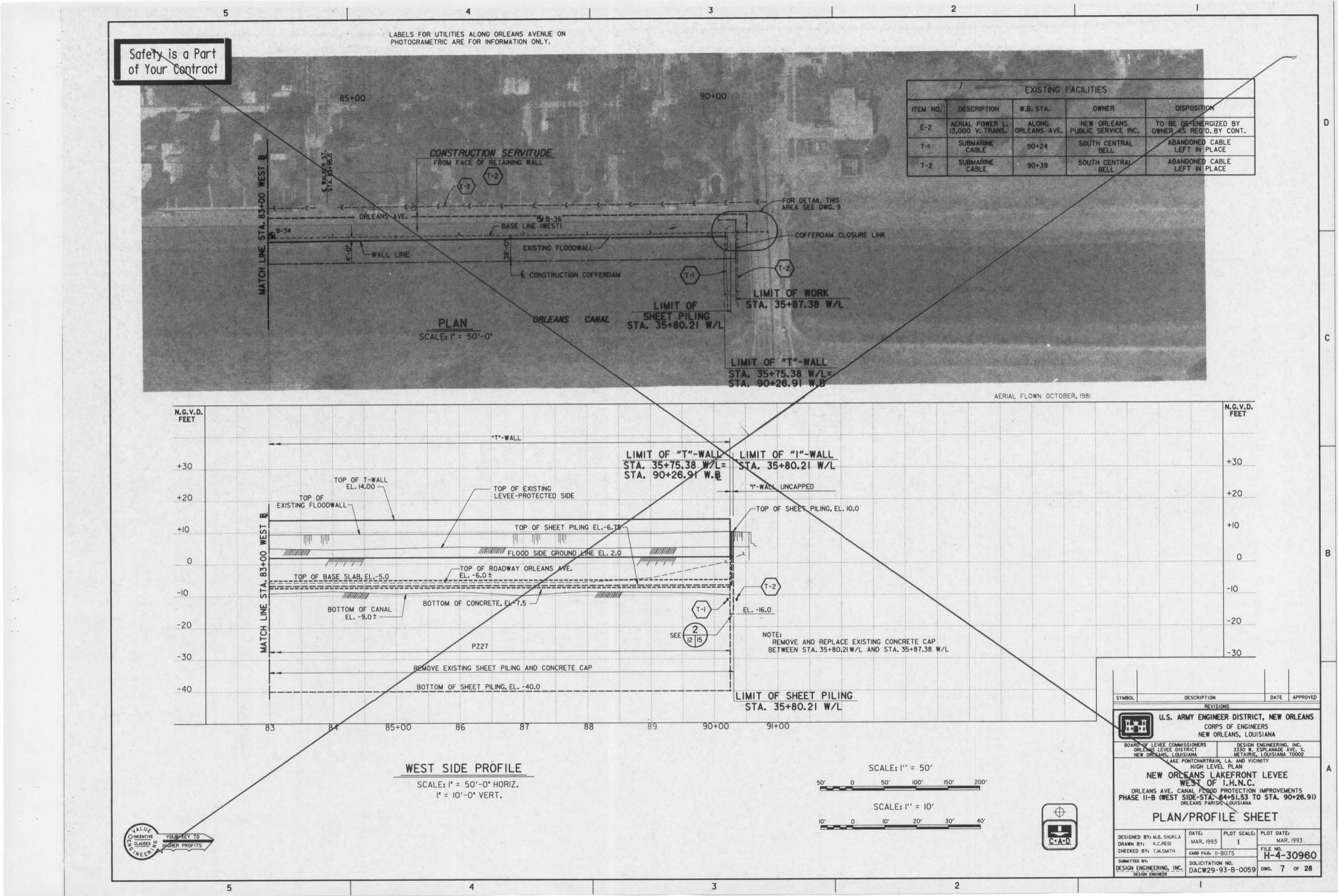


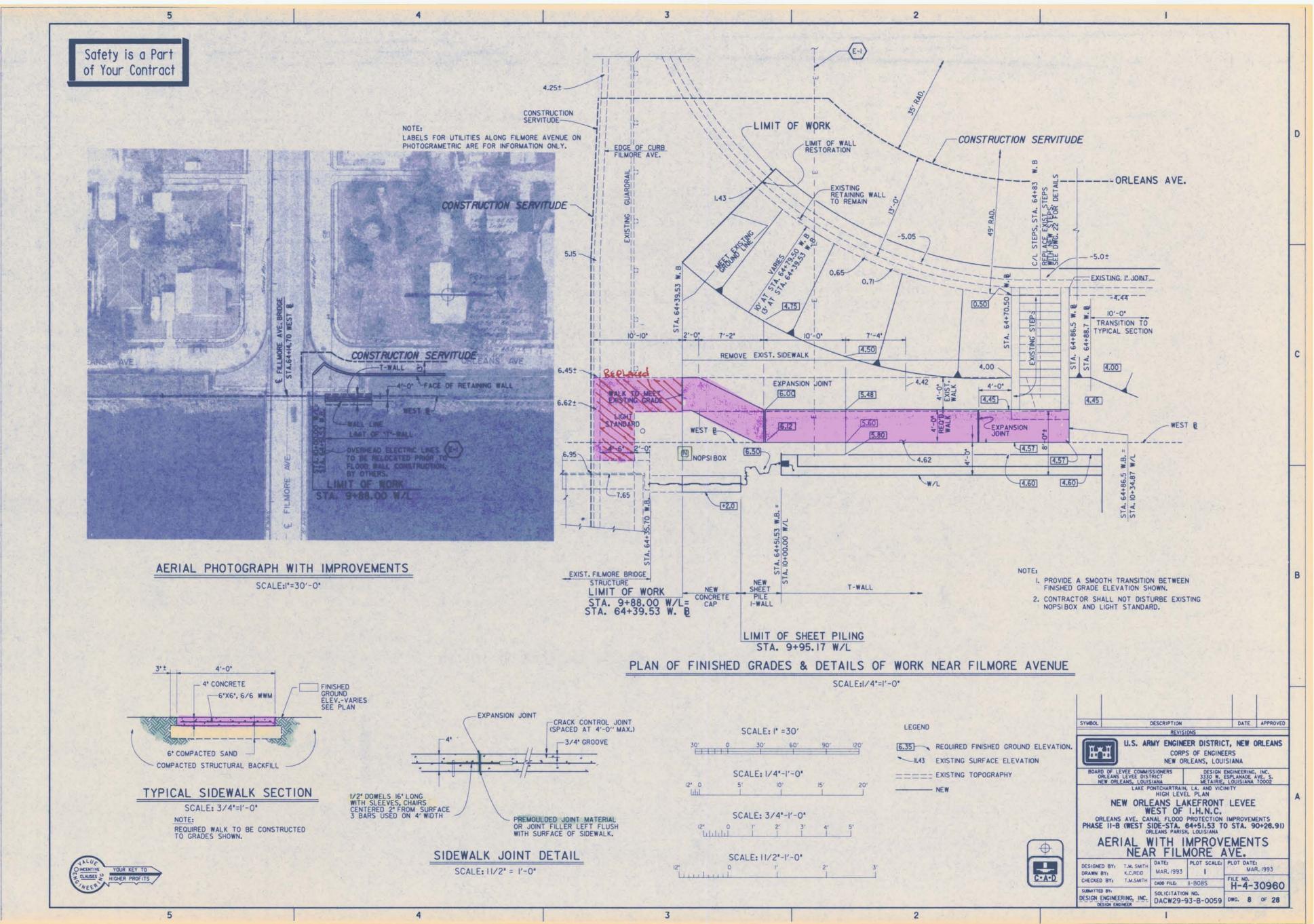


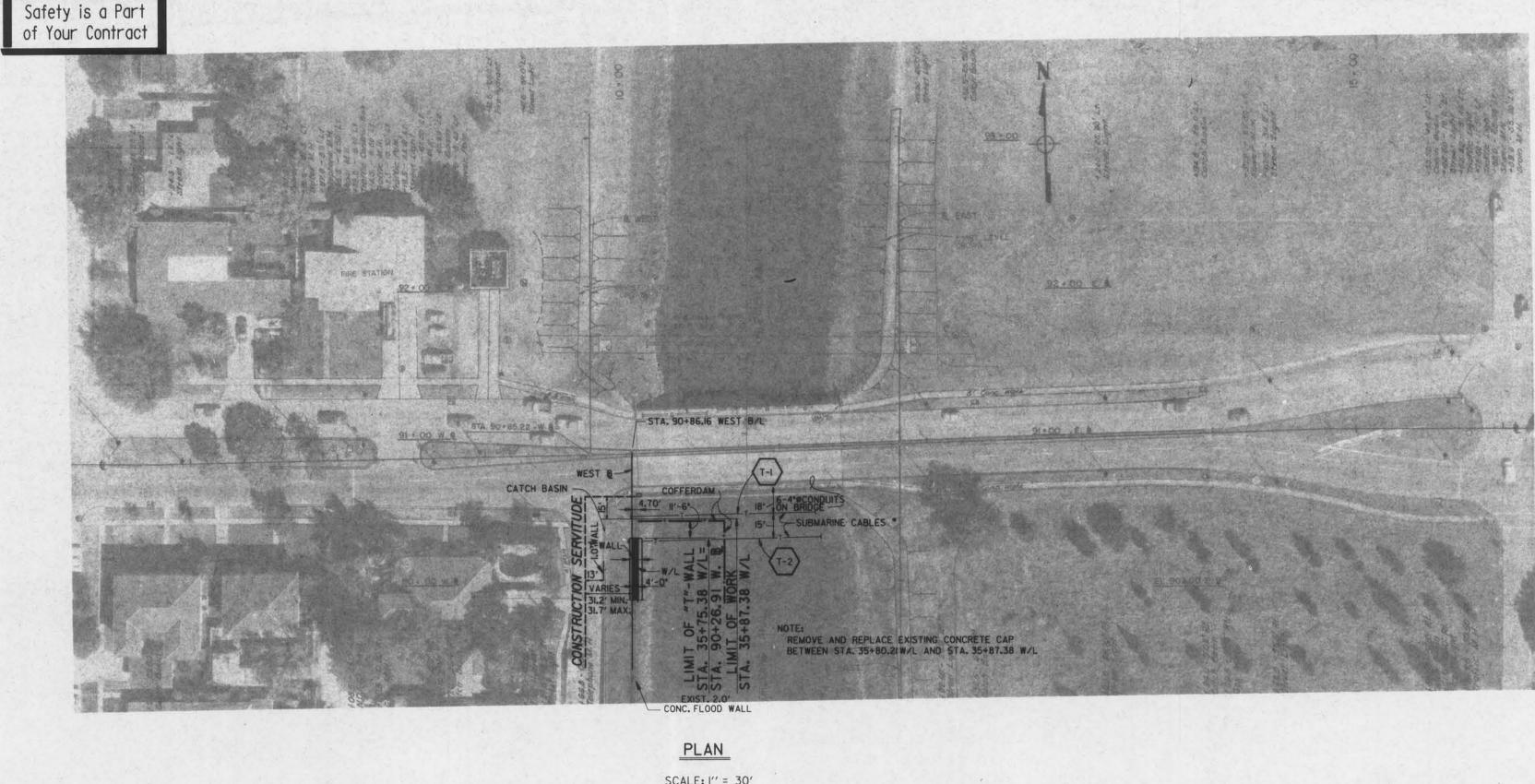












SCALE: I" = 30"

\* SOUTH CENTRAL BELL SUBMARINE CABLES HAVE BEEN DEACTIVATED AND ABANDONED BY SOUTH CENTRAL BELL CO., INC.. REPLACEMENT LINES HAVE BEEN PLACED INTO THE EXISTING 4° Ø CONDUITS ON BRIDGE PRIOR TO CONTRACT AWARD.

CONTRACTOR SHALL DRIVE REQUIRED SHEET PILING THROUGH ABANDONED SUBMARINE CABLES.

LABELS FOR UTILITIES ALONG ROBERT E. LEE BOULEVARD ON PHOTOGRAMETRIC ARE FOR INFORMATION ONLY.



DATE APPROVE U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA

NEW ORLEANS, LOUISIANA

BOARD OF LEVEE COMMISSIONERS DESIGN ENGINEERING, INC.
ORLEANS LEVEE DISTRICT 3330 W. ESPLANADE AVE. S.
METAIRIE, LOUISIANA 70002

LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN

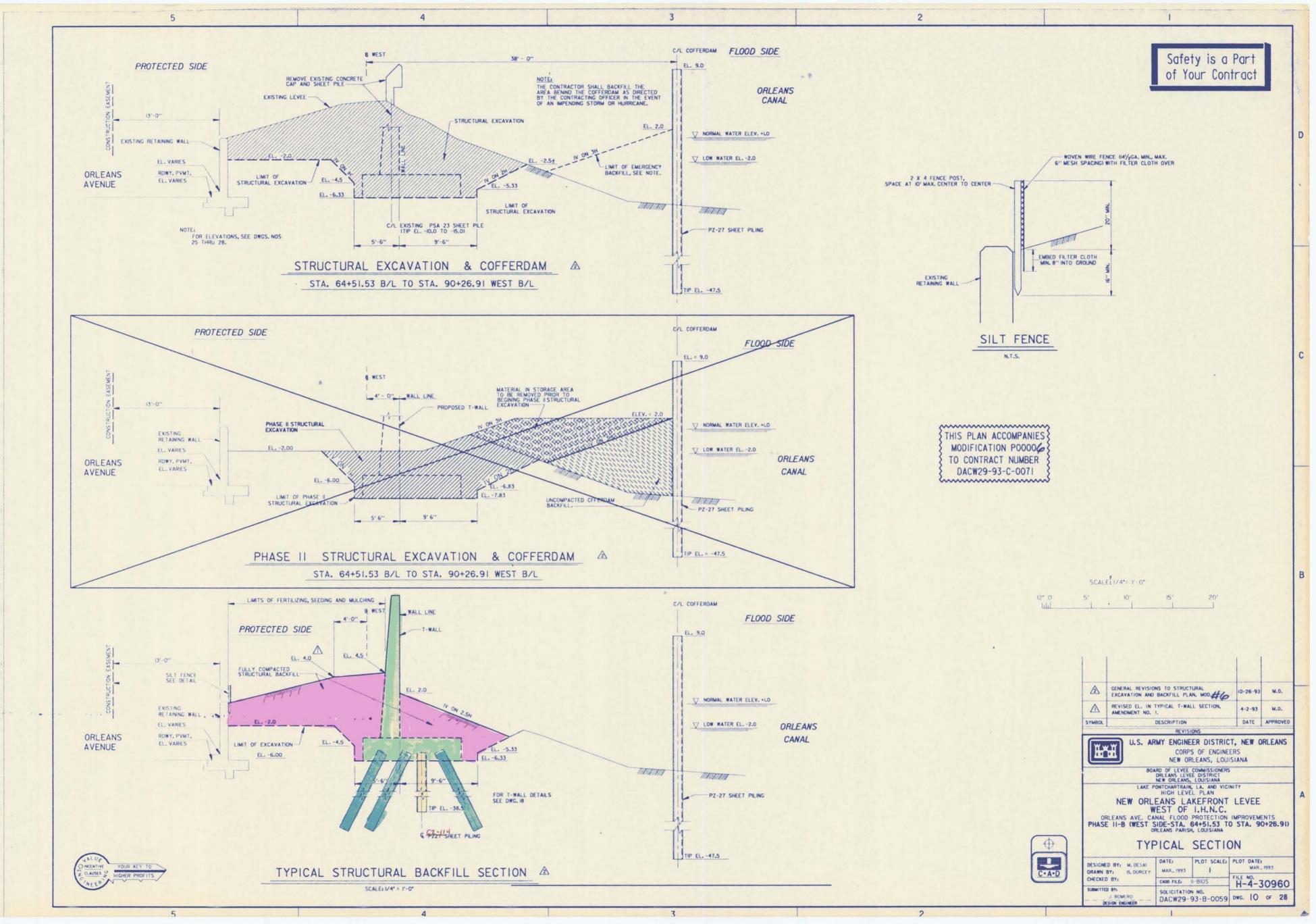
NEW ORLEANS LAKEFRONT LEVEE
WEST OF 1.H.N.C.
ORLEANS AVE. CANAL FLOOD PROTECTION IMPROVEMENTS
PHASE II-B (WEST SIDE-STA. 64+51.53 TO STA. 90+26.91)
ORLEANS PARISH, LOUISIANA

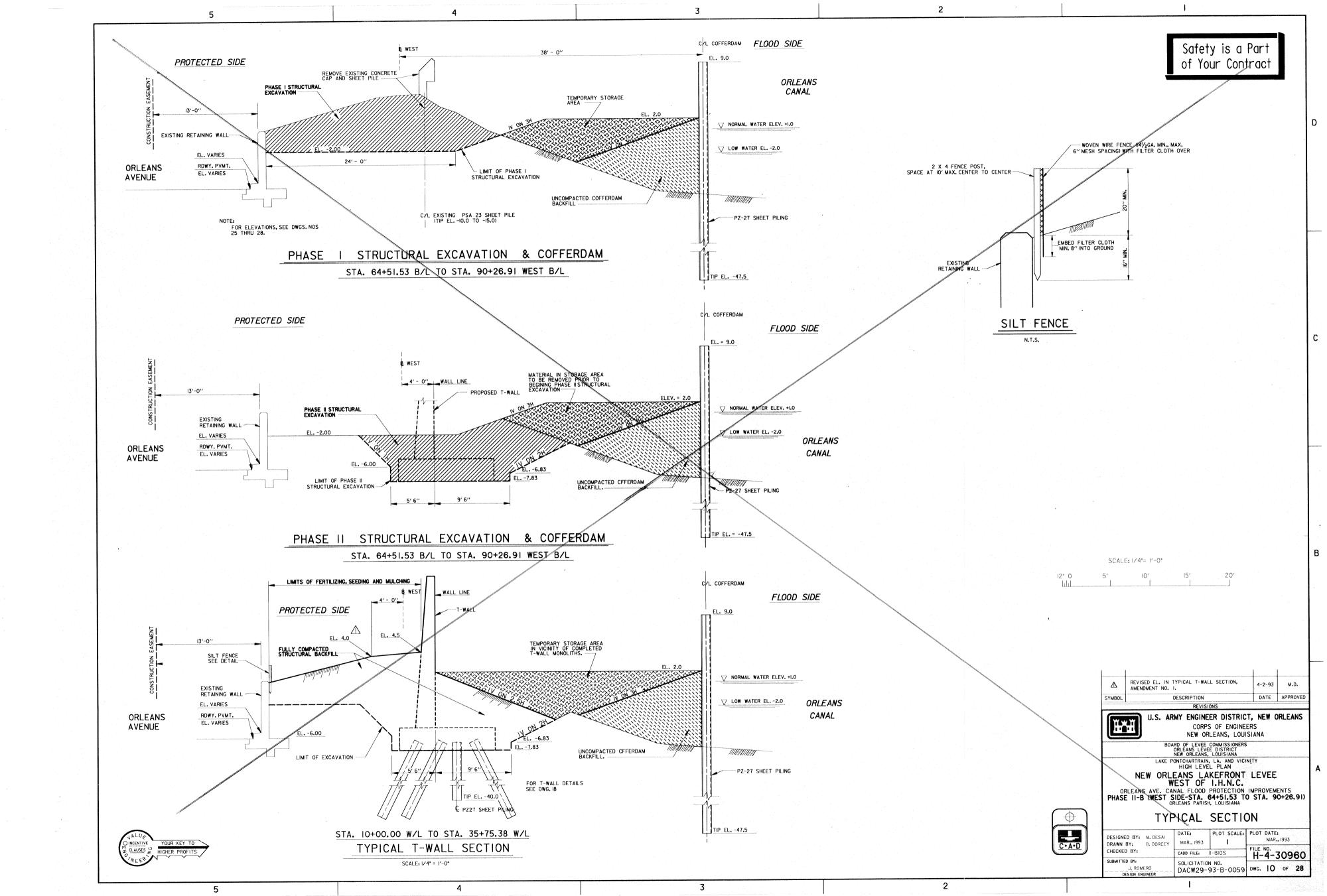
AERIAL WITH IMPROVEMENTS
NEAR ROBERT E. LEE

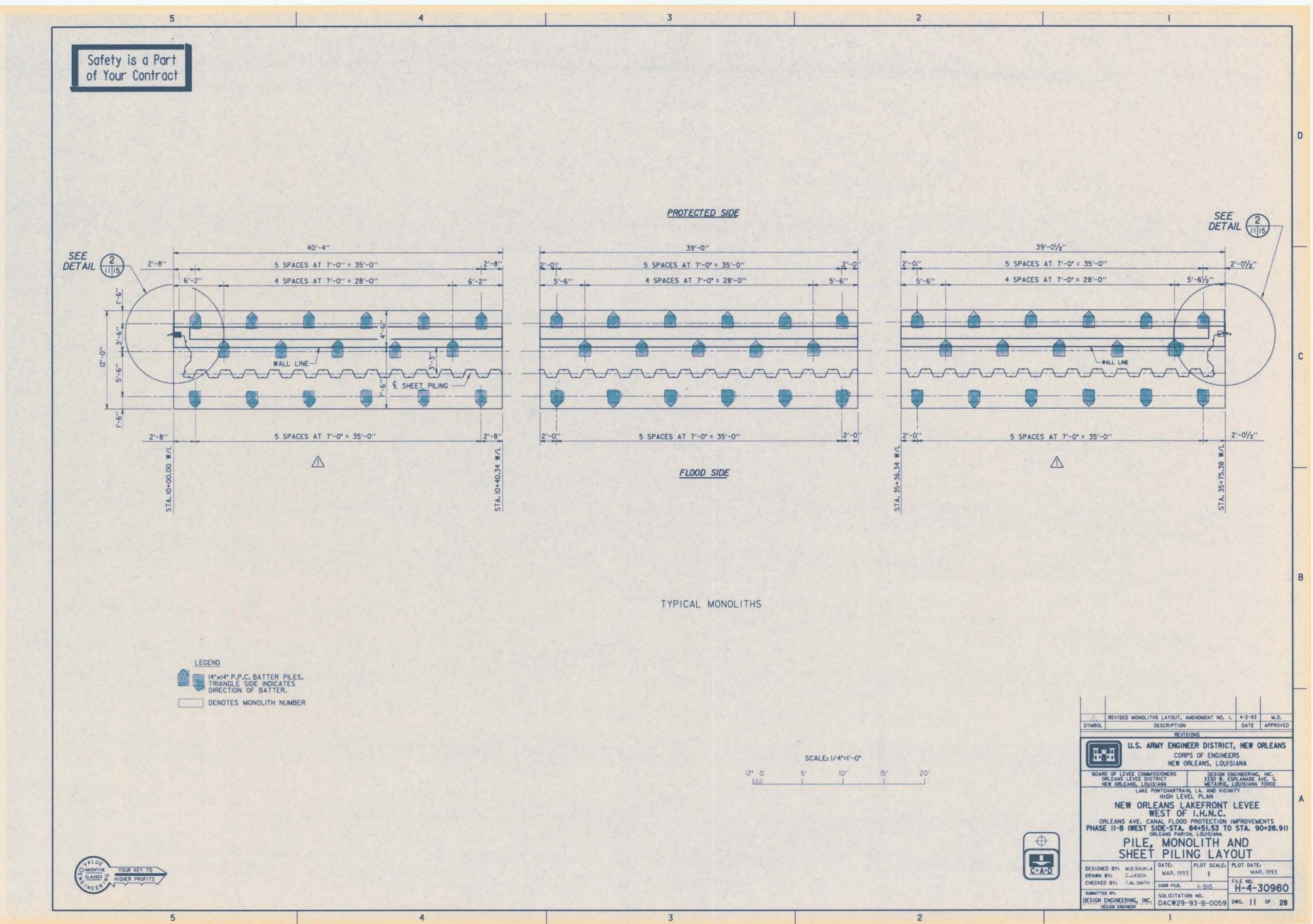
PLOT SCALE: PLOT DATE:
MAR. 1993 DESIGNED BY: T.M. SMITH
DRAWN BY: K.C.REID
CHECKED BY: T.M.SMITH
CA00 FILE MAR. 1993 FILE NO. H-4-30960 CAOO FILE: II-BO9S DESIGN ENGINEERING, INC. DACW29-93-B-0059 DWG. 9 OF 28

5

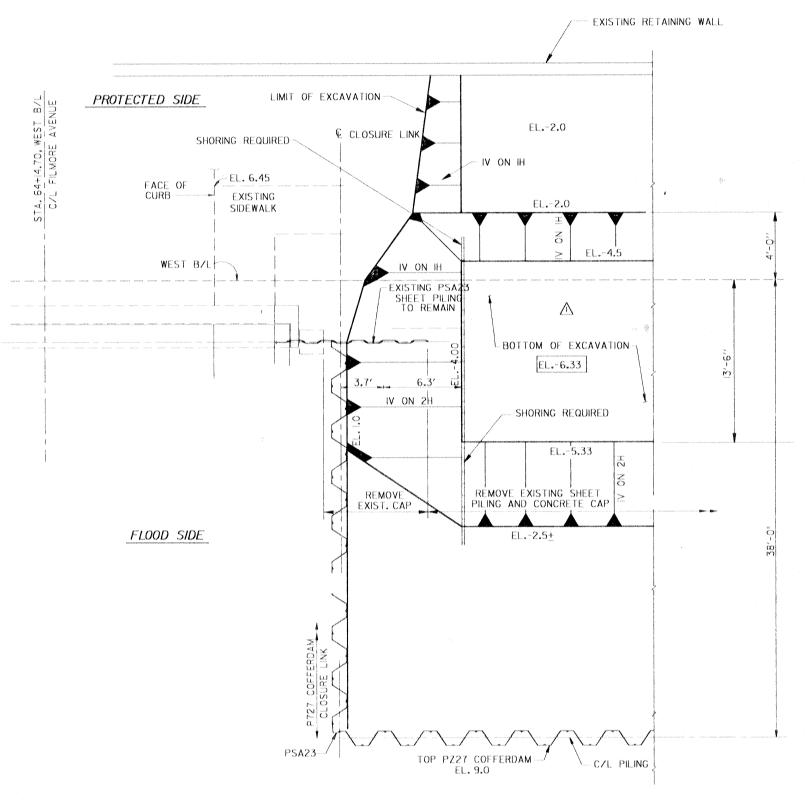
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Safety is a Part of Your Contract

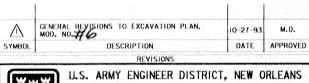


THIS PLAN ACCOMPANIES MODIFICATION POODOG TO CONTRACT NUMBER DACW29-93-C-0071

ORLEANS CANAL

EXCAVATION PLAN AND COFFERDAM FILL LAYOUT

SCALE: 1/4" = 1'-0"



CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA

NEW ORLEANS, LOUISIANA

BOARD OF LEVEE COMMISSIONERS
ORLEANS LEVEE DISTRICT
NEW ORLEANS, LOUISIANA

LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN

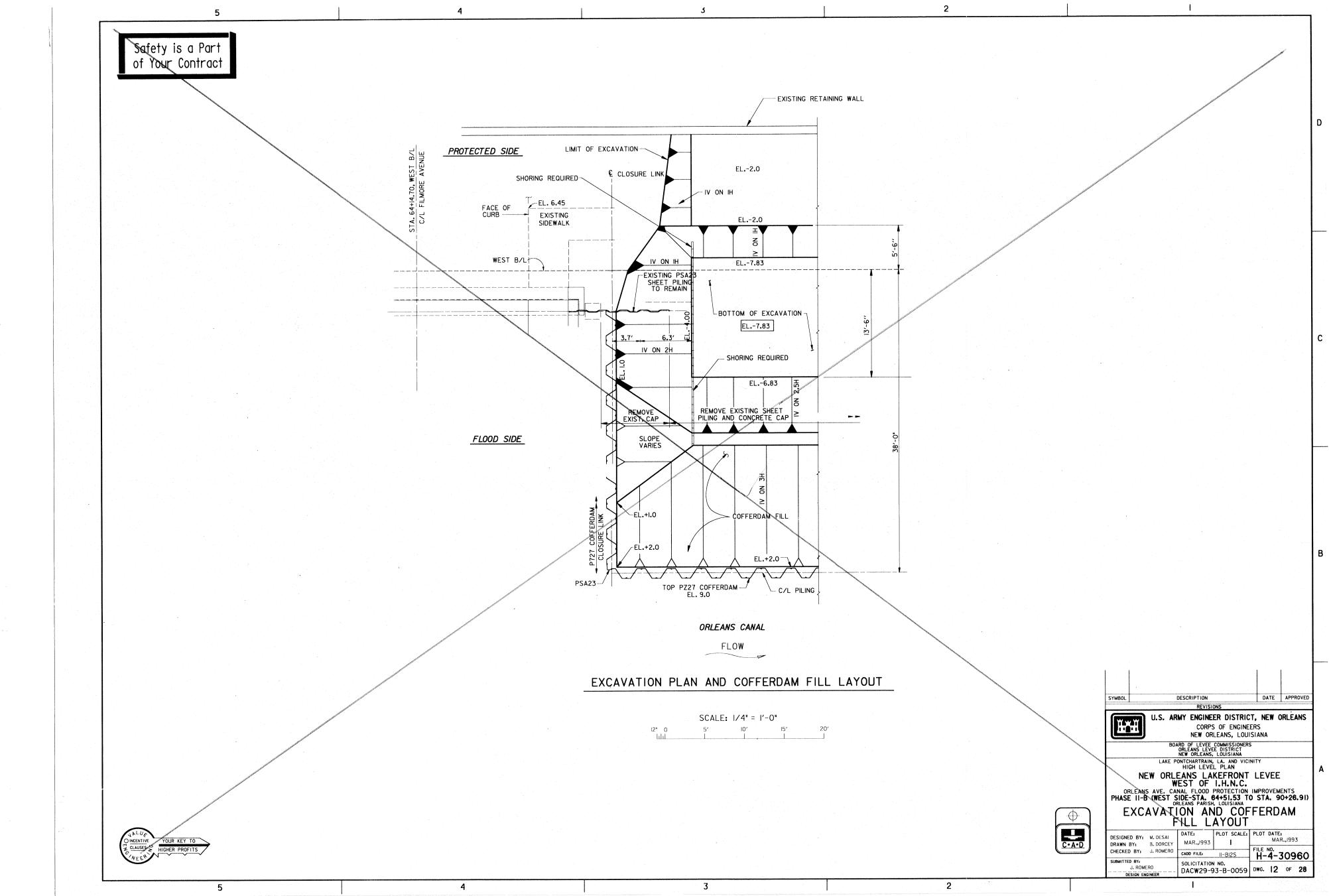
NEW ORLEANS LAKEFRONT LEVEE
WEST OF I.H.N.C.
ORLEANS AVE. CANAL FLOOD PROTECTION IMPROVEMENTS
PHASE II-B (WEST SIDE-STA. 64+51.53 TO STA. 90+26.91)
ORLEANS PARISH, LOUISIANA

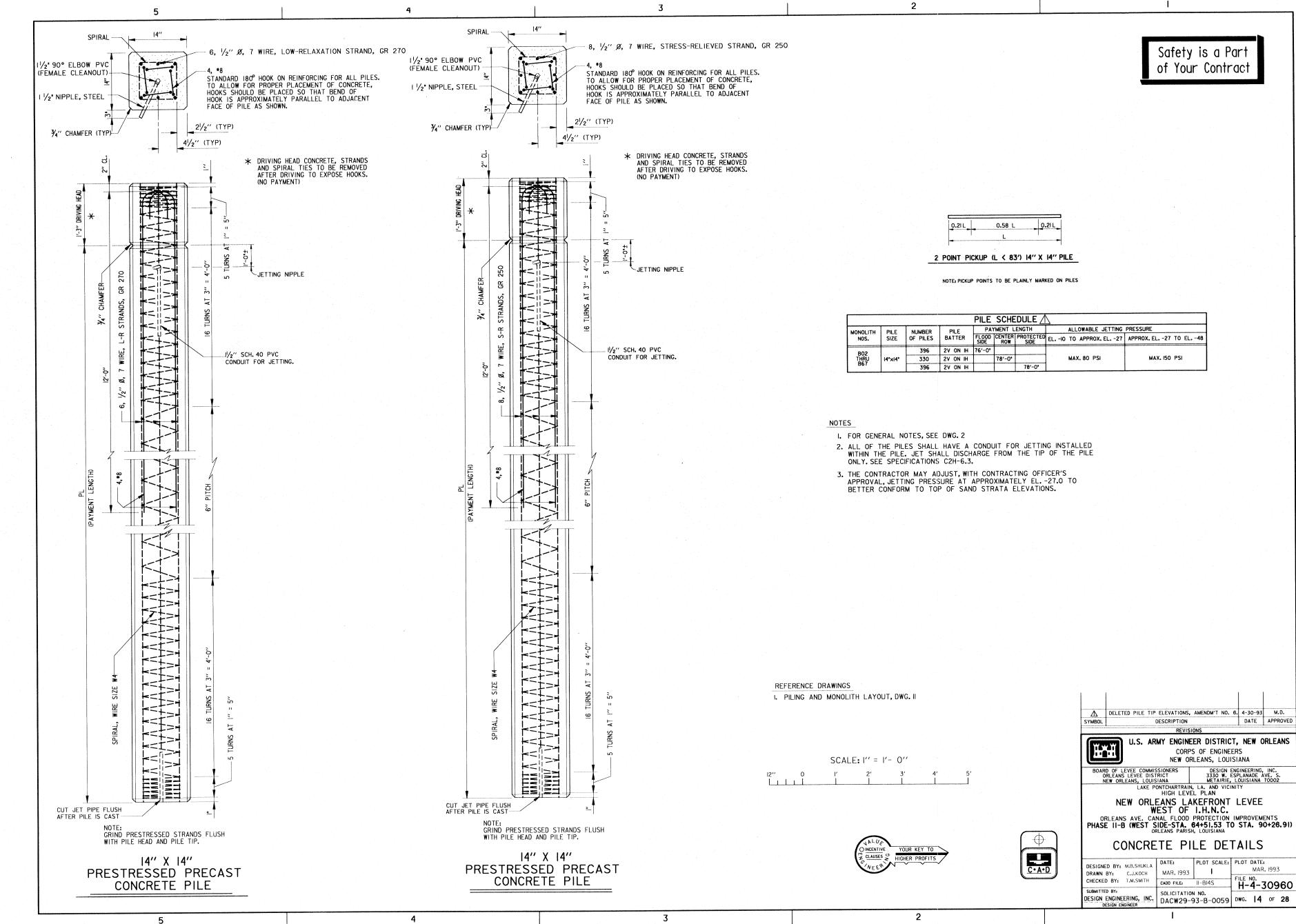
EXCAVATION AND COFFERDAM
FILL LAYOUT

DATE:
PLOT SCALE PLOT DATE:

| DESIGNED BY: M. DESAI DRAWN BY: B, DORCEY CHECKED BY: J. ROMERO DESIGN ENGINEER | DATE: MAR.,1993 | I | FILE NO. H-4-30960 | H-4-30960 |

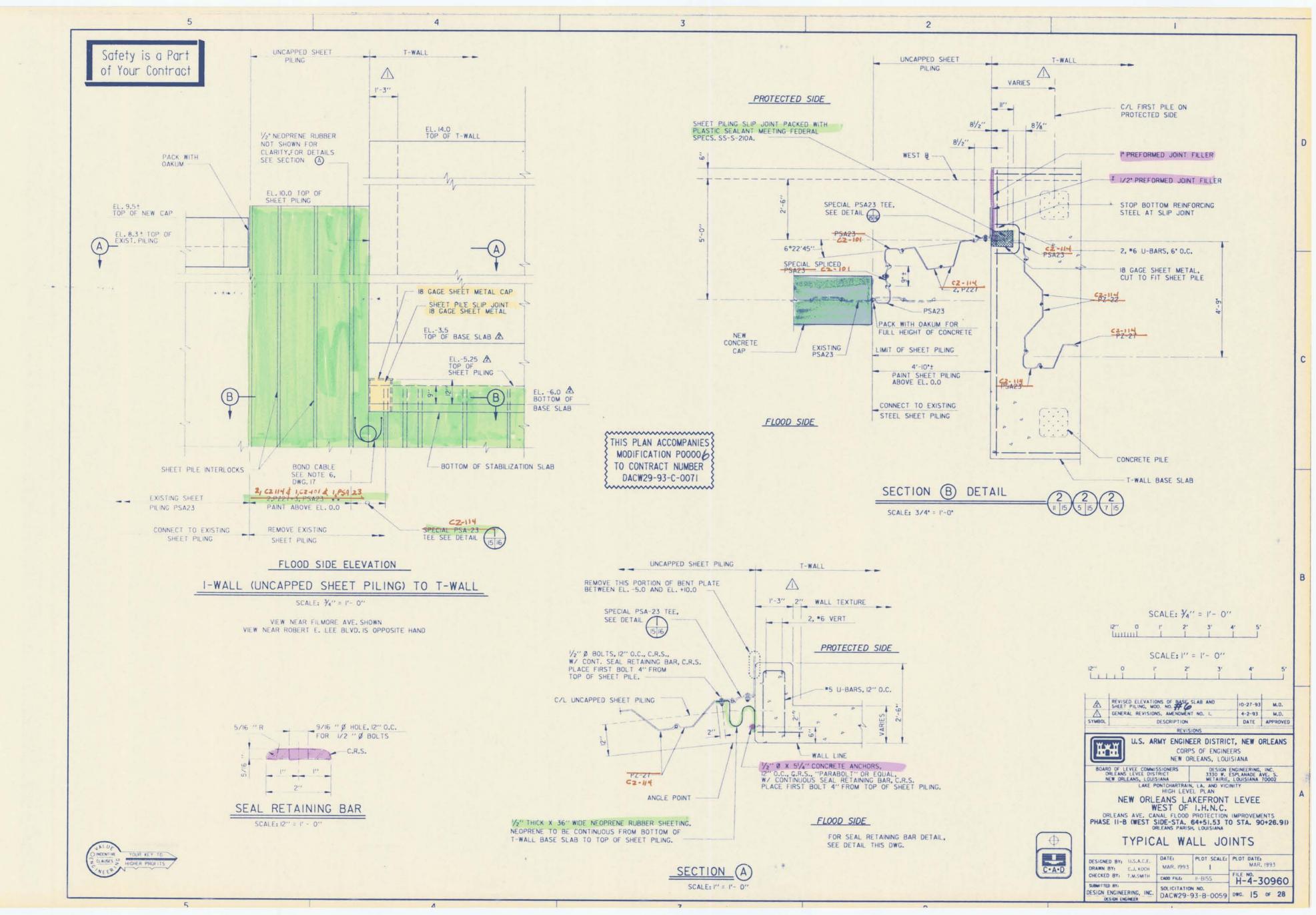
SUBMITTED BY: J. ROMERO DESIGN ENGINEER

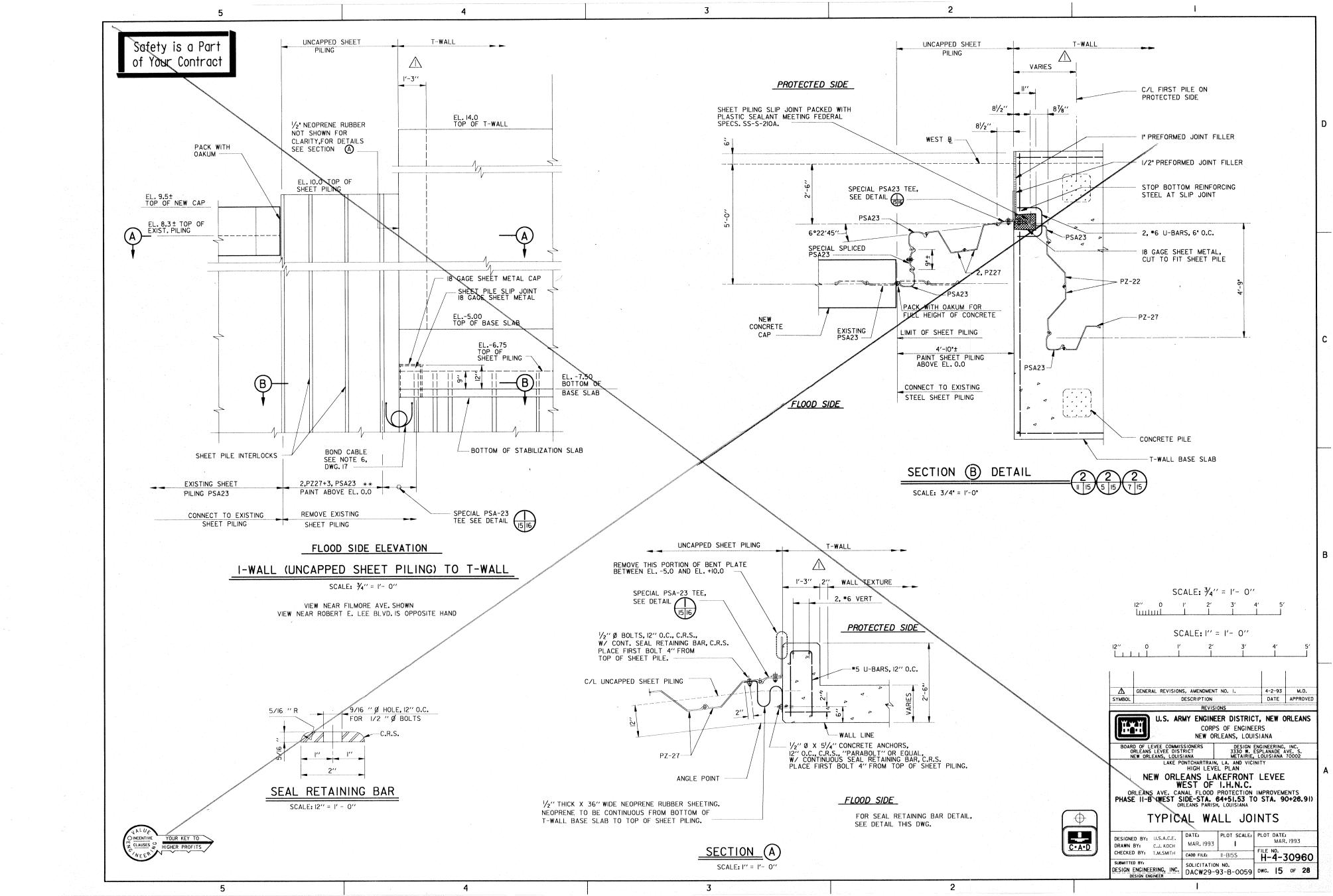


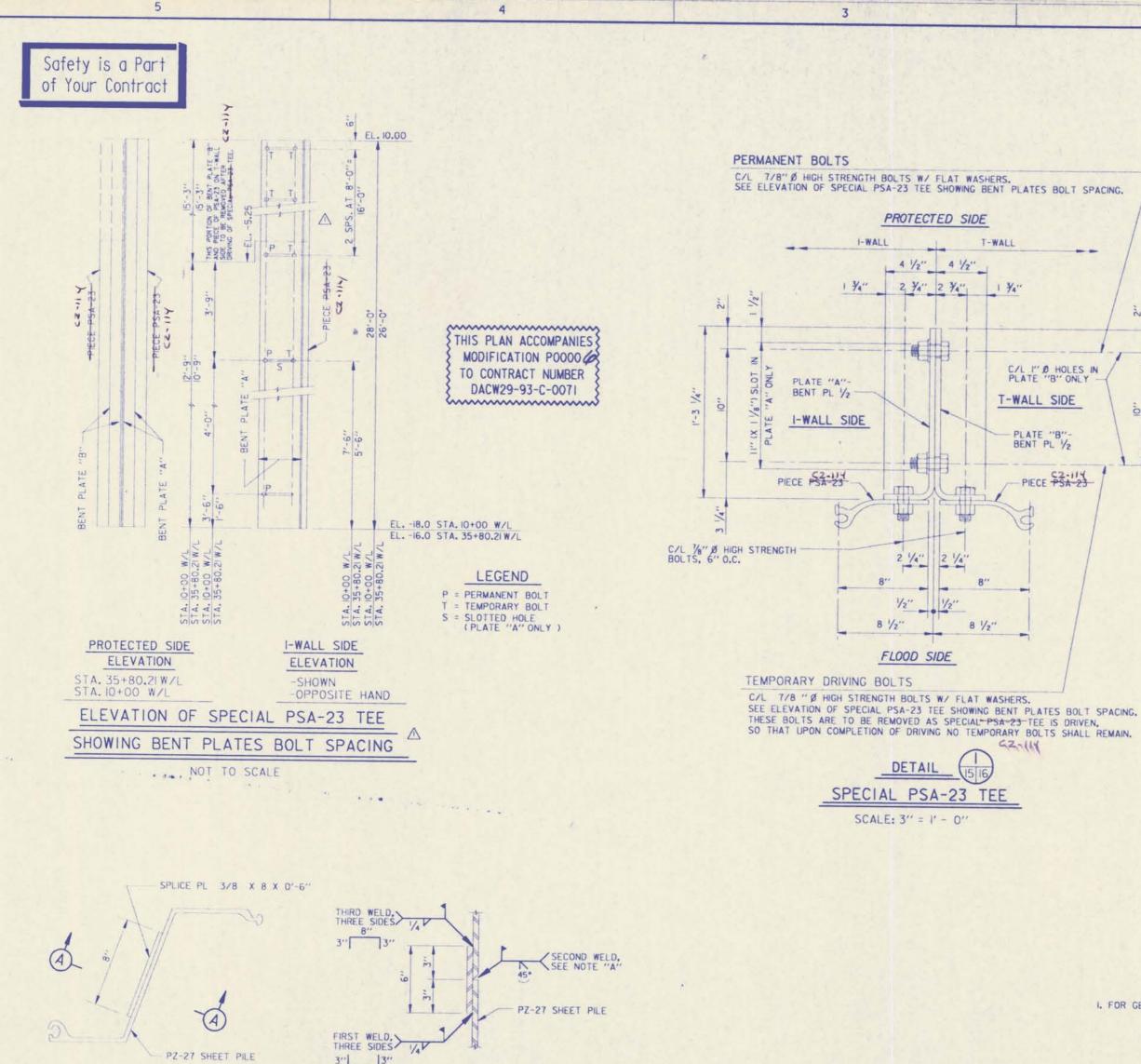


С

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SECTION (A

GROOVE WELD SHALL EXTEND THE FULL LENGTH OF THE SHEET PILE WEB AND FLANGES EXCLUDING THE INTERLOCKS.

SHEET PILE NOTES:

- I. HOLES CUT IN STEEL SHEET PILING FOR PASSING REINFORCING BARS SHALL NOT EXCEED 2"Ø. WHERE HOLES FALL WITHIN THE WEB OF THE STEEL SHEET PILE, THE HOLE SHALL BE SLOTTED 4" HORIZONTALLY TO ACCOMMODATE PASSING THE PENEROPPING RAPS REINFORCING BARS.
- 2. ANY SUBSTITUTIONS SHALL BE SUBMITTED TO THE CONTRACTING OFFICER REPRESENTATIVE FOR APPROVAL.
- 3. STEEL SHEET PILE SURFACE PREPARATION AND PAINTING SHALL BE IN ACCORDANCE WITH SECTION C9A OF THE SPECIFICATIONS. STEEL SHEET PILING NOT ENCASED IN CONCRETE AND EXPOSED TO AIR SHALL BE PAINTED ON BOTH SIDES FROM TOP OF PILING TO A LEYEL 2'-O" BELOW LOWER FINISHED GROUND LINE. SECTION (B), SHEET 15.

NOTES I. FOR GENERAL NOTES, SEE DWG. 2

C-A-D

REVISED DIMENSIONS AND ELEVATIONS OF SPECIAL PSA-23 TEE, MOD. NO. 10-27-93 M.D. DATE APPROVE U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA NEW ORLEANS LAKEFRONT LEVEE
WEST OF I.H.N.C.
ORLEANS AVE. CANAL FLOOD PROTECTION IMPROVEMENTS
PHASE II-B (WEST SIDE-STA. 64+51.53 TO STA. 90+26.91)
ORLEANS PARISH, LOUISIANA

SHEET PILE DETAILS

DESIGNED BY: LLS.A.C.E. DATE: PLOT SCA
DRAWN BY: P.M.KILLEEN MAR. 1993 | 1
CHECKED BY: T.M.SMITH CADD FILE: II-BIGS PLOT SCALES PLOT DATES MAR. 1993 H-4-30960 DESIGN ENGINEERING, INC. DESIGN ENGINEERING, INC. DACW29-93-B-0059 DWG. 16 OF 28

SCALE: 3" = 1' - 0"

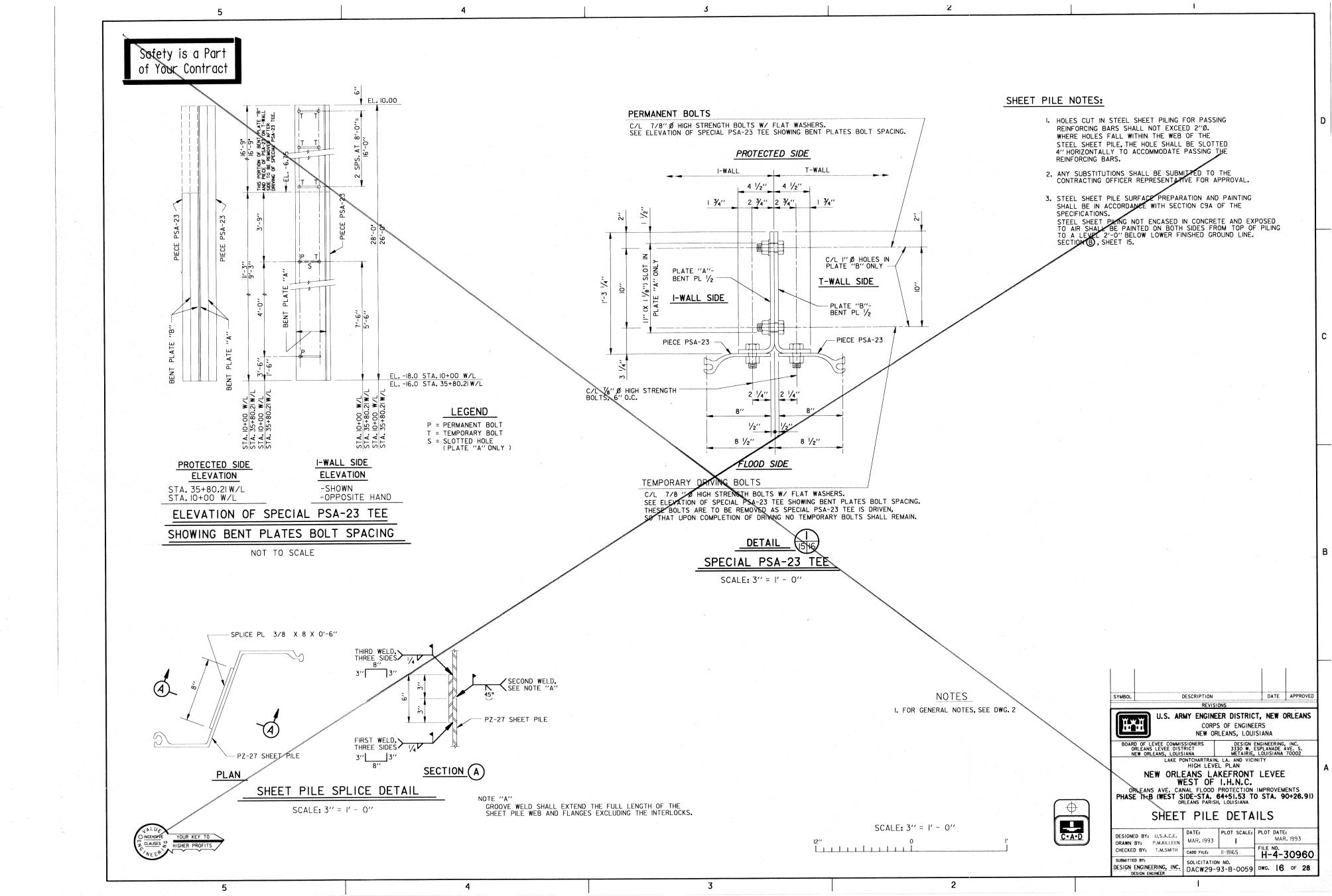
PLATE "B"-BENT PL 1/2

PIECE PSA 23

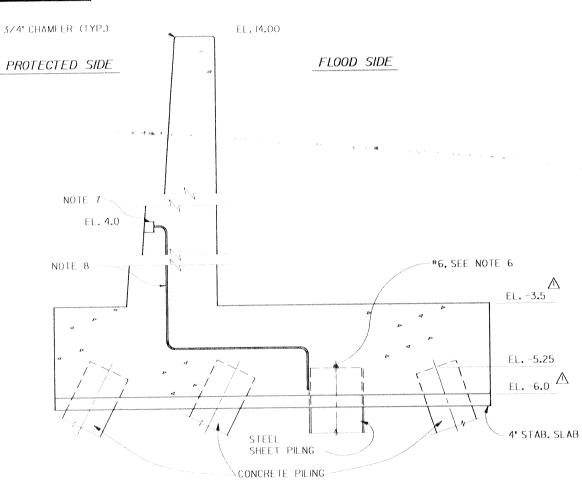
PLAN

SHEET PILE SPLICE DETAIL

SCALE: 3" = 1' - 0"



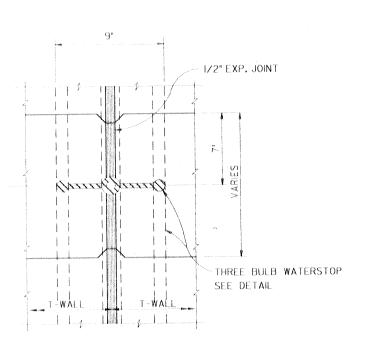
Safety is a Part of Your Contract

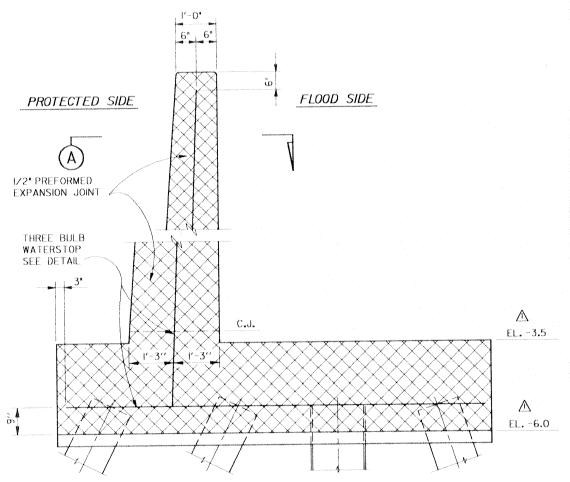


CORROSION PROTECTION DETAIL

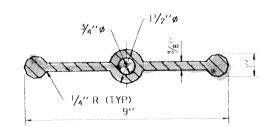
T-TYPE WALL

SCALE: 3/4"=1'-0"





TYPICAL T-WALL JOINT SCALE: 3/4" = 1'-0"



THREE BULB WATERSTOP DETAIL N.T.S.

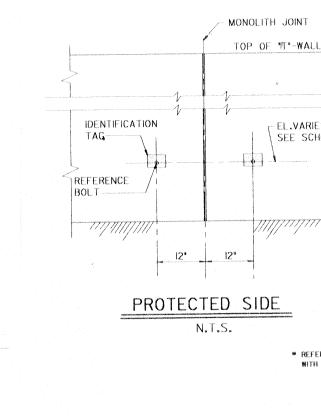
#### "T"-WALL NOTES

- I. FOR GENERAL STRUCTURAL NOTES, SEE SHEET NO. 2.
- 2. ALL HOLES CUT IN STEEL SHEET PILING FOR REINFORCING STEEL OR, ANCHOR BOLTS SHALL NOT EXCEED 2' DIAMETER, UNLESS OTHERWISE INDICATED.
- 3. ALL HOLES CUT THROUGH STEEL SHEET PILING SHALL MISS INTERLOCKS.
- 4. FOR WALL REINFORCEMENT DETAILS, SEE SHEET NO. 18.
- 5. STABILIZATION SLABS ARE REQUIRED.
- 5. STABILIZATION SLABS ARE REQUIRED.

  6. \*6 REINFORCING BAR TO BE WELDED TO THE TOP OF EACH STEEL SHEET PILE. \*6 REINFORCING BAR SHALL NOT EXTEND ACROSS THE MONOLITH JOINT. INSTALL BOND CABLE AT ALL T-WALL AND I-WALL JOINTS AND AT ALL TRANSITIONS FROM T-WALL TO I-WALL JOINTS.

  BOND CABLE SHALL HAVE AN 8" DIAMETER LOOP TO ALLOW FOR STRESSES. BOND CABLES SHALL BE WELDED AS SPECIFIED TO ADJACENT STEEL PILES 12" BELOW THE BOTTOM OF BASE SLAB FOR T-WALL JOINTS, 7" BELOW BOTTOM OF CONCRETE CAP FOR I-WALL JOINTS AND AT TRANSITIONS FROM T-WALL TO I-WALL JOINTS.

  WELDED CONNECTIONS SHALL BE COATED WITH SPLICING EPOXY TO OBTAIN MOISTURE PROOF JOINT. SEE SPECIFICATIONS.
- 7. 6' STANDARD IRON BODY FERRULE WITH BRASS SCREW PLUG. THE FERRULE WILL BE LOCATED AT STA. 10+03 (+/-).
- 8. \*6 REINFORCING BAR WELDED TO STEEL SHEET PILE AND TERMINATED IN THE FERRULE WITHIN 1/2" OF THE COVER.



IO GAGE, C.R.S. IO GAGE IDENTIFICATION TAG 9/16 # HOLE C.R.S., SEE I.D. TAG DETAIL. 11/4" 11/4"

\* REFERENCE TAGS TO BE STAMPED WITH S.R.M. NOS. SHOWN IN SCHEDULE.

EL.VARIES

SEE SCHEDULE

REFERENCE BOLT N.T.S.

REFERENCE BOLT IDENTIFICATION TAG N.T.S.

## SETTLEMENT REFERENCE MARKER DETAILS

6' & HEX. HEAD

S.R.M. NO.*	W/L STA.	FINAL ** ELEVATIONS	S.R.M. NO.	W/L STA.	FINAL ** ELEVATIONS
B-02	10+01.00	6.0	B-36	23+64.69	6.0
B-08	12+72.69	6.0	B-37	23+66.69	6.0
B-09	12+74.69	6.0	B-43	26+37.69	6.0
B-15	15+45.69	6.0	B-44	26+39.69	6.0
B-16	15+47.69	6.0	B-50	29+10.69	6.0
B-22	18+18.69	6.0	B-5I	29+12.69	6.0
8-23	18+20.69	6.0	B-57	31+83.69	6.0
B-29	20+91.69	6.0	B-58	31+85.69	6.0
B-30	20+93.69	6.0	B-67	35+74.38	6.0

• W/L STATIONS ARE APPROXIMATE. LOCATE REFERENCE BOLTS AT NEAREST W/L STATION TO THOSE SHOWN.

\*\* THE CONTRACTOR SHALL TAKE FINAL ELEVATIONS OF ALL SETTLEMENT MARKERS AND SHALL SUBMIT THIS DATA TO THE CONTRACTING OFFICER REPRESENTATIVE (COR).

SE	SETTLEMENT REFERENCE MARKER SCHEDULE						
S.R.M. NO.•	W/L STA.	FINAL ** ELEVATIONS	S.R.M. NO.	W/L STA.	FINAL •• ELEVATIONS		
B-02	10+01.00	5.996	B-36	23+64.69	5.98		
8-08	12+72.69	5.99	B-37	23+66.69	5,992		
8-09	12+74.69	6.005	B-43	26+37.69	5.977		
8-15	15+45.69	5,932	8-44	26+39.69	5.979		
B-16	15+47.69	5,965	B-50	29+10.69	5.97		
8-22	18+18.69	5.942	8-51	29+12.69	5,982		
8-23	18+20.69	5.93	B-57	31+83.69	6.026		
B-29	20+91.69	5.967	B-58	31+85.69	6.015		
8-30	20+93.69	5.975	B-67	35+74.38	6.03		

\* W/L STATIONS ARE APPROXIMATE, LOCATE REFERENCE BOLTS AT NEAREST W/L STATION TO THOSE SHOWN.

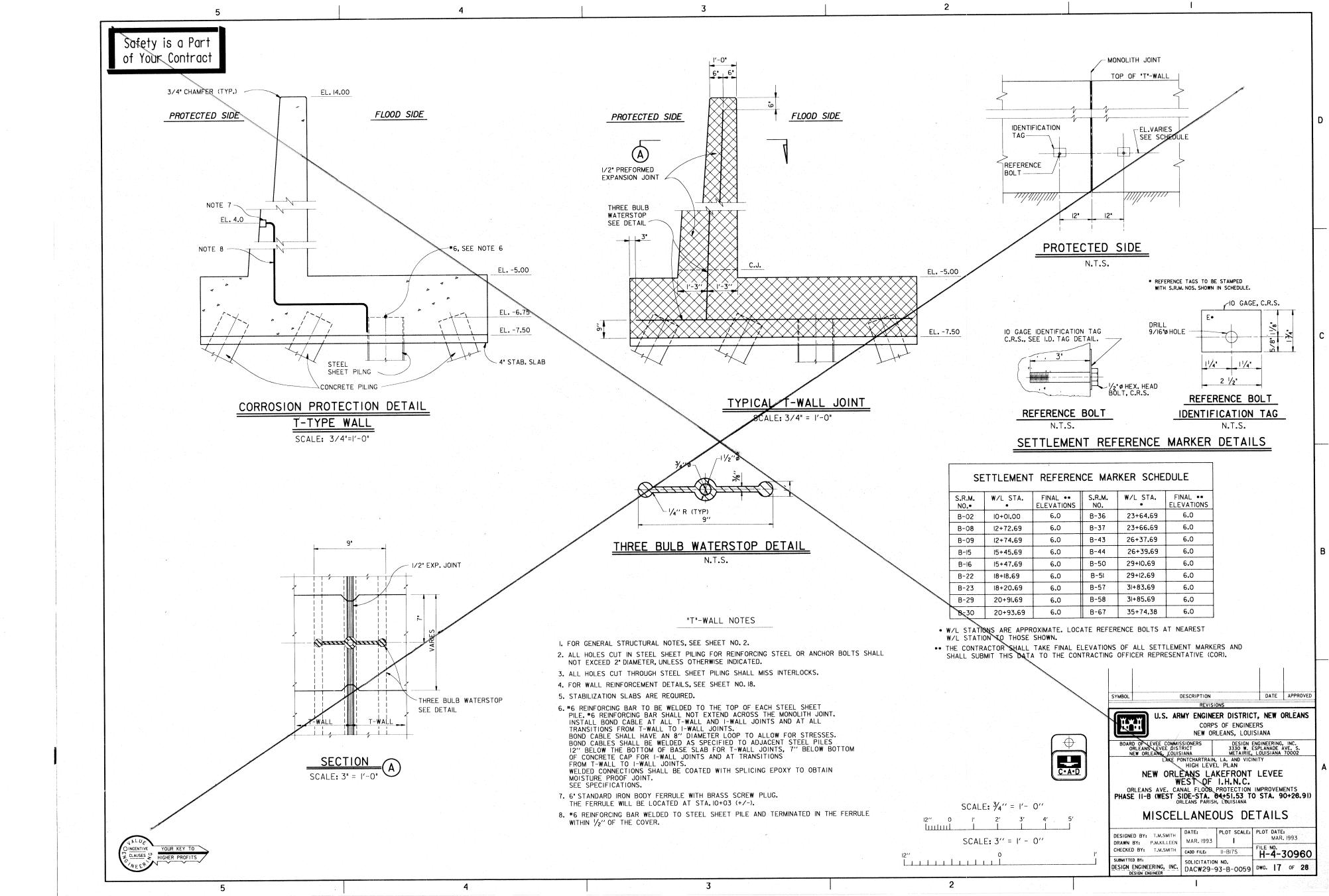
DATE IT, NEW ORLEANS ERS SIANA

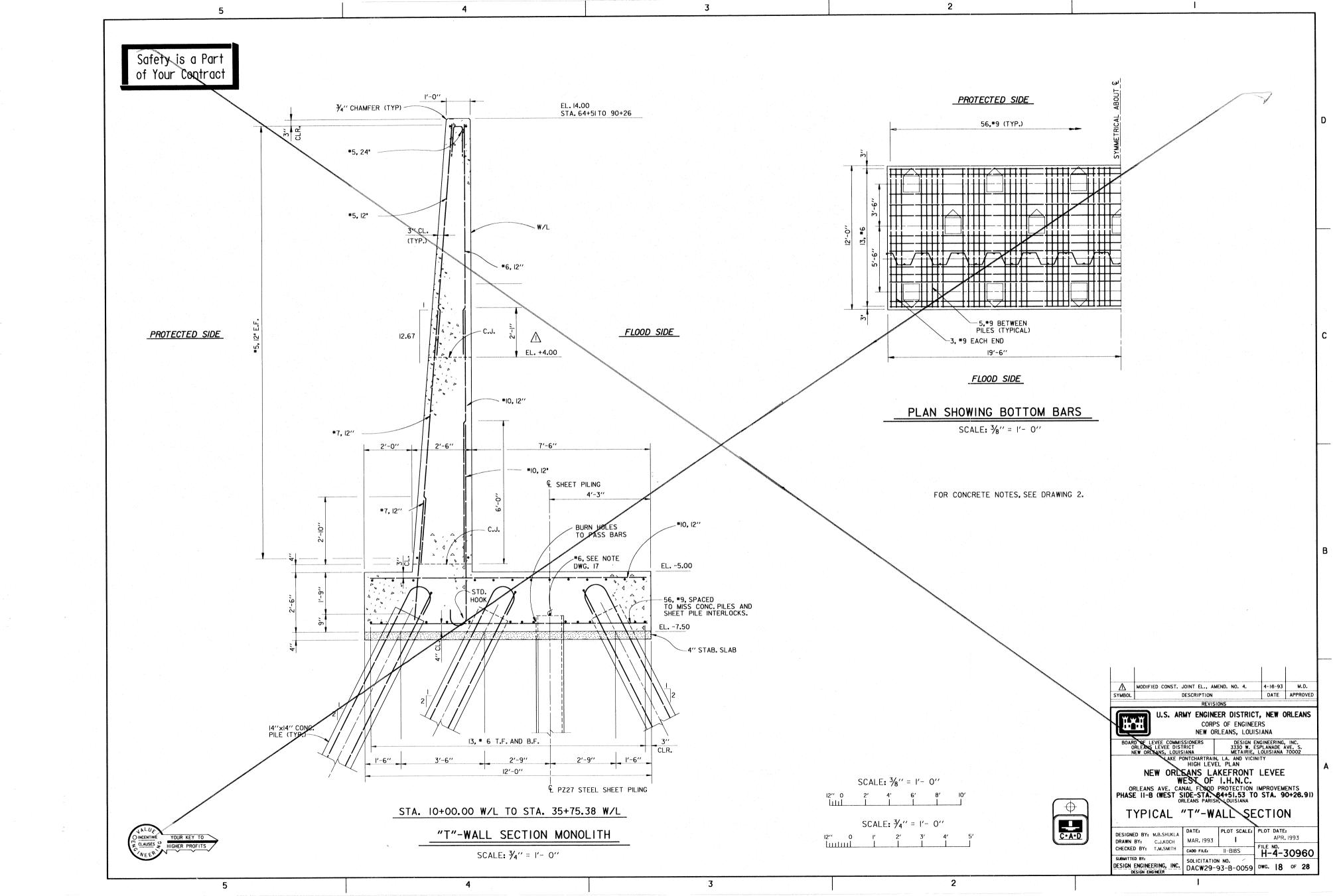
10-27-93

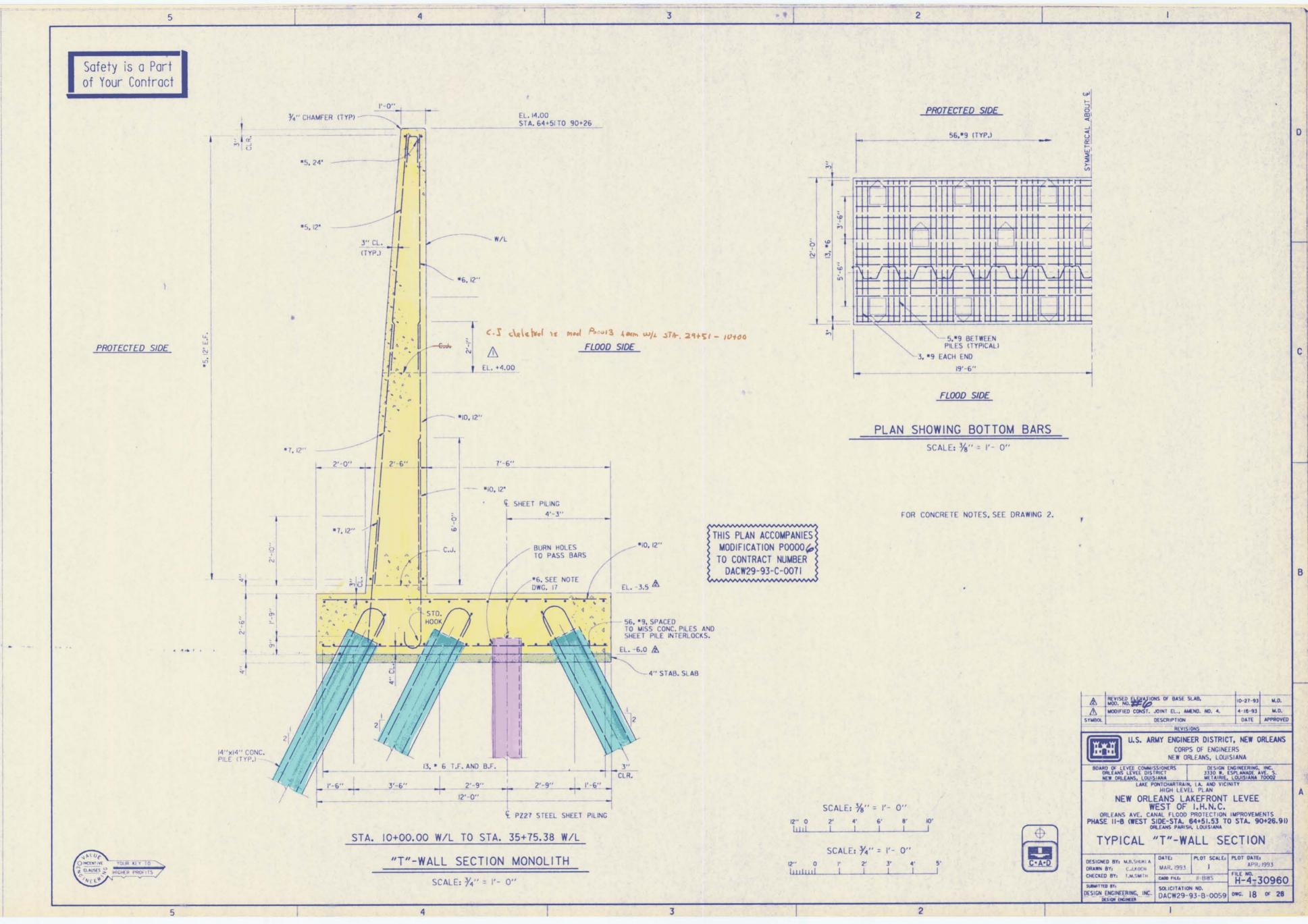
LEVEE IMPROVEMENTS
I STA. 90+26.91) TAILS

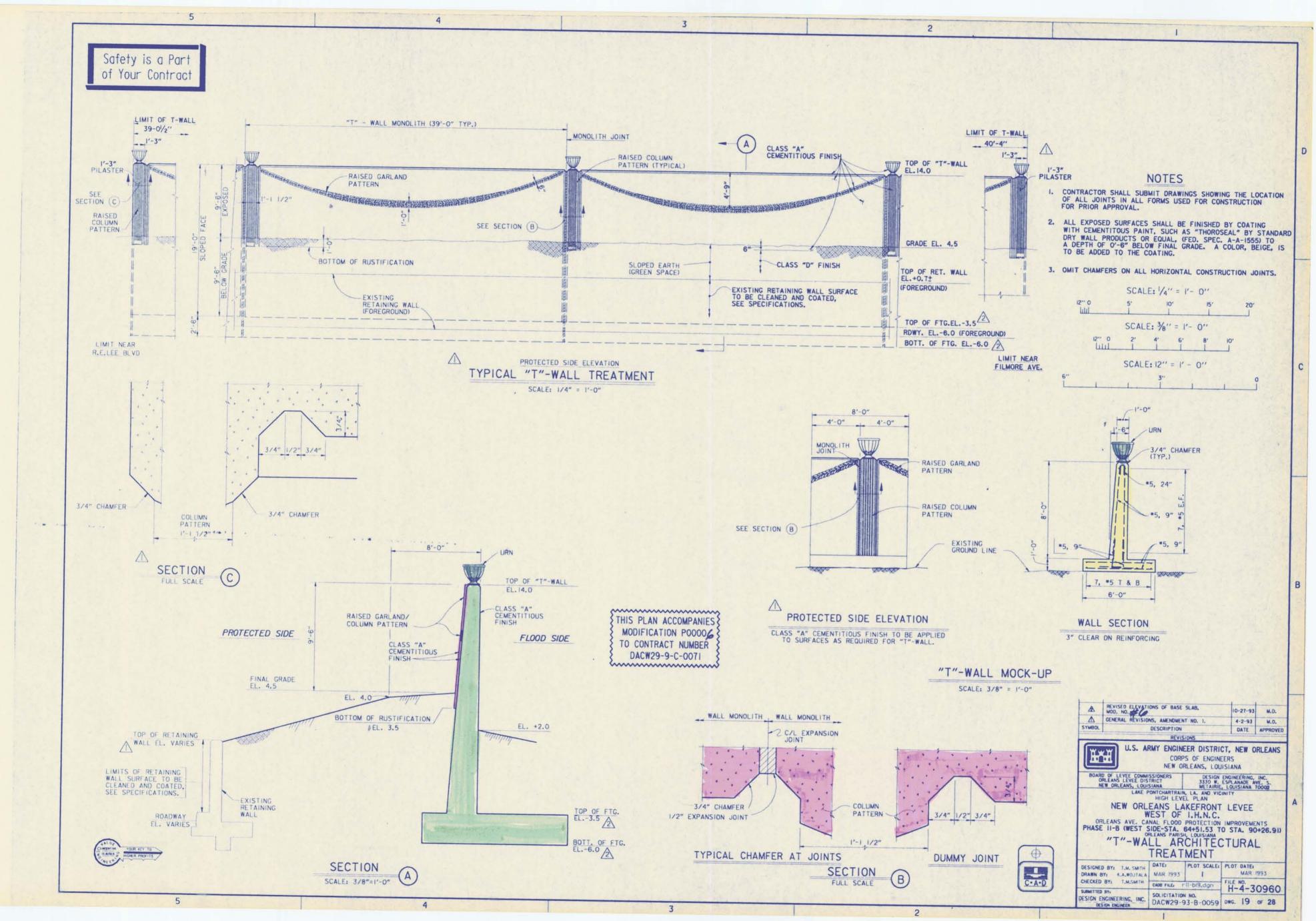
H-4-30960 DWG. 17 OF 28

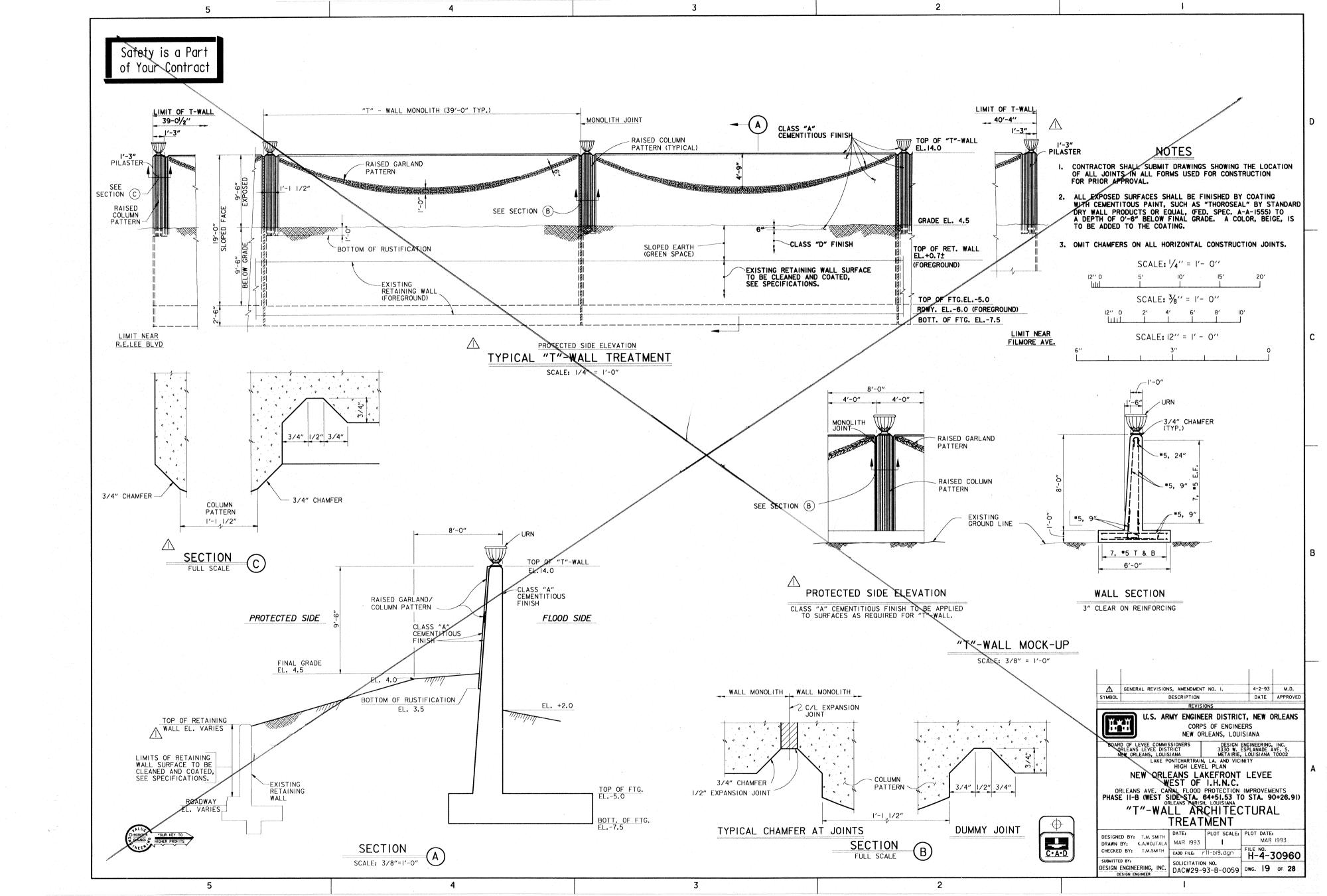
PLOT DATE: MAR. 1993

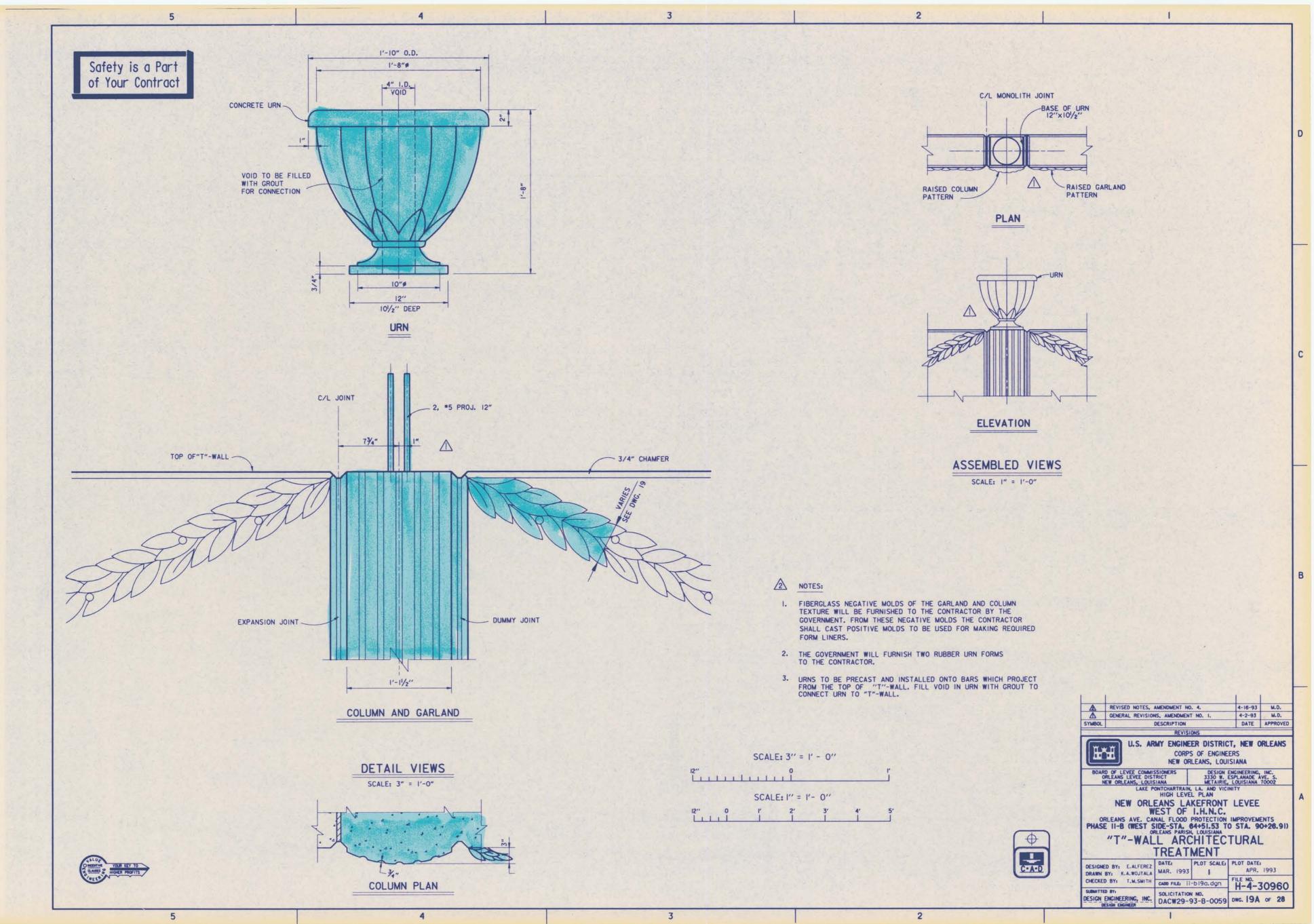












Safety is a Part of Your Contract

> FLOOD SIDE PROTECTED SIDE € PZ27 SHEET PILING 11'-6''+ T-WALL MONOLITH (TO BE CONSTRUCTED) C/L COFFERDAM CLOSURE LINK EL. 9.0 EL. VARIES (HIGH GROUND) V EL. +1.00 NORMAL WATER SURFACE EL. +1.0 EL.-4.0 SHORING positive militarios experience services acquires EL. - 6.33 LIMIT OF EXCAVATION -THIS PLAN ACCOMPANIES MODIFICATION POOOD TO CONTRACT NUMBER 10'-6'' DACW29-93-C-0071 ..... TYPICAL SECTION EXCAVATION LIMITS AT CLOSURE LINK

> > SCALE: |" = 5'-0"

REMOVE EXISTING CONCRETE CAP AFTER COFFERDAM IN PLACE REMOVE EXISTING CONCRETE CAP FOR CLOSURE LINK TIE-IN & CLOSURE LINK EXISTING CONCRETE CAP-PROTECTED SIDE OF COFFERDAM PACK WITH OAKUM EL. +3.0 TO EL. +9.5 -2x6x12'-0" TREATED BOARD
(FROM EL. 9.5 TO -2.5) MOUNTED
ON PZ27 SHEET PILING WITH
34"Ø A307 GALVANIZED BOLTS
SPACED 12" O.C. FLOOD SIDE OF COFFERDAM

> COFFERDAM CLOSURE LINK AT EXISTING SHEET PILING

> > N.T.S.

SCALE: I" = 5"



REVISED BOTTOM OF EXCAVATION, MOD. NO. # 10-27-93 DESCRIPTION SYMBOL REVISIONS U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS

CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA

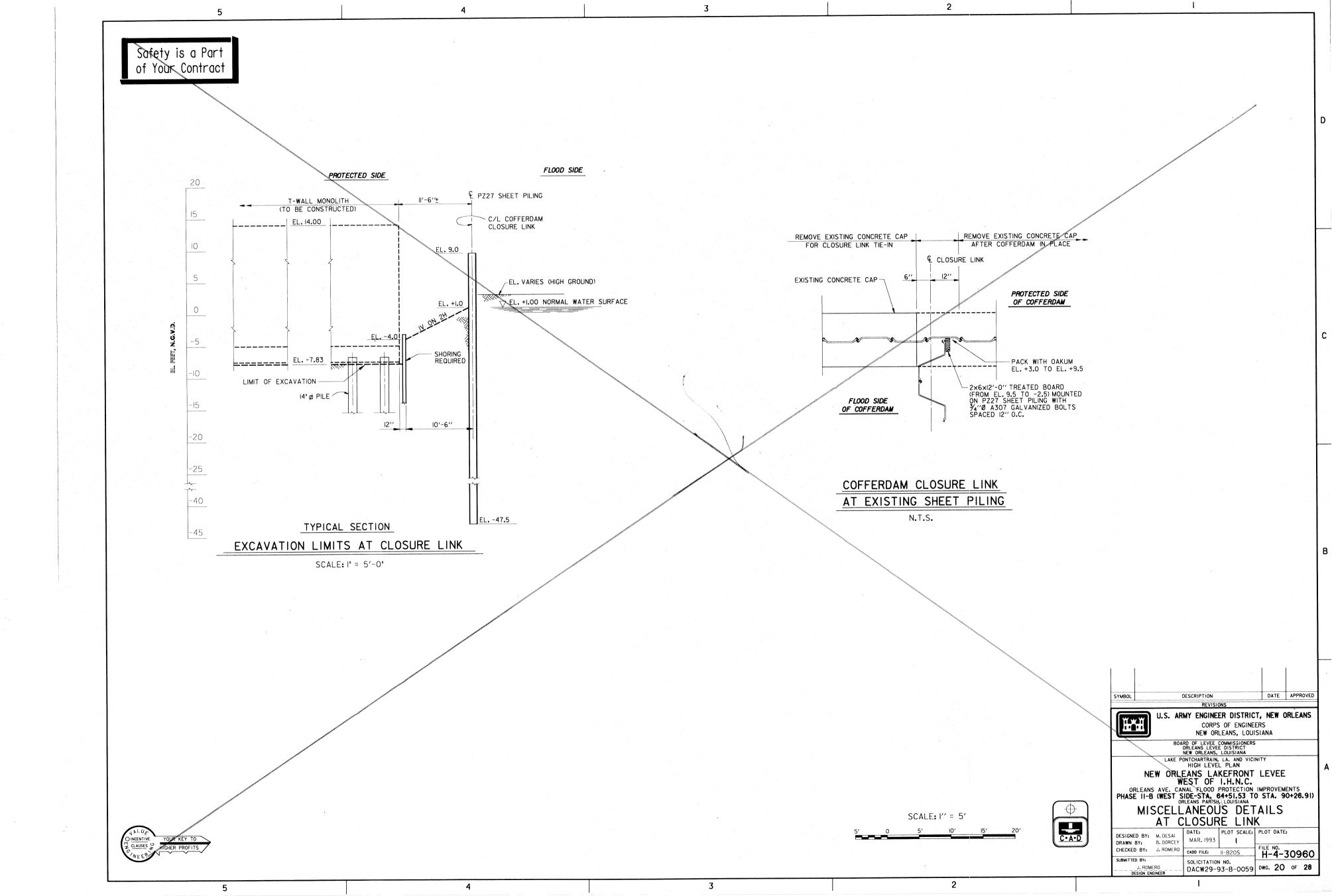
BOARD OF LEVEE COMMISSIONERS ORLEANS LEVEE DISTRICT NEW ORLEANS, LOUISIANA LAKE PONTCHARTRAIN, LA. AND VICINITY HIGH LEVEL PLAN

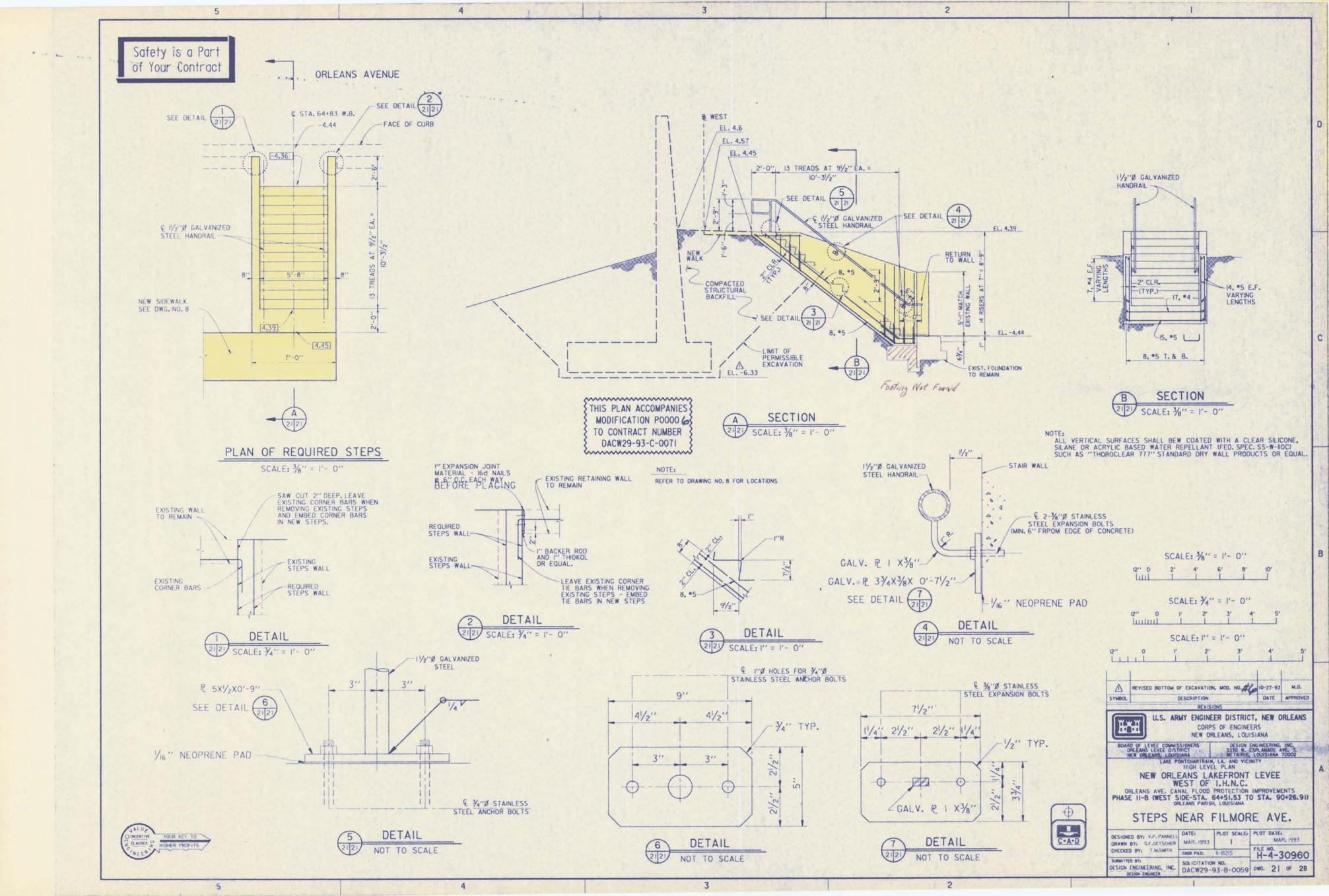
NEW ORLEANS LAKEFRONT LEVEE
WEST OF I.H.N.C.
ORLEANS AVE. CANAL FLOOD PROTECTION IMPROVEMENTS
PHASE II-B (WEST SIDE-STA. 64+51.53 TO STA. 90+26.91)
ORLEANS PARISH, LOUISIANA

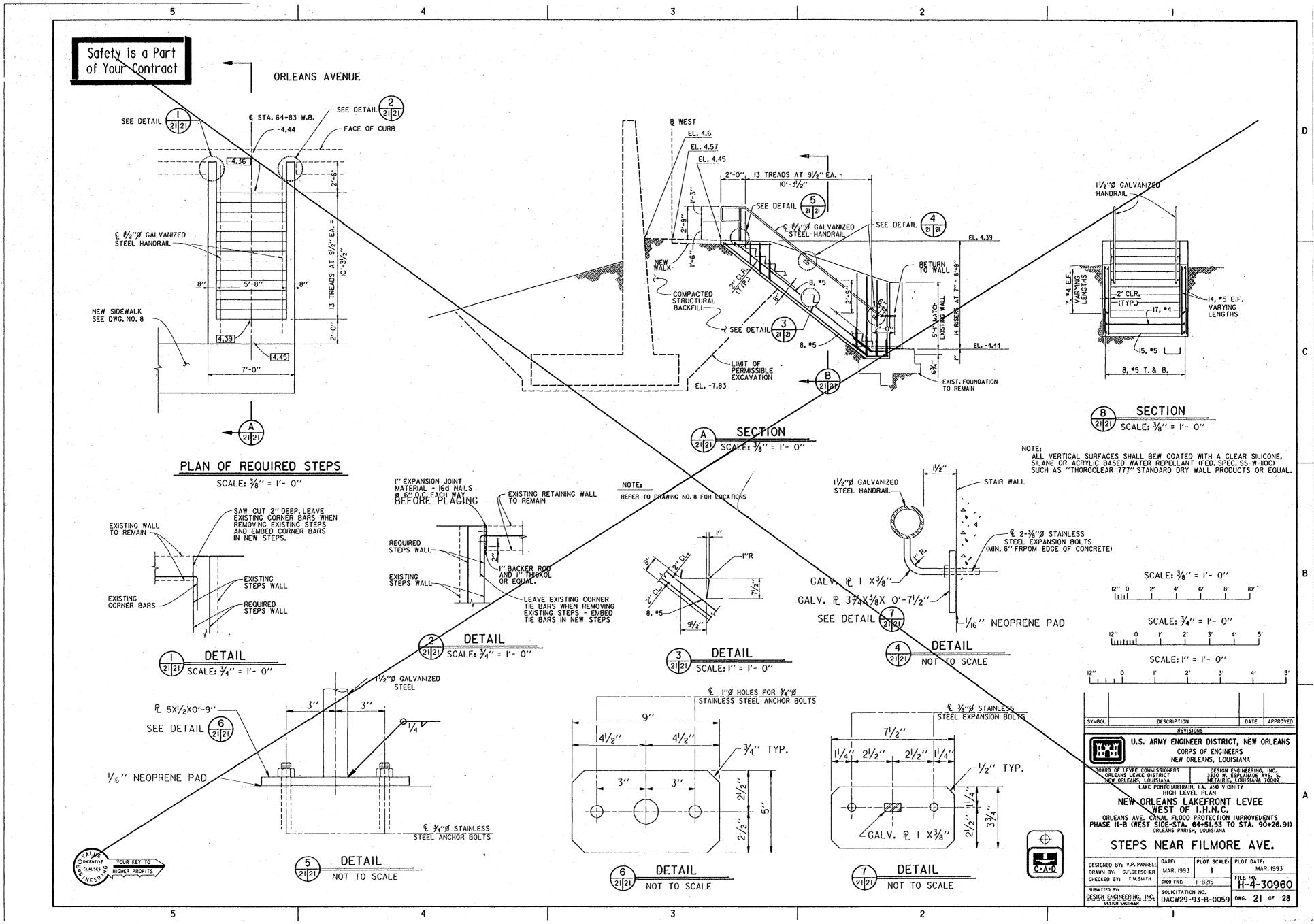
MISCELLANEOUS DETAILS AT CLOSURE LINK

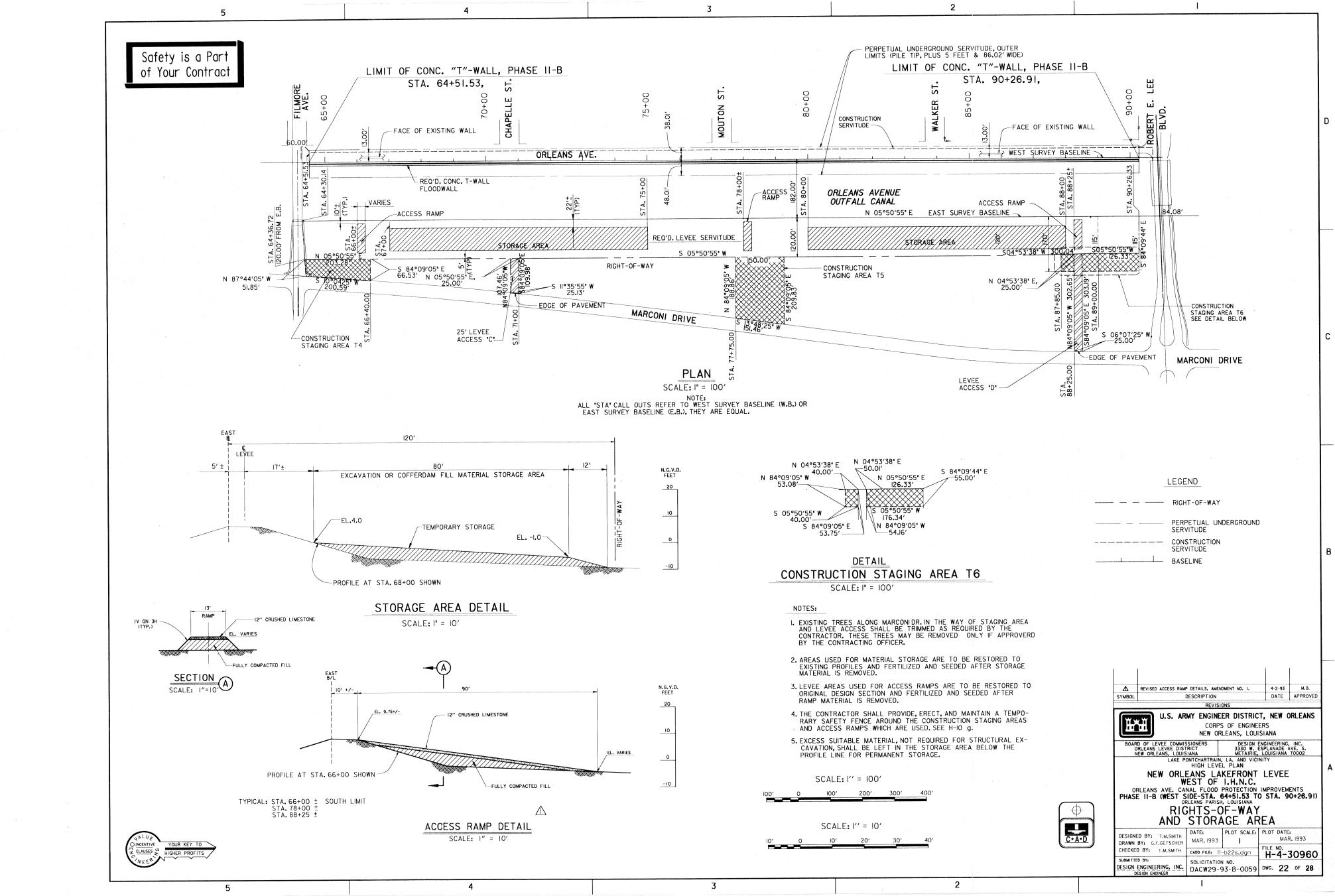
DESIGNED BY: M. DESAL DATE: PLOT SCALE: PLOT DATE: DRAWN BY: B. DORCEY MAR. 1993 CHECKED BY: J. ROMERO CADD FILES II-B2OS H-4-30960 SOLICITATION NO. DACW29-93-B-0059 DWG. 20 OF 28

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LOG OF BORING
EUSTIS ENGINEERING COMPANY
SOIL AND FOUNDATION CONSULTANTS
METAIRIE, LA.

Name of Project: Orleans Levee District, Orleans Avenue Outfall Canal OLB Project No. 2048-0304, New Orleans, Louisiana For: The Board of Commissioners of the Levee District, New Orleans, La. Design Engineering, inc. Metairie, Louisiana Date 20 September 1985 Boring No. 26 Soil Technician A. J. Mayeux Ground Elev. -4.27 Datum NGVD Gr. Water Depth See Text # STANDARD PENETRATION TEST 0.0 0.5 Concrete 0.5 1.25 Sand & shell fill 1 3.0 4.0 1.25 4.0 Wood, organic matter, clay, miscellaneous fill 5.0 6.0 4.0 7.0 Wood w/humus & organic matter 2 7.0 8.0 7.0 10.0 Very soft brown humus w/organic mclay & roots molay & roots 3 II.0 I2.0 I0.0 I3.0 Very soft gray clay w/clayey silt 5 19.0 20.0 16.0 22.5 Very soft gray clay w/silt pockets 6 24.0 25.0 22.5 25.0 Very loose gray clayey sand w/shell fragments & clay pockets
7 25.0 26.5 25.0 27.0 Very loose gray sand w/shell fragments 8 27.5 29.0 27.0 Medium dense gray sand w/shell fragments 20 9 29.5 31.0 Ditto 9 29.5 31.0 DI++o
10 33.0 34.5 35.5 DI++o 19

LOG OF BORING
EUSTIS ENGINEERING COMPANY
SOIL AND FOUNDATION CONSULTANTS
METARIE, LA.

Name of Project: Orleans Levee District, Orleans Avenue Outfall Canal

			Or	leons	Levee District, Orleans Avenue Outfall	Can	al ·		****		
ame	of Pi	10 Jec 1		100,10	olect No. 2048-0304, New Orleans, Louisia	na			îĩ.		
or: The Board of Commissioners of the Levee District, New Orleans, La.								10			
•			- Contract of Cont		ngineering, inc. Metairle, Louisiana	-			//		
	a Na	28 c			n A. J. Mayeux Date 19 Sep	ten	ber 1985				
	-	v5			Datum NGVD Gr. Water Depti				1		
	SAN	PLF	DEPTH	STRATUM		* 5	TANDARD	20	//		
Sample No.	From	Depth-Feet From To				To	VISUAL CLASSIFICATION		PENETRATION TEST		
			0.0	0.2	Asphalt			-	/		
			0.2	0.7	Concrete			70	/_		
			0.7	1.5	Fill (sand & shells)			30	<u> </u>		
			1.5	4.0	Miscellaneous fill (wood, clay, organic						
					matter & shells)						
1	4.5	5.5	4.0	6.0	Extremely soft black humus w/roots			40	1		
					& wood				//		
2	7.5	8.5	6.0	8.5	Extremely soft gray silty clay						
					w/organic matter & wood				//		
3	11.0	12.0	8.5		Very soft gray clay w/some organic			50	/		
					matter			# FEET			
4	14.0	15.0		16.0	Ditto		-	БЕРТН В			
5	19.0	20.0	16.0	24.0	Very soft gray clay w/slit lenses			8			
6	24.0	25.0	24.0	27.0	Soft gray sandy clay w/shell fragments				1.		
7	27.0	28.5	27.0	29.5	Loose gray sand w/shellfragments	1	7				
8	29.5	31.0	29.5		Medium dense gray sand w/shell	5	17	-	1		
					fragments						
9	32.5	34.0			Ditto	4	14		1		
10	35.0	36.5		38.0	Medium dense gray sand w/some clay	3	10				
- 11	38.5	40.0	38.0	42.0	Soft gray clay w/sand pockets &	1	3	-	1		
		1			shell fragments						
12	44.0	45.0	42.0		Medium stiff gray clay w/sand			-	1		
	1	-	1	1							

LOG OF BORING
EUSTIS ENGINEERING COMPANY
SOIL AND FOUNDATION CONSULTANTS
METAIRE, LA Project: Orleans Levee District, Orleans Avenue Outfall Canal O.L.B. Project No. 2048-0304, New Orleans, Louisiana For: The Board of Commissioners of the Levee District, New Orleans, La. Design Engineering, Inc. Metairle, Louislana

omple No.	SAM Depti	PLE n-Feet	OEPTH S	TRATUM	VISUAL CLASSIFICATION	* ST	ANDARD ETRATION TEST	20
	From	10	0.0	0.2	Asphalt	1		
			0.2	0.7	Concrete	-		
			0.7	1.5	Fill (sand & shells)			30
1	2.0	2.5	1.5	4.0	Loose brown humus			
2	5.0	5.5	4.0	6.0	Loose gray clayey slit w/organic			
					matter & wood			40
3	8.0	8.5	6.0		Very soft gray clay w/organic			
					matter & silt			
4	11.0	11.5		13.0	DI++o			
5	14.0	14.5	13.0		Soft gray clay w/slit lenses			_50
6	19.0	19.5		24.0	Ditto			N FEET
7	24.0	24.5	24.0	27.0	Loose gray clayey sand w/shell fragments			M HLAG
8	27.5	29.0	27.0	29.0	Ditto	2	7	8
9	29.0	30.5	29.0		Medium dense gray sand w/shellfragments	6	19	
10	31.5	33.0			Ditto	6	19	
II	34.0	35.5		38.0	Ditto	4	. 12	
12	38.5	40.0	38.0		Medium stiff gray clay w/sand	1.	4	
					pockets & shell fragments			-
13	44.0	44.5			Ditto			
14	49.0	49.5		50.0	Ditto			

LOG OF BORING

Name of Project: Orleans Levee District, Orleans Avenue Outfall Canal O.L.B. Project No. 2048-0304, New Orleans, Louisiana

our		v. <u>-6</u>	· contractor		Datum NGVD Gr. Water Depth	-		20
ample No.	SAN Depti	PLE 1-Feet	DEPTH S	TO	VISUAL CLASSIFICATION		ANDARD ETRATION TEST	
	From	То	0.0	0.2	Asphalt	1	1231	-
			0.2	0.7	Concrete			
			0.7	1.5	Fill (sand & shells)			30
			1.5	2.5	Fill (miscellaneous wood, shells, etc.)			1
1	3.0	4.0	2.5	7.0	Soft brown organic clay w/wood & roots			] -
2	7.0	8.0	7.0	10.0	Soft gray clay w/organic matter & wood			40
3	11.0	12.0	10.0	13.0	Loose gray clayey slit w/trace of		J · '-	
					organic matter			
4	15.0	16.0	13.0		Very soft gray clay w/silt lenses			
5	19.0	20.0		22.0	Ditto			50
6	24.0	25.0	22.0	26.0	Soft gray clay w/slit lenses			- EE
7	26.5	28.0	26.0	28.0	Loose gray clayey sand w/shell	2	7	DEPTH B
					fragments			8
8	28.5	30.0	28.0		Medium dense gray sand w/shell	6	29	
					fragments			
9	31.0	32.5			DI++o	6	22	
10	33.5	35.0		36.0	DI++o	3	14	
11	36.0	37.5	36.0	38.0	Loose gray sand w/shellfragments	2	7	
12	38.5	40.0	38.0		Medium stiff gray clay w/sand	1	4	-
					pockets & shell fragments			
13	44.0	45.0			DI++o			
14	49.0	50.0		50.0	DI++o			_

LOG OF BORING
EUSTIS ENGINEERING COMPANY
SOIL AND FOUNDATION CONSULTANTS
METAIRIE, LA

35.5 37.0 35.5 38.0 Loose gray sand w/sand pockets & shell 2

pockets & shell fragments

| 12 | 38.5 | 40.0 | 38.0 | 42.0 | Soft gray clay w/sand pockets & | shell fragments shell fragments 13 44.0 45.0 42.0 Medium stiff gray clay w/sand

50.0 Ditto

			OL	.B Pro	evee District, Orleans Avenue Outfall Co Ject No. 2048-0304, New Orleans, Louisian	a		
or:	The	Board	of C	ommis	sioners of the Levee District, New Orle	ans	, La.	10_
			Desi	ign En	gineering, inc. Metairle, Louisiana			
orin	a No.	34 s	oli Tec	hnicia	n A. J. Mayeux Date 17 Sep	ter	nber 198	5
		v. 4.7			Datum NGVD Gr. Water Depth	, S	ee Text	- an
Somple	SAN		DEPTH S	STRATUM	VISUAL CLASSIFICATION		TANDARD NETRATION	20
No.	From	То	From	То	VISUAL CLASSIFICATION		TEST	
1	2.0	2.5	0.0	4.0	Stiff tan & gray clay w/slit pockets			-
					& fill			30
2	5.0	5.5	4.0	7.0	Medium compact gray & tan clayey			
					siit w/wood			5.0
3	8.0	8.5	7.0	10.0	oft gray clay			
	11.0	11.5	10.0	12.0	Wood w/some clay		40	
4	14.0	14.5	12.0		Soft gray & brown organic clay			
					w/wood & humus			
5	19.0	19.5		21.0	Ditto			
6	24.0	24.5	21.0	26.0	Very soft gray clay w/slit & roots			50
7	29.0	29.5	26.0		Soft gray clay w/slit lenses			133
8	34.0	34.5		36.0	Ditto			DEPTH IN
9	38.0	38.5	36.0	38.5	Soft gray sandy clay w/shell fragments			96
10	38.5	40.0	38.5	1	Medium dense gray sand w/shell	6	25	
					fragments			
11	41.0	42.5			DI++o	4	16	
12	43.5	45.0		45.0	Ditto 3 II			
13	46.0	47.5	45.0	48.0	Loose gray sand w/shell fragments	3	7	] -
14	48.5	50.0	48.0	50.0	Loose gray sand w/shell fragments &	2	5	
	İ	1	İ	T	clay layers		1	1 -
		<b>†</b>	t	T				1
	ļ			+				-1

LOG OF BORING EUSTIS ENGINEERING COMPANY SOIL AND FOUNDATION CONSULTANTS METAIRIE, LA

pockets & shell fragments

Ditto

50.0

13 49.0 50.0

Name of Project: Orleans Levee District, Orleans Avenue Outfall Canal OLB Project No. 2048-0304, New Orleans, Louisland For: The Board of Commissioners of the Levee District, New Orleans, La.

rour		v. <u>-5</u>		***************************************	Datum NGVD Gr. Water Depth			20
Sample SamPLE Depth-Feet		r-Feet	F	STRATUM .	VISUAL CLASSIFICATION	* STANDARD PENETRATION TEST		-
	From	To	0.0	0.2	Asphalt	т	(£5)	
			0.0	0.7	Concrete	-+		
								30
			0.7	2.0	Fill (sand & shells)			
	2.0	3.0	2.0	3.0	Miscellaneous fill			
	5.0	6.0	3.0	7.0	Extremely soft black & brown humus			
		ļ		-	w/wood & roots	-		40
	8.0	9.0	7.0	9.0	Wood w/humus & clay			
2	11.0	12.0	9.0	13.0	Loose gray clayey slit w/ organic matter			
3	14.0	15.0	13.0		Very soft to soft gray clay w/silt			
					ienses			_50
4	19.0	20.0			Di++o			N FEET
5	24.0	25.0		25.5	Ditto			€ н⊥⊿эс
6	28.0	29.0	25.5	29.0	Loose gray clayey sand w/shell			90
					fragments			_
7	29.0	30.5	29.0	-	Medium dense gray sand w/shell	3	23	
			-		fragments			
8	31.5	33.0		34.0	Ditto	4	18	
9	34.0	35.5	34.0	1	Loose gray sand w/shellfragments	3	7	-
10	38.5	40.0		42.0	Ditto	1	5	
11	43.5	45.0	42.0	1	Medium stiff gray clay w/sand	2	4	
		1		<b>†</b>	pockets & shell fragments			
12	49.0	50.0	<del> </del>	50.0	Ditto			-
12	73.0	30.0	ļ	30.0	D1110			

Notes:

\*\*Number in first column indicates number of blows of I40-lb. hammer dropped 30 in. required to seat 2-in. 0.D. splitspoon sampler 6 in. Number in second column indicates number of blows of I40-lb. hammer dropped 30 in. required to drive 2-in. 0.D. splitspoon sampler Ift. after seating 6 in.

2

While these logs of borings are considered to be representative of subsurface conditions at its respective location on the date shown, it is not warranted that it is representative of subsurface conditions at other locations and times.

LEGEND

0

DATE APPROVED DESCRIPTION SYMBOL REVISIONS

U.S. ARMY ENGINEER DISTRICT, NEW ORLEANS CORPS OF ENGINEERS NEW ORLEANS, LOUISIANA

BOARD OF LEVEE COMMISSIONERS
ORLEANS LEVEE DISTRICT
NEW ORLEANS, LOUISIANA
LAKE PONTCHARTRAIN, LA. AND VICINITY
HIGH LEVEL PLAN

NEW ORLEANS LAKEFRONT LEVEE
WEST OF I.H.N.C.

ORLEANS AVE. CANAL FLOOD PROTECTION IMPROVEMENTS
PHASE II-B (WEST SIDE-STA. 64+51.53 TO STA. 90+26.91)
ORLEANS PARISH, LOUISIANA

SOIL BORING LOGS

DESIGNED BY: DRAWN BY:	EUSTIS P.M.KILLEEN	DATE: MAR. 1993	PLOT SCALE:	PLOT	DATE: MAR. 1993
CHECKED BY:	T.M.SMITH	CADO FILE:	II-B23S	FILE H-	<sup>№</sup> . 4-30960
SUBMITTED BY: DESIGN ENGINEE DESIGN ENG		SOLICITATION DACW29-9	N NO. 93-B-0059		23 of 28

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3

2

			INIII		ED SOIL CLASSIFICATION						
	,		וואוע		LD JOIL CLASSII TOATTON						
MAJOR DIVISION TYPE LETTER SYM SYMBOL BOL TYPICAL NAMES											
S	2 4	CLEAN GRAVEL	GW	0.00	GRAVEL, Well Graded, gravel-sand mixtures, little or no fines						
SOILS is larger	RAVELS than haif of traction r than No.	(Little or No fines)	GP		GRAVEL,Poorly Graded,gravel-sand mixtures,little or no fines						
	GRAVEL e than hal rse fracti per than N	GRAVEL WITH FINES	GM		SILTY GRAVEL,gravel-sand-silt mixtures						
GRAINED of material eve size.	GR/ More the coorse larger sieve si	(Appreciable Amount of Fines)	GC		CLAYEY GRAVEL,gravel-sand-clay mixtures						
25 25	- s	CLEAN SAND	SW	0 0	SAND, Well-Graded, gravelly sands						
28 g	E S IN THE ST ILLITTIE OF CD . CAND Dog				SAND,Poorly-Graded,gravelly sands						
COARSE ore than	More than No. 28 ANDS SANDS (Appropriate than No. 28 ANDS (Appropriate than 19 And 19			0000	LTY SAND,sand-silt mixtures						
O \$ \$	More the course smaller sieve si	(Appreciable Amount of Fines)	SC	%	CLAYEY SAND, sand-clay mixtures						
SOILS	Carrier de Carrier de la proposition de la Principal de la Pri	SILTS AND	ML		SILT & very fine sand, silty or clayey fine sand or clayey silt with slight plasticity						
		CLAYS	CL		LEAN CLAY, Sandy Clay, Silty Clay, of low to medium plasticity						
吊音	Z € 9 < 50)		OL		ORGANIC SILTS, and organic silty clays of low plasticity						
					SILT, fine sandy or silty soil with high plasticity						
NE - re than smaller		CLAYS (Liquid Limit	CH		FAT CLAY, inorganic clay of high plasticity						
FINE Wore			ОН		ORGANIC CLAYS of medium to high plasticity,organic silts						
HIGH	LY ORGANIC	SOILS	P†		PEAT, and other highly organic soil						
	WOOD		Wd		WOOD						
SHELLS		SI	2222	SHELLS							
	NO SAMPLE NS				No Sample Retrieved						
NOTE:	Soils p	ossessin	g char	acte	ristics of two groups are designated by combinations of group symbols.						

COLOR		1.5	CONSISTENCY		MODIFICATIONS	
COLOR	SYMBOL		FOR COHESIVE SOILS	MODIFICATION	SYMBO	
TAN	Т	CONSISTENCY	COHESION IN LBS./SQ.FT. FROM	COHESION IN LBS./SQ.FT. FROM		Tr
YELLOW	Y		UNCONFINED COMPRESSION TEST		Fine	F
RED	R	VERY SOFT	< 250	vSo	Medium	М
BLACK	ВК	SOFT	250-500	So	Coarse	С
GRAY	Gr	MEDIUM	500-1000	М	Concretions	cc
LIGHT GRAY	IGr	STIFF	1000-2000	St	Rootlets	rt
DARK GRAY	dGr	VERY STIFF	2000-4000	vSt	Lignite fragments	Ig
BROWN	Br	HARD	> 4000	Н	Shale fragments	sh
LIGHT BROWN	IBr				Sandstone fragments	sda
DARK BROWN	dBr	60			Shell fragments	sli
BROWNISH-GRAY	brGr	EX	The I will		Organic matter	0
GRAYISH-BROWN	gyBr	INDEX	0x   x		Clay strata or lenses	cs
GREENISH-GRAY	gnGr	<u></u> 40	W 1		Silt strata or lenses	SIS
GRAYISH-GREEN	gyGn	PLASTICITY 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	<b>/</b> - <del> </del> - <del> </del> - <del> </del> - <del> </del> ++		Sand strata or lenses	SS
GREEN	Gn	A 20			Sandy	S
BLUE	ВІ		MH or OH		Gravelly	G
BLUE-GREEN	BIGn	a 7 Z	CL-ML /// ML on OL		Boulders	В
WHITE	Wh	0,0	20 40 60 80 10	0	Slickensides	SL
MOTTLED	Mot		L.LLIQUID LIMIT	-	Wood	Wd
			Cita Cidolo Cimi		0xidized	0×
			DI ACTIOITY CHART			
			PLASTICITY CHART			

NOTES: FIGUR	ES TO LEFT OF BORING UNDER COLUMN " W OR DIO"
Are r	natural water contents in percent dry weight
When	underlined denotes D <sub>10</sub> size in mm*
FIGUR	ES TO LEFT OF BORING UNDER COLUMNS " LL" AND " PL"
Are li	quid and plastic limits, respectively
SYMBO	DLS TO LEFT OF BORING
	Ground-water surface and date observed
©	Denotes location of consolidation test**
<b>S</b>	Denotes location of consolidated-drained direct shear test**
®	Denotes location of consolidated-undrained triaxial compression test**
0	Denotes location of unconsolidated-undrained triaxial compression test*
•	Denotes location of sample subjected to consolidation test and each of the above three types of shear test**
FW	Denotes free water encountered in boring or sample
FIGUR	ES TO RIGHT OF BORING
Are va	lues of cohesion in lbs./sq.ft. from unconfined compression tests
In p	arenthesis are driving resistances in blows per foot determined with a

standard split spoon sampler (1.3/8" I.D., 2" O.D.) and a 140 lb. driving hammer

Where underlined with a solid line denotes laboratory permeability in centimeters per second of undisturbed sample

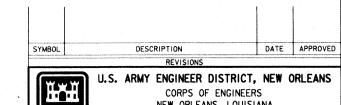
Where underlined with a dashed line denotes laboratory permeability in centimeters per second of sample remoulded to the estimated natural void ratio

\*The D  $_{\rm 10}$  size of a soil is the grain diameter in millimeters of which 10% of the soil is finer, and 90% coarser than  $D_{10}$ .

 $^{**}$ Results of these tests are available for inspection in the U.S. Army Engineer District Office, if these symbols appear beside the boring logs on the drawings.

## TYPICAL NOTES:

- I. While the borings are representative of subsurface conditions at their respective locations and for their respective vertical reaches, local variations characteristic of the subsurface materials of the region are anticipated and, if encountered, such variations will not be considered as differing materially within the purview of the contract clause entitled "Differing Site Conditions".
- 2. Ground-water elevations shown on the boring logs represent ground-water surfaces encountered in such borings on the dates shown. Absence of water surface data on certain borings indicates that no ground-water data are available from the boring but does not necessarily mean that ground-water will not be encountered at the locations or within the vertical reaches of such borings.
- 3. Consistency of cohesive soils shown on the boring logs is based on driller's log and visual examination and is approximate, except within those vertical reaches of the borings where shear strengths from unconfined compression tests are shown.
- 4. Unless otherwise noted:
- a. Undisturbed borings, indicated by the letter "U", are taken with a 5" I.D. Piston Type Sampler.
- b. General type borings are taken with a 1  $7/8^{\prime\prime}$  I.D. Tube Sampler and/or a 1.3/8" I.D. Split Spoon Sampler.



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ORLEANS PARISH, LOUISIANA

SOIL BORING LECEND

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SUIL	DUKIN	G LEGI	LINU
DESIGNED BY: DRAWN BY: U.S.A.C.E.	DATE: MAR. 1993	PLOT SCALE:	MAR. 1993
CHECKED BY:	CADD FILE:	II-B23AS	H-4-30960
SUBMITTED BY: DESIGN ENGINEERING, INC. DESIGN ENGINEER	SOLICITATION DACW29-9	N NO. 93-B-0059	DWG. 23A OF 28

