TEXAS DEPARTMENT OF INSURANCE

Engineering Services / MC 103-3A 333 Guadalupe Street P.O. Box 149104 Austin, Texas 78714-9104 Phone No. (512) 322-2212 Fax No. (512) 463-6693

PRODUCT EVALUATION

FR-02

Effective August 1, 2006

The following product has been evaluated for compliance with the wind loads specified in the International Residential Code (IRC) and the International Building Code (IBC). This product shall be subject to reevaluation 3 years after the effective date.

This product evaluation is not an endorsement of this product or a recommendation that this product be used. The Texas Department of Insurance has not authorized the use of any information contained in the product evaluation for advertising, or other commercial or promotional purpose.

This product evaluation is intended for use by those individuals who are following the design wind load criteria in Chapter 3 of the IRC and Section 1609 of the IBC. The design loads determined for the building or structure shall not exceed the design load rating specified for the products shown in the limitations section of this product evaluation. This product evaluation does not relieve a Texas licensed engineer of his responsibilities as outlined in the Texas Insurance Code, the Texas Administrative Code, and the Texas Engineering Practice Act.

R-Control Structural Insulated Panels (SIPs) as manufactured by:

AFM Corporation 211 River Ridge Circle, #102 Burnsville, Minnesota 55337 (800) 255-0176 www.r-control.com

will be acceptable as an alternative residential construction method in designated catastrophe areas along the Texas Gulf Coast when constructed in accordance with this product evaluation.

PRODUCT DESCRIPTION

R-Control Structural Insulated Panels (SIPs) are factory-built structural insulated panels that are used as structurally insulated wall, roof, and floor panels in buildings. The SIPs are fabricated to fit each specific design situation and are subsequently assembled on the job site to form the structural shell. The SIPs consist of oriented strand board (OSB) facings with an expanded polystyrene (EPS) core. The OSB is bonded to the EPS core with structural grade adhesive. The SIPs may be used as load-bearing and nonload-bearing wall and roof components as well as floor components. The SIPs core thickness is available from $3\frac{1}{2}$ to $11\frac{1}{4}$. The SIPs are available in widths from 4 feet to 8 feet and in lengths from 8 feet to 24 feet.

The top and bottom plates of the SIPs are dimension lumber that is sized to match the EPS core thickness of the SIPs. The dimension lumber top and bottom plates are secured to the OSB face panels with Do-All-Ply® adhesive/sealant and fasteners.

R-Control SIPs are connected to each other at the panel edges using splines which may consist of dimension lumber, engineered wood products, or pre-fabricated beams which have been evaluated by AFM for use with the SIPs. The SIPs are connected together with the splines using fasteners and Do-All-Ply® adhesive/sealant.

Product Identification: Each R-Control SIP shall bear the manufacturer's name and the label of the inspection agency, Underwriters Laboratories, Inc.

INSTALLATION REQUIREMENTS

General: Structures built using the R-Control SIPs shall be designed by a Texas licensed professional engineer. Requirements for the design of the SIPs shall be based on the tables and details specified in this evaluation report and the manufacturer's installation requirements. The tables presented in this evaluation report are for the design of the SIPs for walls, roof, and floors. The design of chords, struts, and connections (such as the attachment of diaphragms to chords and struts, the attachment of the SIPS to the foundation, and the hardware required to resist uplift, shear, and the overturning of the shearwall segments) shall be designed separately. Design drawings shall include instructions for the connection and installation of the panels. The design drawings shall be sealed and dated by the design engineer. The design drawings shall reference the appropriate edition of the wind load standard (ASCE 7) used based on the current building specifications adopted by the Texas Department of Insurance. The basic wind speed and the Building Exposure used for the design shall also be referenced.

Design loads: Design wind loads for the SIPs shall be determined using the wind load requirements for the structure as specified in the building specifications adopted by the Texas Department of Insurance. All loads on the SIPs shall not exceed the allowable loads specified in load design charts.

Load Design Charts: Allowable axial, transverse, racking, header, and diaphragm loads for the SIPs shall be as specified in Tables 1-9 of this evaluation report and as specified in the R-Control Load Design Charts for SIPs (Structural Insulated Panels), dated November 2004. NOTE: The requirements specified in the tables in this evaluation report shall govern if there are any conflicts between the manufacturer's Load Design Charts and the tables and figures in this evaluation report.

Foundation: The foundation is considered to be part of the structure and shall be considered part of the design for the structure. If the foundation is not designed by the engineer responsible for the design of the SIP system, then the design plans shall indicate such. As a minimum, the design plans shall indicate how the SIP system is to be anchored to the foundation. If the foundation is included as part of the design, then the design plans shall include all details and specifications related to the design of the foundation to resist the specified wind loads and shall indicate how the structure is to be anchored to the foundation.

Roof Coverings: The design plans shall indicate the requirements for the roof coverings. The roof coverings shall comply with the building specifications adopted by the Texas Department of Insurance. For roof coverings other than asphalt shingles, the design plans shall specify the design pressure requirements for the roof covering. The roof covering shall be installed as required to resist wind pressure.

Exterior wall coverings: Exterior wall coverings shall be installed as required to resist wind pressure. Products shall comply with the building specifications adopted by the Texas Department of Insurance. The design plans shall specify the design pressure requirements for the exterior wall coverings.

Windows, **doors**, **garage doors**, **and skylights**: Products shall be installed as specified in evaluation reports to resist wind pressure. Products shall comply with the building specifications adopted by the Texas Department of Insurance. The design plans shall specify the design pressured requirements for the products. The design plans shall indicate if the products are required to be windborne debris resistant. Windborne debris resistant products shall be installed as specified in evaluation reports to resist wind pressure and windborne debris.

Shutters: The design plans shall indicate if shutters are required. Products shall be installed as specified in evaluation reports or the building specifications adopted by the Texas Department of Insurance as required to resist wind pressure and windborne debris. Products shall comply with the building specifications adopted by the Texas Department of Insurance. The design plans shall specify the design pressures requirements for the shutters.

INSTALLATION REQUIREMENTS (Continued)

Note: A set of sealed plans, manufacturer's installation instructions, R-Control Load Design Charts for SIPs (Structural Insulated Panels), dated November 2004, and this product evaluation report shall be available to the inspector at the job site at all times. All fasteners shall be corrosion resistant as specified in the International Residential Code (IRC), the International Building Code (IBC), and the Texas Revisions.

Wall - Unity Equation

The equation shown below is to determine design suitability. The equation takes into account the ultimate load for a panel subjected to both axial load and transverse (bending) load conditions.

$$\frac{Design \ Axial \ Load}{Allowable \ Axial \ Load} + \frac{Design \ Transverse \ Load}{Allowable \ Transverse \ Load} \leq 1$$

Note: Refer to Table 1 for Wall Axial Loading Refer to Table 3 for Transverse Loading

Table 1
Wall Axial Loading ^{1,2,3,4}
(See Detail SIP-101TX)

R-Control Structural Insulated Panel										
		7/ ₁₆ " OSB	Thickness							
	Panel Height	EPS Core	Thickness							
		3 ½ " Core	5 ½ " Core							
	8'-0"	2,750	4,000							
	10'-0"	2,500	3,500							
Axial Load (plf)	12'-0"	2,000	3,000							
	14'-0"	-	2,750							
	16'-0"	-	2,500							

¹ Maximum allowable axial load is limited to the loads tabulated for axial condition alone.

Table 2
Shear Loading ^{1,3}
(See Detail SIP-101TX)

R-Control Structural Insulated Panel									
		7/16" OSB Thickness							
	Panel Height	EPS Core Thickness							
		3 ½ " Core	5 ½ " Core						
Racking Shear	See Note 2	335 plf	335 plf						

¹Vertical boundaries (Each end of shearwall segments) require double studs, minimum Douglas Fir-Larch No. 2. Shearwall segments must be anchored to resist overturning.

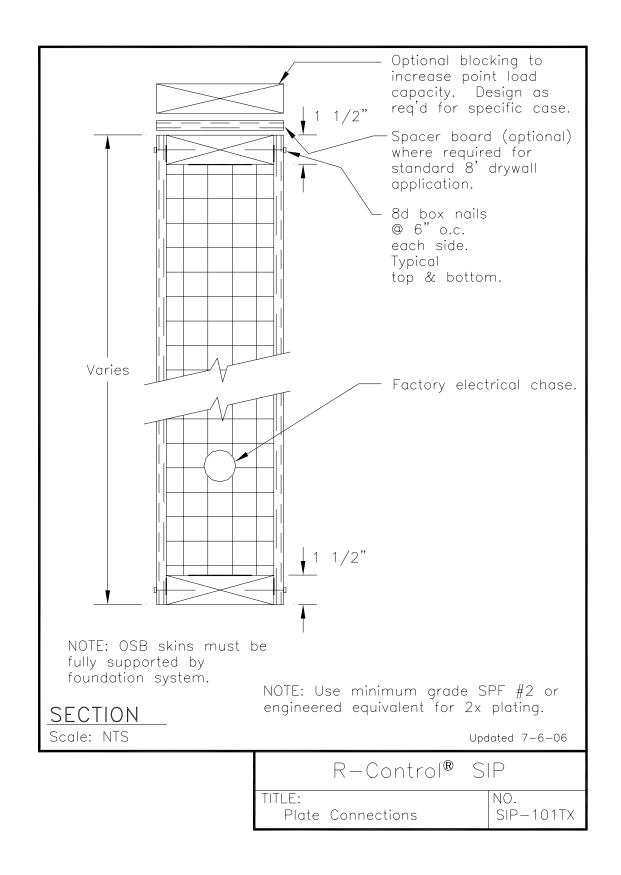
² Ultimate failure load divided by safety factor of 3.0.

³ Values based on a maximum height-to-width ratio of 3 $\frac{1}{2}$:1.

⁴ 2X top plate joints shall be staggered a minimum of 1 foot from panel joints.

² Maximum height-to-width ratio for shearwalls is $3 \frac{1}{2}$:1.

³ 2X top plate joints shall be staggered a minimum of 1 foot from panel joints.



				R-Control Structural I	nsulated Panels								
				7/16	" OSB Thickness								
Pane	l Height		EPS Core Thickness										
			$3\frac{1}{2}$ " Core			$5\frac{1}{2}$ " Core							
Def	lection	L/360	L/240	L/180	L/360	L/240	L/180						
	8'	28	40 1	40 1	42	61 ¹	61 ¹						
(jsd) p	10'	20	30	32 1	32	48	49						
Transverse Load (psf)	12'	15	22	27	26	38	41 1						
Transv	14'	-	-	-	21	31	35						
	16'	-	-	-	17	26	31						

¹Limited to ultimate failure load divided by a safety factor of 3.0.

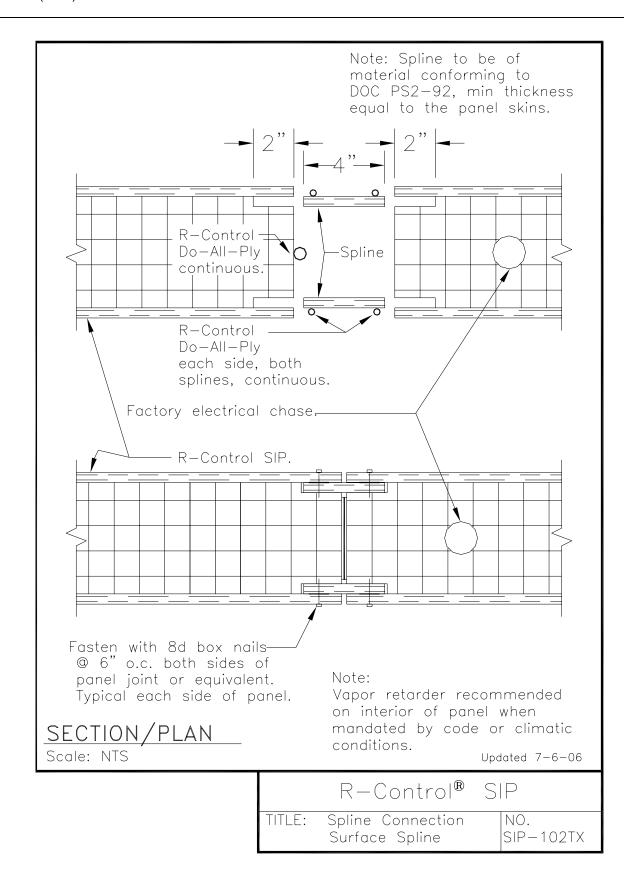
Table 4 ${\it Transverse Loading (psf)}^{\,2} \\ {\it (See Details SIP-102TX and SIP-102gTX)} \\$

	R-Control Structural Insulated Panels															
D (7/ ₁₆ "	OSB Thic	ckness						
	r Floor Span							EPS (Core Thic	kness						
i alici	Оран	3	$\frac{1}{2}$ " Cor	e	5	$\frac{1}{2}$ " Cor	e	7	$\frac{1}{4}$ " Cor	e	9	$\frac{1}{4}$ " Cor	e	1.	1 ½ " Co	re
Defle	ection	L/360	L/240	L/180	L/360	L/240	L/180	L/360	L/240	L/180	L/360	L/240	L/180	L/360	L/240	L/180
	4'	65	80 ¹	80 ¹	89	122	122	92	136	136	107	136	136	104	136	136
(jsd) þi	6'	40	53 ¹	53 ¹	58	81 ¹	81 ¹	64	96 ¹	96 ¹	75	96 ¹	96 ¹	73	96 ¹	96 ¹
Transverse Load (psf)	8'	28	40 1	40 1	42	61 ¹	61 ¹	51	76 ¹	76 ¹	61	76 ¹	76 ¹	60	76 ¹	76 ¹
Transv	10'	20	30	32 1	32	48	49 1	44	64	64	54	64	64	55	64	64
	12'	-	-	-	-	-	-	40	56 ¹	56 ¹	51	56 ¹	56 ¹	55	56 ¹	56 ¹

¹Limited to ultimate failure load divided by a safety factor of 3.0.

² 2X top plate joints shall be staggered a minimum of 1 foot from panel joints.

² Floor panels limited to Group R Occupancies.



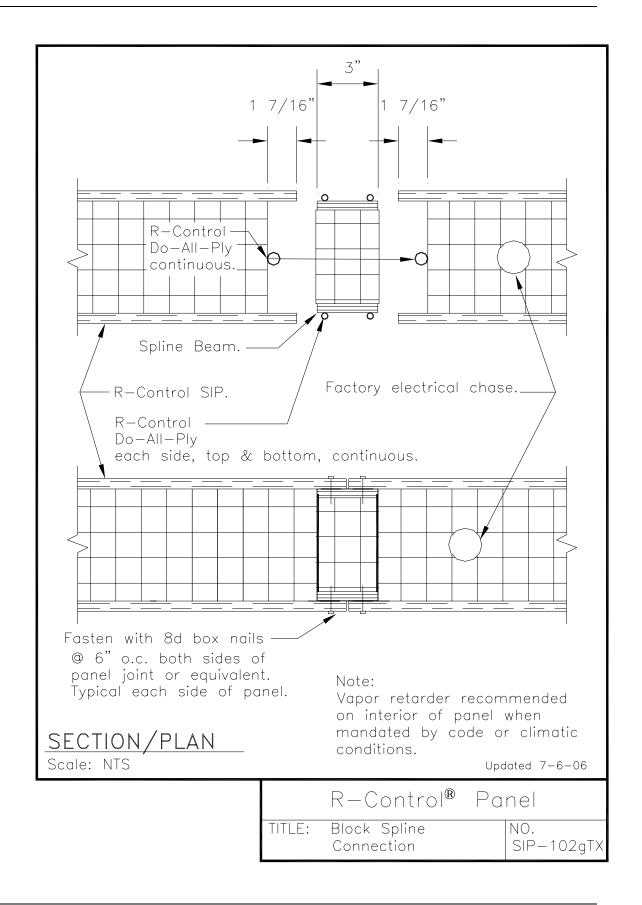


Table 5
Transverse Loading (psf) ^{2,3,4,5}
(See Details SIP-102dTX and SIP-108TX)

					ı	R-Control S	Structural Ins	sulated Par	nels					
Roof	, Wall or						7/ ₁₆ " (OSB Thick	ness					
Floo	r Panel		EPS Core Thickness											
	Span		5 ½ " Co	ore		7 ½ " Co	ore	,	9 ½ " Coı	e		11 $\frac{1}{4}$ " Core		
Det	flection	L/360	L/240	L/180	L/360	L/240	L/180	L/360	L/240	L/180	L/360	L/240	L/180	
	10'	53	79	105	89	109	109	150	174 ¹	174	177	177	177	
	12'	40	59	79	65	91	91	111	145 ¹	145	148	148	148	
sf)	14'	30	45	60	48	72	78 1	84	124	124	115	127	127	
Load (p.	16'	24	35	47	37	55	68 1	65	98	109	89	111	111	
Transverse Load (psf)	18'	19	28	37	28	42	57	51	77	97 1	70	99 1	99 1	
Tr	20'	15	22	30	22	33	44	41	61	82	56	84	89 ¹	
	22'	NP	NP	NP	NP	NP	NP	33	49	66	45	68	81 ¹	
	24'	NP	NP	NP	NP	NP	NP	27	40	54	37	55	74	

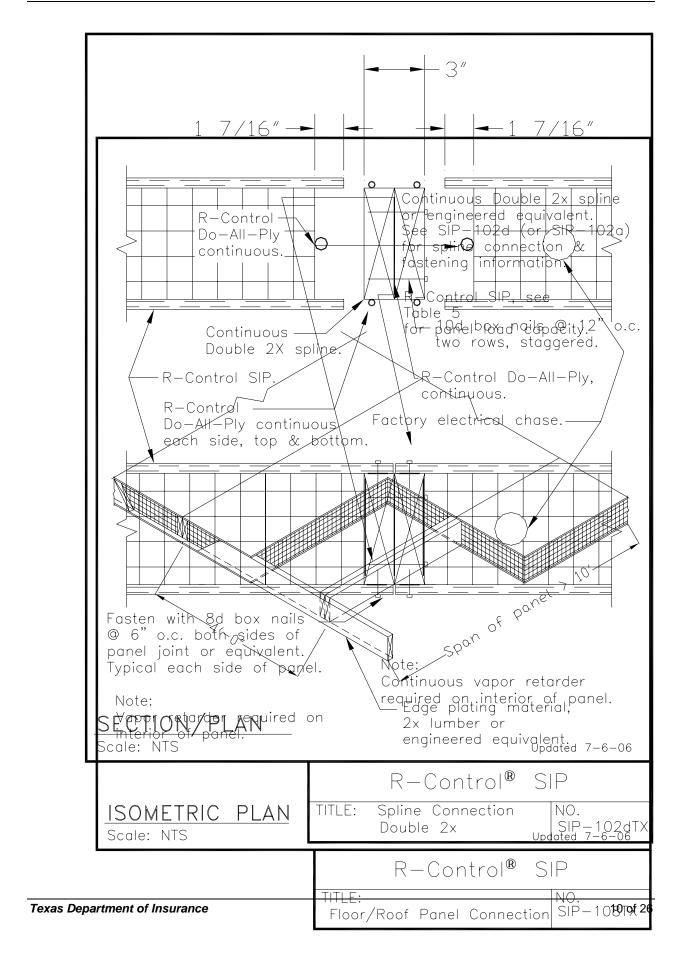
¹Limited to ultimate failure load divided by a safety factor of 3.0.

 $^{^2}$ Roof, wall, and floor panels are framed with continuous doubled nominal 2 inch lumber in the spanning direction. Spaced 4 feet on center, and single nominal 2 inch lumber at panel ends. Lumber is minimum SPF No. 2 grade. Panels below heavy line require Douglas Fir-Larch, No. 2 grade.

³Top facing thickness for floor panels is $\frac{3}{4}$ inch, minimum. As an option, minimum $\frac{7}{16}$ inch top facing may be overlaid with a minimum $\frac{7}{16}$ inch flooring perpendicular to the panels.

 $^{^{4}}$ 2X top plate joints shall be staggered a minimum of 1 foot from panel joints.

⁵ Minimum edge bearing for roof and floor panels is 1 $\frac{1}{2}$ ".



Transverse Loading (psf) 2,3,4,5 (See Details SIP-102bTX and SIP-108aTX)

					R-Control Str	ructural Insulate	d Panels			
						$\frac{7}{16}$ " OSB -				
	or Floor el Span					EPS Core T	hickness			
i aii	ci opan		$7\frac{1}{4}$ " Core			9 ½ " Core			11 $\frac{1}{4}$ " Core	
Def	lection	L/360	L/240	L/180	L/360	L/240	L/180	L/360	L/240	L/180
	10'	81	81 ¹	81 ¹	118	118	118	131	131	131
	12'	63	68	68 ¹	98 1	98	98	109	109	109
sf)	14'	49	58	58 ¹	73	84	84	87	93	93 1
Load (p	16'	38	51 ¹	51 ¹	55	74	74	69	82	82
Transverse Load (psf)	18'	30	45 ¹	45	42	63	65 ¹	55	72	72
Tre	20'	24	37	40 1	33	49	59 ¹	45	65	65
	22'	NP	NP	NP	26	39	49 1	37	55	57 ¹
	24'	NP	NP	NP	21	31	41	30	46	48 1

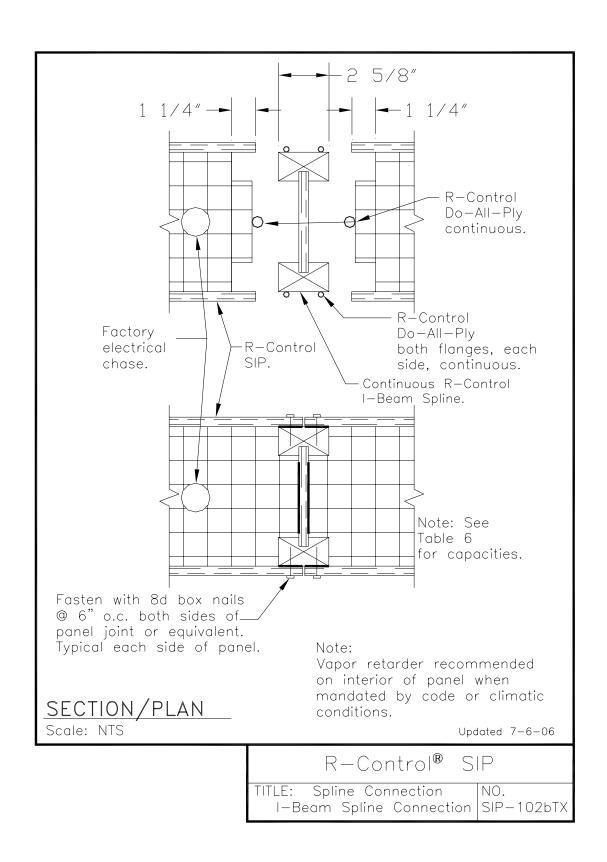
¹Limited to ultimate failure load divided by a safety factor of 3.0.

² Panels require continuous wood I-beams installed in the spanning direction spaced 4 feet on center.

 $^{^3}$ Top facing thickness for floor panels is $\frac{3}{4}$ inch, minimum. As an option, minimum $\frac{7}{16}$ inch top facing may be overlaid with a minimum $\frac{7}{16}$ inch flooring perpendicular to the panels.

⁴ Wood I-joist is SWI-T-34 recognized in ICC-ES legacy report PFC-4801.

⁵ Minimum edge bearing for roof and floor panels is 1 $\frac{1}{2}$ ".



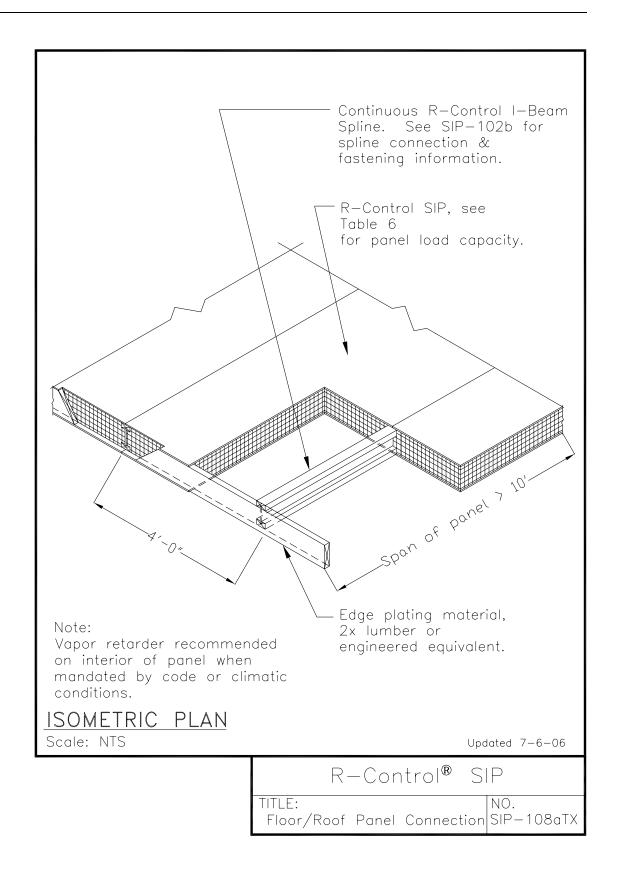


Table 7

Transverse Loading (psf) 2,3,4 (See Details SIP-102cTX and SIP-108bTX)

					F	R-Control S	Structural Ins	sulated Par	nels					
							7/ ₁₆ " (OSB Thickr	ness					
	or Floor el Span		EPS Core Thickness											
			5 ½ " Co	ore		7 ½ " Co	ore	,	9 ½ " Coı	е		11 $\frac{1}{4}$ " Core		
Det	flection	L/360	L/240	L/180	L/360	L/240	L/180	L/360	L/240	L/180	L/360	L/240	L/180	
	10'	76	114	132	101	151	158	117	138	138	149	149	149	
	12'	57	79	96 ¹	72	109	132	88	115	115	124	124	1 124	
sf)	14'	38	57	70 1	54	80	107	68	98	98 1	106	106	106	
Transverse Load (psf)	16'	28	42	54	40	61	81	53	80	86	76	93 1	93 1	
ansverse	18'	21	32	42	31	47	62	42	64	64	64	83 1	83	
Ţ	20'	16	24	32	24	36	49	34	51	52 ¹	50	74 ¹	74 ¹	
	22'	NP	NP	NP	NP	NP	NP	28	42	43 1	40	60	62 ¹	
	24'	NP	NP	NP	NP	NP	NP	23	34	36 ¹	33	49	52	

¹Limited to ultimate failure load divided by a safety factor of 3.0.

 $^{^{2}}$ Panels require continuous insulated spline beams installed in the spanning direction, spaced 4 feet on center.

 $^{^3}$ Top facing thickness for floor panels is $\frac{3}{4}$ inch, minimum. As an option, minimum $\frac{7}{16}$ inch top facing may be overlaid with a minimum $\frac{7}{16}$ inch flooring perpendicular to the panels.

⁴ Minimum edge bearing for roof and floor panels is 1 $\frac{1}{2}$ ".

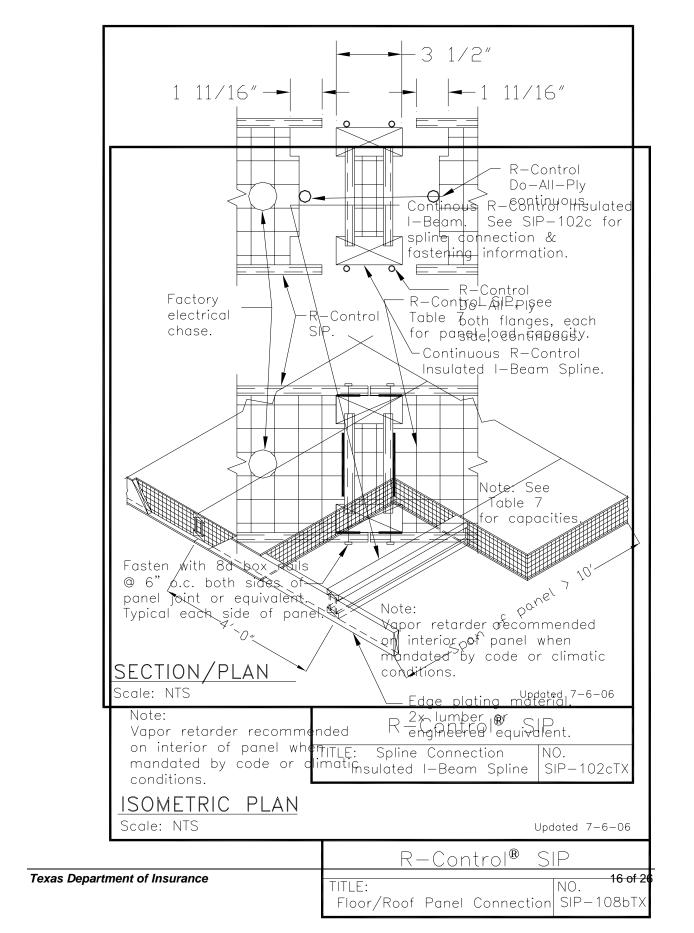


Table 9 ${\it Wall Header Loading (plf)}^{2,3,4} \\ {\it (See Details SIP-112TX, SIP-113TX, SIP-114TX, and SIP-115TX)} \\$

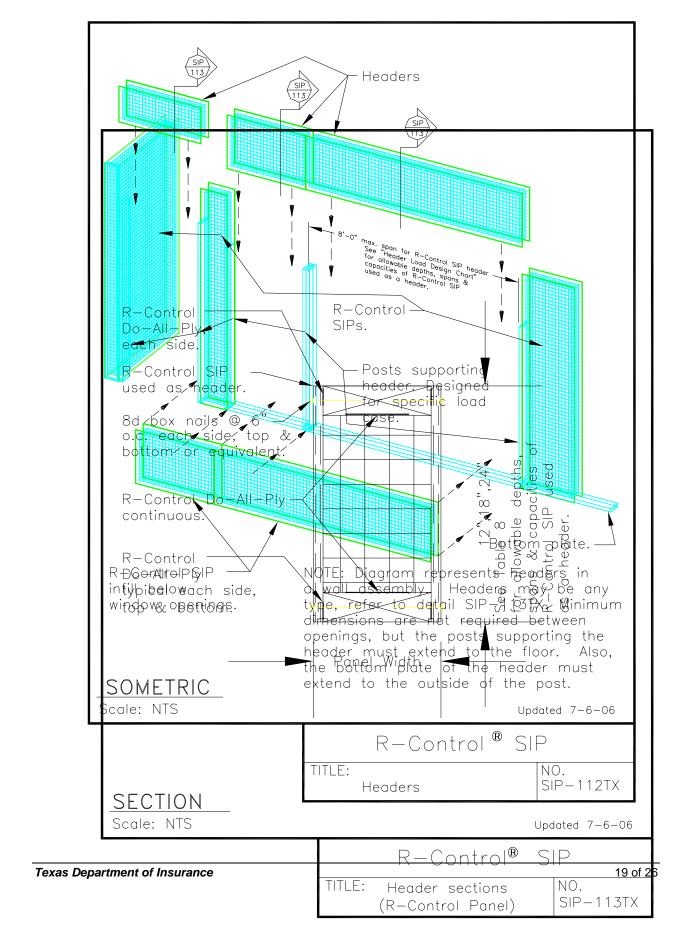
	R-Control Structural Insulated Panels												
	eader Span	Header Depth											
_	pan		12 inches			18 inches			24 inches				
Def	lection	L/480	L/360	L/240	L/480	L/360	L/240	L/480	L/360	L/240			
(plf)	4'	524	703	708	762	773	773	837	837	837			
der Load	6'	319	374	374 ¹	466	466	466	557 ¹	557 ¹	557			
Header	8'	218	248	248	351 ¹	351 ¹	351 ¹	455 ¹	455 ¹	455			

¹Limited to ultimate failure load divided by a safety factor of 3.0.

² Supports and connections shall be designed for each installation.

³ Top and bottom plates shall be Douglas Fir-Larch No. 2.

⁴ See details SIP-112TX, SIP-113TX, SIP-114TX, and SIP-115TX.



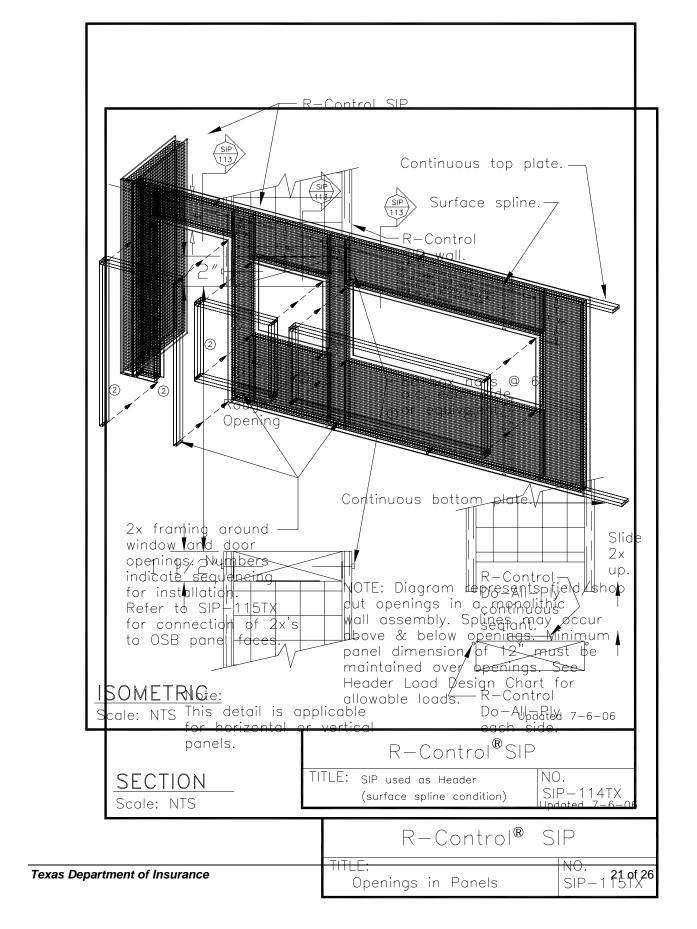


Table 9

Roof/Floor Diaphragm Loading ¹
(See Connection Details SIP-139a, SIP-140TX, and SIP-141TX)

	R-Control Structural Insulated Panel										
	Spacing of R-Control Screw Fasteners 2 at Supported Edges (minimum 1 $^5\!\!/_{\!\! 8}$ " penetration										
- (3 inches	4 inches	6 inches								
7∕ ₁₆ " OSB	Spacing of spline fasteners (8d box or 6d common) at unsupported edges – top										
Thickness	side of panel only – two staggered rows of fasteners on each side of joint										
	3 inches	3 inches	3 inches								
	850 plf ¹	750 plf ¹	500 plf 1								

 $^{^{1}}$ Spline is $\frac{7}{16}$ " OSB x 4.

