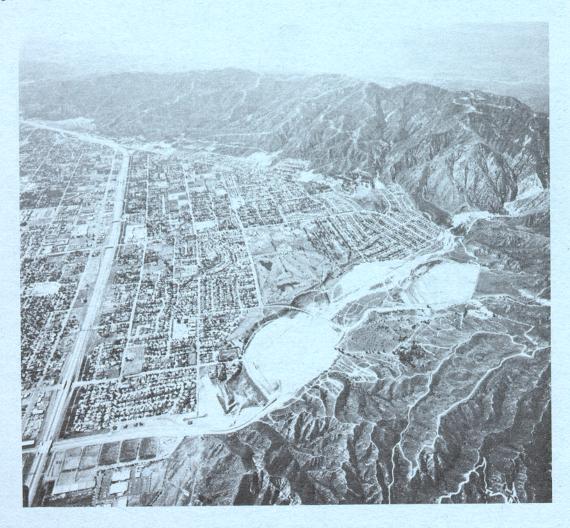


WATER CONTROL MANUAL

LOPEZ DAM PACOIMA WASH California



JANUARY 1986

PERTINENT DATA January 1986

Construction completion date	1 DEC 1954
Stream system	Pacoima Wash
Drainage areasq. miles	34
Reservoir:	
Elevation	4 050 50
Streambed at damft., NGVD	1,253.72
Flood control pool (spillway crest)ft., NGVD	1,272.92
Spillway design surcharge levelft., NGVD	1,292.80
Top of damft., NGVD	1,298.92
Area	
Spillway crestacres	40.4
Spillway design surcharge levelacres	69.9
Top of damacres	80.8
Capacity, gross	
Spillway cresstacre-feet	441 (0.24*)
Spillway design surcharge levelacre-feet	1.566.2 (0.86*)
Top of damacre-feet	2,027.9 (1.12*)
Allowance for sediment (50-year)acre-feet	794 (0.44*)
	(94 (0.44*)
Dam:	Fouther!!!
Type	Earthfill
Height above original streambedft	50
Top lengthft	1,330
Top widthft	_20
Freeboardft	6.1
Slope, upstream and downstream face	1:2
brope, upour cam and downbur cam race	
Slope protection	Loose Rock
	Loose Rock
Slope protection	Loose Rock Broad-crested
Slope protection	
Slope protection	Broad-crested
Slope protection Spillway: Type Crest length Design surcharge ft	Broad-crested 110 19.9
Slope protection Spillway: Type Crest length Design surcharge Design discharge c.ft	Broad-crested
Slope protection Spillway: Type Crest length Design surcharge Design discharge Outlets:	Broad-crested 110 19.9 31,000
Slope protection. Spillway: Type	Broad-crested 110 19.9 31,000 1-5' diameter
Slope protection. Spillway: Type Crest length	Broad-crested 110 19.9 31,000 1-5' diameter 428
Slope protection. Spillway: Type. Crest length. Design surcharge. Design discharge. Outlets: Number and size-diameter. Length. Entrance invert elevation. Spillway: ft. ft. ft. ft. ft. ft. ft. ft	Broad-crested 110 19.9 31,000 1-5' diameter
Slope protection. Spillway: Type Crest length	Broad-crested 110 19.9 31,000 1-5' diameter 428 1,253.92
Slope protection. Spillway: Type. Crest length. Design surcharge. Design discharge. Outlets: Number and size-diameter. Length. Entrance invert elevation. Standard project flood: Duration (inflow).	Broad-crested 110 19.9 31,000 1-5' diameter 428 1,253.92
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Slope protection. Spillway: Type Crest length	Broad-crested 110 19.9 31,000 1-5' diameter 428 1,253.92 3 14,100 (7.78*) 11,200
Slope protection. Spillway: Type Crest length	Broad-crested 110 19.9 31,000 1-5' diameter 428 1,253.92 3 14,100 (7.78*)
Slope protection. Spillway: Type Crest length	Broad-crested 110 19.9 31,000 1-5' diameter 428 1,253.92 3 14,100 (7.78*) 11,200
Slope protection. Spillway: Type Crest length	Broad-crested 110 19.9 31,000 1-5' diameter 428 1,253.92 3 14,100 (7.78*) 11,200 11,200
Slope protection. Spillway: Type. Crest length. Design surcharge. Design discharge. Outlets: Number and size-diameter. Length. Entrance invert elevation. Standard project flood: Duration (inflow). Total volume. Inflow peak. Outflow peak. Probable maximum flood: Duration (inflow). Duration (inflow). Cof.s. Probable maximum flood: Duration (inflow). Duration (inflow). Adays. Total volume. Acre-feet.	Broad-crested 110 19.9 31,000 1-5' diameter 428 1,253.92 14,100 (7.78*) 11,200 11,200 19,900 (10.97*)
Slope protection. Spillway: Type Crest length	Broad-crested 110 19.9 31,000 1-5' diameter 428 1,253.92 14,100 (7.78*) 11,200 11,200 19,900 (10.97*) 30,400
Slope protection. Spillway: Type Crest length	Broad-crested 110 19.9 31,000 1-5' diameter 428 1,253.92 14,100 (7.78*) 11,200 11,200 19,900 (10.97*)
Slope protection. Spillway: Type. Crest length	Broad-crested
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Slope protection. Spillway: Type. Crest length	Broad-crested
Slope protection. Spillway: Type. Crest length	Broad-crested



DEPARTMENT OF THE ARMY SOUTH PACIFIC DIVISION, CORPS OF ENGINEERS

630 Sansome Street, Room 720 San Francisco, California 94111-2206

CESPD-ED-W (1110-2-240b)

MAR 2 0 1331

MEMORANDUM FOR Commander, Los Angeles District Commander, Sacramento District

SUBJECT: Planned Deviations from Approved Water Control Plans

- 1. All planned deviations from approved water control plans for reservoir projects within the South Pacific Division must be coordinated with the Coastal Engineering and Water Management Division at CESPD. Approval must be given prior to implementation of the deviation.
- 2. Emergency deviations do not require prior approval but coordination must still be made as soon as is practical.

OGER E

Brigadie General, U.S. Army

Commanding

ij

WATER CONTROL MANUAL

LOPEZ DAM

PACOIMA WASH, CALIFORNIA

January 1986

Prepared by

U.S. Army Corps of Engineers
Los Angeles District
Reservoir Regulation Unit



Lopez Dam and Reservoir (view from downstream of dam, taken 5/14/85).

NOTICE TO USERS OF THIS MANUAL

Regulations specify that this Water Control Manual be published in looseleaf form, and only those sections, or parts thereof, requiring changes will be revised and printed. Therefore, this copy should be preserved in good condition so that inserts can be made to keep the manual current.

EMERGENCY REGULATION ASSISTANCE PROCEDURES

In the event that unusual conditions arise, contact can be made by telephone to the Reservoir Regulation Unit, Los Angeles District Office (FTS 798-4756 or 213-894-4756). See Plate 5-2 for other important telephone numbers for reservoir regulation assistance.

WATER CONTROL MANUAL

LOPEZ DAM AND RESERVOIR PACOIMA WASH, CALIFORNIA

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Š.	4	River	· · · · · · · · · · · · · · · · · · ·			
C		Gate Opera	ation Schedule	and Instruc	ctions to Da	m Operator

ABBREVIATIONS USED

AF Acre-Feet

ALERT Automatic Local Evaluation in Real-Time

cubic feet per second cfs

COE U.S. Army Corps of Engineers

F Fahrenheit

ft feet

LACDA Los Angeles County Drainage Area

Los Angeles County Department of Public Works LACDPW

Los Angeles County Flood Control District LACFCD

Los Angeles District, U.S. Army Corps of Engineers LAD

National Geodetic Vertical Datum NGVD

NWS National Weather Service PMF Probable Maximum Flood

ppm parts per million SPF Standard Project Flood VHF Very High Frequency

I - INTRODUCTION

1-01 AUTHORIZATION

This Lopez Dam Water Control Manual was prepared in compliance with the following directives: ER 1110-2-240, EM 1110-2-3600, and ETL 1110-2-251.

1-02 PURPOSE AND SCOPE

This water control manual provides a detailed plan for regulation of Lopez Dam and Reservoir on Pacoima Wash for the purpose of flood control. Lopez Dam is located approximately 2.2 miles northeast of the city of San Fernando, California. Major topics in this manual include: authorization, history, and description of the project; watershed characteristics; hydrometeorology; data collection and communication networks; hydrologic forecasting; the water control plan; and responsibilities and coordination for water control management.

1-03 RELATED MANUALS AND REPORTS

Manuals and reports with data and information relevant to the information in this manual are listed in Table 1-1.

1-04 PROJECT OWNER

Lopez Dam and Reservoir was constructed and is owned and operated by the U.S. Army Corps of Engineers, Los Angeles District (LAD).

1-05 OPERATING AGENCIES

- a. The LAD is responsible for the operation and maintenance of the dam, reservoir, and outlet works. The outlet gate is operated manually, as needed, and the dam is not normally manned. The operator for Hansen Dam serves as the Lopez Dam operator when necessary.
- b The Los Angeles County Department of Public Works (LACDPW) is responsible for the operation and maintenance of an adjacent diversion works for the Lopez Spreading Grounds.

1-06 REGULATING AGENCIES

- a. The LAD is responsible for developing the flood control operation plan for Lopez Dam and Reservoir. The LAD is responsible for the flood control regulations and operation of the dam.
- b. The LACDPW is responsible for the regulation of the diversion works for the downstream Lopez Spreading Grounds used for groundwater replenishment.

TABLE 1-1

RELATED MANUALS AND REPORTS

- U.S. Army Corps of Engineers, Los Angeles District, "Hydrology-Pacoima Wash Channel and Lopez Flood-Control Basin," March, 1950.
- U.S. Army Corps of Engineers, Los Angeles District, "Design Memorandum No. 1 on Lopez Flood Control Reservoir," January, 1953.
- U.S. Army Corps of Engineers, Los Angeles District, "Design Memorandum No. 1 on Pacoima Wash Channel Improvement-Lopez Dam to Arleta Avenue," March, 1953.
- 4. U.S. Army Corps of Engineers, Los Angeles District, "Design Memorandum No. 1 on Pacoima Wash Channel Improvement-Arleta Avenue to Tujunga Wash," March, 1953.
- U.S. Army Corps of Engineers, Los Angeles District, "Design Memorandum No.
 2 on Lopez Flood Control Reservoir," February, 1954.
- U.S. Army Corps of Engineers, Los Angeles District, "Design Memorandum No. 3 on Lopez Flood Control Reservoir," February, 1954.
- 7. U.S. Army Corps of Engineers, Los Angeles District, "Operation and Maintenance Manual for Lopez Flood Control Reservoir," July, 1963.
- 8. U.S. Army Corps of Engineers, Los Angeles District, "Lopez Dam and Reservoir-Periodic Inspection and Continuing Evaluation Report No. 1," May, 1970.
- 9. U.S. Army Corps of Engineers, Los Angeles District, "Lopez Dam and Spillway-Supplemental Report on Earthquake Damage," June, 1971.
- 10. U.S. Army Corps of Engineers, Los Angeles District, "Lopez Dam-Dam, Outlet Works and Spillway Periodic Inspection Report No. 2," July, 1975.
- 11. U.S. Army Corps of Engineers, Los Angeles District, "Interim Report on Hydrology and Hydraulic Review of Design Features of Existing Dams for Los Angeles County Drainage Area Dams," June, 1978.
- 12. U.S. Army Corps of Engineers, Los Angeles District, "Lopez Dam-Dam, Outlet Works and Spillway Periodic Inspection Report No. 3," May, 1980.

2-01 LOCATION

Lopez Dam and Reservoir are located on Pacoima Wash in the northcentral part of the San Fernando Valley, about 2.2 miles northeast of the city of San Fernando and entirely within the city and county of Los Angeles, in California. Lopez Dam is approximately 3.5 miles northwest of Hansen Dam. The location of Lopez Dam and Reservoir is shown on Plate 2-1.

2-02 PURPOSE

Lopez Dam and Reservoir are an integral unit on the Pacoima-Tujunga Wash system of tributaries to the Los Angeles River. The purpose of the project, a unit under the approved comprehensive plan for flood control in the Los Angeles County Drainage Area (LACDA), is to provide protection against debrisladen floodwaters for large areas between the dam site and the Los Angeles River. Important improvements in these areas include valuable industrial, business, and residential properties and transportation systems. The dam also forms a headworks to direct flows into the Pacoima Wash channel. The storage allocation for Lopez Reservoir is shown on Plate 2-2.

2-03 PHYSICAL COMPONENTS

- a. Embankment. The earth-fill embankment has a crest length of approximately 1,300 feet and a maximum height of approximately 50 feet above the bed of Pacoima Wash. The crown width at the crest elevation of 1,298.9 is 20 feet. Upstream and downstream slopes of the embankment are 1 vertical on 2 horizontal throughout. The embankment has an impervious inner core with a crest width of 12 feet at elevation 1,292.8 with slopes of 1 vertical to 1 horizontal upstream and downstream. The upstream and downstream portions of the embankment are pervious zones constructed of material excavated from within the reservoir upstream of the dam and rock excavation from the spillway site. A three feet layer of loose rock on the upstream and downstream slopes of the embankment serves as protection against erosion. Information related to the embankment is shown on Plate 2-3. Photographs of the embankment are shown on Figure 2-1.
- b. Spillway. The reinforced concrete spillway structure is located at the left abutment (looking downstream) of the dam. The spillway was designed as a combined outlet and spillway. The design discharge for the downstream channel is 11,000 cubic feet per second (cfs) and for the spillway, the design discharge is 31,000 cfs. The spillway is an uncontrolled broadcrested type with the crest at elevation 1,272.9. The spillway consists of a rectangular channel, extending about 92 feet upstream and about 470 feet downstream from the axis of the dam. The channel converges from a width of 110 feet at the crest, or axis of the dam (station 10+00), to a width of 30 feet at the end of the spillway transition (station 14+70.48) where it joins the Pacoima Wash channel. An overflow section is provided in the right wall of the spillway channel between station 12+13 and station 14+70.48 to spill flows in excess of 14,000 cfs during a spillway design flood.

The upstream approach channel has a reinforced concrete invert and vertical walls diverging upstream from the crest (at the same rate as the downstream convergence) to a channel base width of 122.0 feet at the beginning of the invert. The vertical walls terminate in quadrant walls forming the spillway approach. The tops of these quadrant walls are at elevation 1,293.1. A concrete cutoff wall extending to a depth of 8 feet into rock is constructed beneath the approach channel invert slab at its upstream edge (station 9+10) and along the right side to station 10+10. Details, dimensions, and other information related to the spillway are shown on Plate 2-4 and Plate 2-5. Figure 2-2 shows photographs of the spillway.

c. Reservoir Outlet. A low level outlet is provided to empty the reservoir pool remaining after flow over the spillway has ceased and during low flows. The outlet is a cast-in-place, reinforced concrete conduit with a flat invert and an inside diameter of 60 inches. Other appurtenant features include a low level intake, a perforated intake tower, and a combination gate well and recorder well. Plate 2-6 and Plate 2-7 show pertinent information pertaining to the reservoir outlet.

The low level intake structure is equipped with removable steel grates spaced to permit the entry of water but to prevent the entry of trash and debris. The invert elevation of the low level intake is 1,253.9. The intake tower is a cast-in-place, reinforced concrete structure opening into the conduit. The tower is 5 feet square inside and extends to elevation 1,273.9 (one foot above spillway crest). It is perforated with 4-inch diameter formed openings which permits the pool to be drained down in the event the low level intake becomes inoperative. Photographs of the low level intake structure and the intake tower are shown in Figure 2-3, and Figure 2-4.

The gate well and float recorder well-structure consists of a 2-compartment, reinforced concrete shaft, 10.5 feet by 3 feet and 42 feet deep inside, located in the upstream embankment near the axis of the dam (at station 2+44). A concrete block and steel structure covers the recorder well shaft and houses the recording equipment. Photographs of the gate and recorder structure are shown in Figure 2-5. The recorder well is fed from an intake structure in the reservoir, near the pool drain intake, through a 6-inch diameter steel pipe, encased in concrete. The recorder well compartment is also connected with the gate chamber by a short length of 3-inch diameter cast iron pipe equipped with a gate valve. This connection permits flushing of the recorder well and permits the recorder well to be fed either from the reservoir intake or from the gate well when the 60-inch slide gate is closed.

A heavy-duty circular 60-inch diameter slide gate with bronze seats is installed in the gate well. This gate is raised and lowered by means of a single gear pedestal lift, which is mounted on top of the gate well and equipped with a handcrank mechanism. One inch movement of the gate requires 16 turns of the handcrank. Approximately $2^{1}/2$ inches of the stem is exposed above the lift nut when the gate is closed. The height of the gate opening is determined by measuring the length of the stem above the lift nut.

A 3-inch brass gate valve with nonrising stem is provided at the flanged end of the short 3-inch diameter cast iron pipe connecting the float recorder well and the gate chamber. The valve operating handle is located just below

the floor level of the recorder house. For satisfactory operation of the recorder, this gate valve must be kept closed when the slide gate is open and water is flowing through the conduit.

2-04 RELATED CONTROL FACILITIES

A diversion structure is located near the downstream end of the outlet pipe, immediately upstream of where the outlet pipe discharges into the spillway channel (see Plate 2-4). The diversion structure consists of a reinforced concrete box provided with slots for stop logs and a 30-inch diameter circular slide gate. The diversion structure was constructed with Lopez Dam in 1954 by the COE for the purpose of providing flows to the Lopez Spreading Grounds. The 30-inch gate remains closed and the stop logs are removed at all times except when spreading operations are in progress (Figure 2-6).

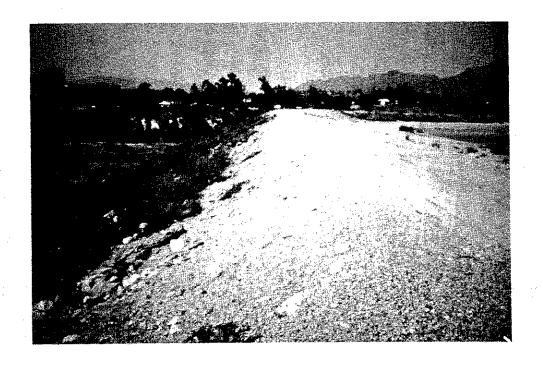
Other facilities located in the vicinity which are impacted by or affect the operation of Lopez Dam and Reservoir are described in Section 3-04.

2-05 REAL ESTATE ACQUISITION

Lopez Dam and Reservoir Project lands comprise 101.4 acres as shown on Plate 2-3.

2-06 PUBLIC FACILITIES

No public facilities are included in the Lopez Dam and Reservoir Project.

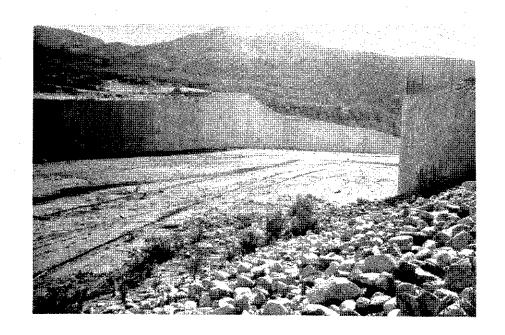


(a) Downstream Slope.



(b) Upstream Slope

Figure 2-1. Photographs of Lopez Dam Embankment.

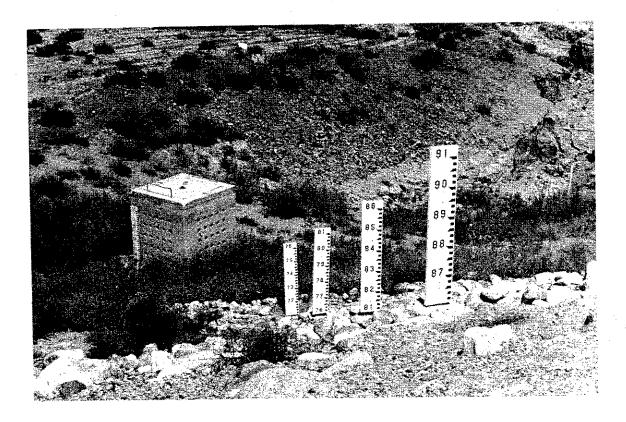


(a) Looking Downstream.

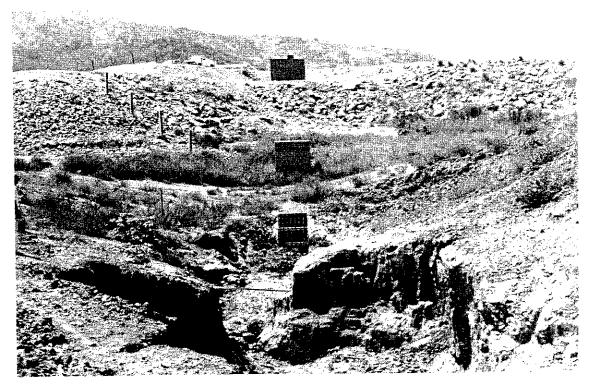


(b) Looking Upstream.

Figure 2-2. Photographs of Lopez Dam Spillway.

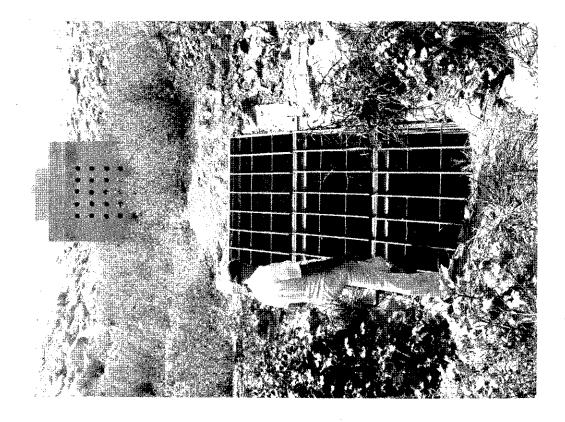


(a) Intake Tower.



(b) Low Level Inlet Structure.

Figure 2-3. Photographs of Lopez Dam Outlet.



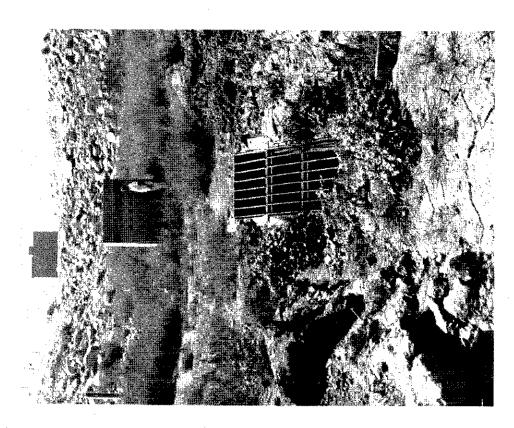
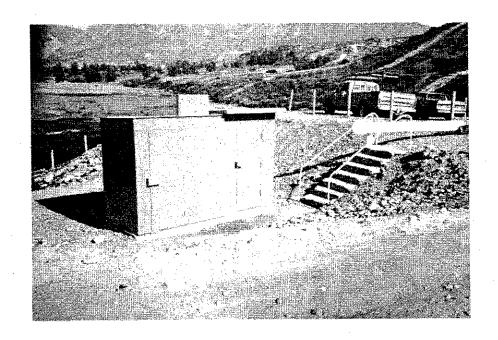
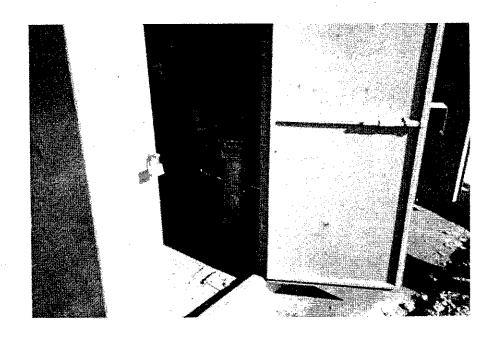


Figure 2.4. Photographs of Lopez Dam Intake Tower and Low Level Inlet Structure.

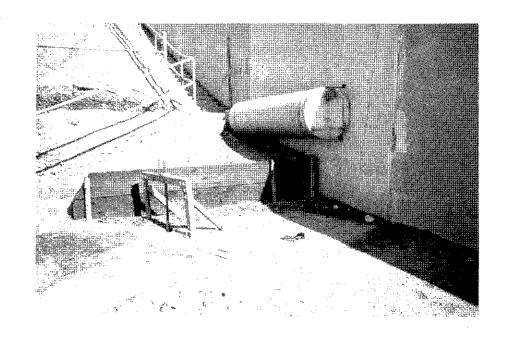


(a) Gate and Recorder Structure.



(b) Handrank Mechanism for Slide Gate.

Figure 2-5. Photographs of Lopez Dam Gate and Recorder Structure.



(a) Diversion Structure, Manhole.(The CMP is used to store equipment.)



(b) Spreading Facility.

Figure 2-6. Photographs of Lopez Spreading Grounds.

3-01 AUTHORIZATION

Lopez Dam was authorized by the Flood Control Act, approved 22 June 1936 (Public Law 738, 74th Congress) and extended and amended by subsequent Flood Control Acts of 1937, 1938, 1941, 1944, and 1946. The plan for construction, in accordance with the recommendations contained in the report dated 11 April 1940 by the Chief of Engineers, and submitted in House Document 838, 76th Congress, 3rd session, was authorized by the Flood Control Act, approved 18 August 1941.

3-02 PLANNING AND DESIGN

The planning for Lopez Dam was conducted by the LAD in conjunction with local interests including the Los Angeles County Flood Control District (LACFCD). A comprehensive plan for flood control in the Los Angeles County drainage area was presented by the LACFCD at a public hearing conducted by the Los Angeles District, U.S. Army Corps of Engineers (LAD) on 31 March 1936. The plan included provisions for improvement of the Pacoima Wash channel. The LAD subsequently began an investigation for flood control alternatives in the Pacoima Wash basin. The investigation was comprised of several alternatives including: locations for Lopez Dam, improvements to the Pacoima Wash channel, and diversion plans for flow from Lopez Canyon.

Design Memorandum No. 1 on Lopez Reservoir was prepared by the LAD in January, 1953. Design Memoranda Nos. 2 and 3 were prepared in February, 1954. Design Memorandum No. 1 contains the following principal departures from the original project-document plan: (1) constructing Lopez Dam at a more economical site on Pacoima Wash; (2) diverting flow from Lopez Canyon into Hansen Reservoir; (3) providing for a debris-control reservoir instead of a flood-control reservoir; (4) increasing the design peak outflow from 3,000 to 11,000 cubic feet per second; and (5) increasing the capacity of Pacoima Wash channel downstream from Lopez Dam. These departures involve changes in dimensions and elevation of the dam and changes in alignment of the channel. The spillway was intended for routine use. The purpose of Lopez Dam is to provide protection against debris-laden floodwaters for large areas between the dam site and the Los Angeles River. Some degree of flood reduction may be realized due to the physical location of the dam.

3-03 CONSTRUCTION

Construction for Lopez Dam started 1 April 1954 with the work being completed and accepted by the U.S. Army Corps of Engineers (COE) on 1 December 1954. The project was constructed by Vinnell Company, Inc. and copies of the construction contract (Contract No. DA-04-353 ENG-3198) and "as-constructed" drawings (File Nos. 145/1 through 145/22) are on file in the COE LAD office.

On 9 February 1971, an earthquake violently shook the San Fernando area in Los Angeles County. The tremors lasted for approximately 60 seconds. The earthquake was rated 6.6 on the Richter magnitude scale at the California Institute of Technology Seismological Laboratory at Pasadena, California, and

the epicenter was located 9 miles from Lopez Dam. The earthquake resulted in a general uplift of the dam and reservoir of approximately 6.9 feet and significant damage was sustained by the embankment, concrete spillway, spillway subdrain pipes, and pool drain conduit. Repairs were made resulting in the project being restored to original "as-constructed" conditions as shown on the as-constructed record drawings dated July 16, 1971, with the exception of the elevation changes due to the general uplift.

3-04 RELATED PROJECTS

Plate 3-1 shows related projects for the entire Los Angeles County drainage area. Related LACDPW water supply facilities in the Pacoima Wash drainage area are shown on Plate 3-2.

- a. Pacoima Dam. Pacoima Dam is located on Pacoima Creek approximately 1.5 miles upstream of the Lopez Dam site. The concrete-arch structure, completed in 1929, is operated and maintained by the LACDPW for flood control and water conservation. Information pertaining to Pacoima Dam and Reservoir is given in Exhibit A.
- b. <u>Harding Street Bridge</u>. The highway bridge across Pacoima Wash at Harding Street is located at the upstream end of the reservoir. It was designed for a discharge of 4,000 cfs at a depth of flow of 6 feet. Harding Street is the primary access route to Pacoima Dam.
- c. Lopez Spreading Grounds. Lopez Spreading Grounds is owned and operated by the LACDPW. The facility is located approximately 1,500 feet downstream of Lopez Dam. The Lopez Dam outlet is used by the LACDPW to divert water into the spreading grounds as discussed in Section 2-04. Photographs of the diversion structure and the spreading facility are shown on Figure 2-5. Pertinent information concerning Lopez Spreading Grounds is listed below:

Maximum basin intake capacity

Maximum basin outlet discharge

Allowable water quality sediment limit

Storage capacity

Percolation rate

Basin gage height limits

Basin gage height limit refers to the depth of water in the basin during spreading operations.

d. Pacoima Wash Channel. The Pacoima Wash channel has been improved from Lopez Dam to the Pacoima Spreading Grounds. The channel improvement projects were completed by the COE as the lead agency with engineering and financial cooperation from the cities of Los Angeles and San Fernando, and the LACDPW. The improvements consisted of channelization, new bridges, drainage confluence and side drainage structures, and utilities relocation. The improved channel cross section varies from rectangular to trapezoidal. The channel capacity is 11,000 cfs at Lopez Dam and increases to 17,000 cfs at the Golden State Freeway crossing as shown on Plate 3-3. The rectangular channel is lined with reinforced concrete. The trapezoidal channel is lined with a combination of reinforced concrete and grouted stone.

e. Pacoima Spreading Grounds. Pacoima Spreading Grounds is owned and operated by the LACDPW. The facility is located approximately 3.5 miles downstream of Lopez Dam. A radial gate (55 feet long and 8.5 feet high) across Pacoima Diversion Channel diverts flows through the intake structure into the spreading grounds. The intake structure consists of four slide gates (each 5 feet x 5 feet). A photograph of the diversion structure is shown on Figure 3-1. An aerial photograph of the spreading facility is shown on Figure 3-2. Pertinent information concerning Pacoima Spreading Grounds is listed below:

Maximum basin intake capacity

600 cfs during storm releases

400 cfs during conservation releases

Maximum basin outlet discharge

Minimal (10 cfs) unless for emergency

Allowable water quality

sediment limit

500 ppm

Storage capacity

Percolation rate

100 cfs

*Basin gage height limit

5.0 ft

*Resin gage height limit refers to the depth of water in the basin december of the depth of water in the basin december.

* Basin gage height limit refers to the depth of water in the basin during spreading operations.

The LACDPW removes the radial gate from the channel and does not lower the gate into the channel when flows reach 1,800 cfs.

- f. Pacoima Diversion Channel. The Pacoima Diversion channel extends from the Pacoima Spreading Grounds to the Tujunga Wash. It is located southwest of and parallel to the Golden State Freeway, as shown on Plate 3-3. This channel is a man-made structure and was constructed as a part of the Lopez Dam Project. The channel capacity is 17,000 cfs. The trapezoidal section of the channel is lined with a combination of reinforced concrete and grouted stone. The rectangular section is lined with reinforced concrete. The section limits are shown on Plate 3-3.
- g. <u>Branford Spreading Grounds</u>. Branford Spreading Grounds are owned and operated by the LACDPW. The facility is located near the Pacoima Diversion Channel/Tujunga Wash confluence. The facility discharges water into the Pacoima Diversion Channel and has no capability of diverting water from the channel. Pertinent information concerning Branford Spreading Grounds is listed below:

Maximum basin intake capacity 1,540 cfs
Maximum basin outlet discharge 1,520 cfs
Storage capacity 250 AF

3-05 MODIFICATIONS TO REGULATION

Design Memorandum No. 1 prepared in January 1953 did not provide for a gate on the outlet conduit. The outlet was designed to drain the low pool after each storm and to discharge material sluiced through Pacoima Dam. The size of the outlet was determined on the basis of free flow for a discharge of 300 cubic feet per second which was the estimated capacity of the sluice outlet of Pacoima Dam.

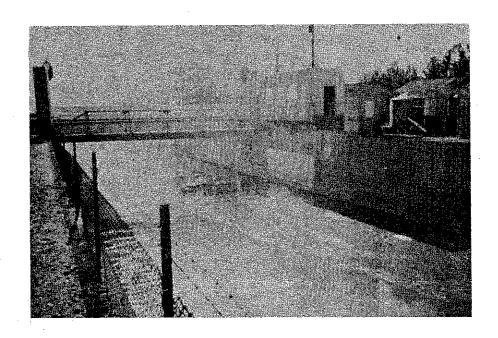


Figure 3-1. Photograph of Pacoima Spreading Grounds Diversion Structure, Looking Downstream.

Figure 3-2. Aerial Photograph of Pacoima Spreading Grounds.

Design Memorandum No. 2 prepared in February 1954 included a gate on the outlet conduit. The gate was added to the design following a model study in which it was determined that conduit flow emerging from the pool drain outlet to join the flow over the spillway would set up undesirable wave turbulence in the downstream spillway channel. The gate allowed the conduit to be closed during spillway flow.

Although no formal water control plan has existed, it was the intent of the original design for the gate to be closed during spillway flow or storms and opened to drain the low pool after each storm or to discharge material sluiced through Pacoima Dam. This procedure has been followed since operation of the dam began in 1954. A temporary modification was in effect from 1959 to 1964. A 1959 agreement allowed the LACDPW to operate the project for water supply purposes. However, the agreement expired in 1964 and has not been renewed. In addition, LAD has allowed temporary closure of the gate to trap material sluiced through Pacoima Dam. This was done with the understanding that LACDPW be responsible for removal of the trapped debris.

This water control manual includes the first formal water control plan for Lopez Dam. The water control plan is consistent with the intent of the original design. The gate will remain fully open except during spillway flow, when it will be fully closed.

3-06 PRINCIPAL REGULATION PROBLEMS

Other than the damage resulting from the February 1971 earthquake described in Section 3-03, there have been no major problems associated with the regulation of Lopez Dam and Reservoir. However, two minor problems have been identified that affect the regulation.

The first problem is associated with non-vegetative debris accumulation. The design for Lopez Dam included a sediment storage space large enough to hold the debris of a 50-year accumulation plus the debris produced by a single flood. Using a one percent debris surface slope projected upstream from the spillway crest, a debris storage allowance of 1,280,000 cubic yards (794 acrefeet) was originally provided. This storage was completely filled with debris by February 1969, only 15 years after completion of the dam. Debris removal is required more frequently due to this higher debris production.

The second problem pertains to seepage that occurs along the downstream toe of Lopez Dam. The seepage occurs when the reservoir level is at the spillway crest or above. The seepage does not appear to be detrimental to the integrity of the dam. However, stability problems could possibly arise at higher water levels or if the water level is maintained at the spillway crest level or above for prolonged time periods. The seepage problem creates a constant need for COE personnel to monitor the dam and limits prolonged reservoir storage. Seepage is also a cause of considerable concern to the local residents.

IV - WATERSHED CHARACTERISTICS

4-01 GENERAL CHARACTERISTICS

Lopez Dam is located on Pacoima Wash in the northcentral part of the San Fernando Valley, about 2.2 miles northeast of the city of San Fernando, and entirely within the city of Los Angeles. The drainage area is 34 square miles, about 15 miles long and from 1.5 to 3 miles wide. Pacoima Dam, owned and operated by the LACDPW, has flood control and conservation storage and is located 1.5 miles upstream of Lopez Dam. The drainage area located between Pacoima Dam and Lopez Dam is 6 square miles.

4-02 TOPOGRAPHY

The elevations in the drainage area range from a peak of 6,532 feet above National Geodetic Vertical Datum (NGVD) on Mount Gleason in the San Gabriel Mountains to 1,250 feet above NGVD at Lopez Dam. The longest water course in the area, Pacoima Wash, is about 19 miles in length with an average gradient of 275 feet per mile.

4-03 GEOLOGY AND SOILS

The soils in the mountains, which are derived mainly from metamorphic and igneous rocks, are shallow and stony. In the valleys, the soil consists of silt, sand, and gravel with some boulders and is relatively deep. The canyon slopes have a sparse cover of live oak, chaparral, and native grass.

4-04 SEDIMENT

The original design for Lopez Dam was based on providing a debris control reservoir to retain the debris of a 50-year accumulation plus the debris produced by a single flood. Using a 1 percent debris surface slope projected upstream from the spillway crest, a debris storage allowance of 1,280,000 cubic yards (794 acre-feet) was originally provided. This storage was completely filled with debris by February 1969, only 15 years after completion of the dam. As a result of this higher debris production, either the spillway crest should be raised 16.5 feet to provide the necessary debris storage, or, more likely, debris should be removed more frequently.

4-05 CLIMATE

The climate of the watershed is characterized by warm, dry summers and cool, moist winters. The climate varies throughout the watershed due to the variation in elevation. The higher elevations are generally slightly cooler and receive more precipitation than the lower portion of the watershed. Nearly all the precipitation occurs during the months of December through March. Elevations above 5,000 feet frequently experience snowfall with the snow usually melting rapidly except on higher peaks and northern slopes. The average annual temperature at the city of San Fernando is 63.6 degrees F. The lowest average monthly temperature is 54.2 degrees F in January and the highest average monthly temperature is 74.6 degrees F in August. The average annual rainfall recorded at the San Fernando station is 16.12 inches. The average monthly rainfall for this station is heaviest in January (3.39 inches)

and is lightest in July (0.02 inches).

4-06 STORMS AND FLOODS

Precipitation in the Lopez Dam drainage area occurs both in the form of general winter storms associated with extratropical cyclones of North Pacific origin and convective type storms generally occurring during the summer. Estimates of probable maximum precipitation furnished by the U.S. Weather Bureau indicate that the highest rate of discharge from the drainage area for Lopez Dam would result from a 6-hour convective type storm.

Precipitation in the area is highly variable as shown by the monthly precipitation recorded at Pacoima Dam (Table 4-1). The variability in the precipitation can be illustrated by inspecting the statistical parameters presented in Table 4-2. A comparison of the highest on record values with the mean or median values shows the wide variation. The four quartile mean values and median values are shown graphically on Plate 4-1.

4-07 RUNOFF CHARACTERISTICS

The annual runoff from the Lopez Dam drainage area is also highly variable. A summary of the outflow from Pacoima Dam is shown in Table 4-3. The annual runoff is also shown on Plate 4-2 (1937-1958) and Plate 4-3 (1959-1976). Selected Pacoima Dam outflow hydrographs are plotted on Plate 4-4. A basin unit hydrograph has been developed and is shown on Plate 4-5.

4-08 WATER QUALITY

The watershed for Lopez Dam lies almost entirely in the unpopulated and undeveloped Angeles National Forest. No agricultural or industrial activity is present upstream from Lopez Reservoir. Under these conditions, water quality of inflow to Lopez Reservoir is expected to be high (low concentrations of heavy metals, pesticides and other pollutants).

In the past, high concentrations of suspended solids and nonvegetative debris from storm runoff and sluicing operations at upstream Pacoima Reservoir have resulted in a continual problem of sediment deposition in Lopez Reservoir. This has necessitated frequent clean-out operations to allow operation of the dam outlet and to ensure that adequate debris storage exists for a potential future flood.

TABLE 4-1

PRECIPITATION DATA RECORDED AT PACOIMA DAM*

(Value in inches)

Year	Oct.	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	TOTAL
1929	0.35	1.66	2.76	1.85	2.02	1.69	2.18	0	0.18	0	0	0.30	12.99
1930	0.16	0 .	6.37	1.04	5.60	0.37	1.72	0.15	0	T	0	0.08	15.49
1931	0.55	3.52	0	4.13	5.96	0.02	2.76	0.84	0.23	0	0.27	0.09	8.37
1932	0.24	2.61	7.36	2.21	9.47	0.22	1.26	0.36	0.36	0	0	0.07	24.16
1933	0.07	0	1.49	11.16	0.08	0.37	0.86	1.03	0.35	0	0.07	0	15.48
1934	0.59	0.19	6.63	4.31	3.33	0.04	0.03	0.01	0.96	0	T 0 16	0.33	16.42
1935	3.74	2.51	6.50	3.57 0.40	2.12 11.28	3.84 2.48	2.50 0.65	0.07 0.03	0 0.04	0.18 0.05	0.14	0.21	25.17 17.79
1936 1937	0.57 2.01	1.38 0.06	0.68 8.55	4.27	8.02	4.83	0.58	1.02	0.05	0.01	0.02	T	29.40
1938	0	0.03	5.01	1.95	10.09	12.84	1.96	0.35	0.30	0	0.06	0.06	32.65
1939	0.27	T	10.27	3.55	1.45	1.96	0.23	0.18	0	Ŏ	0	4.07	21.98
1940	0.20	0.23	0.82	6.97	6.18	1.43	2.17	0.04	0	0	0	0.06	18.13
1941	1.52	0.21	7.72	3.35	13.00	8.98	5.27	0.07	0.03	0.22	0.04	0	40.41
1942	2.00	0.15	5.89	0.36	0.98	1.25	3.04	0.03	0	0	0.79	0	14.49
1943	1.41	0.64	1.20	16.08	4.50	5.22	1.22	0.01	0.03	0	0	0	30.27
1944	0.32	0.28	8.12	1.39	11.80	4.16	1.05	0.18	0.59	0	0	0.09	27.98
1945	0	5.13	1.44	0.09	5.62	4.22	0.59	0.01	0.18	0.12	0.90	0	18.18
1946	1.41	0.36	6.38	0.24	1.38	5.77	0.83	0.37	0	0.12	0	0	16.86
1947	2.50	8.27	6.10	0.75	0.15	1.50	0.33	0.61	0.12	. 0	0.13	0.46	20.92
1948	0.12	0.02	1.70	0	1.78	3.36	1.83	0.17	0.48	0 0	0 T	0	9.46
1949	0.12	0	3.01	2.74	2.37	1.48	0.12	2.14	0.03	T	. 0	0.96	12.01 14.00
1950 1951	0.06 0.61	1.99 1.48	3.31 0.16	2.61 4.21	2.31 0.95	1.09 0.70	2.87	0.29 0.10	0.23 0	. 0	0.74	0.36	11.82
1952	0.99	2.16	7.38	13.10	0.54	9.12	3.06	0	0.01	Ŏ	0	0.11	36.47
1953	0	4.12	3.72	1.77	0.36	1.06	1.81	0.23	0.08	ō	0	0	13.15
1954	Ŏ	0.99	0.27	6.51	2.71	4.79	0.42	0.11	0.07	ŏ	Ö .	ŏ	15.87
1955	0	1.77	1.23	4.40	1.67	0.28	3.04	1.88	0.01	0	0.06	0	14.34
1956	. 0	1.77	2.38	7.19	0.84	0	4.45	1.12	T	0	0.01	0	17.76
1957	1.16	0	0.40	6.30	1.95	2.08	1.33	2.04	0.40	0	0 %	T	15.66
1958	2.25	0.72	4.48	2.53	7.54	6.13	6.06	0.08	0	0	0.03	0.69	30.56
1959	0.37	0.26	0	2.85	4.87	0	0.95	0.02	0.01	T	T	0.07	9.40
1960	0.03	T	1.36	3.10	2.52	1.01	1.62	T	0	0	0	T	9.64
1961	0	5.12	0.26	1.39	0.03	0.61	0.51	0.10	0	0.01	0.58	0.13	8.74
1962	0 26	2.07	2.39	4.01	14.51	1.45	0.12	0.41	T	0	. 0	0	24.96
1963 1964	0.26 0.99	0.03 3.19	T 0.06	0.83 3.09	4.10 T	3.04 3.02	3.00	0.07	0.67	0	T	1.11	13.11
1965	0.75	2.39	2.64	0.72	0.62	2.01	1.91 6.86	0.06 T	0.31	0.17	T 0.02	T 1.98	12.63 18.22
1966	0	15.66	4.68	1.00	1.38	0.35	0.00	0.33	0.00	T T	0.02	0.61	24.01
1967	0.15	3.93	7.91	6.98	0.17	4.89	5.73	0.65	T	Ö	0	1.58	31.99
1968	0	6.96	1.06	1.66	1.95	3.50	0.63	T	T	0	0.12	0.03	15.91
1969	0.50	0.38	1.56	16.55	9.45	1.49	1.45	0.02	0.25	0.12	0	T	31.77
1970	0	2.25	0.11	2.05	4.20	5.84	T	0.02	0.12	T	Ō	0	14.59
1971	0.03	6.75	6.76	1.53	0.94	1.06	0.88	1.38	0.05	0.02	0	0.15	19.55
1972	0.51	0.71	8.24	T	0.21	0	0.14	0.08	0.06	T	0.11	0.03	10.09
1973	1.60	2.39	2.08	4.07	12.70	4.02	0.08	0.01	0.09	0 .	T	T	27.04
1974	0.54	2.01	0.79	8.69	0	4.22	0.62	0.01	0.03	T	0 ,	0	16.91
1975	1.05	0.13	3.48	0.29	3.08	5.08	3.35	0.12	0.04	0	0	0.10	16.72
1976	0.62	0.01	0.78	0.00	4.25	1.92	1.54	0.24	0.21	0.01	0.34	4.33	
1977	2.97	0.58	0.67	4.96	0.13	2.15	-0	5.16	0.04	0		T	19.56
1978 1979	0.02 0.16	0.23 2.72	5.13	7.79	8.01	14.01	2.93	T	T	0	0	0.98	39.10
1980	1.20	0.70	1.83 0.55	8.20 7.41	4.05	5.74	0.03	0.34	0.16	0.04	0.07	0.02	23.32
1981	0	0.70	0.85	3.49	12.52 1.38	5.59 4.63	0.69 1.49	0.85 T	T 0	0.04	0	0	29.55 11.84
1982	0.59	2.72	0.28	2.70	1.00	6.59	2.74	0.06	0.21	0	0	1.32	18.21
1983	0.32	4.92	1.85	6.46	5.68	12.00	4.41	0.19	0.02	Ö -	4.12	2.10	42.07
			1000		•		: '- '-			- .		_,	
x	0.65	1.97	3.10	4.08	3.99	3.47	1.74	0.46	0.13	0.02	0.21	0.40	20.21
8	0.84	2.74	2.91	3.78	4.06	3.25	1.67	0.85	0.19	0.05	0.69	0.90	8.46
xy	0.40 2.69	1.15	2.26	2.99	2.80	2.53	1.26	0.22	0.07	0.01 4.09	0.06 4.81	0.16	18.64
		2.82	2,21	2.20	2.32	2.21	2.24	3.38	2.91			3.83	1.49

LACDPW STATION NO. 33A

x = normal mean s = normal standard deviation $x_y = log-normal mean$ $s_y = log-normal standard deviation$

TABLE 4-2

STATISTICAL PARAMETERS FOR PRECIPITATION DATA RECORDED AT PACOIMA DAM*

(Values in inches unless noted otherwise)

Parameter	8	Oct.	Nov.	Dec.	Jan.	Feb.	Mar.	Apr.	May	Jun.	Jul.	Aug.	Sep.	Total
Mean	o	0.65	1.97	3.10	4.08	3.99	3.47	1.74	94.0	0.13	0.02	0.21	0,40	20.21
Median	0	0.32	66.0	1.85	3.10	2.31	2.48	1.26	0.11	0.04	0	0 ,	0.03	17.79
First Quartile Mean		0.004	0.03	0.25	0.53	0.36	0.41	0.17	0.01	0	0	0	0	11.65
Second Quartile Mean		0.18	94.0	1.28	2.33	1.60	1.71	0.85	0.07	0.02	0	0	0.004	16.02
Third Quartile Mean		0.61	2.03	3.64	4.36	4.09	4.11	1.95	0.27	0.11	0	0.03	0.11	21.05
Fourth Quartile Mean	·	1.91	5.62	7.53	9.47	10.35	7.96	4.16	1.57	0.41	0.08	0.86	1.58	33.02
Highest on Record		3.74	15.66	10.27	16.55	14.51	14.01	6.86	5.16	96.0	0.22	4.12	4.33	42.07
Lowest on Record	ord 0			0	0	0	0	0	0	0	0	0	0	8.74
Percentage of Years with Measurable Precipitation		78.2	87.3	92.7	94.6	h•96	94.6	9.46	87.3	69.1	21.8	38.2	52.7	100.00

*LACDPW STATION NO. 33A

TABLE 4-3
PACOIMA DAM OUTFLOW*

Season	Annual Outflow (AF)	Instantaneo Peak Outflow (cfs)	us <u>Date of Peak</u>
1937-38 1938-39 1938-40 1940-41 1940-41 1941-42 1942-43 1943-44 1945-46 1945-46 1945-48 1946-47 1948-49 1950-51 1950-51 1953-54 1953-54 1955-56 1956-57 1958-59 1958-60 1961-62 1965-63 1965-63 1965-66 1965-68 1965-68 1965-68 1965-68 1965-68 1965-68 1965-71 1971-72 1971-72 1971-73 1971-73 1975-76 1976-77	26796 3080 3133 25942 2032 20407 15167 4911 2904 6029 335 740 1019 69 14325 3500 2941 737 1252 773 15808 708 271 11 6279 228 722 1041 15214 23600 3833 42998 2308 4994 802 7383 4154 2526 1614 507 not available	2062 66 169 430 97 598 326 397 241 237 8 10 231 634 163 292 39 66 47 420 242 4 0 511 24 117 564 197 105 2715 153 85 90 1540 460 83 66 470	Date of Peak 3-2 1-20 2-4 3-5 7-15 1-23 3-2 2-2 2-5 1-7 6-29 6-24 4-11 9-30 1-18 11-17 4-5 4-21 5-17 5-7 2-5 6-18 8-1 N/A 4-7 9-25 6-15 5-7 11-23 7-6 11-22 2-25 3-3 12-26 2-11 2-11 1-8 8-19 6-6 4-4
		ŭ	

N/A - Not Applicable

^{*} Outflow data provided by Los Angeles County Department of Public Works (LACDPW) for Pacoima Creek Flume below Pacoima Dam. LACDPW Station No. F/188B-R.

4-09 CHANNEL AND FLOODWAY CHARACTERISTICS

The basin channel upstream of Lopez Dam is natural with no channel improvements for flood control. Flood control channel improvements have been constructed on Pacoima Wash downstream of Lopez Dam as described in Section 3-04. The channel has been divided into three sections in terms of nomenclature:

Section	<u>Name</u>	Description
1	Pacoima Creek	Watershed up to Pacoima Dam
2	Pacoima Wash	Pacoima Dam to Pacoima Spreading Grounds
3	Pacoima Diversion	Pacoima Spreading Grounds to Tujunga Wash
		Channel

A profile of the channel from Tujunga Wash to the watershed crest is shown on Plate 4-6 (Sta. 0+00-Sta. 80+00) and Plate 4-7 (Sta. 80+00-Sta. 160+80).

4-10 UPSTREAM STRUCTURES

<u>Paccima Dam.</u> Paccima Dam is located on Paccima Creek approximately 1.5 miles upstream of the Lopez Dam site (see Plate 3-1). The concrete-arch structure, completed in 1929, is operated and maintained by the LACDPW for flood control and water conservation. Information pertaining to Paccima Dam and Reservoir is given in Exhibit A.

4-11 DOWNSTREAM STRUCTURES

- (a) <u>Hansen Dam</u>. Located along Tujunga Wash, 9 miles above its confluence with Los Angeles River (see Plate 3-1), Hansen Dam is a major flood-control facility owned, operated, and maintained by the LAD, as part of the LACDA flood-control project. Hansen Dam controls floods on the downstream portions of Los Angeles River, as well as on Tujunga Wash, immediately downstream of Hansen Dam. Pertinent data for Hansen Dam are included in Exhibit B.
- (b) Sepulveda Dam. Sepulveda Dam is a major flood control dam owned, operated and maintained by the LAD. It is located along Los Angeles River, 43 miles above the mouth of the river, and 6 miles above the confluence of Tujunga Wash and Los Angeles River. The dam is in the southcentral portion of the San Fernando Valley, just northwest of the junction of the Ventura Freeway (U.S. Highway 101) and the San Diego Freeway (Interstate Highway 405) (see Plate 3-1). Pertinent data for Sepulveda Dam are included in Exhibit B.

4-12 ECONOMIC DATA

a. <u>Population</u>. The watershed for Lopez Dam lies almost completely in the Angeles National Forest. This is an unpopulated and unincorporated part of Los Angeles County. The downstream area is located in the cities of San Fernando and Pacoima. The population estimates below are from the State of California, Department of Finance, Population Research Unit, and are of January, 1984:

Pacoima	74,662
San Fernando	18,966

- b. Agriculture. The downstream area was once primarily an agricultural and ranch area. The postwar era has brought increasing urbanization to the area which has virtually replaced all agriculture and ranching.
- c. <u>Industry</u>. The downstream area is heavily residential and supports general office and commercial development. There are a number of business/industrial parks. Most of the manufacturing is light industry.
- d. Flood Damages. Since completion of the project, flood damages prevented through fiscal year 1984 are estimated to be \$159,584,000. The reason this much damage has been prevented is because Lopez Dam serves as a debris collection basin. If this debris was allowed to flow downstream it would damage the channel lining thus causing greater flood damages for a particular event than if the channel lining was intact.

V - DATA COLLECTION AND COMMUNICATION NETWORKS

5-01 HYDROMETEOROLOGICAL STATIONS

a. <u>Facilities</u>. Plate 3-2 shows the precipitation, reservoir, and stream gages in the watershed above Lopez Dam, plus the stream gage on Pacoima Diversion Channel at Branford Street. These gages, along with their latitudes, longitudes, and elevations, are listed in Table 5-1.

There is one precipitation gage (Pacoima Dam) within the Lopez Dam watershed and it is a Los Angeles telemetry system gage. The water level in Pacoima Reservoir and at the Pacoima Outlet channel gage is also reported by the telemetry system. The closest downstream channel gage is the "Pacoima Diversion Channel at Branford Street" gage operated by the LACDPW. The Branford Street gage is not on the telemetry system. A rating table for the gage is shown on Plate 5-1.

b. Reporting. Telemetry gages report in real-time. Data from the gages are either recorded locally onto charts or tapes (recording type gages) or are observed daily or more frequently (non-recording gages).

Of those which report via telemetry, there are two types:

- (1) Interrogated Gages. The Los Angeles telemetry system gages, owned and operated by the COE are interrogated (usually once each hour or more frequently during storm conditions) by a radio located at the LAD Office in downtown Los Angeles. The data are compiled and processed by the LAD's Water Control Data System Computer for printout and for hydrologic forecasting.
- (2) Event-Reporting Gages. The telemetry gages, installed by the LACDPW in cooperation with National Weather Service (NWS) report via Very High Frequency (VHF) radio on an event basis. Example events reported include incremental changes in accumulated rainfall depths or predetermined incremental changes in streamflow or reservoir gage heights. The greater the intensity of rainfall or the change in gage height, the more frequent the gage reports.

These gages form a part of a network, labeled by the NWS, as the ALERT (Automatic Local Evaluation in Real-Time) system. The LAD monitors these gages, along with its own interrogative telemetry gages.

c. Maintenance. Each operating agency is responsible for the maintenance of its own gages and/or telemetry radio equipment. In many cases, the gage is owned by the USGS, and the telemetry attachments are owned by the LAD or LACDPW.

5-02 WATER QUALITY STATIONS

There are no water quality stations in the watershed above Lopez Dam.

TABLE 5-1

HYDROMETEOROLOGICAL STATIONS

Description	Recording and nonrecording precipitation, temperature, reservoir stage, and reservoir discharge. Operated by LACDPW. On LAD telemetry system.	Recording reservoir stage. Operated by LAD. Not on LAD telemetry system.	Recording streamflow. Operated by LACDPW. Not on LAD telemetry system.
Elevation	1650	1249	855
Longitude	118, 23, 51"	118, 24, 30"	118, 25' 13"
Latitude	34, 20, 05"	34, 18, 15"	34, 14, 07"
Name	Pacoima Dam	Lopez Dam	Pacoima Diversion Channel at Branford Street
Plate 3-2 Designation	#	#2	#3

5-03 SEDIMENT STATIONS

There are no sediment stations in the watershed above Lopez Dam. There are no sediment ranges in Lopez Reservoir.

5-04 RECORDING HYDROLOGIC DATA

Each agency maintains records of its own data. The NWS data are placed in archives at the National Climatic Center in Asheville, North Carolina. Precipitation and other data are published monthly by the National Climatic Center in Climatological Data and Hourly Precipitation Data.

The State of California, Department of Water Resources, publishes the data from the ALERT telemetry gage network on a monthly basis. The LACDPW maintains their recording and non-recording data bases and furnish data to other agencies upon request. The LAD maintains a data base from its recording and telemetry gages and provides selected data to the NWS for publication. Real Time Reports received from the ALERT gages and the Los Angeles Telemetry System gages are stored in a database on the Water Control Data System Computer.

5-05 COMMUNICATION NETWORK

The LAD maintains a voice radio communication network for its entire operations activities. This routinely includes communications between the District Office and the various dam tenders, as well as vehicles in the field.

During periods of significant runoff, communications to the dam tenders becomes vital. The existing radio network, which has proven itself reliable, is backed up by a second radio network; and both of these are backed up by the local telephone system.

Power at the District Office is backed up by an emergency generator system; and if all fails at the District Office, there is a complete radio system at the LAD Base Yard. The Base Yard is located a few miles east of the District Office.

5-06 COMMUNICATION WITH PROJECT

a. Regulating Office with Project Office. During the flood season (15 November through 15 April), a routine radio call is made at least once each weekday from the LAD Office to the dam tender for Hansen Dam. The Hansen Dam operator is also the operator for Lopez Dam. This "Morning Report" is usually made at 0800 hours, Monday through Friday. Other routine or non-routine radio or telephone calls are made as needed. There are no telephone or electrical services at Lopez Dam. Direct communication with the operator while he is at Lopez Dam is possible by calling his Mobile Radio (WUK 4121).

In the event that all communications with the COE District Office, including the COE Base Yard, should be interrupted, a set of "Standing Instructions to Dam Tender" have been compiled for Lopez Dam and a copy of these instructions are included in this manual. The COE organization chart and important phone numbers for reservoir operations decisions at Lopez Dam are given in Plate 5-2.

- b. Between Project Offices and Others. No routine communication exists between Lopez Dam and other agencies.
- c. Between Regulating Office and Others. Before and during the earliest stages of any reservoir impoundment, the COE notifies offices of other agencies and selected private interests of the impending rises in the reservoir water surface elevation and corresponding outflow. A list of agencies to notify, with applicable office and home telephone numbers, is published annually in the LAD's <u>Instructions for Reservoir Operations Center Personnel</u> (the so-called "Orange Book"). During major runoff events, the LAD Reservoir Operations Center is in constant contact with the LACDPW Hydraulics Branch to fully coordinate the operations of both agencies. The LACDPW is directly tied into the LAD radio and telephone system. The LAD Reservoir Operations Center is also in direct radio contact with channel observor's dispatched to patrol the Los Angeles River during large floods.

5-07 PROJECT REPORTING INSTRUCTIONS

During periods of water operations, communications between the LAD Office and each affected dam tender are made on a frequent basis. Normal communications occur once each hour and more frequent communications are sometimes required. If a gate change is required, the Reservoir Operations Center (ROC) staff provide the radio operator at the LAD Office with the gate change instructions. These instructions are then broadcast to the dam tender. When the gate change is completed, the dam tender calls back to the District radio operator with information on the change. The radio operator then informs the ROC engineer who initiated the change. The dam tender records pertinent information associated with the gate change on the form shown on Figure 9-1. This report form is subsequently submitted to the LAD Office.

Other special instructions to dam tenders are conducted in a similar manner. This network of radio communications is also used by the dam tender to report any failure of machinery or other equipment or any other unusual conditions at the dam.

5-08 WARNINGS

The responsibility for issuing all weather watches and warnings and all flood and flash flood watches and warnings rests with the NWS. Local emergency officials of cities and counties are responsible for issuing any other public warnings including unusual overflows, evacuations, unsafe roads or bridges, and toxic spills. The COE is responsible for providing these officials with up-to-date information, and forecasts where possible, of water rises within Lopez Reservoir and release rates into the channel downstream of Lopez Dam. The LAD ROC should notify the Los Angeles Police Department (Foothill Division) to initiate evacuation if a dam break is imminent.

VI - HYDROLOGIC FORECASTS

6-01 GENERAL

a. Role of Corps of Engineers. The LAD does not make any formal hydrologic forecasts, published or unpublished, for Lopez Dam. Despite the lack of formal hydrologic forecasts, the LAD does carefully monitor the reservoir water surface elevation in Lopez Reservoir, and does notify other agencies of any significant changes or anticipated changes as described in Section 5-06.

The LAD continues to improve its monitoring capabilities of conditions not only at Lopez Dam, but in adjacent watersheds. Improved and increased numbers of automatic telemetry rain and stream gages help in this manner not only directly, but also in the development of computerized rainfall-runoff forecast models. The long-term goal of the LAD is to be able to provide relatively accurate predictions of inflows and reservoir surface elevations as far in advance as possible. It is intended that these predictions will become accurate and reliable enough that they can be shared with the NWS, the LACDPW, city and county emergency officials, and others to be used as a basis for reservoir systems operations during the upcoming years.

The LAD Meteorologist prepares special quantitative precipitation forecasts for Los Angeles River drainages and other watersheds including the Lopez Dam watershed. These are used in determining the potential for significant runoff into Lopez and other reservoirs. Research is progressing into the direct incorporation of these quantitative precipitation forecasts into the rainfall-runoff forecast models being developed.

b. Role of Other Agencies. No agency has any specific forecast responsibility for water surface elevations in Lopez Reservoir or for discharges on Pacoima Wash, either upstream or downstream of Lopez Dam. The NWS issues Flash Flood Warnings for rivers and other watercourses in the San Fernando Valley.

The LAD does receive real-time weather reports and forecasts, as well as historical weather data, from the NWS. This is accomplished by means of weather facsimile pictures and teletype data and forecasts transmitted by the NWS and received by a LAD facsimile recorder and teletype printer. Close coordination is maintained with the NWS forecast office located in Los Angeles.

Historical precipitation and streamflow data are available from the LACDPW. These data, while not of use in real-time, are important to studies of historical storms and floods which aid in the development and refinement of computerized rainfall-runoff forecast models.

6-02 FLOOD CONDITIONS FORECASTS

Forecasts of flood hydrographs are currently not made. However, routine evaluation of precipitation, resulting inflow, and forecast precipitation provides valuable subjective predictions of flood situations. Using such information, the LAD Reservoir Operations Center can evaluate if an ongoing flood will increase or decrease over the next 24 hours.

6-03 CONSERVATION PURPOSE FORECASTS

Since Lopez Dam is strictly a flood control facility, forecasts for other purposes including water conservation are not made.

6-04 LONG-RANGE FORECASTS

Since the watershed above Lopez Dam is relatively small (34 square miles), and since water is impounded behind Lopez Dam for short time periods, there is little direct need for long-range forecasts in the operation of Lopez Dam. Only in the event of major impoundment at Lopez Reservoir, as well as simultaneously at other reservoirs affecting the downstream channel and Los Angeles River, would a forecast of more than one day be of immediate significance to the operation of Lopez Dam. In such a case, the forecast of another impending major storm or lack of such storm might influence the release rate of water from Lopez Dam. The primary consideration of the release rates from all of the dams in the Los Angeles River system is to prevent or minimize downstream damages.

VII - WATER CONTROL PLAN

7-01 GENERAL OBJECTIVES

Lopez Dam and Reservoir are integral flood control facilities in the Pacoima-Tujunga Wash system of tributaries to the Los Angeles River. The purpose of the dam, a unit under the approved comprehensive plan for flood control in the LACDA, is to provide protection against debris-laden floodwaters for large areas between the dam site and Los Angeles River. Important improvements in these areas include valuable industrial, business, and residential properties and transportation systems. A secondary use for the dam is that it also forms a headworks to direct flows into the Pacoima Wash Channel and/or the Lopez Spreading Grounds operated by the LACDPW.

7-02 MAJOR CONSTRAINTS

Several physical constraints at Lopez Dam result in regulation limitations, including:

- a. Seepage occurs along the downstream toe of the dam when the reservoir level is at the spillway crest or above. The seepage has not been detrimental to the integrity of the dam. However, with pool elevations exceeding a 12-foot spillway surcharge, some instability may develop. Prolonged reservoir storage should be limited and COE personnel should monitor the dam whenever the reservoir contains water, especially at water levels above the spillway crest.
- b. The capacity of the reservoir outlet conduit is relatively small compared to the standard project flood (SPF) discharge of 11,200 cfs and the probable maximum flood (PMF) discharge of 30,400 cfs. The maximum discharge capacity of the outlet for the water level at the spillway crest is 422 cfs.
- c. The reservoir outlet discharges directly into the spillway channel which creates turbulence and affects the spillway flow. The outlet gate should be closed during flow over the spillway to eliminate spillway flow interference.
- d. Greater amounts of non-vegetative debris accumulate in the reservoir than was originally anticipated. The original design was based on retaining a 50-year accumulation of debris plus the debris produced by a major flood. The debris allowance was determined to be 794 acre-feet. This storage was completely filled with debris by February, 1969, only 15 years after completion of the dam. The greater amount of debris production results in decreased flood control storage and debris removal on a more frequent basis is required.
- e. Vegetative debris can plug the low level inlet to the reservoir drain under certain conditions. No structural improvements have been incorporated into the project for the specific purpose of controlling vegetative debris. Reservoir ponding will not occur while the outlet gate is at the full-open position until the reservoir content reaches approximately 45 acre-feet and the inflow exceeds 144 cfs.

f. Long term reservoir impoundment could be a safety hazard due to the fact that the reservoir is located in a residential area. There is a potential for children drowning, as well as several other safety concerns.

7-03 OVERALL PLAN FOR WATER CONTROL

Due to the very limited flood control capability of Lopez Dam, the overall plan for water control at Lopez Dam has been formulated to address the following objectives and criteria: (a) minimization of sedimentation; (b) minimization of pool depths to reduce seepage through the embankment; (c) maintenance of a debris pool to avoid clogging the inlet trash rack on the outlet works with vegetative and miscellaneous debris; (d) gate closure during spillway flow to avoid outlet flow disturbance of flow in the spillway channel; and (e) ease of operation while providing for flexibility.

The objectives of the overall water control plan can be achieved by operating the reservoir outlet as if it were uncontrolled during all conditions except during spillway flow. For all inflow conditions when the water surface is below the spillway crest, the outlet gate is to be full-open. The outlet gate should be closed at all times when spillway flow is occurring. After spillway flow has ceased, the outlet gate is to be opened to the full-open position to allow draining of the pool.

7-04 STANDING INSTRUCTIONS TO DAM TENDER

The standing instructions to the dam tender for the regulation of Lopez Dam and Reservoir are given in Table 7-1. At all times when there is no spillway flow (water surface elevation less than 1,273 ft.-NGVD), the reservoir outlet gate should be set at the full-open position of 5 feet. At all times during spillway flow, the gate should be closed to avoid interference with spillway flow. After spillway flow has ceased, and unless instructed otherwise, the gate opening should be reset to the full-open position.

The dam operator should follow the communication guidelines for normal and emergency operating conditions given below:

- a. Communication with the District Office is available:
- 1. Notify the Reservoir Operations Center when a gate change will be required according the the schedule given in Table 7-1.
 - 2. Notify the ROC if unable to set the gates as instructed.
 - b. Communication with the District Office is not available:
 - 1. Try to reestablish communication through the LACDPW.
- 2. Allow a period of one-half hour to pass to reestablish communication with the District Office. If after one-half hour communication is not reestablished, follow the gate operation schedule given in Table 7-1.

TABLE 7-1

LOPEZ DAM RESERVOIR REGULATION SCHEDULE

Step No.	When Reservoir Water Surface is Between Elevations (feet - NGVD)	Gate Setting (feet of opening)	Computed Outlet Discharge (cfs)	Spillway Discharge (cfs)
1	1,254-1,273	5.0	0-422	0
2	1,273-1,299	0	0	0-44,500

NOTES: Spillway Crest Elevation 1,273 ft, NGVD.
Top of Dam Elevation 1,299 ft, NGVD.

Follow Step 1 at all times when there is no spillway flow

Follow Step 2 at all times during spillway flow

DAM OPERATOR INSTRUCTIONS

- 1. Communication with the District Office is available.
 - a. Notify the Reservoir Operations Center when a gate change will be required according to the schedule.
 - b. Notify the Reservoir Operations Center if unable to set the gate as instructed.
- 2. Communication with the District Office is not available.
 - a. Try to reestablish communication through the Los Angeles County Department of Public Works.
 - b. Allow a period of one half hour to pass to reestablish communication with the District Office. If after one half hour communication is not reestablished, follow the gate operation schedule.

7-05 FLOOD CONTROL

Because of the limited storage capacity of Lopez Dam and the large channel capacity downstream of the dam, the flood control provided by the dam is due to the configuration of the dam and channel rather than to the operation of the outlet gate. The dam acts to divert flows coming from the unimproved Pacoima Wash upstream into the concrete lined channel downstream. capacity of the downstream channel is about the same as the SPF Inflow Peak for Lopez Dam. Therefore, the water control plan for Lopez Dam has been developed considering the supplemental objectives and criteria discussed in Section 7-03 and the constraints listed in Section 7-02. The maximum operational benefits will be obtained by following the reservoir regulation schedule given in Table 7-1. The basic water control plan specifies that the reservoir outlet gate remain in the full-open position at all times unless spillway flow is occurring. The gate is to be closed during spillway flow to insure optimal flow conditions in the spillway channel. After spillway flow has ceased, and as a general rule, the reservoir outlet is to be opened again to the full-open position to allow draining of the pool. The reservoir pool drawdown from the spillway crest to the outlet invert, assuming no inflow into the reservoir, and the gate in the full-open position, is approximately 30 hours. It may be desirable to delay the drawdown time during certain conditions. The drawdown times for select gate openings are provided in table 7-2.

Maximum open channel flow of the outlet conduit is 144 cfs. Based on available information, this is probably less than the debris carrying discharge. Therefore, there is no requirement to close the outlet gate at the start of inflow in order to build a debris pool, because once inflow becomes greater than 144 cfs and debris starts to form, reservoir impoundment will begin, and a debris pool will build up.

7-06 RECREATION

There are no existing recreational facilities or activities associated with Lopez Dam and Reservoir.

7-07 WATER QUALITY

Since flood control operation of Lopez Dam has limited effect on water quality, no special provisions for water quality control are included in normal regulation. At various times in the past, LAD has cooperated with LACDPW in modifying its gate operating schedule to trap sediment being sluiced from upstream Pacoima Reservoir behind Lopez Dam. A 1959 agreement allowed the LACDPW to operate the outlet for purposes of lowering the concentration of suspended solids in water being delivered to downstream spreading grounds. This agreement expired in 1964.

7-08 FISH AND WILDLIFE

The Lopez Reservoir encompasses approximately 80 acres of vacant land. No vegetation exists within the reservoir basin. The margins of the basin have a scattered covering of tree tobacco (Nicotiana glauca) and broom baccharis

TABLE 7-2
LOPEZ DAM POOL DRAWDOWN

Gate Opening (feet)	•	·	Drawdown Time* (hours)
0.5			332.0
1.0			113.5
1.5			66.5
2.0			49.0
2.5			42.5
3.0			38.0
. 3.5			35.0
4.0			32.5
4.5	•		31.0
5.0			29.5

^{*}Assuming reservoir inflow equal to zero and pool initially full at spillway crest.

(Baccharis sarothroides), which intergrades into the California coastal scrub of the surrounding hills.

The basin does not provide suitable habitat for wildlife. The lack of cover and intermittent flooding make the area unsuitable for most species. During periods in which the reservoir is holding water the basin provides marginal habitat for wading birds and migrating water fowl.

7-09 WATER SUPPLY

The Lopez Dam water control plan does not provide for regulation for water supply. In some instances LAD has coordinated regulation of Lopez Dam with LACDPW's water supply operations of upstream Pacoima Dam and downstream spreading grounds.

Currently no formal agreement exists with LACDPW with respect to the operation of Lopez Dam. However, from 1959 to 1964, LACDPW operated the gated outlet under license from LAD in order to maintain a small debris pool behind Lopez Dam. Suspended solids in water conservation releases from Pacoima Dam settled out in this pool resulting in higher quality water released to downstream spreading grounds. The agreement required LACDPW to remove accumulated sediments resulting from their operations. Because LACDPW felt the cost of sediment removal exceeded the water supply benefits, LACDPW allowed the license to expire without renewal in 1964.

LAD has also periodically cooperated with LACDPW in allowing closure of the gated outlet at Lopez Dam and trapping sediment sluiced from Pacoima Reservoir in Lopez Reservoir. This operation facilitates removal of sediment from Pacoima Reservoir and has been allowed on the condition LACDPW remove the debris deposited in Lopez Reservoir.

7-10 DEVIATION FROM NORMAL REGULATION

There may be instances when it is necessary for the operation of Lopez Dam to deviate from the established flood control plan. Prior approval of deviations is required from District Engineer, LAD, except for emergencies and minor deviations as discussed in subparagraphs a and b, below.

- a. <u>Emergencies</u>. Some emergencies that can be expected are: drownings and other accidents, and failure of operation facilities. Necessary action under emergency conditions should be taken immediately unless such action would create equal or worse conditions. The Reservoir Regulation Unit, LAD, is to be informed of any deviations as soon as practical.
- b. Unplanned Minor Deviations. There are unplanned instances that create a temporary need for minor deviations from the normal regulation of the reservoir, although they are not considered emergencies. Construction activities account for the major portion of such incidents and often include utility stream crossings, facility maintenance, bank protection work, and channel maintenance and major construction contracts. Changes in releases are sometimes necessary for maintenance and inspection. Requests for changes of release rates are generally for a few hours to a few days. Each request is

analyzed on its own merits. Consideration is given to upstream watershed conditions, potential flood threat, conditions of reservoirs, and possible alternative measures. In the interest of maintaining good public relations, the requests are usually complied with, providing there are no foreseen adverse effects on the overall operation of the project for the authorized purposes. Approval for these minor deviations will normally be obtained from the Reservoir Regulation Unit, LAD, by telephone with subsequent written confirmation.

c. <u>Planned Deviations</u>. These are planned instances, which require deviations from the normal regulation. Each condition is to be analyzed on its own merits. Sufficient data on flood potential, reservoir and watershed conditions, possible alternative measures, benefits to be expected, and probable effects on other projects and useful purposes will be presented by letter or telephone to Reservoir Regulation Unit, LAD, along with recommendations for approval.

7-11 WATER CONTROL PLANNING TOOLS

Specific planning tools have been utilized in the development of the flood control plan. These tools are also used to evaluate and set operation rules for planned deviations and also to facilitate operation of the dam during emergencies and unplanned minor deviations. Water control planning tools used for Lopez Dam and Reservoir include:

- a. Pool Drawdown Table (Table 7-2),
- b. Outlet Rating Curve and Table (Plate 7-1 and Table 7-3),
- c. Spillway Discharge Curve and Table (Plate 7-2 and Table 7-4), and
- d. Area-Capacity Curves and Tables (Plate 7-3 and Table 7-5).

Table 7-3. Lopez Dam Outlet Discharge in Cubic Feet Per Second

ELEVATION				GATE O	PENING I	N FEET				·
	0.5	1.0	1.5	2.0	2.5	3.0	3.5	4.0	4.5	5.0
1254	0	0	0	0	0	0	0	0	0	0
1255	6	10	10	10	10	10	10	10	10	10
1256	. 9	21	30	30	30	30	30	30	30	30
1257	11	27	49	62	62	. 62	62	62	62	62
1258	12	32	60	82	99	99	99	99	99	99
1259	13	36	67	92	116	130	144	144	144	144
1260	14	40	73	101	127	147	162	170	175	186
1261	14	44	80	109	138	160	178	191	206	232
1262	15	47	85	117	147	171	194	211	233	266
1263	16	49	90	124	155	182	208	230	256	288
1264	17	52	94	131	164	192	221	248	276	306
1265	18	54	99	138	171	203	233	264	293	321
1266	18	55	102	144	180	213	246	279	308	336
1267	19	57	106	149	188	223	258	292	322	350
1268	20	58	110	155	195	232	269	304	334	363
1269	20	60	113	160	202	241	280	315	346	376
1270	21	61	116	165	210	250	290	326	358	388
1271 .	21	62	119	170	217	258	300	337	368	400
1272	22	64	122	175	224	267	309.	347	379	411
1273*	22	66	124	180	231	275	319	356	388	422
1274	23	67	127	185	237	283	327	365	400	433
1275	23	68	130	189	243	291	335	373	410	444
1276	24	70	132	194	249	299	344	381	420	454
127	24	71	135	198	255	306	352	389	429	464
1278	25	72	138	202	261	313	360	398	438	474
1279	26	74	140	206	266	320	367	406	448	484
1280	26	76	143	210	272	326	374	414	456	493
1281	27	78	146	214	277	333	381	422	464	502
1282	27	79	148	218	282	339	388	430	472	510
1283	27	81	151	222	287	344	394	438	480	519
1284	28	82	153	225	292	350	401	446	487	527
1285	. 28	84	156	228	297	356	406	453	495	535
1286	28	86	158	232	302	361	412	460	502	543
128	29	87	160	235	306	365	418	467	510	551
1288	29	88	163	238	310	370	425	474	518	559
1289	29	89	166	241	314	375	431	480	525	567
1290	30	90	168	244	319	380	437	486	532	574
1291	30	91	170	247	323	386	443	492	539	582
1292	30	92	172	250	326	391	449	499	547	589
1293	30	93	175	253	330	396	455	506	554	596
1294	3.1	94	178	256	334	401	461	512	561	603
1295	31	- 95	180	259	338	406	467	518	568	610
1296	31	96	182	262	341	411	473	525	575	617
1297	31	96	184	265	344	416	479	532	582	624
1298	32	97	186	267	346	420	484	538	589	631
1299**	32	98	187	270	348	424	490	545	596	638
	~	-	-	•	_		-			J .

Note: Elevation given is water surface elevation in feet above NGVD.

^{*}Spillway Crest

^{**}Top of Dam

Table 7-4
LOPEZ DAM SPILLWAY DISCHARGE

Elevation	Discharge (cfs)	<u>Elevation</u>	Discharge (cfs)
1273.0	0	1286.5	16,688
1273.5	119	1287.0	17,603
1274.0	336	1287.5	18,554
1274.5	617	1288.0	19,522
1275.0	950	1288.5	20,506
1275.5	1,328	1289.0	21,507
1276.0	1,746	1289.5	22,523
1276.5	2,200	1290.0	23,554
1277.0	2,688	1290.5	24,601
1277.5	3,208	1291.0	25,663
1278.0	3 ,7 57	1291.5	26,739
1278.5	4,334	1292.0	27,830
1279.0	4,939	1292.5	28,936
1279.5	5,569	1293.0	30 , 056
1280.0	6,224	1293.5	31,190
1280.5	6,902	1294.0	32,338
1281.0	7,604	1294.5	33,500
1281.5	8,328	1295.0	34,676
1282.0	9,073	1295.5	35,864
1282.5	9,840	1296.0	37,067
1283.0	10,626	1296.5	38,282
1283.5	11,433	1297.0	39,510
1284.0	12,260	1297.5	40,751
1284.5	13, 105	1298.0	42,005
1285.0	13,969	1298.5	43,271
1285.5	14,851	*1299.0	44,550
1286.0	15,751		•

Note: Elevation given is water surface elevation in feet above NGVD.

^{*}Top of Dam

TABLE 7-5 LOPEZ DAM RESERVOIR AREA AND CAPACITY

Elevation	Capacity (acre-feet)	Area (acres)	Elevation	Capacity (acre-feet)	Area (acres)
1254	`O	0	1277	622.7	48.4
1255	2.1	3.7	1278	671.8	49.7
1256	7.7	7.6	1279	722.1	51.1
1257	17.2	11.3	1280	774.1	52.8
1258	29.9	14.0	1281	827.8	54.6
1259	45.2	16.4	1282	883.3	56.5
1260	62.9	18.9	1283	940.7	58.2
1261	83.0	21.2	1284	999.6	59.6
1262	104.9	22.7	1285	1059.8	60.8
1263	128.4	24.1	1286	1121.2	61.9
1264	153.2	25.5	1287	1183.7	63.1
1265	179.3	26.8	1288	1247.3	64.1
1266	206.8	28.1	 1289	1311.9	65.1
1267	235.9	29.6	1290	1377.5	66.0
1268	265.9	31.1	1291	1444.0	67.0
1269	297.9	32.8	1292	1511.4	67.9
1270	331.6	34.7	1293	1579•9	69.2
1271	367.3	36.6	1294	1649.8	70.7
1272	404.8	38.4	1295	1721.4	72.4
* 1273	441.1	40.4	1296	1794.7	74.3
1274	485.6	42.6	1297	1870.0	76.4
1275	529.4	44.9	1298	1947.6	79.0
1276	575.2	46.6	** 1299	2027.9	80.8

Notes: (1) Table based on survey dated March, 1979.
(2) Elevation given is water surface elevation in feet above NGVD.

^{*}Spillway Crest
**Top of Dam

VIII - EFFECT OF WATER CONTROL PLAN

8-01 GENERAL

The operation of Lopez Dam has resulted in a reduction in the magnitude of flows along the lower portion of Pacoima Wash. The dam has successfully provided a headworks to direct flows into the downstream flood control channel. Indirect benefits have also resulted from operations by controlling flows for conservation diversions into Lopez Spreading Grounds and the downstream channel.

8-02 FLOOD CONTROL

- a. Spillway Design Flood. The spillway of a dam must be designed in order to pass, without danger to the structure of the dam or threat of overtopping the dam, the greatest rate of discharge that could be expected from the most severe combination of rainfall and runoff conditions that could reasonably occur. This hypothetical flood is called the PMF.
- (1) Original Criteria. The spillway at Lopez Dam was designed for a peak outflow of 32,000 cfs, with 21 feet of surcharge. An additional 5 feet of freeboard to handle runup by waves from the water surface was incorporated, which set the top of the dam at elevation 1,298.9 feet.

The original spillway design flood resulted from a 24-hour stable orographic-type storm. The average precipitation depth over the drainage area for the probable maximum storm was 21.5 inches with an effective total precipitation of 17.9 inches. The probable maximum flood (PMF) peak inflow to Lopez Dam was 32,000 cfs. The total 2-day flood volume, including base flow, was 37,800 acre-feet. The maximum water surface elevation obtained during the PMF routing was 1293.9 feet, approximately 5 feet below the top of the dam.

(2) Revised Criteria. In a subsequent 1978 study, the adequacy of the Lopez Dam spillway was reviewed under modern criteria. This led to the development of a revised PMF resulting from a probable maximum storm based on a 6-hour convective-type storm. The average depth of precipitation for 1/2, 1, 3, and 6 hours during the probable maximum storm for the drainage area above Lopez Dam were 4.1, 5.9, 8.7, and 11.3 inches, respectively. The total flood volume, including base flow, was 19,900 acre-feet.

The revised PMF (peak inflow of 30,400 cfs) was routed, assuming that the outlet conduit was plugged and that the debris storage pool was filled. The peak outflow would be 30,200 cfs and the maximum pool elevation would be 1292.8 feet. This provides 6.1 feet of freeboard which exceeds the required freeboard of 5 feet. The inflow, outflow, and water surface elevation hydrographs for the revised spillway design routing are shown in Plate 8-1.

b. Standard Project Flood. The SPF represents the runoff event that would result from the most severe combination of rainfall and watershed conditions that are considered reasonably characteristic for the region in

question. The COE generally applies the SPF as the criteria for protecting urban areas. Thus, since approximately 1952, the SPF has been used as the Reservoir Design Flood for the construction of new dams.

For the rainfall to be used in the determination of the SPF at a given site, a Standard Project Storm is normally selected as the most severe reasonably characteristic storm of record within a climatically homogeneous region surrounding the site, and is then transposed to the drainage area above the target site.

The SPF has been routed through the reservoir assuming that the reservoir outlet was closed and that the debris storage of 794 acre-feet was full. The SPF was based on a general type storm having a duration of 48 hours. The average precipitation depth over the drainage area is 21.2 inches with an effective total precipitation of 7.8 inches. The SPF peak inflow and outflow are both 11,200 cfs. The total flood volume, excluding base flow, is 14,100 acre-feet. The maximum water surface elevation obtained during the SPF routing is 1,283.2 feet, approximately 16 feet below the top of the dam. Plate 8-2 shows the inflow, outflow, and water surface elevation hydrographs for the SPF routing.

8-03 WATER QUALITY

At most times during most years, inflows are not detained in the reservoir, but pass immediately downstream, with little change in water quality. Impoundment of water occurs in the winter storm season for debris control purposes. These impoundments are of such short duration that little adverse effect on water quality will occur. In fact, whatever effect would probably be beneficial as suspended solids and debris would settle out and result in higher quality releases downstream. In most instances, water flowing from Lopez Reservoir should meet Federal and state water quality standards and be suitable for the identified beneficial uses, primarily groundwater recharge.

8-04 FISH AND WILDLIFE

Any impacts on biological resources resulting from the new flood control operation plan are expected to be minor. No vegetation exists within the reservoir basin. The margins of the basin have a scattered covering of tree tobacco (Nicotiana glauca) and broom baccharis (Baccharis Sarothroides). The existing vegetation either tolerates inundation well or can quickly reestablish itself after inundation. Prolonged inundation does not occur within the reservoir basin.

The basin does not provide suitable habitat for wildlife. The lack of cover and intermittent flooding makes the area unsuitable for most species. There are no State or Federally listed threatened or endangered species within the general vicinity of the reservoir.

8-05 FREQUENCIES

a. <u>Peak Inflow and Outflow Probabilities</u>. Plate 8-3 is a graph of the inflow and outflow frequencies at Lopez Dam, computed from a December 1984 LACDA review study. The values from which these curves were derived are

listed in Table 8-1. The inflow curve is affected by the Water Control Plan for Pacoima Dam, as discussed in Section 4-07. The inflow and outflow frequencies at Pacoima Dam are presented on Plate 8-4. Table 8-2 contains the values from which the curves on Plate 8-3 were derived. The inflow curve, is of course not affected by the Water Control Plan for Lopez Dam, which has bearing only upon regulation of the outflow and consequently the impoundment of water behind the dam.

The outflow curve of Plate 8-3, on the other hand, does reflect the Lopez Dam Water Control Plan. The sharp break in the slope of the curve at water surface elevation 1,273 feet reflects the fact that the water surface elevation in Lopez Dam has reached the spillway crest 1,273. The outflow rate increases rapidly for any additional rise in the reservoir water surface above elevation 1,273 feet.

b. Pool Elevation Duration and Frequency. Plate 8-5 is the computed filling frequency curve for Lopez Dam. Plate 8-6 shows a similar curve for Pacoima Dam. The curves on Plate 8-5 and Plate 8-6 are based upon, and have been adjusted for, 1984 conditions. These conditions include percent of impervious cover in the drainage area above Lopez and Pacoima Reservoirs, runoff routing conditions, and the gate operation schedule of the Water Control Plan for Lopez and Pacoima Dams. The values from which the curves of Plates 8-5 and 8-6 were constructed are listed in Tables 8-1 and 8-2, respectively. As with the outflow frequency curve (Plate 8-3), the relatively sharp change in slope of the filling frequency curve for Lopez Dam (Plate 8-5) reflects the fact that the outflow rate increases rapidly as the reservoir water surface elevation in Lopez Dam rises above elevation 1,273 feet, therefore, the rate of additional impoundment of water within the reservoir is reduced for a given increase in inflow.

8-06 OTHER STUDIES

- a. Discharge-frequency values presented in this manual were derived from ongoing (1984) investigations in the COE LACDA study. The "Interim Report on Hydrology and Hydraulic Review of Design Features of Existing Dams for Los Angeles County Drainage Area Dams," dated June 1978, presents the derivation of the Probable Maximum and Standard Project Floods used in this manual.
- b. No flood plain management studies addressing the downstream channel have been conducted by the COE since the downstream channel was constructed. Several flood insurance studies have been completed to date by the COE and LACDPW for the Federal Emergency Management Agency. These studies show no downstream flood problem. Currently (1984) the COE is conducting an ongoing review study of the entire LACDA system in order to reassess the adequacy of flood protection provided by the downstream channels.

TABLE 8-1

INFLOW, OUTFLOW, AND FILLING FREQUENCY VALUES, LOPEZ RESERVOIR

RETURN PERIOD (Years):	2	5	10	25	50	100	200	500
INFLOW (cfs)	156	323	616	1,130	2,720	2,990	3,390	4,630
OUTFLOW (cfs)	70	115	233	296	2,710	2,990	3,380	4,580
FILLING (max elevation, feet above NGVD)	1,257	1,258	1,261	1,264	1,276	1,277	1,277	1,279

NOTES:

- 1. These preliminary data values, which represent 1984 conditions, were obtained from a December 1984 LACDA review study performed by the Hydrologic Engineering Section of the U.S. Army Corps of Engineers, Los Angeles District.
- 2. Inflow and outflow frequency curves, drawn from the data values listed in this table, appear on Plate 8-3.
- 3. A filling frequency curve, drawn from the data values listed in this table, appears on Plate 8-5.

TABLE 8-2

INFLOW, OUTFLOW, AND FILLING FREQUENCY VALUES, PACOIMA RESERVOIR

RETURN PERIOD (Years):	2	5	10	25	50	100	200	500
INFLOW (cfs)	204	843	1,600	2,740	7,110	8,920	10,600	12,500
OUTFLOW (cfs)	200	405	799	1,370	2,720	2,990	3,390	4,630
FILLING (max elevation, feet above NGVD)	1,900	1,905	1,910	1,920	1,959	1,968	1,981	1,995

NOTES:

- 1. These preliminary data values, which represent 1984 conditions, were obtained from a December 1984 LACDA review study performed by the Hydrologic Engineering Section of the U.S. Army Corps of Engineers, Los Angeles District.
- 2. Inflow and outflow frequency curves, drawn from the data values listed in this table, appear on Plate 8-4.
- 3. A filling frequency curve, drawn from the data values listed in this table, appears on Plate 8-6.

IX - WATER CONTROL MANAGEMENT

9-01 RESPONSIBILITIES AND ORGANIZATION

a. Corps of Engineers. Lopez Dam is owned, operated, and maintained by the LAD, which has complete regulatory responsibility for the dam and the reservoir.

Reservoir operations at Lopez Dam and other COE facilities in the LAD are conducted by the Reservoir Regulation Unit of the Hydrologic Engineering Section. Plate 5-2 shows an organizational chart depicting the chain of command for Reservoir Regulation.

Gate regulation instructions to the dam tender are issued by the Reservoir Regulation Unit (see Sections 5-06 and 5-07). In the event that communications between the Reservoir Regulation Unit and Lopez Dam are interrupted, a set of Standing Instructions to Dam Tender are included in Section 7-04. Dam tenders are part of the Operation Branch, in the Construction-Operations Division.

- b. Other Federal Agencies. The COE has complete responsibility for the operation of Lopez Dam; and although the COE receives data and information from other Federal and local agencies and informs these agencies of major decisions affecting Lopez Dam, no other agency has any responsibility in the operation of Lopez Dam. The U.S. Geological Survey operates stream gages in the LACDA.
- c. State and County Agencies. LACDPW has maintenance responsibility for the Pacoima channel downstream of Lopez Dam and maintains and operates a number of projects in the drainage area. See Section 3-04.

9-02 INTERAGENCY COORDINATION

The COE coordinates with other Federal, State, County, and local organizations, as well as with the press, concerning the water control for Lopez Reservoir.

- a. Local Press and Corps of Engineers Bulletins. The Public Affairs Office of the LAD, is responsible for interfacing with the press regarding operations at Lopez Dam and flows in the channel downstream of the dam. This is accomplished through interviews and the occasional issuance of press releases. The COE does not broadly issue flood watches or warnings or other status reports or forecasts to the general public. These are the responsibility of the NWS.
- b. National Weather Service. The COE utilizes NWS data and forecasts in the operation of Lopez Dam, including the real-time telemetry data from gages installed in the watershed by the LACDPW in cooperation with the NWS. The COE shares data with the NWS and other agencies both on a real time basis and after the fact.

- c. U.S. Geological Survey. The COE receives streamflow data from the U.S. Geological Survey, primarily on a historical basis in southern California. The COE coordinates with the U.S. Geological Survey in many different ways, and shares its data with the Geological Survey.
- d. Los Angeles County Department of Public Works. The COE and LACDPW closely coordinate the operation of their reservoir projects and the maintenance and patrolling of their channels in the LACDA.

9-03 REPORTS

The LAD prepares and files several types of reports.

Each month during the runoff season, November through April, a flood situation and runoff potential report is prepared and sent to the South Pacific Division of the COE.

Four specific forms are also prepared in conjunction with the District's reservoir operation at Lopez Dam. A copy of each of these forms is included as Figures 9-01 through 9-04. These include: Flood Control Basin Operation Report (prepared by each dam tender), Record of Calls (both radio and telephone), Record of Data from Digital Recorders, and Reservoir Computations.

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Figure 9-02

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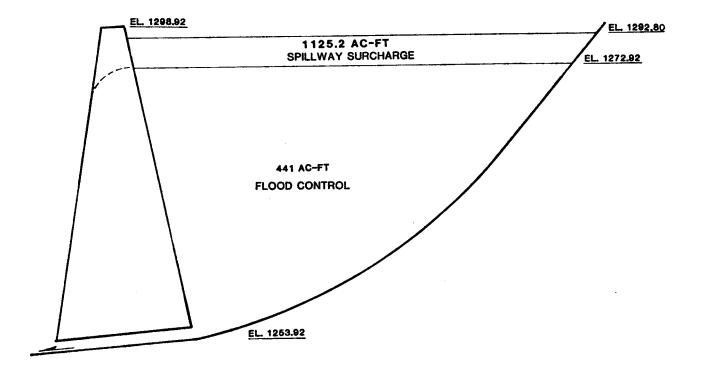
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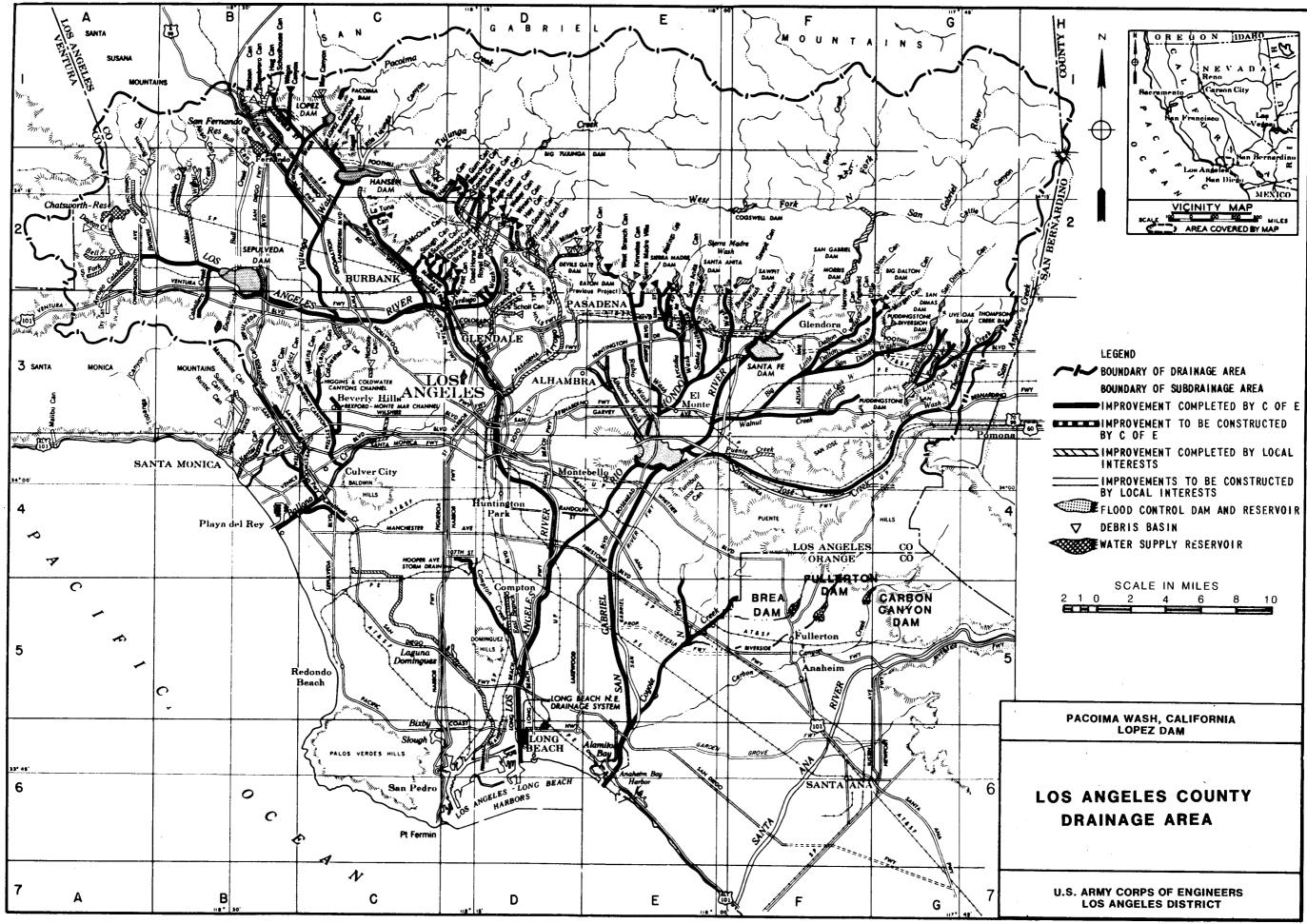
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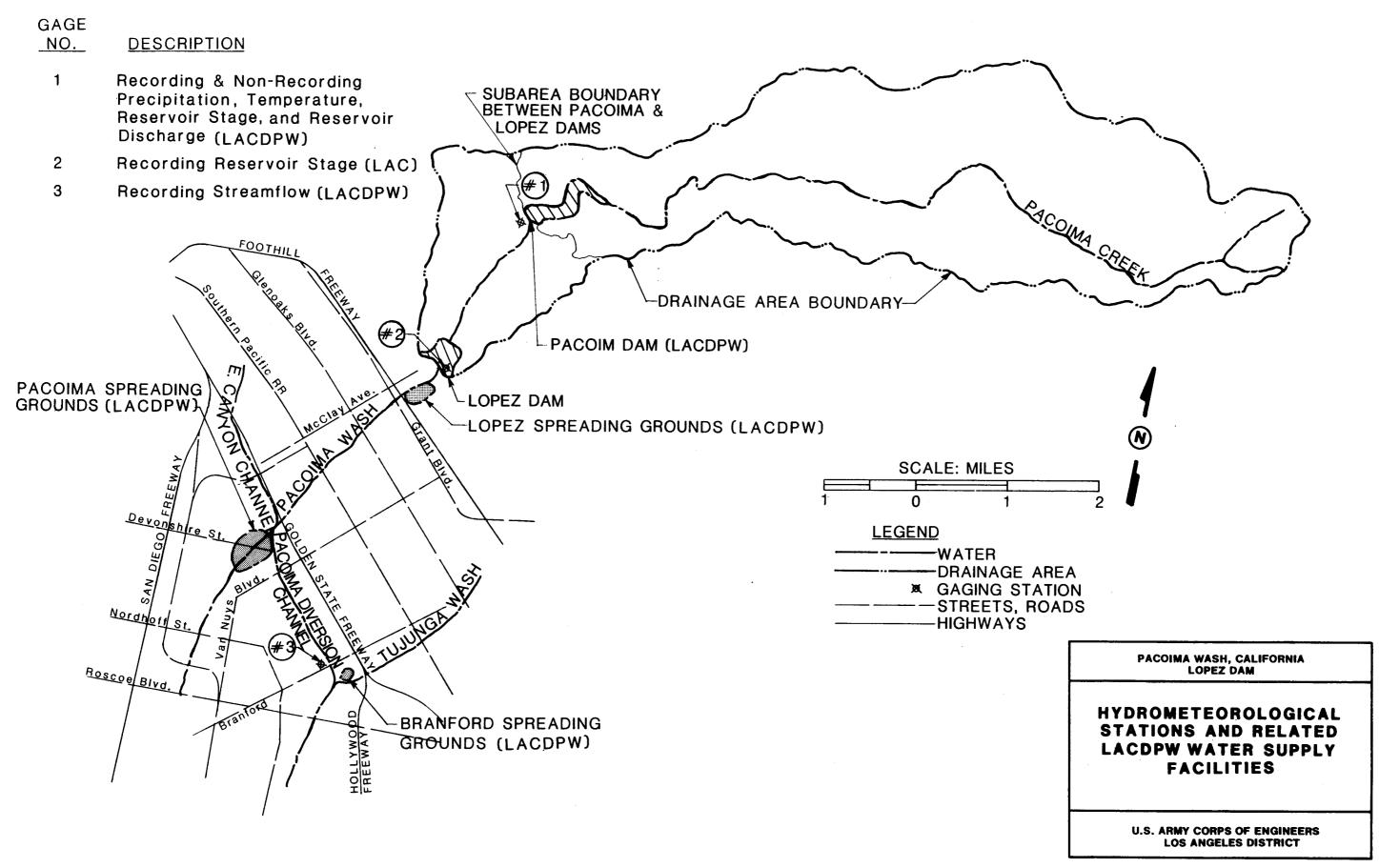
PLATE 2-1

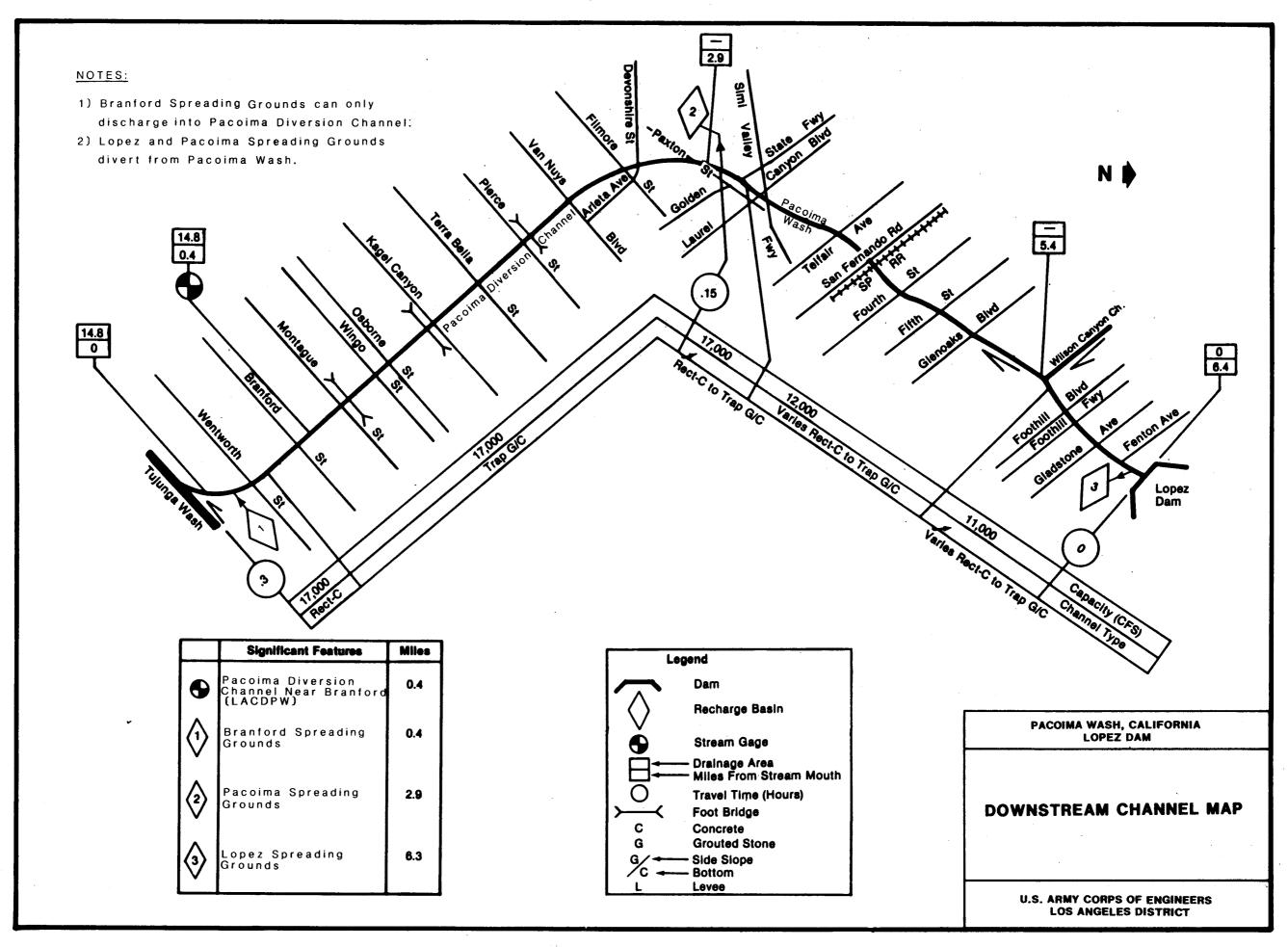


STORAGE ALLOCATION
DIAGRAM

U.S. ARMY CORPS OF ENGINEERS
LOS ANGELES DISTRICT







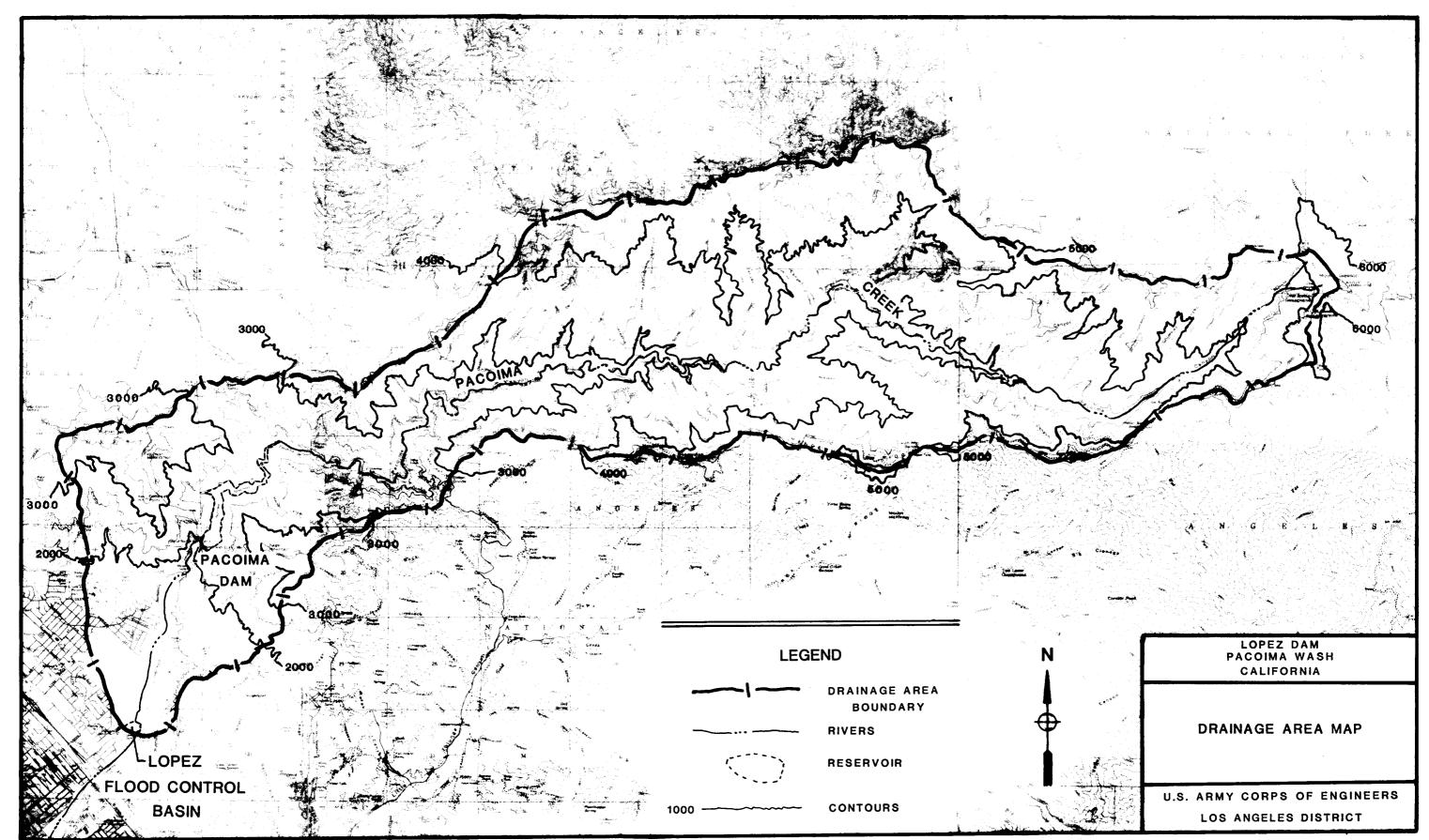
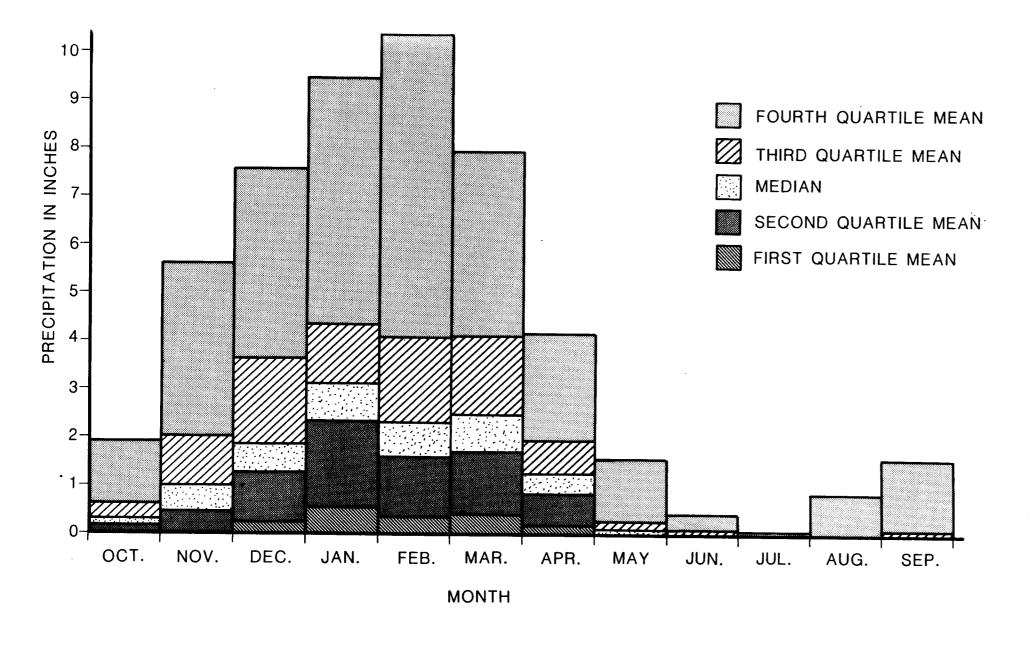


PLATE 3-4



See Table 4-2 for the information used to develop this Plate

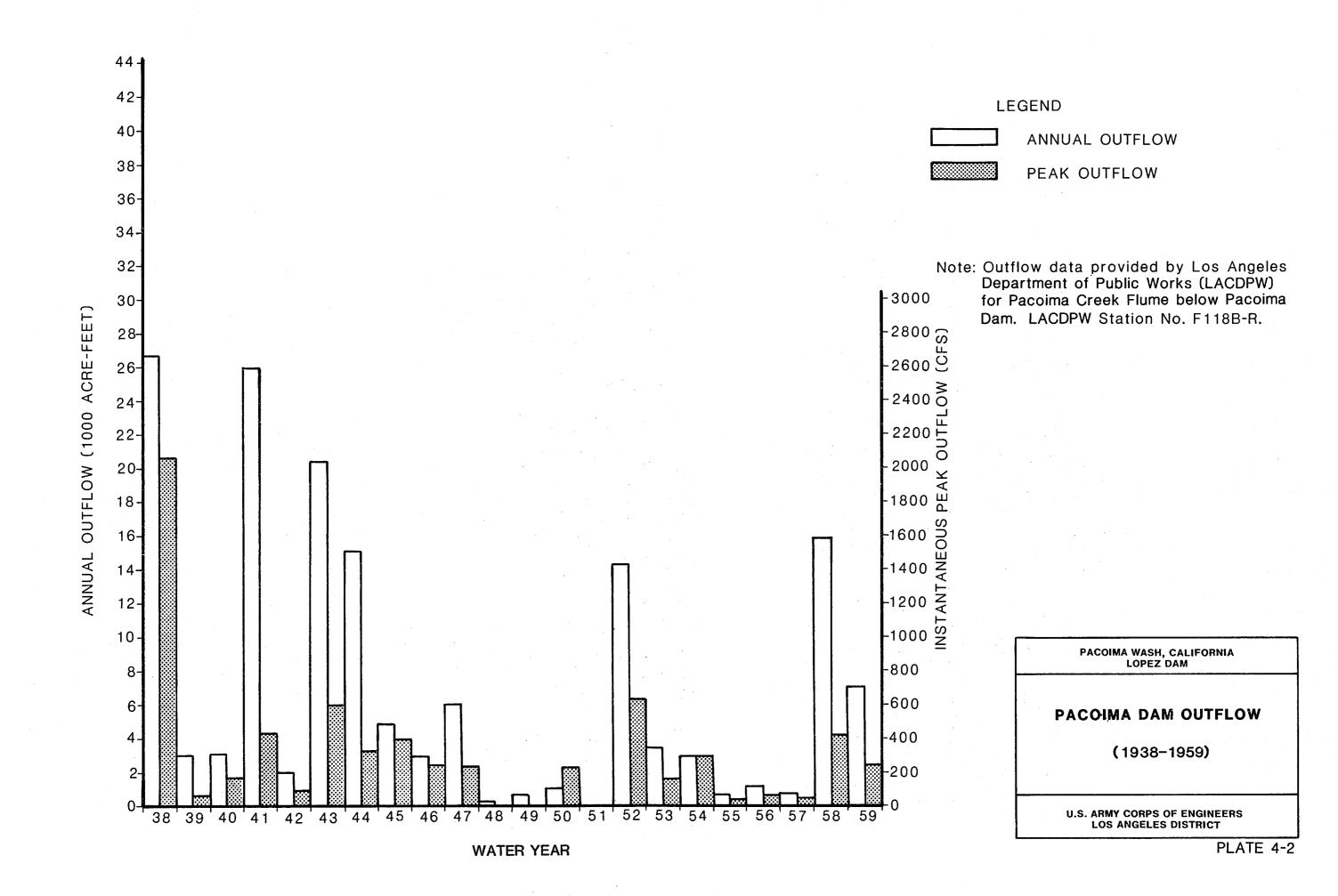
PACOIMA WASH, CALIFORNIA
LOPEZ DAM

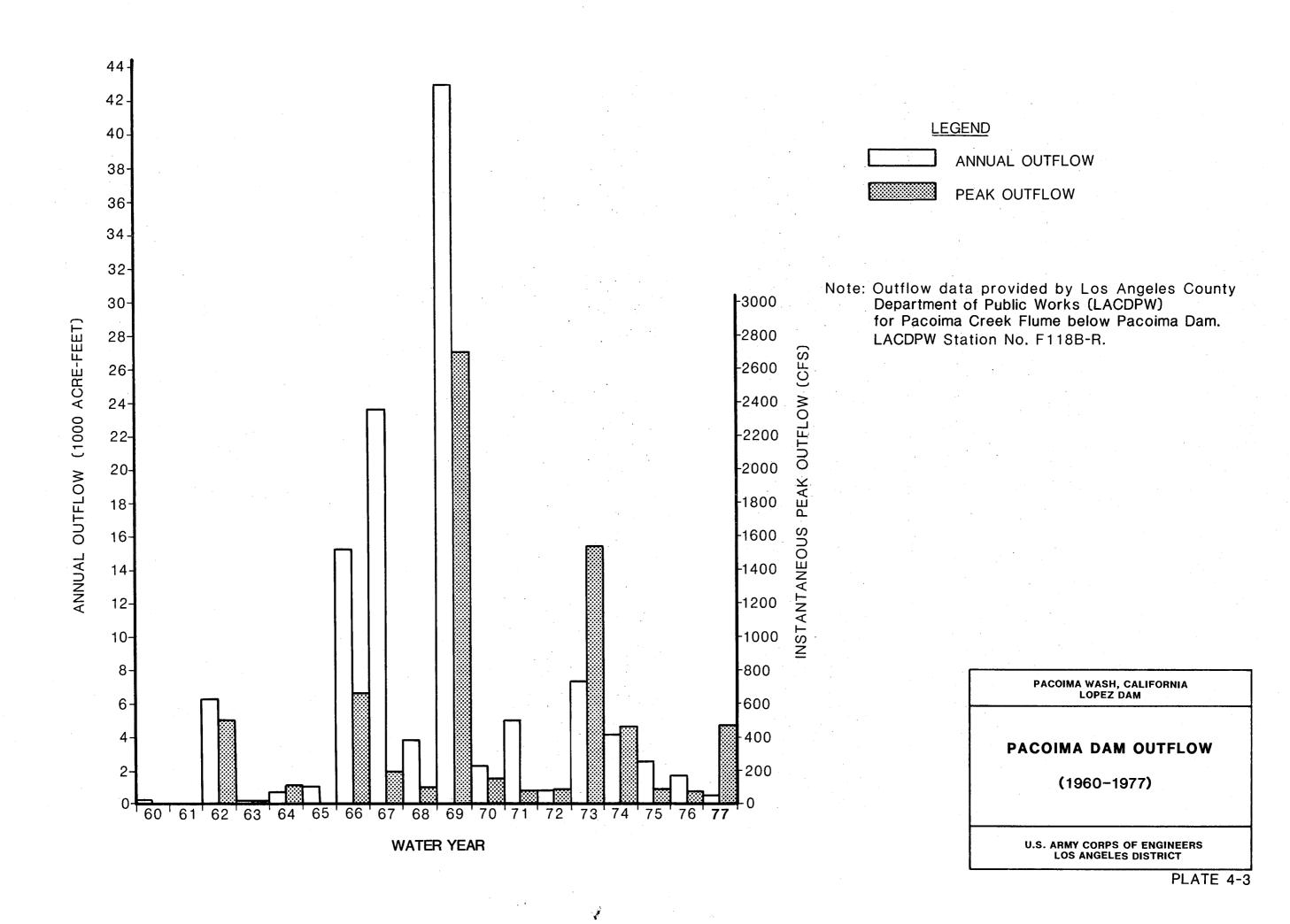
STATISTICAL PRECIPITATION

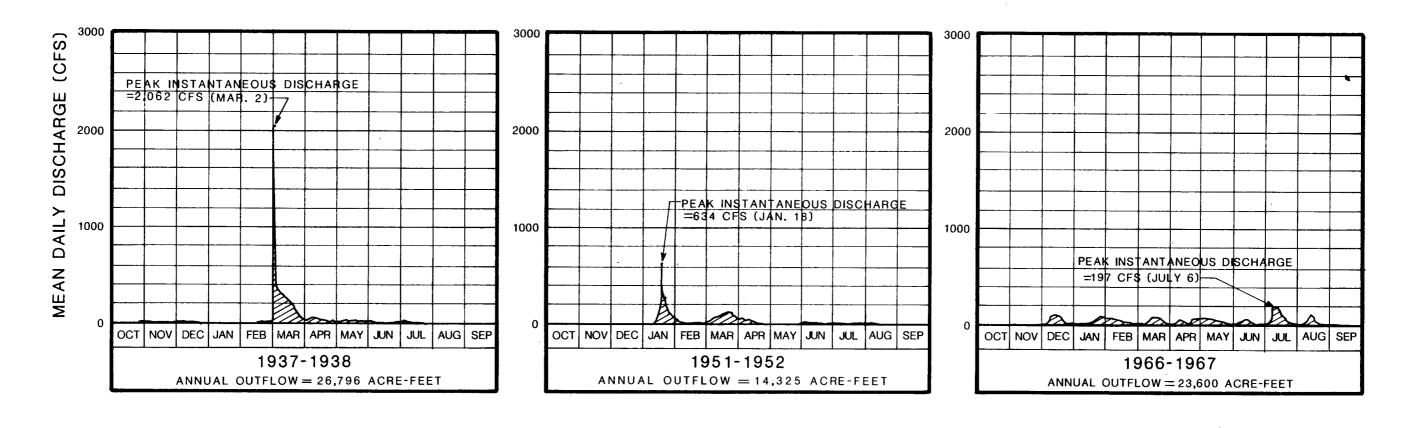
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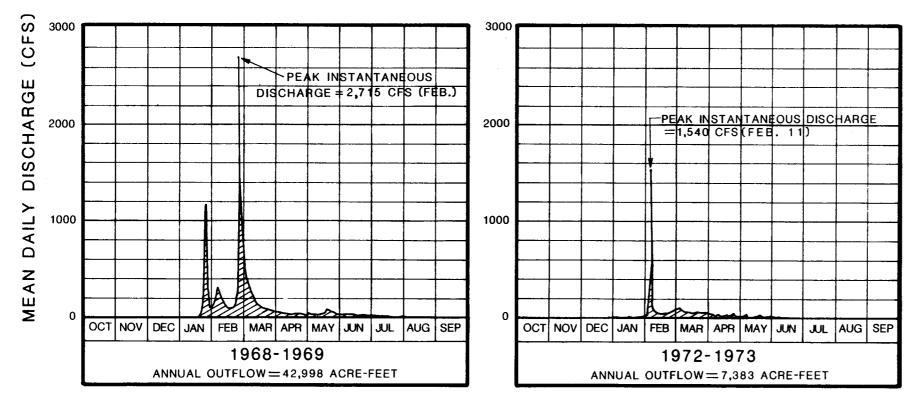
U.S. ARMY CORPS OF ENGINEERS
LOS ANGELES DISTRICT

PLATE 4-1

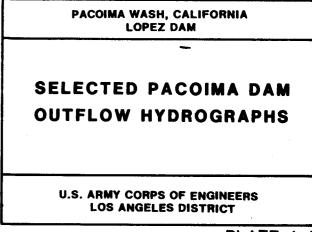


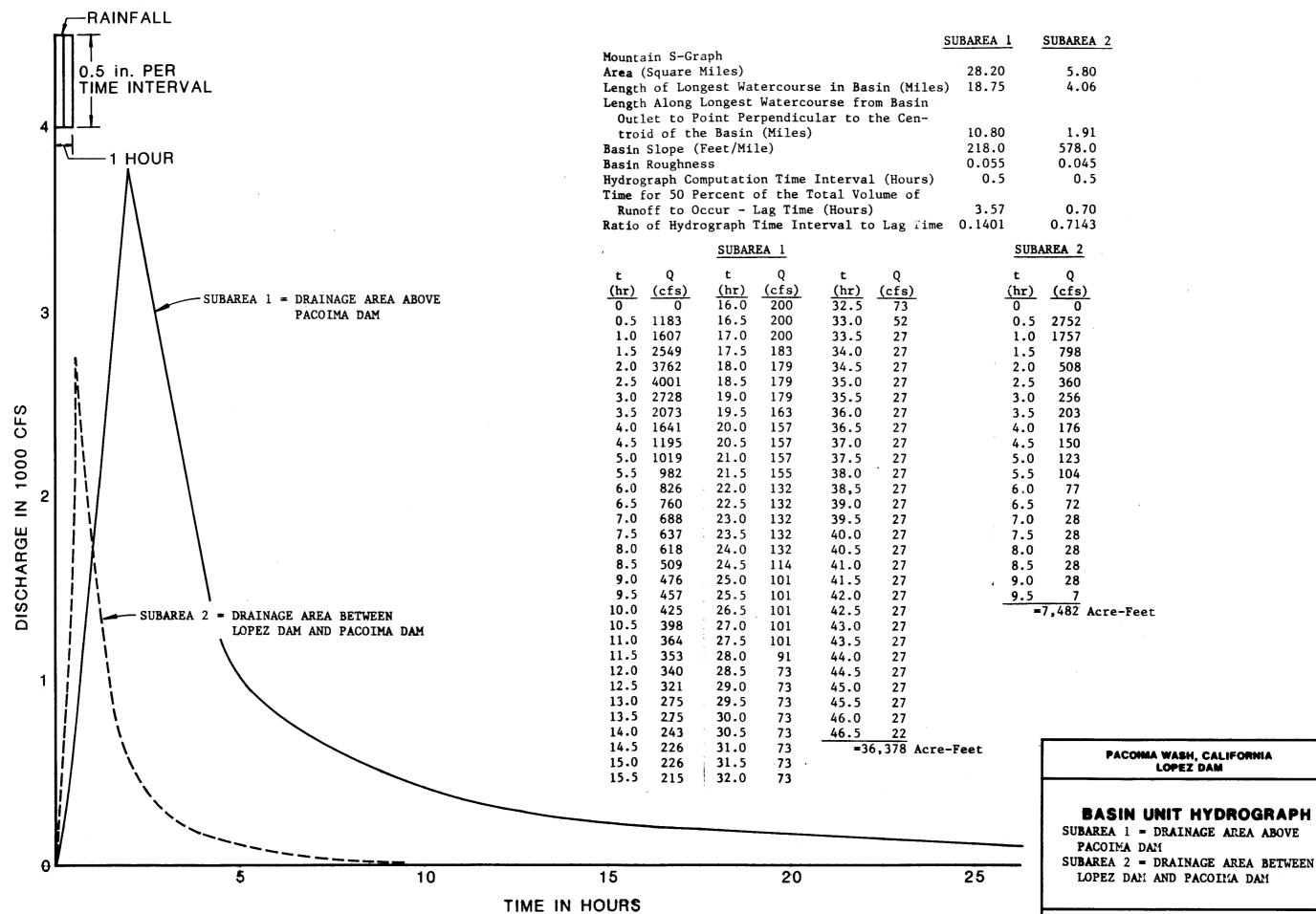




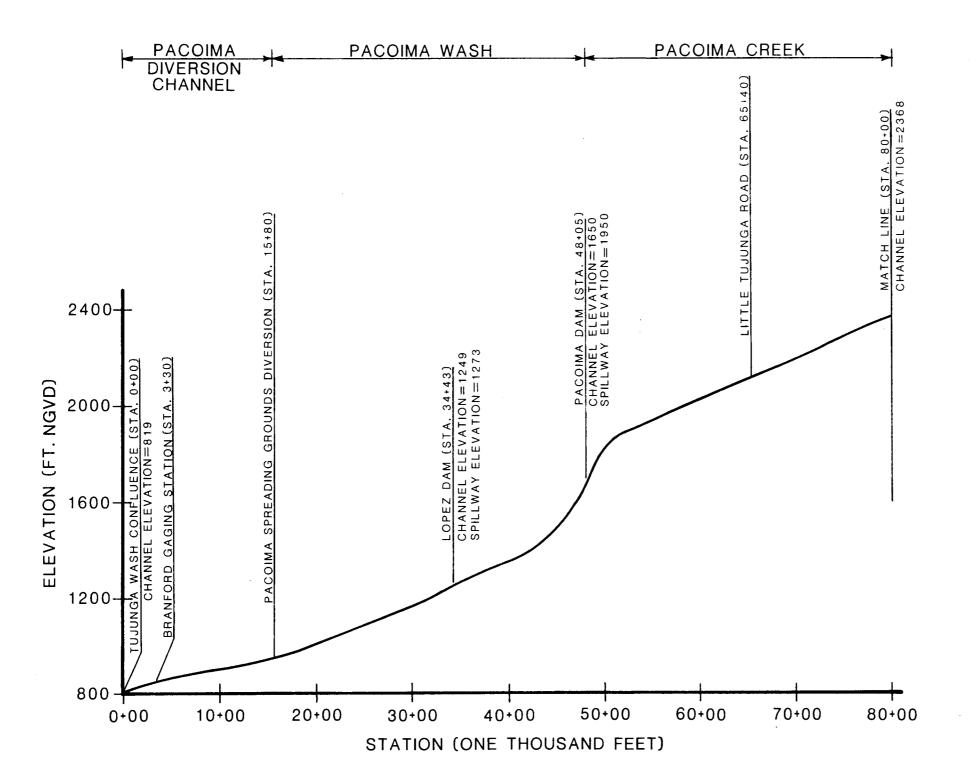


Note: Hydrographs were compiled from records provided by (LACDPW) for Pacoima Creek Flume below Pacoima Dam.





U.S. ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT



NOTE: Channel elevations taken from U.S.G.S. Quadrangle maps (Van Nuys, San Fernando, Sunland, Agua Dulce, Condor Peak, and Acton.)

PACOIMA WASH, CALIFORNIA LOPEZ DAM

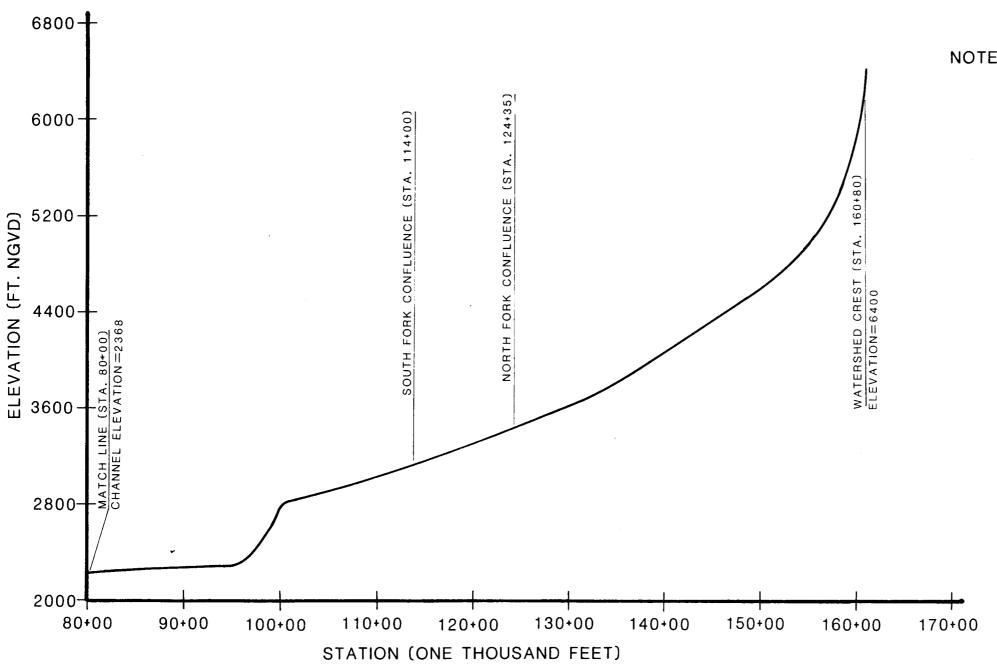
PACOIMA CHANNEL PROFILE

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U.S. ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT

PLATE 4-6





NOTE: Channel elevations taken from U.S.G.S. Quadrangle maps (Van Nuys, San Fernando, Sunland, Agua Dulce, Condor Park, and Acton.)

PACOIMA WASH, CALIFORNIA LOPEZ DAM

PACOIMA CHANNEL PROFILE

(STA. 80+00 - STA. 160+80)

U.S. ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT

PLATE 4-7

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.4	22.0	24.0	26.0	28.0	30.0	32.0	34.0	36.0	38.0	40.0
.5	42.0	44.8	47.6	50.4	53.2	56.0	58.8	61.6	64.4	67.2
.6 .7	70.0 106.0	73.6 110.2	77.2	80.8	84.4	88.0	91.6	95.2	98.8	102.4
8	148.0	152.7	114.4 157.4	118.6 162.1	122.8 166.8	127.0 171.5	131.2 176.2	135.4 180.9	139.6	143.8
.9	195.0	200.1	205.2	210.3	215.4	220.5	225.6	230.7	185.6 235.8	190.3
1.0	246.0	251.5	257.0	262.5	268.0	273.5	279.0	284.5	290.0	295.5
1.1	301.0	306.7	312.4	318.1	323.8	329.5	335.2	340.9	346.6	352.3
1.2	358.0 418.0	364.0	370.0	376.0	382.0	388.0	394.0	400.0	406.0	412.0
1.3	418.0	424.4 488.4	430.8 494.8	437.2 501.2	443.6 507.6	450.0 514.0	456.4 520.4	462.8 526.8	469.2 533.2	475.6
1.5	546.0	552.9	559.8	566.7	573.6	580.5	587.4	594.3	601.2	539.6 608.1
1.6	615.0	622.3	629.6	636.9	644.2	651.5	658.8	666.1	673.4	680.7
1.7	688.0	695.5	703.0	710.5	718.0	725.5	733.0	740.5	748.0	755.5
1.8	763.0	770.7	778.4	786.1	793.8	801.5	809.2	816.9	824.6	832.3
1.9 2.0	840.0 920.0	848.0 928.5	856.0 937.0	864.0 945.5	872.0 954.0	880.0 962.5	888.0 971.0	896.0	904.0	912.0
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2.3	1190.0	1200.0	1210.0	1220.0	1230.0	1240.0	1250.0	1260.0	1270.0	1280.0
2.4	1290.0	1300.0	1310.0	1320.0	1330.0	1340.0	1350.0	1360.0	1370.0	1380.0
2.5	1390.0 1490.0	1400.0 1500.0	1410.0 1510.0	1420.0	1430.0	1440.0	1450.0	1460.0	1470.0	1480.0
2.7	1590.0	1601.0	1611.0	1520.0 1622.0	1530.0 1632.0	1540.0 1643.0	1550.0 1653.0	1560.0 1664.0	1570.0 1674.0	1580.0 1685.0
2.8	1695.0	1706.0	1717.0	1728.0	1739.0	1750.0	1761.0	1772.0	1783.0	1794.0
2.9	1805.0	1817.0	1828.0	1840.0	1851.0	1863.0	1874.0	1886.0	1897.0	1909.0
3.0	1920.0	1932.0	1943.0	1955.0	1966.0	1978.0	1989.0	2001.0	2012.0	2024.0
3.1	2035.0	2047.0	2058.0	2070.0	2081.0	2093.0	2104.0	2116.0	2127.0	2139.0
3.2	2150.0 2270.0	2162.0 2283.0	2174.0 22 95. 0	2186.0 2308.0	2198.0 2320.0	2210.0 2333.0	2222.0 2345.0	2234.0	22 46. 0 2370.0	2258.0
3.4	2395.0	2408.0	2420.0	2433.0	2445.0	2458.0	2470.0	2483.0	2495.0	2383.0 2508.0
3.5	2520.0	2533.0	2546.0	2559.0	2572.0	2585.0	2598.0	2611.0	2624.0	2637.0
3.6	2650.0	2663.0	2676.0	2689.0	2702.0	2715.0	2728.0	2741.0	2754.0	2767.0
3.7	2780.0	2793.0	2806.0	2819.0	2832.0	2845.0	2858.0	2871.0	2884.0	2897.0
3.8 3.9	2910.0 3040.0	2923.0	2936.0	2949.0	2962.0	2975.0	2988.0	3001.0	3014.0	3027.0
4.0	3176.0	3054.0 3190.0	3067.0 320 3 .0	3081 .0 3217 . 0	3094.0 3230.0	3108.0 3244.0	3122.0 3257.0	3135.0 3271.0	3149.0 3284.0	3162.0 3298.0
4.1	3311.0	3325.0	3338.0	3352.0	3365.0	3379.0	3392.0	3406.0	3419.0	3433.0
4.2	3446.0	3460.0	3474.0	3488.0	3502.0	3516.0	3530.0	3544.0	3558.0	3572.0
4.3	3586.0	3600.0	3614.0	3628.0	3642.0	3656.0	3670.0	3684.0	3698.0	3712.0
4.4	3726.0	3741.0	3756.0	3771.0	3786.0	3801.0	3816.0	3831.0	3846.0	3861.0
4.5	3876.0 4026.0	3891.0 4042.0	3906.0 40 5 8.0	3921.0 4074.0	3936.0 4090.0	3951.0 4106.0	3966.0 4122.0	3981.0 4138.0	3996.0 4154.0	4011.0 4170.0
4.7	4186.0	4202.0	4218.0	4234.0	4250.0	4266.0	4282.0	4298.0	4314.0	4330.0
4.8	4346.0	4364.0	4381.0	4399.0	4417.0	4434.0	4452.0	4470.0	4488.0	4505.0
4.9	4523.0	4541.0	4558.0	4576.0	4594.0	4611.0	4629.0	4647.0	4665.0	4682.0
5.0	4700.0							l		

Note: Rating Table provided by LACDPW.

Pacoima Diversion at Branford Street,
F305-R, Rating Curve 54I, March, 1954.

PACOIMA WASH, CALIFORNIA LOPEZ DAM

PACOIMA DIVERSION CHANNEL AT BRANFORD STREET RATING TABLE

U.S. ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT

PLATE 5-1

CHAIN OF COMMAND FOR RESERVOIR OPERATIONS DECISIONS

Corps of Engineers Los Angeles District

Title

District Engineer

Office Phone Number:

(213) 894-5300 FTS 798-5300

Water Control Decisions

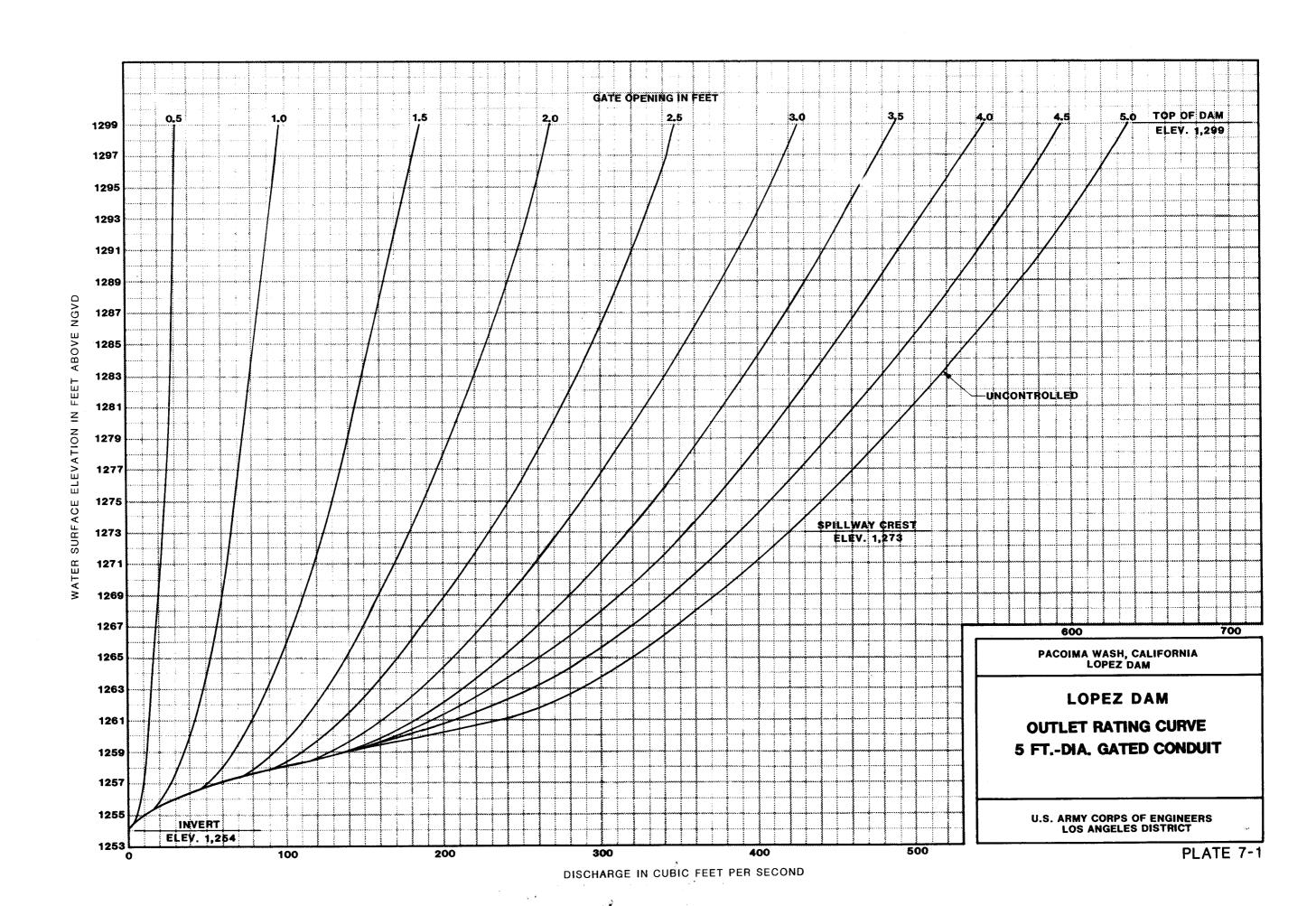
Gate Operations

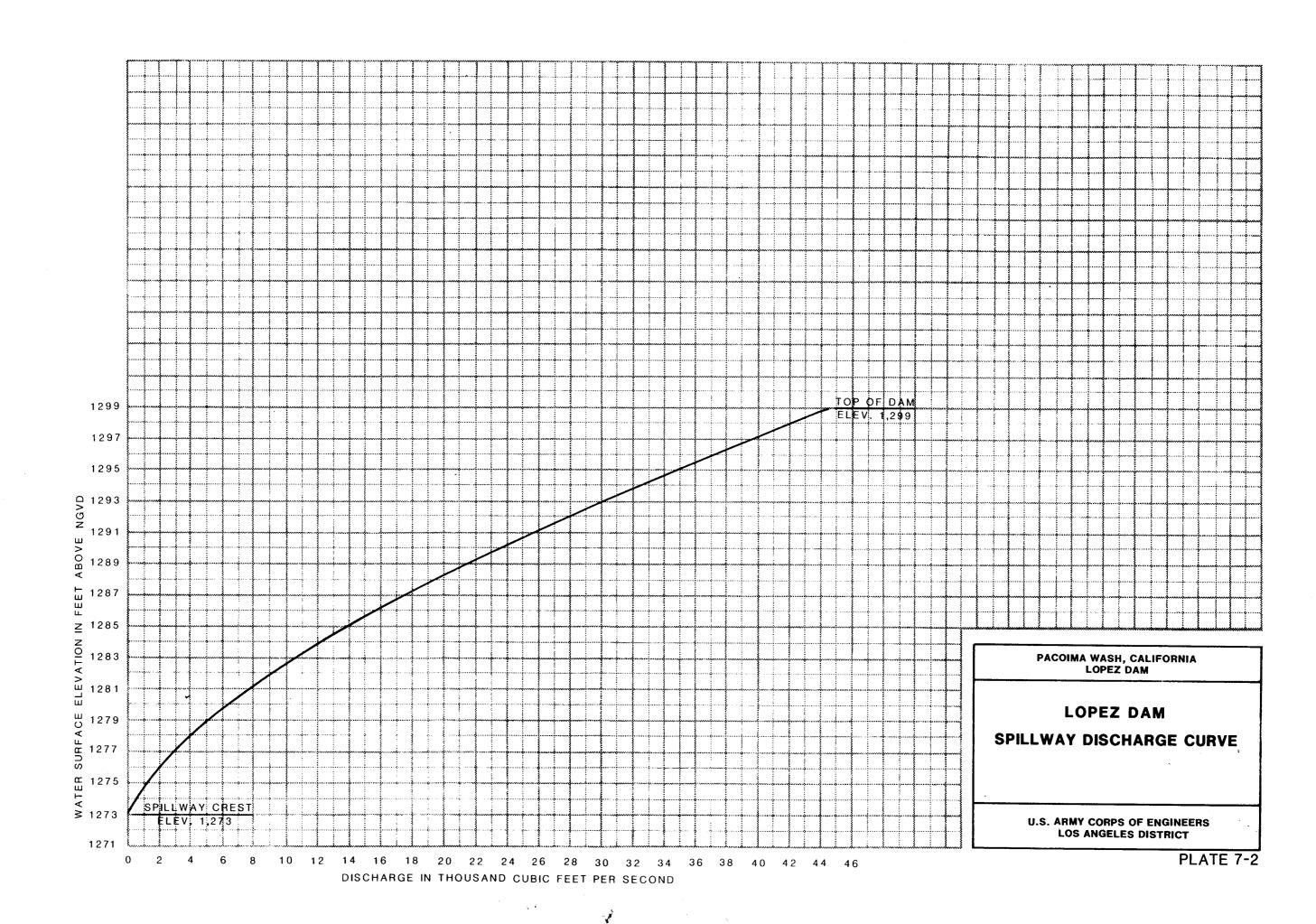
<u>Title</u>	<u>P</u>	hone:	<u>Title</u>		Phone:
Chief, Engineering Division		894-5470 798-5470	Chief, Construction- Operations Division		894-5600 798-5600
Chief, Hydrology & Hydraulics Branch		894-5520 798-5520	Chief, Operations Branch		894-5620 798-5620
Chief, Hydrologic Engineering Sectio	(213) n FTS	894-4753 798-4753	Chief, Operations and Maintenance Section	(213)	283-2757
Chief, Reservoir Regulation Unit		894-4756 798-4756	Dam Tender Foreman	(213)	283-2757
			Lopez Dam Tender at Hansen Dam	(818)	767-3810

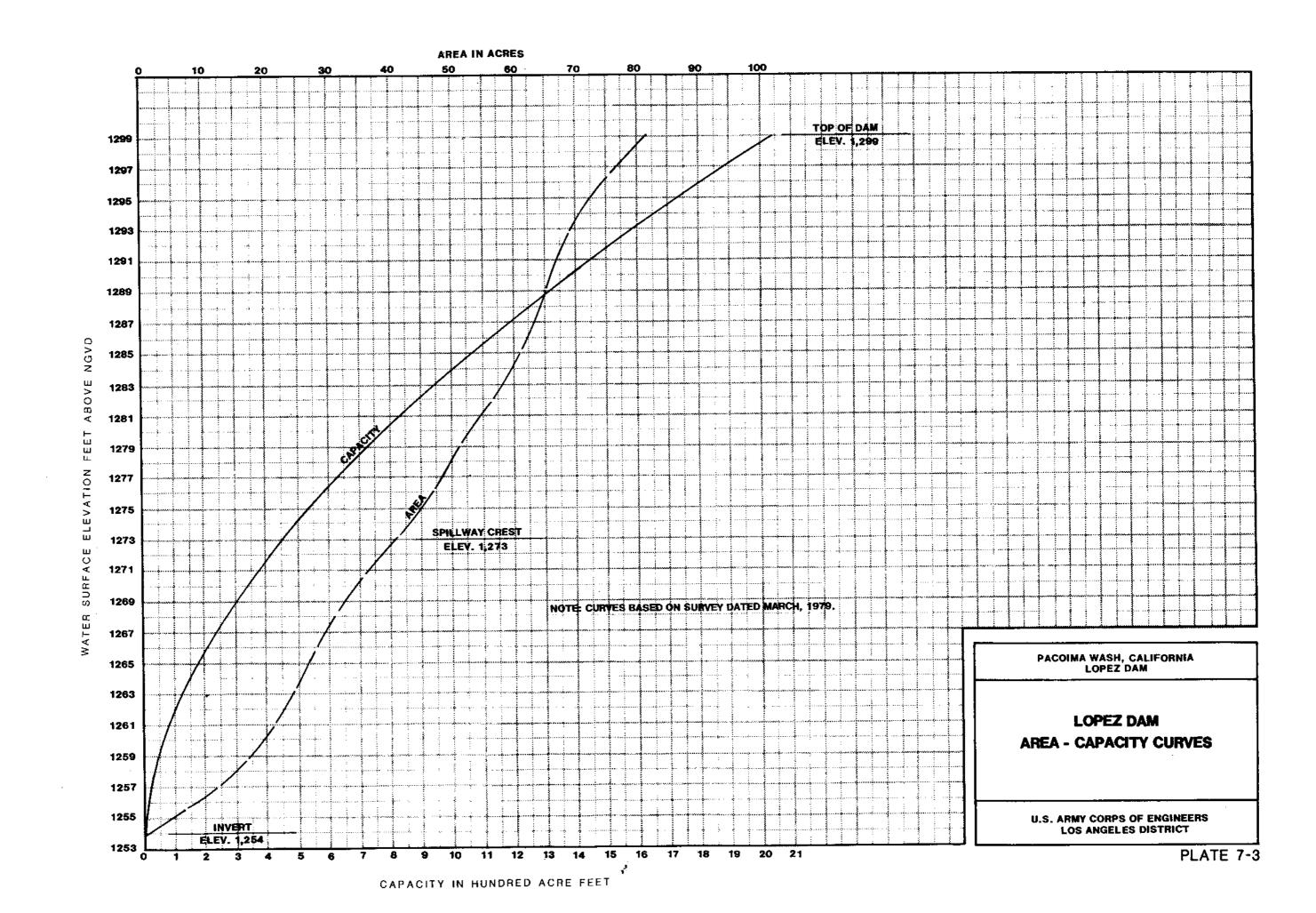
PACOIMA WASH, CALIFORNIA LOPEZ DAM

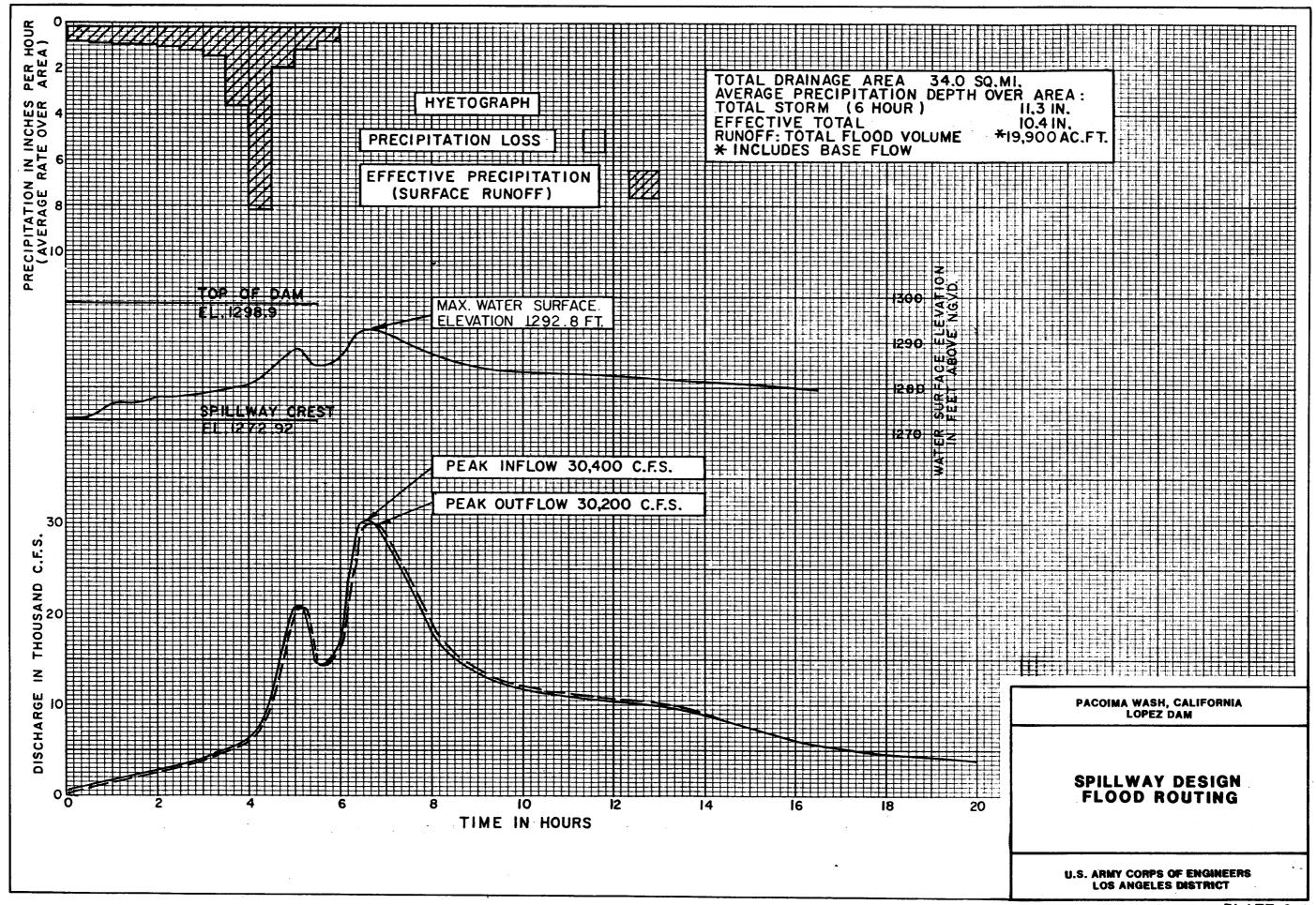
CORPS OF ENGINEERS
ORGANIZATION CHART AND
IMPORTANT PHONE NUMBERS
FOR RESERVIOR OPERATIONS

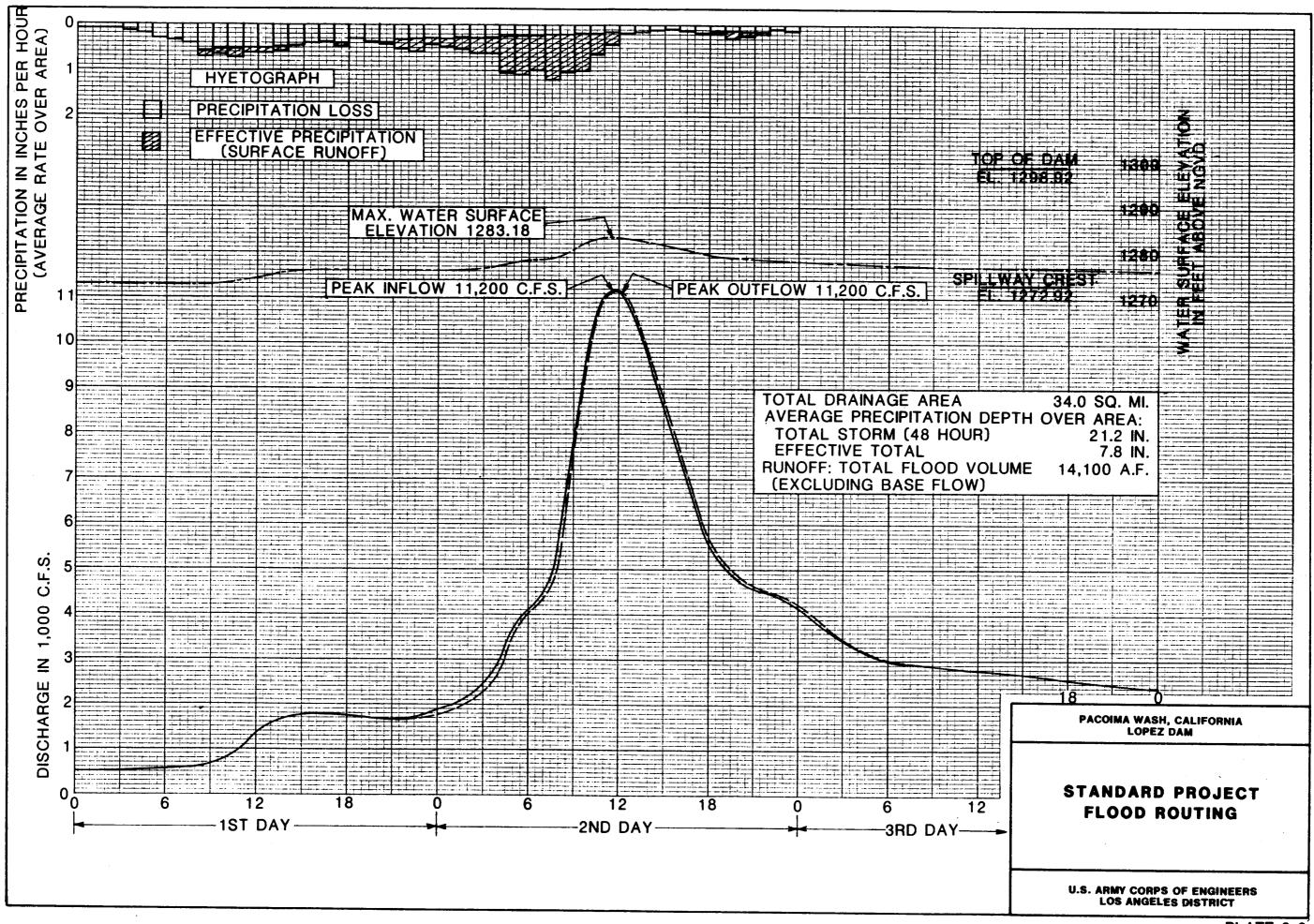
U.S. ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT

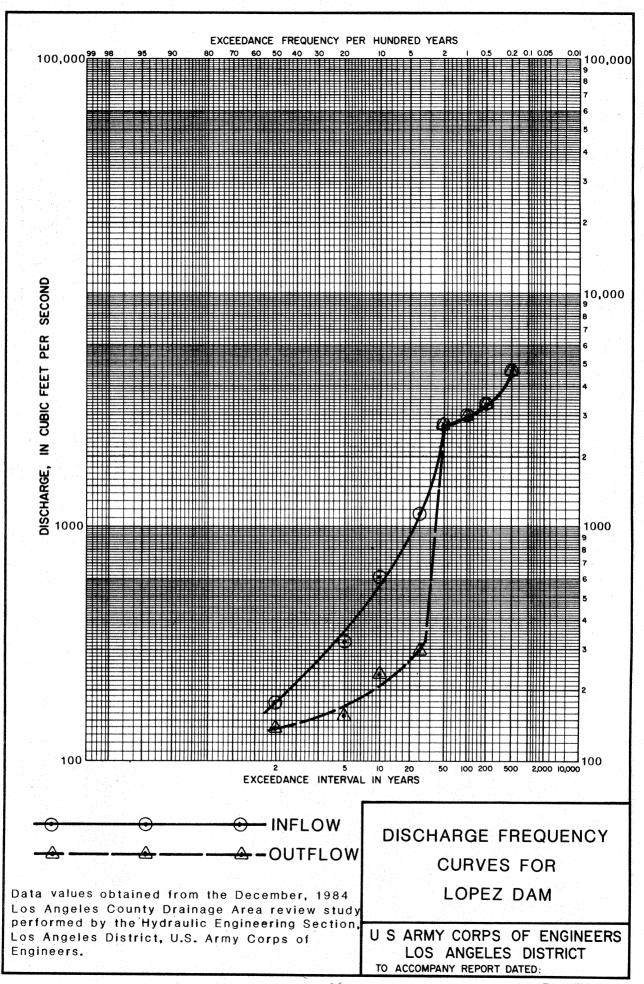


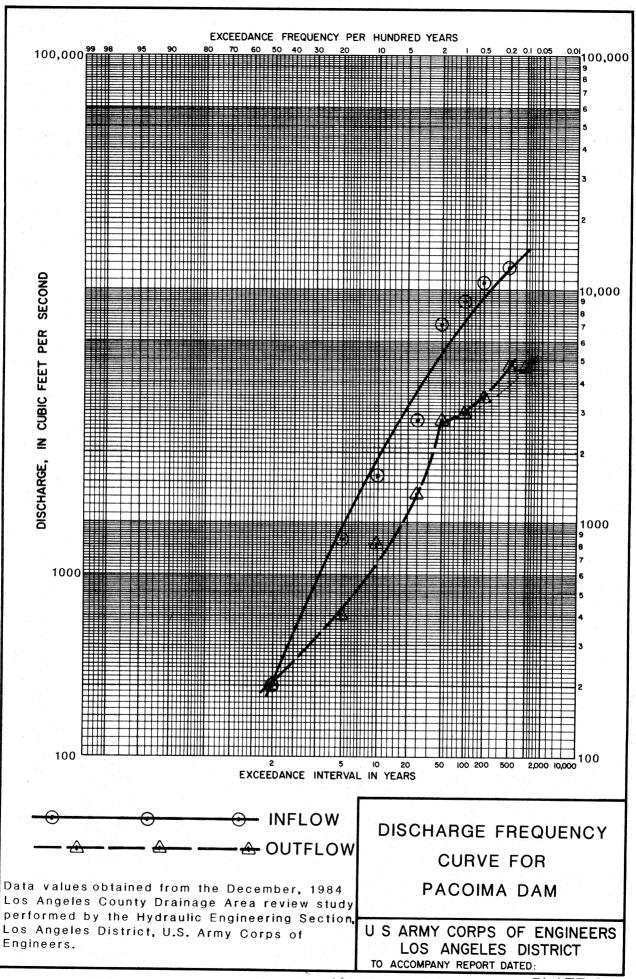


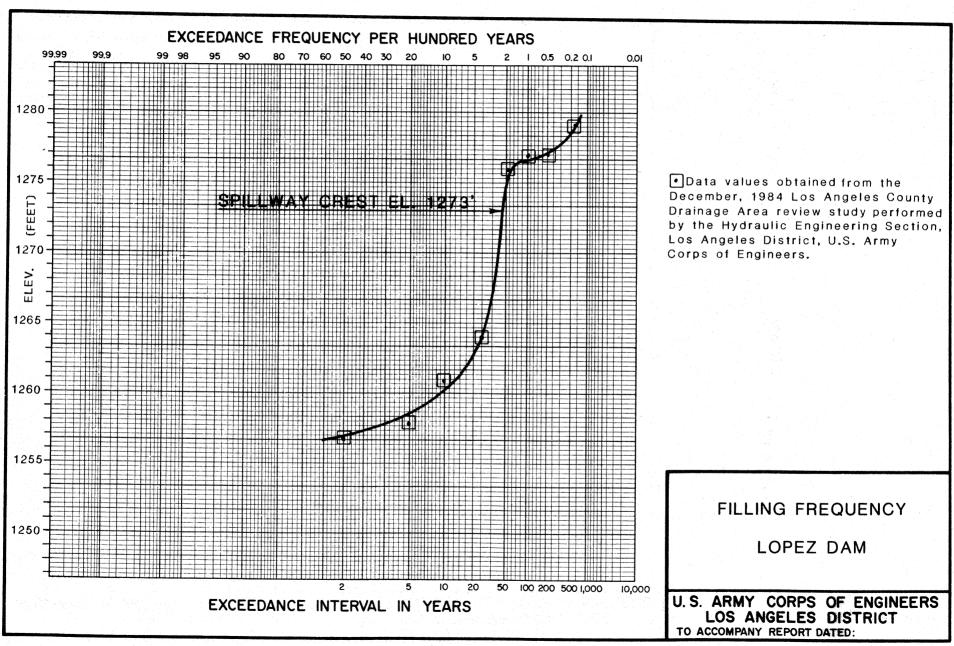


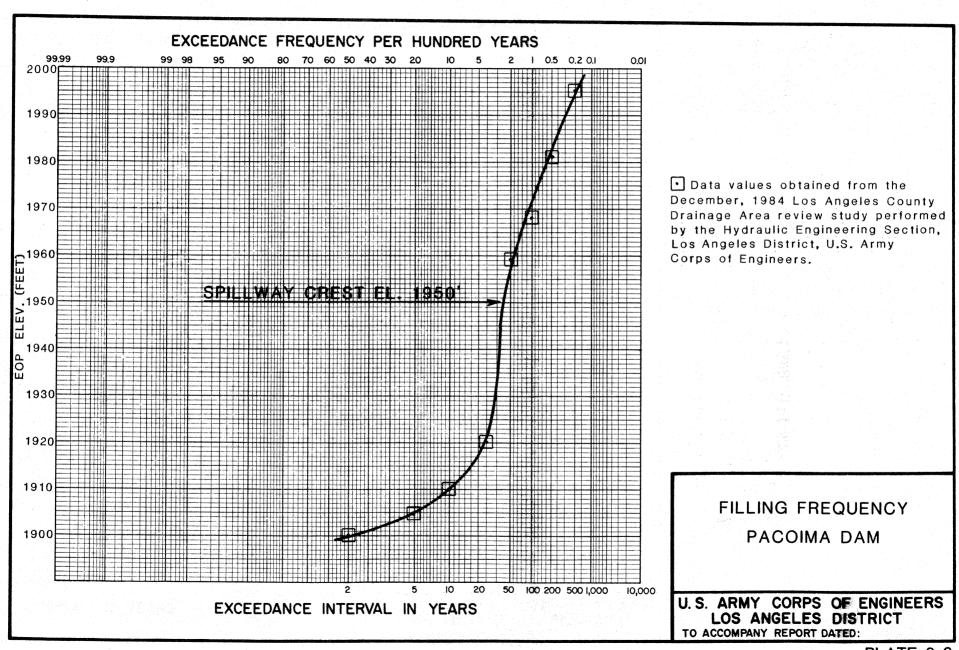










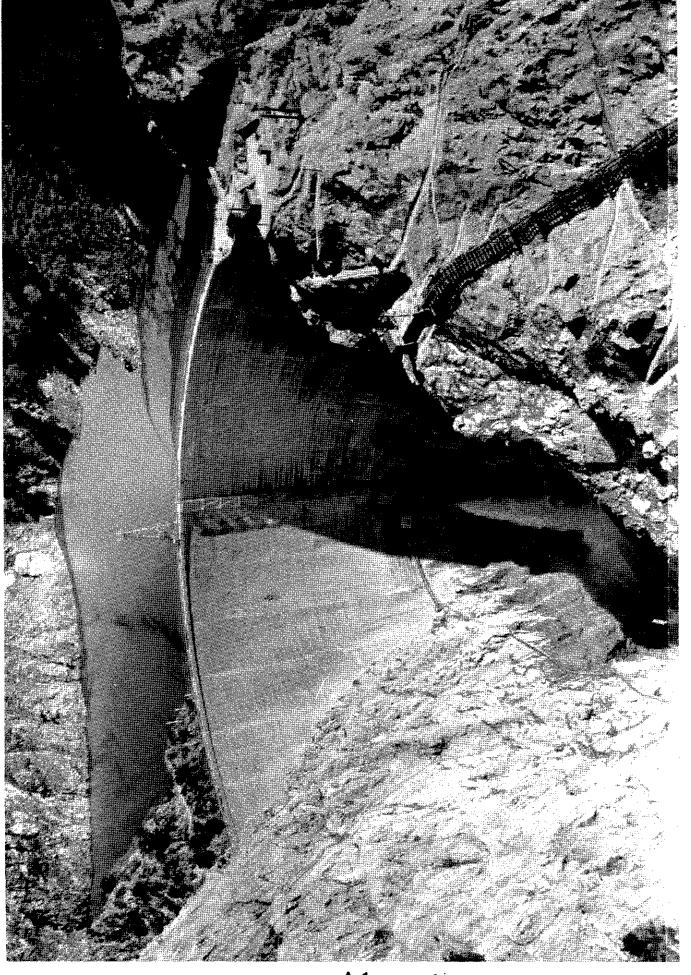


WATER CONTROL MANUAL LOPEZ DAM

EXHIBIT A
PERTINENT DATA
PACOIMA DAM AND RESERVOIR

PERTINENT DATA PACOIMA DAM AND RESERVOIR

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PACOIMA DAM LOS ANGELES COUNTY FLOOD CONTROL DISTRICT

PERTINENT DATA SHEET

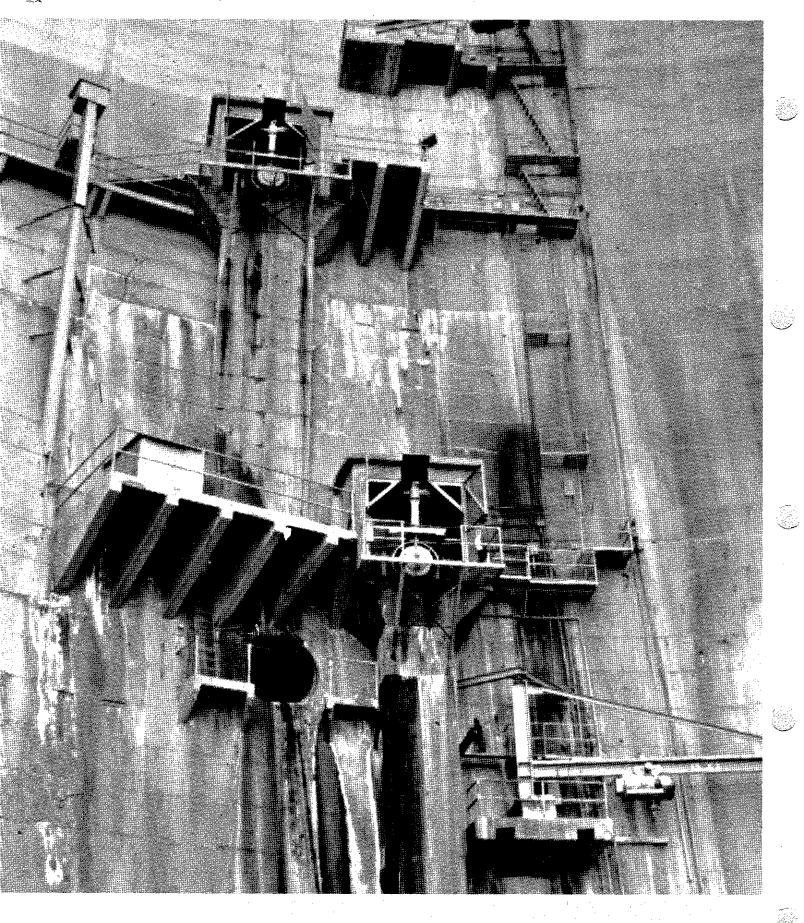
GENERAL							
NAME LOCATION STREAM PURPOSE TYPE	- PACOIMA DAM - PACOIMA CANYON - PACOIMA CREEK - FLOOD CONTROL A - CONCRETE, CONS	AND CONSERV	TION			- 28.2 SQ. MI. F	ROM ELEV. 1950 VE MEAN SEA LEV
CONSTRUCT	ION:						
COST COST PER COST PER	ARCH 1925 - FEBRUARY 1929 A.F. F.C. A.F. CONSERV. D ON ORIGINAL CAPA		738.28 206.85 407.09	DESI RESI	TRUCTED BY - BENT GNER - CONSTANT A DENT ENGINEER - K UTATIONS - B.F. J K.J. H	NGLE ARCH CO.	
DIMENSION	S:	<u></u>					
CREST HT. CREST LEN CREST WID HEIGHT OF BASE THIC	TH PARAPET WALL	3' 6	65.0 FT. 72.0 FT. 40.0 FT. 10.4 FT. 0.75 FT.	TOTA CHAF	L VOLUME OF EXCAV L VOLUME ON CONCE ACTER OF FOUNDATI	RETE .	105,927 C.Y. 226,140 C.Y.
OUTLETS:							
<u>no</u> .	TYPE	SIZE		ISER OR INLET	OUTLET ELEV	MAX. DISCH. AT SPILLWAY ELEV	_
FLOOD OPE	RATION VALUES						
2 2A 3 3A 4	HOLLOW JET GATE VALVE HOLLOW JET GATE VALVE HOLLOW JET GATE VALVE	30" 30" 30" 30" 30"		1850.2 1850.0 1850.5	1700.0 1700.0 1750.0 1750.0 1800.0 1800.0	350 319 281	
SERVICE VA	I.VES				÷	•	
A1 A1A A1B A1C A2 S.G.	GATE VALVE GATE VALVE GATE VALVE GATE VALVE HOLLOW JET SLUICE GATE	18" 8" 8" 18" 10" 5' x 5'		1669.7 1669.7 1669.7 1669.7 1825.0 1739.9	1694.0 1694.0 1694.0 1694.0 1703.0 1739.9	 33 1712.0	
SPILLWAY	түре		SILL ELEV.	NO. 1	SPILLWAY ORIG. CA A.F.		
1 2	TUNNEL TUNNEL		1950 .00 1989 . 95		(6060.0)	10,780	(COMBINED)
ELEVATION	vs:						
		ELEV.					
	STREAMBED (CAVATION	1650.00 1642.60					

- (1) CONSTRUCTED 1947. NEW TRASH RACK INSTALLED OCTOBER 1960. RISER INSTALLED NOVEMBER 1971. RISER REMOVED AUG. 1983.
 (2) RISER CONSTRUCTED DECEMBER 1947. MODIFIED IN 1964. EXTENDED IN OCTOBER 1969.
 (3) GUNITE RISERS CONSTRUCTED JANUARY 1939. RISER NO. 3 EXTENDED NOVEMBER 1971.

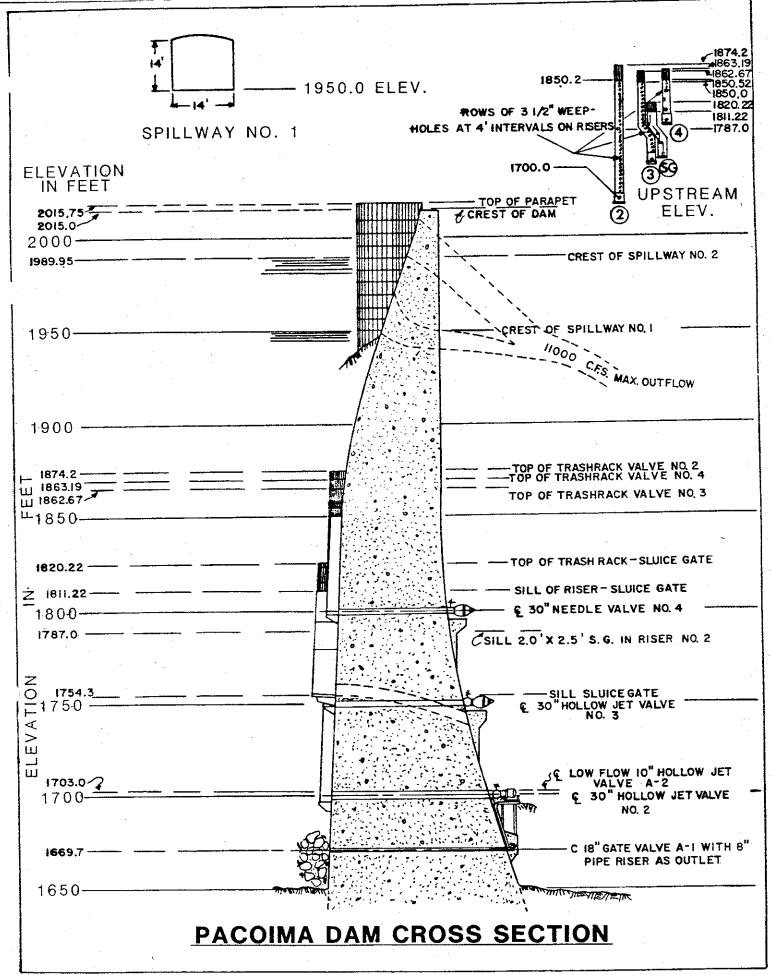
- HOLLOW JET INSTALLED JANUARY 1956.

 LATEST STATE APPROVAL JUNE 8, 1977. WATER MAY BE IMPOUNDED TO ELEVATION 1950 FEET.

 1950.035 USGS = 1950.0 SPILLWAY DATUM. SPILLWAY DATUM SHOWN. (4)



PACOIMA DAM OUTLET WORKS



PACOIMA DAM

LOS ANGELES COUNTY DEPARTMENT OF PUBLIC WORKS

EMERGENCY TELEPHONE LIST

District Operations Center (M-F, 8:00 am - 4:30 pm)	 (213)	226-4191
District 24-Hour Number	(213)	226-4308

Department of Public Works

MEMORANDUM

TO: Mr. Orville E. McCollom

January 31, 1985

FROM: N. C. Datywler

Hydraulic Division

File No. 64.121

Pacoima Dam and Reservoir

Operation Plan

Recommendation

Approve the following operating plan for Pacoima Dam. It is recommended that this operating plan take effect upon completion of the current spillway tunnel extension project.

Background

Due to sluicing of Pacoima Reservoir in 1983 and the current spillway tunnel extension project, several new operating schemes for the dam and reservoir were studied. The proposed operating plan is outlined below.

Operating Plan -

Holding Pool

Minimum holding pool is Elevation 1880.0 feet.

Rising Reservoir

Storm inflow will be ponded to Elevation 1920.0 feet. Above 1920.0 feet, outflow will approximate inflow to maximum outlet capacity (874 cfs at Elevation 1920.0 feet).

Once spillway flow is achieved (Elevation 1950.0 feet), the valves will be closed in increments so that the combined outflow from the valves and spillway will produce a uniform outflow hydrograph. All valves will be closed above Elevation 1954.0 feet. At no time will the combination of total valve plus spillway outflow be greater than inflow to the reservoir.

Falling Reservoir

When possible, releases greater than inflow will be maintained until the reservoir water surface has receded to Elevation 1927.0 feet. At that time, outflow may be reduced to a conservable rate or inflow, whichever is greater. At the discretion of the operation engineer, reservoir outflow may be reduced to inflow at elevations greater than 1927.0 feet to increase water conservation benefits.

Mr. Orville E. McCollom Page 2 January 31, 1985

Water Conservation Pool

Nonstorm spreading releases greater than inflow should be initiated when Elevation 1927.0 feet is achieved.

Discussion

The 10-, 25-, 50-year and capital event runoff frequency inflow hydrographs were routed through the dam with the goal of increasing water conservation benefits, preventing spillway flows for events with less than a 10-year frequency, and keeping the reservoir debris cone upstream a satisfactory distance. Alternate discharge schedules, which begin releases at lower elevations, do not significantly decrease the peak outflows for events greater than a 10-year frequency.

Runof f Frequency	Resultant W.S. Proposed Plan	Maximum Release
10	1950.4	969
25	1962.5	1761
50	1976.3	2230
Capital	2009.0	10,342

A 1,500 acre-foot water conservation pool is provided for between Elevation 1880.0 feet and 1927.0 feet. If need be, the reservoir can be drawn from 1927.0 feet to Elevation 1920.0 feet in approximately four hours using all valves with no inflow.

The Sedimentation Section of Hydraulic Division concurs with the elevation selected for minimum holding pool (Elevation 1880.0 feet) with regards to sedimentation. They expect no difficulties in valve operations due to sediment for at least several years with the absence of very large debrisproducing storms.

Brett L. Forrester Operations Section Extension 4191

BLF:bmc

cc: Operation and Maintenance (2) (Noyes, Remillard)
 Water Conservation
 SP&DIG
 Hydraulic (2) (Mitchell, Files)
 General Files

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945	7029.	7038.	7047.	7056.	7065.	7074.	7083.	7092.	7101.	7110.	9.01
700	71.19.	7128.	7137	7146.	7156.	7165.	71.74.	7183.	7192.	7201.	80°6
007.	72.10	7219.	7228	7237.	7247	7256	7265.	7274.	7283.	7292.	9.16
008	7302	7311.	7320.	7329.	7339.	7348.	7357.	7366.	7375.	7385.	9.23
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002	7676.	7685	7695	7704.	7714.	7723.	1733.	7742.	7752.	7762.	9.55
	7771.	7781.	7750.	7800.	7810.	7819.	.7829.	7839.	7848.	7858.	69.63
2004	7867	7877	7887	7897.	7906.	7916.	7926.	7935.	7945.	7955.	9.71
005	7965	7974	7984.	1994.	8004	8014.	8023.	8033.	8043.	8053.	9.19
006	8062	80.72	8082	8092.	8102.	8112	8122.	8132.	8141.	8151.	9.86
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600	82.61	8271.	. 8281.	8291.	8301.	8311.	8321.	8331.	8341.	8351.	10.02
	8361.	83.71	8381.	8391.	8401.	8411.	8421.	8431.	8441.	8452	10.10
010	8462	8472	8482	8492	8502.	8513.	8523.	8533.	8543.	8553.	10.18
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SPILLMAY ELEVATION	= 1950.0		٠.
CREST FLEVATION	= 2015.0		

LOS ANGELES COUNTY FLCCD CONTROL DISTRICI - HYDRAULIC DIVISION PAGE 4
INTERPOLATED ELEVATION-AREA TABULATION FOR PACCIMA DAM
GASED ON 64-T31. DATED 08/04/83 SCALE 1"= 10.0 ET. 1 SQ.IN= 1.000 UNITS

POINT	ELEVATION	AREA	
NUMBER	(FEET)	(ACRES)	BY JS 09/12/83
<u> </u>	******	****	•
1	1748.00	0.0	
2	1749.00	0.02	
	1750.00	0.03_	
4	1751.60	0.04	
5	1752-00	0-04	
6_ 7	1753.00	0.04	*
	1754.00	0.04	
8 9	1755.00 1756.00	0.06 0.09	•
10	1757.00	0.14	
11	1758.00	0.20	
12	1759.00	0.28	
13	1760.00	0.36	
14	1761-00	0.44	
15	1762-00	0.53	
16	1763.CO	0-63	
17	1764-00	0.72	
18	1765-00	0.82	
19		0.91	
20		1.01	
21	1768.00	1.10	
22		1.19	to the state of th
23		1.27	
24		1.35	
25		1.42	
26		1.49	
27		1.57	
28		1.65	
29	1776.CO	1.73	
30		1.82	en de la composition de la composition La composition de la
31	1778-00	1.91	
32	1779.00	2.00	
33	1780-00	2.09	
34		2-18	
35		2.27	
3.6		2-35	
37		2.43	
38		2.51	
39		2.59	
40	= -	2.67	
41		2.75	
42		2.83	
43		2.93	
44	and the second s	3.05	
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46		3.31	
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49	*	3.71	
50		3.83	
>1	1798.00	3.95	

LOS ANGELES COUNTY FLOOD CUNTROL DISTRICT - HYDRAULIC DIVISION PAGE 5
INTERPOLATED ELEVATION-AREA TABULATION FOR PACCIMA DAM
MASED ON 64-131 DATED 08/04/83 SCALE 1"= 10.0 FT. 1 SC.IN= 1.000 UNITS

POI NT	ELEVATION	AREA			***************************************	· · · · · · · · · · · · · · · · · · ·		±M.
NUMBER ——— 李本文文字	(FEET) <u>*******</u>	(ACRES)	BY J	IS (09/12/83			
52	1799.00	4-06						• 1
53	1800.00	4.17						
54_	1801-00	4-28_						
55	1802.00	4.39						
56	1803.00	4.50	•		* *.	• • • • • • • • • • • • • • • • • • •	\$ 1	
57_	1804-00	4.62						
58	1805.00	4.73						
59	1806.00	4-85			•			
60_	1807.00	4.98			 		T - 1-2	
61	1808.00	5.11						
62	1809.00	5-25						
63_	1810.00	5,39	<u> </u>	·				
64	1811.00	5-54						
65	1812.00	5-69						
66	1813.00	5.84_						يساع والمنافرة وأكسو
68	1814.00	6.00						
69_	1815.00 1816.00_	6.15				1.1	- · · · · · · · · · · · · · · · · · · ·	100
70	1817-00	6.30						: i—
71	1818.00	6.61						
	1819.00_	6.79			•			
73	1820.00	6.99					بالمناسب والمناج	
74	1821-00	7.22				1.		
75	1822.00	7.48						
76	1823.00	7.75		···,			·	
77		8.02						
78	1825.00	8.29			$(x_1, \dots, x_n) \in \mathcal{X}_{n+1}$	in the second second		
79	1826-00	8.56						
80	1827.00	8.81	· ·					
81	1828.00	9.07	·					
82		9.33						
83	1830.00	9.61						
84	1831-00	9.90						
85	1832.00	10-22						
86	1833.00	10.55			t et al f			
87	1834.00	10.91						
88 89	1835-00	11.28						
90	1836.00 1837.00	11.67 12.06_	•			4 - 4	4 A.1	
91	1838.00	12.46						
92	1839.00	12-40		1.	4			
93	1840.00	13.21					Professional	e policina de la composición dela composición de la composición de la composición dela composición del composición de la composición del composición de la composición del com
94	1841.00	13.54						والمراجع المستعددة
95	1842.00	13-84				•		
96	1843.00	14-11	. *		•			
97	1844-00	14-34						18 (18 18 18 18 18 18 18 18 18 18 18 18 18 18 1
(98	1845.00	14.54					#	
99	1846.00	14.71						
100	1847-00	14.86						
101		15.00						
1.02	1849.00	15.14						

LOS ANGELES COUNTY FLOED CONTROL DISTRICT - HYDRAULIC DIVISION PAGE 6
INTERPOLATED ELEVATION-AREA TABULATION FOR PACCIMA DAM
BASED DN 64-131 DATED 08/04/83 SCALE 1#= .10.0 FT. 1 SQ.IN= 1.000 UNITS

-									
	POINT	ELEVATION	AREA						
	NUMBER	(FEET)	(ACRES)	BY JS	09/12/8	3			
•	*****	*****	****		J // 12/0	•			
	1 03	1850.00	15.30						
	104	1851.CO	15.48						
	105	1852.00	15.69				; *		
	106	1853.00	15.90						
	107	1854.00	16.12	•	•				
	108_	1855.00	16.32						
	1.09	1856.00	16.52		•				
	110	1857.00	16.70						
	111_	1858.00	16.87	•					
	112	1859.00	17.05		· · · · · · · · · · · · · · · · · · ·				
	113	1860.00	17.22	i.	•				
	114	1861-00	17.41						
	115	1862-00	17.60						
	116	1863-00	17.81		*,				
	117	1864.00	18.03						* +
	118	1865.00	18.26			~~~~~~			
	119	1866.00	18-51		•				
	120	1867-00	18.76						
	121	1868.00	19.03						e e manager agains
	122	1869.00	19.29						
	123_	1870-00	19.54						
	124	1871.00	19.79						`
	125	1872.00	20.04						
_	126	1873-00	20.30						
	127	1874-00	20.59						
	128	1875.00	20.90					•	4
	129	1876.00	21.26	·		•			4.
	130	1877.00	21.63						er merk kroen kroen geren in de grande.
	131	1878.00	21-98		-				
	132_	1879.00	22,29						
	133	1880.00	22.54						
	134	1881.00	22.69			and the second of the			
	135	1882,00	22.80_						÷
	136	1883.00	22.92						
	137	1884-00	23.10		•				•
•	138_	1885.00	23-40		•				-
	139	1886.00	23.86						
	140	1887-00	24.43			•			
	141	1888.00	25.07				•		
	142	1889.00	25.72						
	1 43	1890.00	26.32						
**	144	1891-00	26.83						
	145	1892.00	27.27						
	146	1893.00	27.66						
	147	1894-00	28-02	•		•			•
	148	1895.00	28.39						
	149	1896-00	28.78						• •••
	150	1897-00	29.19						
	151	1898.00	29.61					• • • • • • • • • • • • • • • • • • •	:
	152	1899.00	30.05			. *			
	153	1900.00	30.49						

LOS ANGELES COUNTY FLOCO CONTROL DISTRICT - HYDRAULIC DIVISION PAGE 7
INTERPOLATED ELEVATION-AREA TABULATION FOR PACGIMA DAM
MASED ON 64-T31... DATED 08/04/83 SCALE 1"= 10.0 FT. 1 SQ.IN= 1.000 UNITS

POINT Number	ELEVATION (FEET)	AREA		·				aranda Sana			
*****	**************************************	(ACRES)	BY.	JS	09/12/83		18 . 18 .				
154	1901.00	30 . 92				·- ·					
155	1902.00	31.35					erin (†				
156_	1903.00_	31.80				•	•				
157	1904.00	32.26									
158	1905.00	32.74			· ·						
159_	1906.00_	33.26_									
160	1907.00	33.78			•				· · · · · · · · · · · · · · · · · · ·	- 5	
161	1908.00	34.31					er die er Gebeure				4.00
- 162	1909.00	34.83				٠.	•				
163	1910-00	35.33							••		· ×
164	1911.00	35.80						41.			
165_	1912.00_	36-25	ir								
166		36.70					7				
167	1914-00	37.17									
- <u>168</u>	1915-00	37.68_		•					•	÷ .	
169	1916.00	38.24									· - '
170 171	1917.00	38.84	* **			1.5	*				
172	1918.00	39.46									
173	1919.00	40+09								····	
<u> 174</u>	1920.00	40.71					1000				
175	1921.00 1922.00	41.31		·····							
176	1923.00	41-87				• .					
177	1924.00	42-41									1.
178	1925.00	42,90 43.36	·					**********			
179	1926-00	43.78					and the second				
180	1927-00	44.17		,							
181	1928.00	44.55						- 		· .	
182	1929.00	44.93		4							
183	1930,00	45.33									1.1
184	1931.00	45.76									
1 85	1932-00	46.22		,						1.	
1 86	1933-00	46,71						11	1 41		
187	1934.00	47.24									
188	1935.00	47-81					j.			*. *	
189	1936,00	48.42			•	****			100		
190	1937.00	49-06						. —			
191	1938-00	49-71		-						100	
192	1939-00	50-36_					4000			1.	
193	1940.00	51.00							• • • • • • • • • • • • • • • • • • • •		
194	1941-00	51.63					s**	e de la companya de			
195	1942.00	52.25							$\mathcal{I}_{(1)}(f,s)$		
196	1943.00	52-89					·				44 -
197 198_	1944.00	53.57	٠								A C
199	1945-00	54-32_	· · · · · · · · · · · · · · · · · · ·								
200	1946-00	55-15		-							
201	1547.00	56-02									
202	1948-00	56.91_									
203	1949.00 1950.00	57.75									
204		58.53									
	1951.00	59.20		<u>.</u>			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		· [25

LOS ANGELES COUNTY FLOOD CONTROL DISTRICT — HYDRAULIC DIVISION PAGE 8 INTERPOLATED ELEVATION—AREA TABULATION FOR PACGIMA DAM BASED UN 64-131 _____DATED. 08/04/83 SCALE_1"= ___10-0_FI. 1 SQ-IN= 1.000 UNITS

POINT ELEVATION	N AREA	
NUMBER (FEET)		BY JS 09/12/83
***** ******		51 65 67,12,63
205 1952.00		*
206 1953.00		
207 1954.00		
208 1955.00	61.55	
209 1956-00	62.24	
210 1957-00	62.96	
211 1958.00	63.64	
212 1959.00	64.24	
213 1960.00	64.69	
214 1961-00		
215 1962.00	65.10	
216 1563.00	65-18	
217 : 1964.00	65.28	
218 1965.00		
219 1966.00		
220 1967.00		
221 1968.00		
222 1969.00		
223 1970-00		
2.24 1971.00		
2.25 1972.00		
226 1973.00		
227 1974.00	68.42	
228 1575.00		
229 1976.00		
230 1977.00		
231 1978.00	74.96_	
232 1979.00		** * * * * * * * * * * * * * * * * * *
233 1980.00	78.74	
234 1981.00		randra de la companya de la company La companya de la co
235 1982.00		
236 1983.00	81.66	
237 1984,00		
238 1985.00		
239 1986.00		
240 1987.00	0 83.99	
241 2 1988-00		
242 1989.00		
243 1990.00	0 86-25	
244 1991.00		
245 1992.00		
246 1993.00		•
247 1994-00		*
248 1995-00		
249 1996-00		
250 1997.00		
251 1998-00		
252 1999.00	·· ·····	
253 2000-00		The second secon
254 2001-00		
2552002.00	95-07	

LOS ANGELES COUNTY FLCCD CONTROL DISTRICT - HYDRAULIC DIVISION PAGE 9
INTERPOLATED ELEVATION-AREA TABULATION FOR PAGOIMA DAM
EASED ON 64-131 ____DATED_08/04/83 SCALE_1"= ____10.0 FT. I SQ.IN= ____1.000 UNITS

POINT NUMBER	ELEVATION (FEET)	AREA (ACRES)	BY JS	09/12/83		· 1	<u>~</u>
256 257 258	2003.00 2004.00 2005.00	95.89 96.70 97.50	•				
259 260 261	2006-00 2007-00 2008-00	98•26 99•02 99•78					
262 263 264	2009.00 2010.00 2011.00	100.58 101.42 102.34			•		
265 266 267 268	2012-00 2013-00 2014-00 2015-00	103.31 104.32 105.37 106.43					<u> </u>
	2013.00	100.49				 	
-							
<u> </u>		-					
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1 of		100	18,	224:	275	276	27.B	. c	281.	281.	283.	284 284	285	2H6-	288.	2 # 8	289.	290.	292.	- 543	293.	296	296.	297	29B	298	300	301	302	303	304	
Page	£\$	50	18.	224.	269	270,	272	27.2	274.	275.	277.	277.	278,	279.	281,	282	282	283	285.	286	287.	- x - x - x - x - x - x - x - x - x - x	289.	240	291.	291	293	294	245	296	297.	
ď.	E, ALVE	u.	1.8. 8.8.	224.	242	263.	265 266.	344	267	268.	270.	271	271.	272.	274.	275	275.	276.	27.8	279	279.	280,	2H2 +	2#3	2H3.	284. 285.	286.	286	287. 288.	2 89	540 .	
	111	- °	18.	224.	256	255	256 257	a y c	259	260,	261	262	263,	263.	265	266.	267	267	269.	270	270.	271.	273.	276	274.	275	277.	- [27H	. 1	280.	
Valve	(11-01-10)	9	E E	224.	243	244.	24.5	27.7	248.	24B.	250.	757	251.	252.	254	254.	255.	256.	257.	25.8	259,	260.	261.	262		263.		245		.]		
Jet Va	1977 (75	. # E	224.	73.6	237	30	. 076	240.	241.	243.	243.	244.	245.	244.	247	24H.	249	250.	751	251.	252	254.	256	255.	256.	257.	25.8	•	. 1		•
	JANUARY	0,2	=======================================	224.	226	227 228	229	086	23.1	23.5	233.	234	234.	235	34.	237.		239	١.	24.1	241.	242	243.	244.		245	١.	24.7	24H	.		7
Hollow	u .	6.5	PH.	214:	215	216.	217 218.	2.0	١.		١.	222.			١.	225.		226.	228	228		230.	١.	231.		233	1	235.		3.5	37.	
30.	FFFFCTIV	\$0	# 46 # 86	١.	203	203	. 1 .	400	١.			209		210.	١.	212.		213		215		21.7.		218.		ે જ	<u>.</u>	21.				
ь <u>ө</u>	TABLE	5.5	18.	-	9.2	200	. .	4	١.	96.	١.	9.8			١.	201		202		204		205.	١.	207.		20H. 2		210. 2			12.	
Table	ATING	5.0	H. E.	E .	H	£3.	H2.		ž.	8.5°		46. 1	٠			a a		90.		92. 2		93. 2		95.	١.	96. 2		9.1.		.]	***	
Rating	* * * *	4.5	18.		65	165. 1	67.	241	6.A.	169, 1] 2	70. 1		171, 1	72.	173.	_	174	3.	75. 1	1	1./6.		7H 1	ļ .			НО.			-	
Ø	VALVE	4.0	18.		1 6 7	50.			١.	52.		53. 1	_	54.	١.	56.	l _	- C - C - C - C - C - C - C - C - C - C	١.	78. 1	١.		100	1. 1				11 : 29			·	
lve #	JET VA	3.5	18.	1.	132.	32. 1	1	137		135, 1		136 1		37. 1	17	× **	<u>ا</u> .	-`-	1.	140. 1	\ \ .		41.	71 - 69	Γ.		1.	44. 17	-		<u> </u>	
		30	1. 6.8.		4	114.	777	71.	1	117.	ł	18.		118.	1.	114	١.	20, 13	١.		١.	22. 14].	_]-		1=]_	
Dam Va	30" אטווטא	1GES)	1.8. 6.8.		96	96. 1	. .		L	98, 13	١.	11 .66		100, 11	1.	,	[•	١.	7. 121	-	٠,	1.	04. 12	l.	_	105. 12	5. 12	=:	12	1	
Paçoima	4 HI E	4	18.	}	77.	7H.		90	│.	96.		AO. 9		BO. 10].	. 100		82, 101]_	82. 102			H3. 103	93. 10	١.	. س	H4. 10	 				
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	2 KA	IAHILAIFO DISCHARGE HCENT VALVE OPFNING 10 15 20	18.1		341 5	00 00 00 00 00 00 00 00 00 00 00 00 00		· ·		40.		40.		. •].	1.4	⇃.	. •		24. 62].	, 63,	١.			5.				
	VALVF #	PENCENT 5 10	± 0 0		211. 3	20. 3										7			╿.	4.2				,	4.5			£ 7		42		
}	Z.			0.2	2	2 2	20,	30	5(202	~	507	20.	S.	20.	i.	. 2.	7.	~	~	21	7.	21.	Ċ	21.	71.	21.	-	12.5	22	22	
	PACOTMA DAM	RESERVOIR FLEVATION (FEET)	1451.00 1852.00	1854.00	H55.00	1856,00	1858 00 1859,00	00 0487	1 461.00	1863,00	864,00	1865,00	1866,00	1867,00	1469.00	00 07.81	1471 00	1872,00	1874.00	1875.00	1476.00	A77.00	1879,00	00 0881	HH1.00	18 H 2 , 00	1884.00	885.00	1 446.00	888,00 888,00	HH4.00	
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			o i	í c	c 6	307	308	309	310	310	312		11	314.	314	326	317.	317.	ž 3	320.	320	321.	322
£\$	36	201		1	200	300	301	302.	302.	303	305	1	305	306.	307.	30.8	308	310	311	312	313	314	314.
FOALVES		1000	ène y		292	203	293.	294	295	296.	297.		29.H	599.	299	301	302	302	304	304	305	306.	307.
123	- ;	080	• 111	25.2	283.	243	284.	285	2H5.	286.	- E E		ZHH	289.	290	791.	292	293	294.	295.	295.	296.	.162
EFFECTIVE JANUARY 1977 (01-07-77)	ş	268		270	270	271.	272.	273.	273.	274.	275.		276	277.	277.	279.			78.	282.	283.		
1977	3,	261		242	263	263	264.	265	265.	266.	797		26.8	249.	269	271.	271.	272	273.	274.	275.	275.	276.
ANUAR Y	5	250.	190	2,5	25.2	24.3	253.	254	755.	255.	257.		127	25H.	25H.	260.	260.	261.	262	263.	264.		265.
TIVF ÷	3	237.		23.8	239.	240	240.	24.1	242.	242.	243		2444	548.	245	246.	24.7	24A	276	249.	250.	251.	251.
EFFFG	9	224	, , , ,	225	226	226.	227.	227	228.	22H.	230		230-2	231.	231.	23.2	233	234	23.5	235	236.	P36.	247.
*** RATING TARLE	7	212		213	213.	214.	512	215	216.	216.	217		2.11	2) A.	2.19.	220.	221.	221.	222.	223.	223.	224.	224.
RATING	50	199		200	201	201	202	202	203	204	, i.e.		205	204.	207	207.	20.H	20B	209	210	210.	211.	211.
급 주 북	3	182.		183	184	1 # 4	1 A4 .	185	185.	186.	187		187	188.	* 5 % C	189	190	190	5 6	161	192	197.	143.
JET VALVE	07	164.	4	165	165	166	• yy :	167.	167.	168	16. 16.		169	169.	170.	1.10	1711.	171		173.	1 73	173.	1 74.
	3.5	145.	77.1	144	147	147.	147.	148	148.	148	149			150.	50	151	151	152		153	163	154.	154
₩UTTUM aŭa HUTTUM	å.	126.	, ,	127	127	127.	128.	12B.	12 A.	129.	129.		138	130.	130	131.	131	132.	13.7	132.	133	133,	33
1	25 %	106		107	107	107	107.	1044	108.	108	66	•	109	109.	110	110	110.	1111		===	112.	112.	112.
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2 RATING TAHLE	RCENT VALVE DPENING	7,4	3,	4.5	99	47.	66	67.4	.99	ęų.	\$66		999	66.	67.	67.0	67	67.	, 4 , 4	¥,	6.H.	6 H.	6н.
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M VALV	T. S.	22.	c	22	22	22	35.	22.	22.	22.	22.		22	22.	22,	22,	23	23		23,	, ,	7.3	23
PACHIMA HAM VALVE	RESERVITE FLEVATION	1889.00	00000	1891 00	1892.00	1843,00	1894,00	1895,00	1896,06	1897.00	1494.00		1900,00	1901.00	1402,00	1904.00	1905,00	1906.00	1907.00	1909,00	00.0191	1911.00	1912.00
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324.	325	326.	3273	327.	324	329.	330.	330	188	
317.	317	31H.	319	320.	320	321.	322	77	323	
309	310.	310.	-	312.	312	313.	314.	115	315.	
299.	300	300	301	302	302	303.	304	307	305	
286	287.	2HH.	2 R H -	289.	28.9	290	291	707	292	
27 B.	279.	279.	240	2H0.	281	2H2.	2H2.	283	244.	
267.	267.	268.	269	269.	270.	270.	271.	27.2	272.	
253	254.	254.	2.5	255.	256	257.	257.	25.H	258.	
234.	239	240.	240	241.	2414	242.	243	243	244.	
226.	224.	227.	227.	228.	229.	229.	230.	230	231.	
213	213.	214.	214.	715.	215.	216.	2).6.	217	217.	
1944	1950	195	196.	196.	196.	197.	197.	198.	19н.	
175	175.	176.	176.	177.	177.	177.) (H.	174.	1 79.	
155.	1561	156.	156.	156.	157	157.	15H.	158.	158.	
134.	135.	135	135,	136.	136.	136.	137.	137.	137.	
113,	11.3.	114.	1.4.	114,	114.	115.	115	115.	115.	
91.		26	1	92.	42.	92.	43	4.4	63.	
69	.64	, 64	64.	.69	70.	70.	٤	70.	70.	
44.	44.	46	44	e ly 19	47.	47.	47.	41.	1.1	
23.	23.	23.	7.3.	23.	23.	7.1.	23.	24.	24,	
1415.00	1916.00	1917.00	1918,00	1919,00	1920.00	1921.00	1422,00	1923,00	1924,00	

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3.0		100	331°	ત	C (2 2	C.	335	336.	# # # # # #	¥.	339	340	0 to 10 to 1	34.2	37.3	1	446	45.	· -	47	7 7 6	34.11	346	380	•							.	5 5,		S
Page	S:	96	323,	324	325	375	327.	328	32H.	926	330	331	332		334.		335	936	15	•	HEE	* O C C	340	341	342				•				e			
ď	F.ALVE	g.	315.	316	317.	317.	319.	319	320.	321	322.	323	324.	324	326.	766	327	328	329		055	330	332	332	333				-							
	171	8.5	305.	300	307.	307	309.	1			١.	313	313.	314.	315.		3,75	317.	318.	•	319	320.	321.	322.	322	•										
	11-10-101	90	92.	ŀ		200				297	١.	99	١.	300	Ι.		707		Ι.				307.		309											;
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30.	,	90	544	244	245	245	244	247	747	4. 4. C.	5.49	250	250	251	252		25.2	253	447		255	255.	256.	25.7	257.	٠.									•	
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#5	JFT V	35	5H. 1	, 6		90	140.	, 1	. [4	161.	1,62	, ,	79	163, 1				240	4	n C	65.	66. 1	166.				,									
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	F # 2	TABILATED DISCHARGE ICESS PERCENT VALVE UPENING 5 10 15 20 26	47.	47.	47.		÷ ;	14.7	=	4.X.	¥ .		¥ ₹	4.H.	3	•	407	o 5	405	6.6	07	4.0	4.04	į ģ	. v											-
	VAI.V	P.F.R.	24.	24.	24.	2.5	7.	7.0	,	74.	2,2		2,4,2	74.	7/2	.	2.5	24	2.4.	52		25.	ξ.											-		
÷	PACOTHA DAM VALVE	ERVILLE VATION (FEFT)	00	0.0	υu	90	e o		5	9	5 6	S			9	E	00	<u> </u>	9	9	6	g	Ç		: :											
•	PACOI	RESELVOIR FLEVATION (FEFT)	1924,00	1925,00	1926.00	1927.00	1429.00	1930 00	1931.00	1932,00	1434.00		00.45.91	1937.00	OC WEST	1434	1940,00	1941,00	194,3 00	1944,00	00 3701	1946.00	1947.00	1944 00		חוס חבה ו										
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Pacoima Dam Valve #3 Rating Table - 30" Hollow Jet Valve

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ES		\$	25	, H	157.	226.	22H	230.	231.	232	234		236.	23.4	240.	24.1	24.2	244	244.	24.7 24.8	24.9	nc)	25.1	253	254	633.	256.	25 H	259	L'IIC)
F. ALVES		06	25.	78	157. 219.	220.	222	224.	225.	226	22K.		230.	232	233°.	235	236	23H.	240.	241.	243	;	245	246.	247.		249.	251	252	
(22-20-10)		æ 90	0.	48	157. 212.	213.	215	217.	218.	219.	220.		222.	224	225.	227.	22H.	231.	232.	233.	235	730.	236.	234	234.		241.	263	744.	17.19
	:	£	25.	1	202.	203	502	207	20B	209	211.		213.	2.46	215.	217.	218	221.	222.	223	224	5	226.	22B	229	.,,,	230.	232	233.	e E C y
		75	5.0	7	157.	197.	555	201.	202	203	204		204.	2011	210.	71.7	212.	213.	215.	216.	214.	* T	219	22.1	222.	•(//	273	225	226.	•
		07.	°, %	1	157.	149.	55	192.	193	194.	196.		19H.	357	200.	202	203	204.	206.	207.	209.	50.	210	212	213.	7.	214.	215.	217.	
		6.9	25.	THE STATE OF THE S	15%	179. 140.	181	1H2.	143.	185.	186.		187.	149	190.	100	192	194.	105	196.	198	841	199	201	201	202	203	202	205	(3)0.
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l		\$	° 4.	7.1	135. 136.	137.	138	134.	140.	7 <u>7</u> 7	142.	• 31	143	144	145.	1771	44	148.	149.	150.	151.	152.	152.	153.	154.	154.	155.	156.	157.	2
		04	25.0	18	122.	123,	124	175.	126.	126.	128.		129	130	130.	123	132	133.	134	135	136.	136.	137	137. 138.	134.	139.	139.	140	141.	-4%
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	RESFRYILE	ELEVATION (PEET)	1850.00	1852.00	1853.00 1854.00	1855,00	1452.00	1858.00 1859.00	1860,00	1861,000	1863.00	1804+00	1865.00	1867.00	1869.00		1871.00	1473,00	1875.00	1876.00	1474.00) H/Y.00	1840.00	1441 1442 00	1883.00] ни4.00	1885.00	1866.00	1888.00	1849.00
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MA DA	PACHIMA NAM VALVE	*	3 RATING TABLE		30"	30" HILLIIM	.HF.T	VALVE	新公安	RATING	HATING TABLE	EFFEGT1VE	1	IANITARY	1411	11-10-101	_	E ALVE	F.S	
RESERVITE) —	AMII AT	STO CE	TAMILATED DISCHARGE	(CFS)		-													
FLEVATION (FEET)	P.F.R.	CENT V	ALVE O	PERCENT VALVE OPFNING 5 10 15 20	1	30	35	Û\$	£ *	'n	55	90	ţ	e.	33	OH .	- ₹	0.6	\$6	100
1889.00	19.	37.	55.	73.	91.	104.	125.	142.	157.	172.	183.	194.	206.	21%	227.	234.	245.	253.	260.	266.
1840.00	91	37.	56.	74.	91,	100	126	1424	158	173.	1844	195	207	21He	22 He	2354	2454	254	747	267
1891.00	-6-	37.	5. 5.	74.	92.	109.	126.	143.	158.	174.	. 85	195	207.		228 220	235	246.	255	261.	26 E
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1894.00	61	, , , , ,	26.	75.	33.		127.	1.4.5	140.	1.16	187.	197.	210	221.	231	238.	249.	257.	264.	271.
90 407	2	2	5.7	7.5	0.3		124.	145.	161.	176.	147.	198.	210.	222.	23.5	239.	250.	258.	265.	272
1896.00	5	3H.	2.5	75.	6.6	111	128	145.	141	177.	188.	661	211.	223	232	240.		259	266.	273.
1897.00	2	æ. Æ.		75.	• 76	111	129.	146.	162.	177	. 68.	200	212.	224.	233	240	251	260	267	274.
494,00	2 2	3 X	57.5	9,9	76	112	130	147.	163.		190.	201.	213.	225.	235	242.	253.	262.	269.	275.
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1900,00	5 3	, S.	2 4	97.	977		131	14.9	166	180	161	20%	215.	221.	236.	244.	255.	264.	2711.	277.
00.7050	7 7	, de	 			13.	131.	14 H.	164	180.	192.	203	215	227.	237.	244.	256.	264.	271.	27H.
403.00	÷	36	λ,	17.	96	114	171	14.1	557	- E	255	702	45.5	22.H.Z.	4 c	2472	12.5	7	2,2	2.40 2.40
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1906,00	20.	34.	54.	7.B.	97.	115.	G. 1	50.	167		•	\$ 200		231	170	0 H 7 C	260	26.0	276.	2.8.3
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00 Hub 0				, H		9	ጎ (ጣ	151	16.8	, ¥	196.	207	220	232.	543	250.	262.	270	274.	285
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1921.00	21.	41.	61.	H 1.	101	120.	139,	157,	174.	191.	203.	213	228	24.7	262	260	272.	281	2 H Y	296
1422.00	2.5	41.	62.	82°	101	121.	139.	157	175	142	25.	2 2	230	247	253	2614		. 5	290	297
00	7	4 1 5	23	#2 #3	107	127	1607	158.	1,4,1	163	205	217	230	243,	254.	262,	274.	283	5.00	298 •
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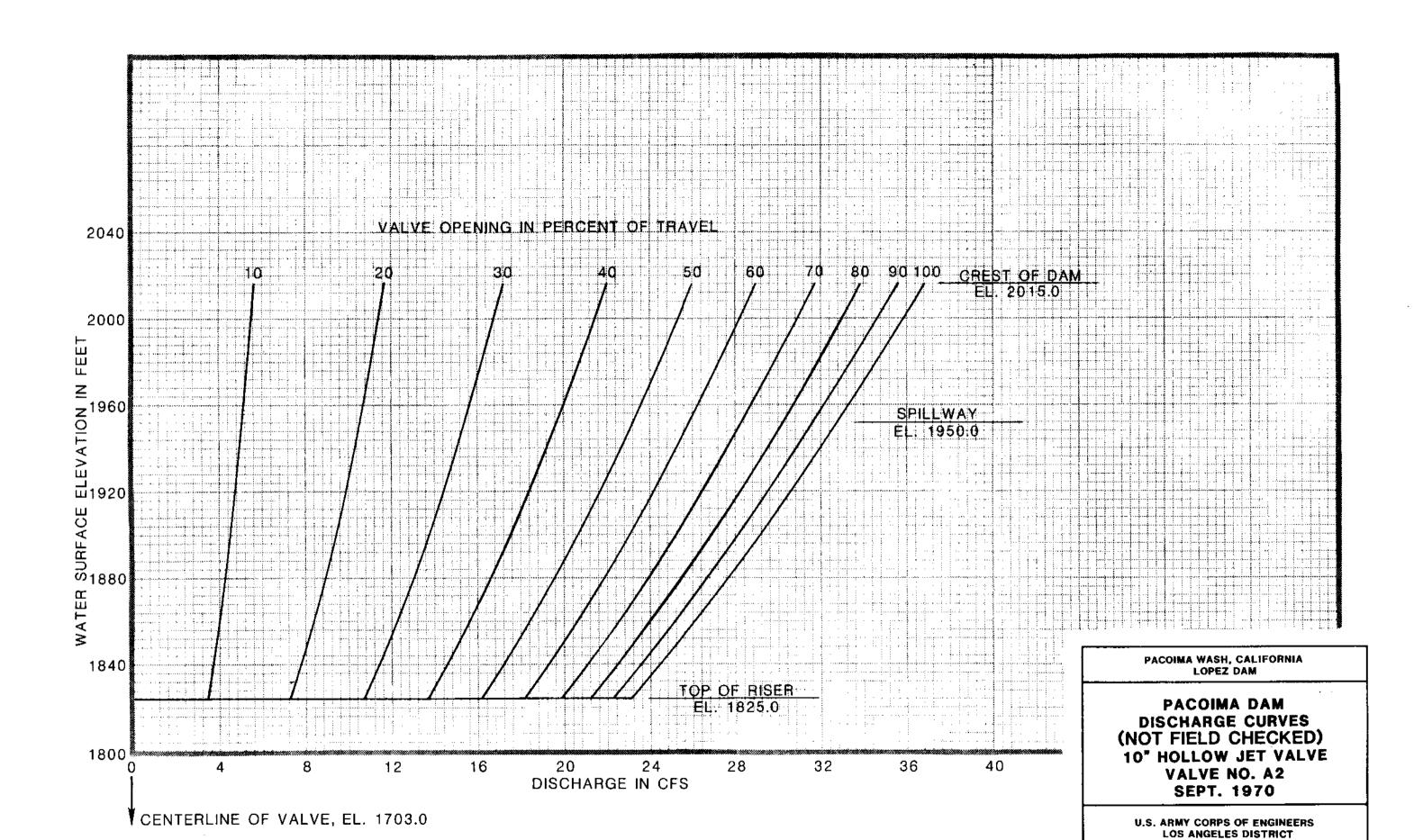
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Jet Valve	1477		75	\$98	254	255.	256,	257.	25B	259.	259	241.	262.	262.	263.	266		265	266.	267	268.	269-	269.	270.	271,	212									
	IANUARY.		70	243.	244	245.	245.	24.7.	24.7	24.H.	949	250	251.	251.	252	2,3		254	522	, 55 25 25	25%	25.7	25H.	25.4 25.4 25.4 25.4 25.4 26.4 26.4 26.4 26.4 26.4 26.4 26.4 26	240.	761									
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	FFFECTIVE		90	217.	218	218.	230.	220.	221.	221.	222.	223.	224.			1		227	227.	229	229.	230		237.	232.	233		ŀ							
- 30	TAHLF		, 85 7	205.	2064	506.	207	20H.	209.	209	210.	211.	212.	212.	213.	1		Ì		216				218. 219.		220			· .						
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	*		4.5	176.	1764	177.	177. 178.	13.	179.	179.	180.	181	181	١.				1		185.		186	187.		١.	189									
Rating	VALVE *		04	158,	159	159.	160.	1.61	161.		162	١.	163.			165		165		167.	1	168			١.	20									
#3	JET VA		3.5	140.	141	141.	141.		42	143.	143.		. 44.	١.		44			• 1		.	484		64		50					-		2		
/alve	30" HULLOW		30	121.	122	122.	122.	123.	123.	124.	124.	125.	. 25.	125	126.	126.	•	-27-	127.	128	128,	128	129.	200		30.					٠.				
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	VALVÉ	ĪΔ	PEKCFNT 5 10	21.	21.	21.	21.	 	21.	21,	23.	21.	21.5	21.	22.	22.		22	22.		72.	22.	22.	22.	22.	22									
	PACOINA DAM VALVE	HESERVILL	FLEVATION	1924.00	1925,00	1926,00	1927.00	1929.00	1930.00	1931,00	1932,00	1934.00	1935.00	1936.00	1937.00	1938 00		1940.00	00.1441	1942.00	1944.00	1965,00		1947,00		1950.00	•								
$\frac{\mathbf{V}_{i}^{(i)}}{2\pi}$	ŧ	†		l		ŀ		ĭ		ł		į		l		İ		l		٠	L		1		l	l		į		ı		l		1	5. 12.

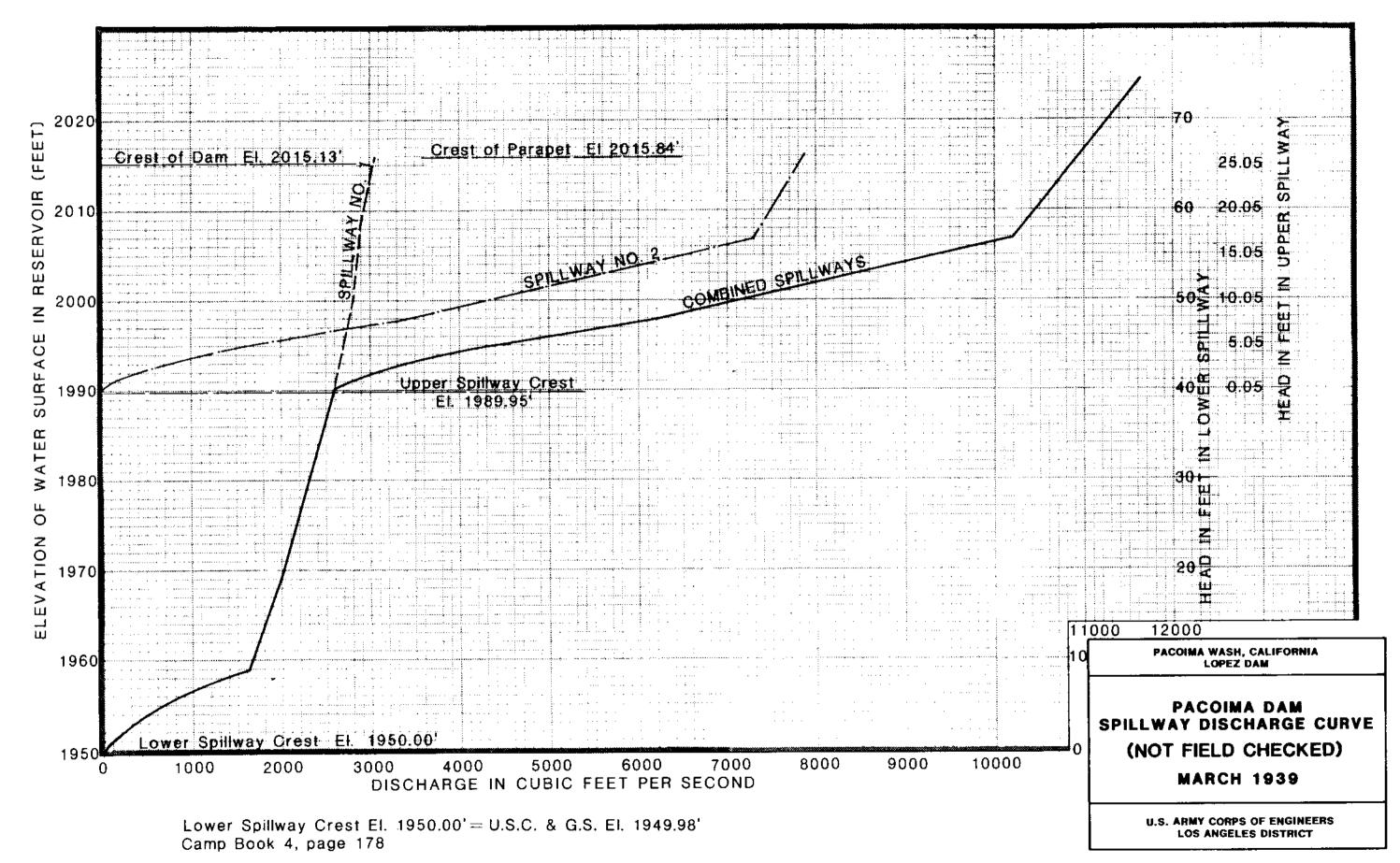
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Pacoima Dam Valve #4 Rating Table - 30' Hollow Jet Valve

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	S.	56	2116	212.	214.	215.	217.	218.	219.	220.	223.	224	225.	224.	22.	224.	230,	282	234.	235.	5 34.	237	234.	240.	241.	243.	244.	245	244.	24 H	546	
-1	F. ALVES	6	206+	207	208	209	211.	212.	213	215.	217.	218	219.	250	222,	223.	224.	225	227.	229.	230	231	233	234.	235	237	23A.	239	240	242	243	•,
	-17	- 5#	199	200	201	202	204	205	206	207. 204.	209	210.5	212.	213,	215	216.	217.	219	2204	221.	222.	223	225	22.6.	227	229.	230,	737	232	2.2	234.	
	(01-01-11)	HO.	190.	191	192	193	195	196	197,	. HG 1	200	201	202	203	205.	206	207	20 K	210.	211.	212.	213.	215.	215	216.	21 K.	219.	220	221.	223	224.	
- 1	161	75	184.	185	186.	1 H / •	189.	190	191.	192	196.	195	196.	197.	199.	200.	201	202	203	204.	505	206	20H	209	210.	2.2	212.	213	214.	216.	2) 7,	
	RATING TARLE FFFECTIVE JANUARY	07.	176.	177	178	179	181	182	143	1 H 4	166.	187	187.	188.	190.	191.	192.	143	195	196	197	107	199	200	201.	200	203	2014	205	206.	20H+	
	TIVE.	54	167.	167.	168.	170.	171.	172.	173.	174.	1764	176.	177.	178	140	181	182.	1 x 3	1.84.	145.	18.6	187	188.	24.	190.	25	193.	193	194.	195.	197.	
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	RATING	5.0	139.	140.	141.	147.	143.	144.	144.	145	147.	147	14н.	146.	150	151.	152.	152.	154.	155.	155.	156.	157	158	150	159.	161.	1614	162.	163.	164.	
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	VALVE	64	114:	114.	115.	116.	117.	118	118.	911	120.	121.	121.	122.	123	124.	124.	125.	126.	127.	127.	12H.	129.	124	130.	130	132.	132.	133.	133	134.	٠.,
- 1	7. V	*	101	101	102.	107	103.	104.	104	105.	104.	107.	107) OH.	100	109.	110.	110	Ē	112	<u>-</u>	113.		114.	115.		116.	117.	117.	æ 3	119.	
- 1	## L OY	30	H7.	H 7.	88	£ 7	74	90	90	6 6	92.	92.	6.6	63.	76	96	95	95.	9.4	٠, ٥	0.7	E o	H G	à	66	000	101	101	101	102.	103.	
	- 30" HIE.	1	73.	7.1.	74.	7.4	75.	75.	76.	76.	77.	77.	7.	7.H.	:	79.	BO.	C. G	=	ā	#2.	. B.2	#3.	30	H.3.	# # # # # # # # # # # # # # # # # # #	84.	x Y	85.	E E	H.	
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	VE #4 RATING TABLE -	PERCENT VALVE OPENING 5 10 15 20	30.	30.	30.	ç ç	30.	· ·	-	<u>.</u> .	3.1.		32.	800	32.	20	32.			, 7	33	37.		76	34.	34.	34.	34.	35.	10 10 10 10 10 10 10 10 10 10 10 10 10	5	•
	4 VALVI	PEKO	15.	15,	-	٠. د	2	-	2	4.	14.	1.4	16.	4.	19.	4	9	.] 6	16.	,		<u>'</u>			17.	7.	<u>:</u>	1.1	17.		T.	
	PACOLHA DAM VALVE #4 RATING TAHLF	FLEVATION (FEFT)	1889.00	00.0980	1891.00	1892.00	1494.00	00 3580	1896.00	1897,00	1849.00	00 0001	1901.00	1902.00	1404,00	00 9001	1906.00	1907.00	1909,00	0000	1911-00	1912,00	1914.00	90	1916.00	1917,00	1919.00	00 0001	1421.00	1922.00	1424.00	
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		Ω.,	Pacoima		Dam	Valv	# 4		Rating	Table	ŧ	30. H	Hollow	v Jet	Valve	•		Pag	ဇ	of 3
PACOLMA DAM	>	# +# H	RATING TABLE	TABLE	1.	שטיי אחנות	W JFT V	VALVE	**	RATING	TABLE	1	EFFECTIVE .	JANIJARY	7 1977	101-07	127-70-	E, ALVE	/F.S	
RESERVILH FLFVATION (FFET)		ARILLAI PENI V 10	15 015	TABILLATED DISCHARGE PERCENT VALVE OPENING 5 10 15 20	LGES 25	30	3.5	4.0	4.5	0.9	5.8	99	99	0.2	31	F.O.	- *	0.5	56	100
424.00	18.	35.	52.	.69	966	103	119,	134.	149	164.	175.	185.	197.	20H.	217.	224.	234.	243	244.	256.
1925.00	18	3,6	63	70.	A7.	103	119.	135	150	165	175	186.	197.	209	218	225.	235	244	250	257.
1926.00	61	भ । (* त	en e	0.0	oc a	103	120	135	151	165.	176.	186.	198	209	200	226.	236.	245	252.	259.
928.00		36	53	71.	X	10.6	121	: 2	152	167		188	200	172	220	227	;뒤	24.7	: 1	260
1429.00	18.	34.	53.	711.	8.8.	105.	121.	137.	152.	167.	17H•	189.	200	212.	221.	228.	239.	247.	254.	261.
0.00	Į.	36	5.4	7.	B H B	105	122	138	153	168		189	201	213	222	229	260	268	चंदर	242
1931.00	ж. •	36.	54.	71.	89	106.	122.	134.	154.	164.	1#0 20	190.	202	213.	223.	230.	241.	24.9	256.	764
1932,00	.	e d	7.	2.0	* J * J * X	900	123.	139	15.6	170.	181	192	204	212	222	232	24.32	₹₫	: #	245
1934.00	- H	34.	54.	72.	ŝ	107.	123.	140.	155.	171.	182.	192.	204.	216.	225.	233*	244.	252	259.	266.
00 9801	3	2.7	r.	7.3	90	107.	124.	140.	156.	171	182	193	205	217.	226	233	245	253	260	267.
1936.00	4	3.	25.	1.	66	10.8	124.	141.	157.	172.	183.	194.	206.	21H+	227	234.	245.	254	261.	268.
1937,00	<u> </u>	. 60	55.		91.	10H•		7	157	2	184.	194	207.	21H.	22A	235	246.	255	262.	269.
1939,00	51	<u>.</u>	56.	73	9 3	1001	2 2	142	15 H.	174.	185	196.	20H	220.	230	237.	248.	257.	264.	271.
	į			ì	ć		. 40	17.3	9	174	4 1	147	200	221.	230.	238.	2692	258	265	277
000 - 70	7 9		44	77.	0.0	001	127.	143.	159.	175.	186	197.	210.	221.	731.	239.	250.	259.	266.	273.
1942,00	6		56.	7,4	95	110	127.	(144.	160	176.	187.	# 5 T	210.	222.	232	239.	251.	260.	267	274.
00.2701	0 3	4 K	5,6	4.4.	93	9:	127	145.	191	177.	1.8H	199	212	224.	35.	241:	253.	261.	269.	276.
	:	•		•	•						,	4		6	4	,	963	24.2	270	277
1945-00	7	3 % 6	25	75	93.		128	145	797	1 1	180	20.	213	22.5	235	243	254.	263		277.
1947.00	5	 		\$	94.	112.	129.	146.	163	179.	190.	201.	214.	226.	236	244.	255	264	271.	27K
1948-00	9.	3H.	2 2	49.	95.		7-	12.	184	140.	191	303.	25.	22±.	238.	245.	257,	264	273,	280.
		. =	. u	7.	ď	113	17	14.8.	164.	180.	192.	203.	216	22H.	239	246	258	267	274	787
77.00		100										L					L		•	
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	DI	SCHA	RGE	IN CU	BIC F	EET F	PER S	ECON	D	
Gage Height (feet)	0.00	0.01	0.02	0.03	0.04	0.05	0.06	0.97	0.08	0.09
.0	0	0.6	1.2	1.8	2.4	3.0	3.6	4.2	4.8	5.4
.1	6.0	6.7		8.1	8.8	9.5	10.2	10.9	11.6	12.3
.2	13.0	13.7 ;	14.4	15.1	15.8	16.5	17.2	17.9	18.6	19.3 27.2
.3	20.0	20.8 : 28.8	21.6 29.6	22.4 30.4	23.2 31.2	24.0 32.0	24.8 32.8	25.6 33.6	26. 4 34.4	35.2
	36.0	36.8	37.7	38.5	39.4	40.2	41.1	41.9	42.8	43.6
.5 .6 .7	44.5	45.3	46.2	47.0	47.9	48.7	49.6	50.4	51.3	52.1
.7	53.0	53.9	54.8	5 5.7	56.6	57.5	58.4	59.3	60.2	61.1
.8	62.0	62.9	63.8	64.7	65.6	66.5	67.4	68.3	69.2	70.1
.9	71.0	71.9	72.9	73.8	74.8	75.7	76.7 86.2	77.6 87.1	78.6 88.1	79.5 89.0
1.0 1.1	80.5 90.0	81.4 91.0	82.4 92.0	83.3 93.0	84.3 94.0	85.2 95.0	96.0	97.0	99.0	99.0
1.2	100.0	101.0	102.0	103.0	104.0	105.0	106.0	107.0	108.0	109.0
1.3	110.0	111.0	112.0	113.0	114.0	115.0	116.0	117.0	118.0	119.0
1.4	120.0	121.0	122.0	123.0	124.0	125.0	126.0	127.0	128.0	129.0
1.5	130.0	131.1	132.2	133.3	134.4	135.5	136.6	137.7	138.8 149.8	139. 9 150.9
1.6	141.0 152.0	142.1 153.0	143.2 154.0	144.3 155.0	145.4 156.0	146.5 157.0	147.6 158.0	148.7 159.0	160.0	161.0
1.7 1.8	162.0	163.1	164.2	165.3	166.4	167.5	168.6	169.7	170.8	171.9
	173.0	174.1	175.2	176.3	177.4		179.6	130.7	181.8	132.9
2.0	184.0	185.3	186.6	187.9	189.2	190.5	191.8	193.1	194.4	195.7
2.1	197.0	198.3	199.6	200.9	202.2	203.5	204.8	206.1	207.4	208.7
2.2	210.0	211.3	212.6	213.9	215.2	216.5	217.8 231.4	219.1 232.8	220.4 234.2	221.7
	223.0 237.0	224.4	225.8 240.0	227.2	228.6 243.0	230.0 244.5	246.0	247.5	249.0	250.5
2.4	252.0	253.6	255.2	256.8	258.4	260.0	261.6	263.2	264.8	266.4
2.6	268.0	269.7	271.4	273.1	274.8	276.5	278.2	279.9	281.6	283.3
2.7	285.0	286.7	288.4	290.1	291.8	293.5	295.2	296.9	298.6	300.3
2.8	302.0	303.7	305.4	307.1	308.8	310.5	312.2	313.9	315.6	317.3 334.3
	319.0	320.7 337.8	322.4 339.6	324.1 341.4	325.8 343.2	327.5 345.0	329.2 346.8	330.9 348.5	332.6 350.4	352.2
3.0 3.1	354.0	355.8	357.6	359.4	361.2	363.0	364.8	366.6	368.4	370.2
3.2	372.0	373.9	375.8	377.7	379.6	381.5	383.4	385.3	387.2	389.1
3.3	391.0	392.9	394.8	396.7	398.6	400.5	402.4	404.3	406.2	408.1
3.4	410.0	412.0	414.0	416.0	418.0	420.0 440.0	422.0	424.0 444.0	426.0 446.0	428.0 448.0
3.5	430.0	432.0 452.0	434.0 454.0	436.0 456.0	438.0 458.0	460.0	442.0 462.0	464.0	466.0	468.0
3.6 3.7	450.0 470.0	472.0	474.0	476.0	478.0	480.0	482.0	484.0	496.0	488.0
3.8	490.0	492.1	494.2	496.3	498.4	500.5	502.6	504.7	506.8	508.9
3.9	511.0	513.1	515.2	517.3	519.4	521.5	523.6	525.7	527.8	529.9
4.0	532.0	534.2	536.4	538.6	540.8	543.0	545.2 567.8	547.4 570.1	549.6 572.4	551.8 574.7
4.1	554.0	556.3	558.6	560.9	563.2 586.2	565.5 588.5	590.8	593.1	595.4	597.7
4.2 4.3	577.0 600.0	579.3 602.3	581.6 604.6	583.9 606.9	639.2	611.5	613.8	616.1	618.4	620.7
4.4	623.0	625.3	627.6	629.9	632.2	634.5	636.8	639.1	541 <u>.1</u>	643 <u>.7</u>
4.5	646.0	648.3	650.6	652.9	655.2	657.5	659.8	662.1		666.7
4.6	669.0	671.4	673.8	676.2	678.6	681.0	683.4	685.8	688.2	690.6 714.6
4.7	693.0	695.4	697.8	700.2	702.6 726.6	705.0 72 9. 0	707.4 731.4	709.8 733.8	712.2 736.2	738.6
4.8 4.9	717.0 741.0	719.4 743.4	721.8 745.8	724.2	750.6	753.0	755.4	757.8	760.2	762.6
5.0	765.0	767.4	769.8	772.2	774.6	777.0	779.4	781.8	784.2	786.6
5.1	789.0	791.4	793.8	796.2	798.6	801.0	803.4	805.8	808.2	810.6
5.2	813.0	815.4	817.8	820.2	822.6	825.0	827.4	829.8	832.2	834.6
5.3	837.0	839.5	842.0	844.5	847.0	849.5	852.0	854.5 879.5	857.0 882.0	859.5 884.5
5.4	862.0	864.5	867.0 892.0	869.5 894.5	872.0 897.0	874.5 899.5	902.0	904.5	907.0	909.5
5.5 5.6	887.0	889.5 914.6	917.2	919.8	922.4	925.0	927.6	930.2	932.8	935.4
5.7	938.0	940.6	943.2	945.8	948.4	951.0	953.6	956.2	958.8	961.4
5.8	964.0	966.6	969.2	971.8	974.4	977.0	979.6	982.2	984.8	987.4
5.9	990.0	992.6	995.2	997.8	1000.4	1003.0	1005.6	1008.2	1010.8	1013.4
6.0	1016.0	1018.6	1021.2	1023.8	1026.4	1029.0	1031.6 1057.6	1034.2 1060.2	1036.8 1062.8	1039.4 1065.4
6.1	1042.0	1044.6	1047.2 1073.2	1049.8	1052.4	1055.0 1081.0	1083.6	1086.2	1088.8	1091.4
6.2 6.3	1068.0	1070.5 1096.6	1073.2	1101.8	1104.4	1107.0	1109.6	1112.2	1114.8	1117.4
0.2	1120.0	1122.6	1125.2	1127.8	1130.4	1133.0	1135.6	1138.2	1140.8	1143.4

Note: Rating Table provided by LACDPW.

Pacoima Creek Flume below Pacoima Dam,
Sta. No. F118B-R, Rating Curve 44I.

Does not include Spillway Flow

PACOIMA WASH, CALIFORNIA LOPEZ DAM

PACOIMA CREEK FLUME BELOW PACOIMA DAM RATING TABLE

U.S. ARMY CORPS OF ENGINEERS LOS ANGELES DISTRICT

WATER CONTROL MANUAL LOPEZ DAM

EXHIBIT B
PERTINENT DATA FOR OTHER
RESERVOIRS AFFECTING LOS ANGELES RIVER

EXHIBIT B PERTINENT DATA FOR OTHER RESERVOIRS AFFECTING LOS ANGELES RIVER

Item	Contents	Page
1 2	Sepulveda Dam and ReservoirPertinent Data Hansen Dam and ReservoirPertinent Data	B-1 B-3

SEPULVEDA DAM AND RESERVOIR LOS ANGELES COUNTY, CALIFORNIA

PERTINENT DATA May 1985

Stream system	Los Angeles River
Drainage area	152
Reservoir:	·
Elevation	
Top of spillway gates (in raised position)ft., NGVD	710
Flood control poolft., NGVD.	710
Snillway design sunchange lovel	713.5
Spillway design surcharge levelft., NGVD	716.7
Top of damft., NGVD	725+
Spillway gates basin to automatically lower.ft., NGVD	712.0
Spillway gates complete automatic loweringft., NGVD	715.0
Area	* 4
Spillway gates (in raised position)acres	1,335
Flood control poolacres	1,529
Spillway crestacres	- •
Spillway design surcharge levelacres	~ 765
Top of damacres	
Capacity, gross	2,447
Spillway gates (in raised position)acre-feet	17,425 (2.15*)
Flood control poolacre-feet	22,493 (2.77*)
Spillway crestacre-feet	6,857 (0.85*)
Spillway design surcharge levelacre-feet	27,563 (3.40*)
Top of damacre-feet	44,727 (5.52*)
Allowance for sediment (50-year)acre-feet	9
Dam:	
Type	Earthfill
Height above original streambedft	
Top lengthft	57
Top widthft	15,440
Freehoard	_30
Freeboardft	7.4
Spillway:	
Type	Concrete ogee
Crest lengthft	399
Crest elevationft., NGVD	700
Design surchargeft	17.6
Design dischargec.f.s	108,900
Outlets:	100,900
Uncontrolled	
Size	h 6 m - 6 5 m
Entrance invert elevationft., NGVD	4 - 6'W x 6.5'H
Controlled	668
Controllednumber	4
Sizeft	6'W x 9'H
Gate type	Vertical lift
Entrance invert elevationft., NGVD	668

SEPULVEDA DAM AND RESERVOIR LOS ANGELES COUNTY, CALIFORNIA

PERTINENT DATA (continued) May 1985

Conduits - (Rectangular) Ungated Gated Length	40
Maximum capacity at spillway crest	16,500
Standard project flood:	16,500
Duration (inflow)days	3
Total volume (including base flow)acre-feet	68,200 (8.41*)
Inflow peakc.f.s	50,000
Probable maximum flood:	31,000
Th	
Duration (inflow)days	14
Duration (inflow)days Total volumeacre-feet	·
Total volumeacre-feet	163,200 (20.13*)
Duration (inflow)days Total volumeacre-feet. Inflow peakc.f.s Historic maximums:	·
Total volumeacre-feet Inflow peakc.f.s Historic maximums:	163,200 (20.13*) 114,000
Total volumeacre-feet. Inflow peakc.f.s. Historic maximums: Maximum inflowc.f.s.	163,200 (20.13*) 114,000 58,970
Total volume	163,200 (20.13*) 114,000 58,970 2-16-80
Total volume	163,200 (20.13*) 114,000 58,970 2-16-80 15,100
Total volume	163,200 (20.13*) 114,000 58,970 2-16-80 15,100 2-16-80
Total volume	163,200 (20.13*) 114,000 58,970 2-16-80 15,100

^{*} inches of runoff

⁺ December 1980 survey shows variation in elevation of top of dam from 723.7 feet northeast of Control House to 725.5 feet southwest of Control House.

HANSEN DAM AND RESERVOIR LOS ANGELES COUNTY, CALIFORNIA

PERTINENT DATA June 1984

	the state of the s
Stream system	Tujunga Wash 147.4
Reservoir:	17.47
Elevation	1 010 5
Debris poolft., NGVD	1,010.5
Flood control pool (spillway crest)ft., NGVD	1,060
Spillway design surcharge levelft., NGVD	1,081.22
Top of damft., NGVD	1,087
Area	
Debris poolacres	142.4
Spillway crestacres	781.4
Spillway design surcharge levelacres	1,061.5
Top of damacres	1,136.0
Capacity, gross	
Debris poolacre-feet	1,329 (0.17*)
Spillway crestacre-feet	25,446.1 (3.24*)
Spillway design surcharge levelacre-feet	44,990 (5.72*)
Top of damacre-feet	51,360 (6.53*)
Allowance for sediment (50-year)acre-feet	10,500 (1.34*)
	21,000 (2.67*)
Allowance for sediment (100-year)acre-feet	21,000 (2.07*)
Dam:	m44.0333
Type	Earthfill
Height above original streambedft	97
Top lengthft	10,475
Top widthft	30
Freeboardft	5.8
Spillway:	Overflow conc
Type	Ungated ogee
Crest lengthft	284
Design surchargeft	21.22
Design dischargec.f.s	99,700
Outlets:	
Uncontrolled	
Number and sizeft	2 - 8'W x 6'H
Entrance invert elevationft., NGVD	1,011
Controlled	1,011
	Vertical lift
Gate type	8 - 5'W x 8'H
Sizeft	
Entrance invert elevationft., NGVD	990
Conduitsnumber	10
Size	$2 - 8'W \times 6'H$
	8 - 5'W x 8'H
Lengthft	265
Maximum capacity at spillway crestc.f.s	22,000
Regulated capacity at spillway crestc.f.s	22,000

HANSEN DAM AND RESERVOIR LOS ANGELES COUNTY, CALIFORNIA

PERTINENT DATA (continued) June 1984

Standard project flood: Duration (inflow)days	14
Total volumeacre-feet	92,500 (11.77*)
Inflow peakc.f.s	53,000
Probable maximum flood:	33,000
Duration (inflow)days	1
Total volumeacre-feet.	76,800 (9.77*)
Inflow peakc.f.s	130,000
Historic maximums:	: - ·
Maximum releasec.f.s.	12,371
Date	3-3-83
Maximum water surface elevationft., NGVD	1,039.70
Date	3-2-83
	J = 9 J

^{*} inches of runoff

WATER CONTROL MANUAL LOPEZ DAM

EXHIBIT C

LOPEZ DAM RESERVOIR

REGULATION SCHEDULE

AND

DAM OPERATOR INSTRUCTIONS

EXHIBIT C

LOPEZ DAM RESERVOIR REGULATION SCHEDULE

Step	When Reservoir Water Surface is Between Elevations (feet - NGVD)	Gate Setting (feet of opening)	Computed Outlet Discharge (cfs)	Spillway Discharge (cfs)
1	1,254-1,273	5.0	0-422	0
2	1,273-1,299	0	0	0-44,500

NOTES: Spillway Crest Elevation 1,273 ft, NGVD Top of Dam Elevation 1,299 ft, NGVD

Follow Step 1 at all times when there is no spillway flow

Follow Step 2 at all times during spillway flow

DAM OPERATOR INSTRUCTIONS

- 1. Communication with the District Office is available.
 - a. Notify the Reservoir Operations Center when a gate change will be required according to the schedule.
 - b. Notify the Reservoir Operations Center if unable to set the gate as instructed.
- 2. Communication with the District Office is not available.
 - a. Try to reestablish communication through the Los Angeles County Department of Public Works.
 - b. Allow a period of one half hour to pass to reestablish communication with the District Office. If after one half hour communication is not reestablished, follow the gate operation schedule.