

Measurements of Reynolds stress profiles in unstratified tidal flow

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Abstract. In this paper we present a method for measuring profiles of turbulence quantities using a broadband acoustic doppler current profiler (ADCP). The method follows previous work on the continental shelf and extends the analysis to develop estimates of the errors associated with the estimation methods. ADCP data was collected in an unstratified channel and the results of the analysis are compared to theory. This comparison shows that the method provides an estimate of the Reynolds stresses, which is unbiased by Doppler noise, and an estimate of the turbulent kinetic energy (**TKE**) which is biased by an amount proportional to the Doppler noise. The noise in each of these quantities as well as the bias in the TKE match well with the theoretical values produced by the error analysis. The quantification of profiles of Reynolds stresses simultaneous with the measurement of mean velocity profiles allows for extensive analysis of the turbulence of the flow. In this paper, we examine the relation between the turbulence and the mean flow through the calculation of u_* , the friction velocity, and C_d , the coefficient of drag. Finally, we calculate quantities of particular interest in turbulence modeling and analysis, the characteristic lengthscales, including a lengthscale which represents the stream-wise scale of the eddies which dominate the Reynolds stresses.

1. Introduction

Turbulence in the presence of stratification and shear plays an important role in the dynamics of shallow tidal flows like those found in estuaries. It can play an integral role in regulating barotropic and baroclinic shear flows [Peters, 1997; Stacey, 1996] and can affect biological processes as well (Koseff et al., 1993). Models of turbulence have been developed with varying degrees of sophistication and success [Mellor and Yamada, 1982; Lehfeldt and Bloss, 1988; Nunes-Vaz and Simpson, 1994]. Our ability to fully evaluate these models has been limited by a lack of comprehensive turbulence data sets from estuarine flows.

Until the last decade, field measurements in estuaries were done using current and salinity meters moored at points around the estuary [Dyer, 1980; Bowden and Howe, 1963]. These studies provided excellent time resolution, but were unable to capture spatial structures in the flows. Schroder and Siedler [1989] deployed a fixed tripod on the bed to measure small-scale velocity fluctu-

ations and profiled the water column every 30 min with a sonde to measure mean velocity and salinity. In each of these studies, turbulence measurements were limited to a couple of points near the bed or near the surface.

More recently, profiling instruments have been used to capture vertical variability in both the velocity and salinity fields. Acoustic Doppler current profilers (ADCPs) were used by Burau et al. [1993] to study the residual flow fields in northern San Francisco Bay. Another study using the profiling ability of the ADCPs was done by Geyer [1993], who looked at three-dimensional flows around a headland. In general, acoustic Doppler current profilers have allowed researchers to gather comprehensive data sets on the evolution of a water column. These data sets typically resolve only the mean quantities and do not address turbulent fluctuations.

A comprehensive look at vertical mixing in an estuary was given by Farmer and Smith [1980] who used acoustic backscatter to resolve the displacement of a sharp density interface in Knight Inlet. Profiles of the small-scale shear (along with an assumption of isotropy at those scales) provide estimates of the dissipation in the flow [Seim and Gregg, 1995]. Measurements with a microstructure shear probe were also used by Peters, [1997] in combination with profiles of the mean velocity to estimate eddy viscosity and diffusivity [Bwch, 1977]. Similar measurements have been performed by Imberger [Imberger and Head, 1994] in a variety of conditions in lakes around the world.

Gargett and Moum [1995] measured mixing efficiency

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in tidal fronts and compared direct measurement of buoyancy flux using a towed conductivity-temperature depth profiler (CTD) and ADCP to values inferred from a microscale profiler. *Gargett* [1994] has also developed a method of estimating dissipation from the large scales. She used a single-beam acoustic current profiler to measure the instantaneous fluctuating vertical velocities and defined an estimator of the dissipation based on the variance of these measurements.

A similar method to extract turbulence statistics directly from the large scales was used by *Lohrmann et al.* [1990] on the continental shelf. Using a pulse-to-pulse coherent acoustic Doppler current profiler, they developed ensemble profiles of Reynolds stress and eddy viscosity. In a similar experiment, *van Haren et al.* [1994] used a 1.2 MHz narrowband ADCP to measure eddy fluxes above a sloping bottom on the Scotian Shelf; they attributed the measured fluxes to internal wave instability. We should note here, however, a difficulty in using the narrowband ADCP for these types of measurements which arises due to biases in the noise levels between beams. Also using an ADCP, *Plueddemann* [1987] examined Reynolds stresses due to internal waves in the upper ocean. Finally, *Lu* [1997] used an ADCP in the Cordova Channel near Vancouver Island to measure Reynolds stresses and turbulent kinetic energy.

The technique used to resolve turbulence statistics in these last studies was similar to the one developed in this paper. One of the great benefits of this method for measuring turbulent mixing (besides being noninvasive) is that it measures the time evolution of Reynolds stresses and mixing coefficients throughout the entire water column. In this paper, the analysis is extended to include a quantification of the error associated with the measurements. Measurements from an unstratified channel flow will be used to both test the method (through comparison with theory) and to examine the turbulent characteristics of an unstratified tidal flow. The Reynolds stress profiles will be extrapolated to the bed in order to estimate values of the friction velocity. These values will then be compared to those calculated by assuming a log-layer profile for the mean velocities. Profiles of relevant turbulent lengthscales will be calculated from both the Reynolds stress profiles and the autocorrelations of the instantaneous velocity measurements. Finally, we will discuss the generalization of this approach to other flows and conditions.

2. Turbulence Measurements

The data set we will discuss was collected with a 1200 kHz broadband acoustic Doppler current profiler (BB-ADCP) from RD Instruments. The BB-ADCP uses a pair of broadband encoded pulses to measure velocity throughout a water column. The pulses are transmitted from a transducer, which then functions as a receiver to collect the signal that is reflected off particles which move with the currents. The motions of the reflectors

create a Doppler-shifted reflected signal in which the relative phase shift between the two reflected signals is proportional to the velocity of the reflector. Velocity measurements are done along each of four beams, which are arrayed in a Janus configuration (Figure 1). In this configuration, calculation of the two horizontal velocity components is possible as

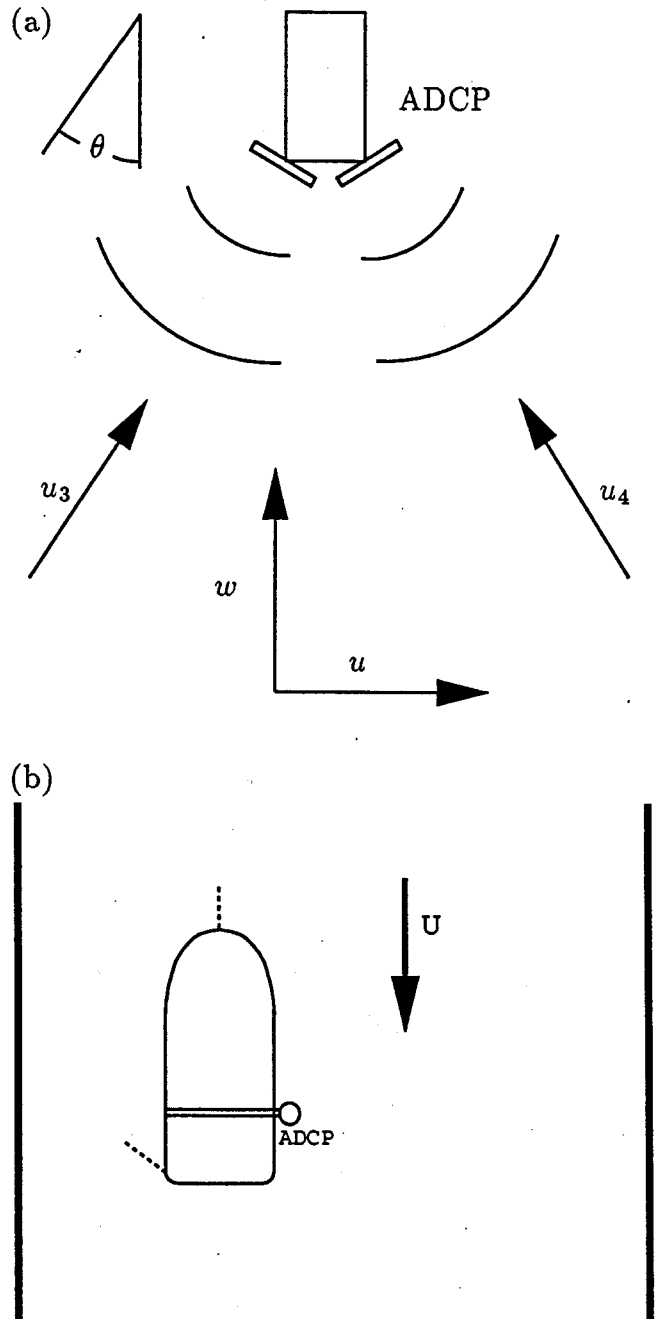


Figure 1. Experiment configurations. (a) Janus configuration of ADCP beams. For the instrument deployed at Three Mile Slough, $\theta = 20^\circ$. (b) Configuration of boat and ADCP deployed at Three Mile Slough. Two-point anchoring was from the bow and the port-side stern (dotted lines). Channel walls (solid lines) not to scale.

$$u = \frac{(u_3 - u_4)}{2 \sin \theta} \quad (1a)$$

$$v = \frac{(u_1 - u_2)}{2 \sin \theta} \quad (1b)$$

where u_1 and u_2 are the beams into and out of the plane of Figure 1a.

The method used to calculate the turbulence statistics was outlined by Tropea [1981] for use with a one-dimensional laser Doppler anemometer and applied to ADCP data by Lohrmann *et al.* [1990]. The technique relies on the along-beam variances of the velocity measurements and, as such, will be referred to as the variance technique for resolving turbulent quantities.

The direct calculation of correlations between the along-beam velocities will not be used to resolve the Reynolds stresses due to inhomogeneity in the instantaneous velocity fields. As will be discussed further below, opposite beams sample the flow at locations that are separated by several meters. In the instantaneous velocity fields, there will be variations in the flow on scales defined by the turbulent eddies. Therefore, at an instant in time, it is possible (in fact, likely) that one beam will be sampling one eddy while the opposite beam will be sampling a different eddy entirely, rendering direct calculation of the correlation meaningless. The variance technique relies only on combining the statistics (mean and variance) of opposite beams. Therefore we require homogeneity between the beams only in the mean and variance of the velocity signal, an assumption that will be discussed below.

3. Definition of Method

The ADCP has two pairs of opposite beams, each inclined at 20 degrees to the vertical (other units may have 30 degree beams). As a result, as shown in Figure 1a, each beam measures a velocity which is actually a weighted sum of the local horizontal and vertical velocity. For beam 3 (as numbered in Figure 1a), the velocity measured, u_3 , is given by

$$u_3 = u \sin \theta + w \cos \theta \quad (2a)$$

where u is the horizontal velocity in the plane formed by beams 3 and 4, w is the vertical velocity, and θ is the angle the beams make with the vertical. Similarly, we can see that the velocity measured by beam 4 (opposite beam 3) is given by

$$u_4 = -u \sin \theta + w \cos \theta \quad (2b)$$

Separating each velocity into a mean, where the mean is taken over some chosen averaging period, and a fluctuating quantity as

$$u = \bar{u} + u' \quad (3a)$$

$$w = \bar{w} + w' \quad (3b)$$

$$u_3 = \bar{u}_3 + u_3' \quad (4a)$$

$$u_4 = \bar{u}_4 + u_4' \quad (4b)$$

allows us to calculate the variance of the along-beam velocities as

$$\overline{u_3'^2} = \overline{u'^2 \sin^2 \theta} + \overline{w'^2 \cos^2 \theta} + 2\overline{u'w'} \sin \theta \cos \theta \quad (5a)$$

$$\overline{u_4'^2} = \overline{u'^2 \sin^2 \theta} + \overline{w'^2 \cos^2 \theta} - 2\overline{u'w'} \sin \theta \cos \theta \quad (5b)$$

with the only difference between the two expressions being the sign on the term containing $\overline{u'w'}$.

Finally, by taking the difference of the variance of opposite beams, we can calculate the Reynolds stress exactly as

$$\overline{u'w'} = \frac{\overline{u_3'^2} - \overline{u_4'^2}}{4 \sin \theta \cos \theta} \quad (6a)$$

Similarly, the cross-stream Reynolds stress is given by

$$\overline{v'w'} = \frac{\overline{u_1'^2} - \overline{u_2'^2}}{4 \sin \theta \cos \theta} \quad (6b)$$

Using the variances as defined in (5a) and (5b), we can also eliminate the cross terms by summing the variances:

$$\overline{u_u^2} = \frac{1}{2}(\overline{u_3'^2} + \overline{u_4'^2}) = \overline{(u'^2 \sin^2 \theta + w'^2 \cos^2 \theta)} \quad (7a)$$

$$\overline{u_v^2} = \frac{1}{2}(\overline{u_1'^2} + \overline{u_2'^2}) = \overline{(v'^2 \sin^2 \theta + w'^2 \cos^2 \theta)} \quad (7b)$$

Using the anisotropy in the turbulent kinetic energy field (as given below, (12a) and (12b), for an unstratified channel flow), we can calculate q^2 or any of its components, using the quantities in (7a) and (7b).

4. Observations and Comparison to Theory

The observations presented in this paper were collected in Three Mile Slough, a straight, narrow channel in the Sacramento-San Joaquin Delta which is tidally active but contains fresh water through most of the year. The channel is oriented directly north-south and is 3 miles long (4.8 km) and about 100 m wide [Lai, 1988]. The channel actually connects the Sacramento and San Joaquin Rivers, making it unique due to the tidal forcing occurring at both ends (the tide propagates up both the Sacramento and San Joaquin Rivers). The prevailing winds in the summer months are east-west and therefore there is very little wave activity on the slough. For these reasons, it provided a good environment to examine unstratified tidal flow using the analysis technique shown above.

The data collection took place for 2 hours starting just after the maximum of an ebb tide (flow to the south) in August 1994. A 1200 kHz BB-ADCP (described above) was deployed in the downwards-looking mode from an anchored whaler (Figure 1b) and collected every ping with a frequency of about 1 Hz. The instrument was used in "mode 4" [RD Instruments, 1995], which provided a data set in which the noise

characteristics are independent of the velocities being measured (see below, section 4.1.1 for additional discussion). Although there were no wind waves to create boat motion, there were occasional boat wakes which would be reflected in some portions of the data. The channel was approximately 9 m deep (noted from the boat's depth sounder; its data were not logged throughout the experiment) which allowed for the resolution of twenty-nine 25 cm depth cells. Although no salinity data were collected during this experiment, the channel was most likely fresh, and there should have been no effects of stratification, allowing the measurements to be compared to theory.

At its simplest, estuarine flow is an open-channel flow. The theory of turbulence in open channels is well developed for the case of steady, unstratified flow [Nezu and Nakagawa, 1993]. For this type of flow (with a logarithmic mean velocity profile), the total shear stress (the sum of the Reynolds and viscous stresses) in the flow can be derived analytically as

$$\frac{\tau}{\rho} = -\overline{u'w'} + \nu \frac{\partial \bar{u}}{\partial z} = u_*^2 \left(1 - \frac{z}{H}\right) \quad (8)$$

where τ/ρ is the total shear stress, ν is the molecular viscosity, and u_* is a quantity known as the friction velocity and is defined by the above equation (at $z = 0$). The friction velocity is the fundamental turbulent velocity scale for channel flow. It is usually scaled by the depth-averaged mean velocity using a constant coefficient C_d as

$$u_* = C_d^{1/2} \langle \bar{u} \rangle \quad (9)$$

with a typical value of C_d being 0.0025 and angle brackets indicating a depth-averaged quantity.

The vertical distributions of other turbulence statistics have been analyzed by Nezu and Nakagawa [1993]. Assuming a local balance of production and dissipation, they give

$$\overline{u'^2} = 5.29 u_*^2 \exp\left(-2\frac{z}{H}\right) \quad (10a)$$

$$\overline{v'^2} = 2.66 u_*^2 \exp\left(-2\frac{z}{H}\right) \quad (10b)$$

$$\overline{w'^2} = 1.61 u_*^2 \exp\left(-2\frac{z}{H}\right) \quad (10c)$$

From which it follows that

$$q^2 = 9.56 u_*^2 \exp\left(-2\frac{z}{H}\right) \quad (11)$$

Using these expressions, we can define the anisotropy ratios of the turbulent field as

$$\frac{\overline{v'^2}}{\overline{u'^2}} = 0.30 \quad (12a)$$

$$\frac{\overline{w'^2}}{\overline{u'^2}} = 0.50 \quad (12b)$$

The expressions presented above will serve as a baseline with which to compare the measurements from

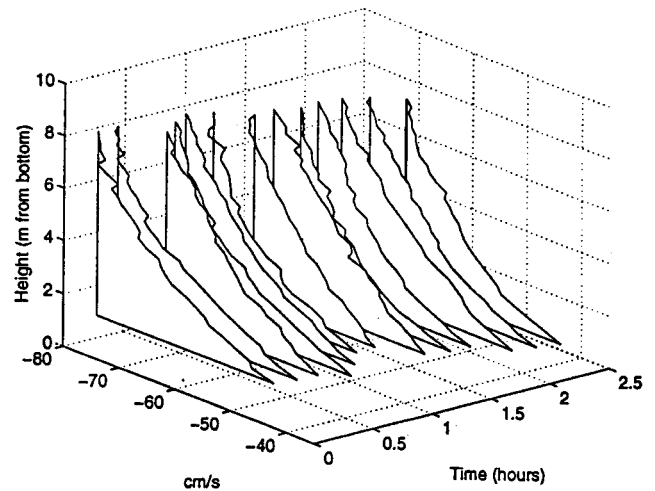


Figure 2. Along-channel mean velocity profiles. Entire Three Mile Slough data set is displayed.

Three Mile Slough. Because Three Mile Slough is an unstratified, prismatic channel, we would expect the theoretical profiles to be reproduced by the data collected there.

Using a 10-min window, mean velocities and Reynolds stresses were calculated using (6). A plot of the mean velocities (Figure 2) shows that we were sampling during a slightly decelerating phase of the tide. The normalized profiles of turbulence statistics display the behavior expected in unstratified channel flow. First of all, the Reynolds stresses profiles from various times in the collection track the theoretical value quite well (Figure 3). In normalizing the Reynolds stresses, a linear extrapolation of each profile has been extended to the bed to define the friction velocity squared. An alternate

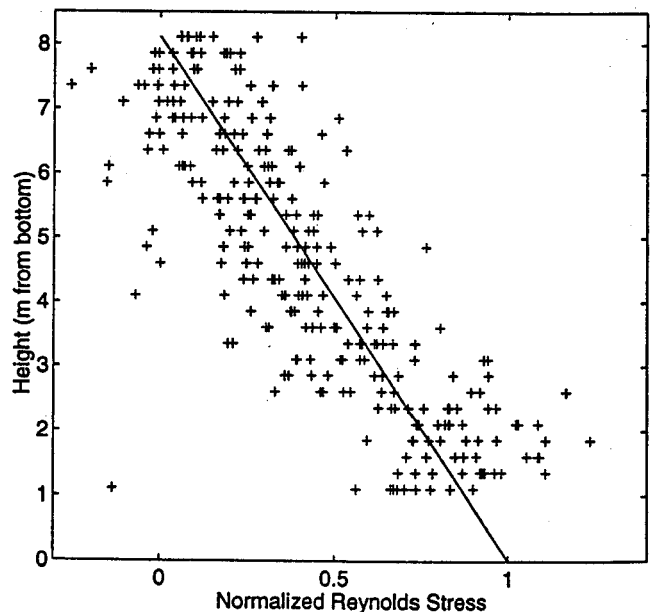


Figure 3. Reynolds stress profiles, normalized by u_*^2 , for Three Mile Slough data. Solid line is theoretical curve.

estimate of u_* is available from the mean velocities; this approach is discussed further below in section 5.

The turbulent kinetic energy can be calculated by assuming the anisotropy of the components is represented by the unstratified values reported by *Nezu and Nakagawa* [1993, equations 12a and 12b] and using the expressions for d_u^2 and d_v^2 (equations (7a) and (7b)). Before calculating the turbulent kinetic energy, the bias in the values of d_u^2 and d_v^2 must be estimated. This bias can be estimated in two ways, which will be discussed below when analyzing the bias (section 4.1.1). Once the bias is removed, d_u^2 and d_v^2 can each be used to calculate q^2 (giving two independent estimates). These values are then normalized by u_*^2 , resulting in excellent agreement with theory throughout the profile (Figure 4). This result lends further support to the validity of our approach because (consistent with (11)) the turbulent kinetic energy (q^2) has been scaled by the friction velocity (u_*^2), which was based on the measured Reynolds stress profiles, indicating consistency within our method. Additionally, the consistency between the profiles of q^2 calculated using d_u^2 and d_v^2 indicates a consistency between the four beams of our instrument.

In Figures 4a and 4b, there are two profiles which lie above the rest (these are most clear in Figure 4b), indicating increased bias of these measurements. This bias is due to boat motion, an issue that will be addressed further in the discussion of errors, section 4.2.

4.1. Error Analysis

Generally, the error in an individual ADCP estimate is considered to be Gaussian white noise which is quantified as a per-ping uncertainty (or a standard deviation of the noise). Referring to Figure 1a, the Reynolds

stresses are given exactly by (6a) and (6b). When we use the measured data to calculate these quantities, we need to consider the effect of noise. To be specific, define the measured signal from beam i as

$$x_i = u_i + N \quad (13)$$

where N is a Gaussian white noise random variable and u_i is the actual velocity along beam i .

Each of these quantities will be modeled as a random variable for the purpose of this analysis. The actual equations used to calculate the turbulence quantities, however, will be based on the sample mean and variance. That is, the mean value of the velocity along beam i , \bar{u}_i , will be estimated as

$$\hat{u}_i = \frac{1}{M} \sum_{m=0}^M x_i(m) \quad (14)$$

where we have used the convention that a "hat" indicates an estimator of the mean of a quantity and we have assumed an ensemble of M realizations (i.e., M is the number of samples during the 10-min window used to calculate means and variances).

Similarly, the variance along beam i , $\overline{u_i'^2}$, will be estimated as

$$\begin{aligned} \widehat{u_i'^2} &= \frac{1}{M} \sum_{m=0}^M x_i'^2(m) \\ &= \frac{1}{M} \sum_{m=0}^M (x_i(m) - \bar{x}_i)^2 \\ &= \frac{1}{M} \sum_{m=0}^M (x_i(m) - \hat{u}_i)^2 \end{aligned} \quad (15)$$

In other words, the velocity variance will be estimated as the sample mean of the square of the fluctuating component of the velocity measurements.

With these conventions, our estimators of the turbulence quantities described above become

$$\widehat{u'w'} = \frac{1}{4 \cos \theta \sin \theta} [\widehat{u_3'^2} - \widehat{u_4'^2}] \quad (16)$$

$$\widehat{d_u^2} = \frac{1}{2} [\widehat{u_3'^2} + \widehat{u_4'^2}] \quad (17)$$

The errors that result when using these estimators are of two types: bias and spread. A bias in the estimator means that the quantity that you are calculating is shifted from the true value, such that even averaging an infinite number of observations will result in an error in the estimate. A spread in the estimator is what is typically considered noise and represents a widening of the distribution of the estimator. In this case, averaging of the observations can help to reduce the error in the estimator.

4.1.1. Calculation of bias. The bias of an esti-

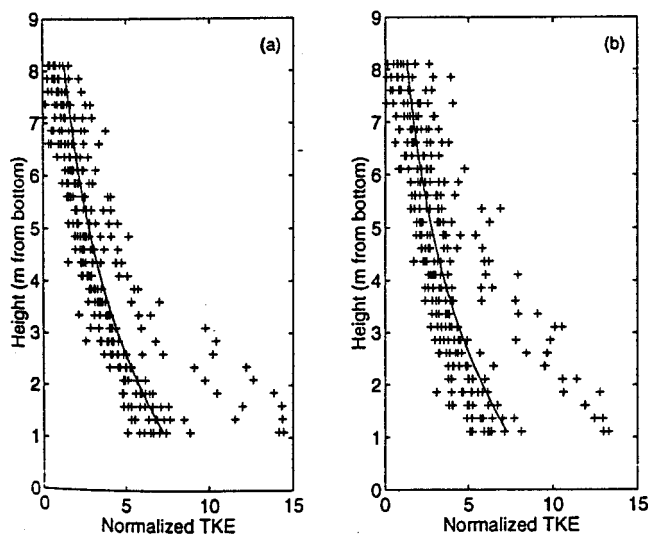


Figure 4. Turbulent kinetic energy profiles, normalized by u_*^2 (as extrapolated from Reynolds stress profiles), for Three Mile Slough data. Noise bias has been removed. Solid line is theoretical curve. Calculation based on (a) d_u^2 and (b) d_v^2 .

mator is the difference between the expected value of the estimator and the quantity it is attempting to measure. To be specific, the bias in an estimator of the variable y is given by $E(\hat{y}) - E(y)$, where we have defined $E(x)$ to be the expected value of the variable x [Bendat and Piersol, 1986].

Taking the expected value of (16) and (17) gives

$$E(\widehat{u'w'}) = \frac{1}{4 \cos \theta \sin \theta} [E(\widehat{u_3'^2}) - E(\widehat{u_4'^2})] \quad (18)$$

$$E(\widehat{d_u^2}) = \frac{1}{2} [E(\widehat{u_3'^2}) + E(\widehat{u_4'^2})] \quad (19)$$

Examining (15), we see that the expected value of $\widehat{u_i'^2}$ is equivalent to the variance in the measurements of velocity. That is,

$$\begin{aligned} E(\widehat{u_i'^2}) &= \text{Var}(x_i) \\ &= \text{Var}(u_i) + \text{Var}(N) \\ &\quad + 2 \text{Cov}(u_i, N) \end{aligned} \quad (20)$$

where we have applied (13). When operating in mode 4, the BB-ADCP produces velocity measurements for which the error is dominated by Doppler "self-noise," not flow-dependent components. The flow-dependent errors are very small relative to the self-noise, and the noise can be assumed to be independent of the velocities being measured. This should be contrasted with modes 5 and 8, where flow-dependent errors are comparable to the self-noise and independence between flow parameters (such as turbulent fluctuations) and noise can not be assumed. Because our data were collected in mode 4, we can safely assume that $\text{Cov}(u_i, N) = 0$. Thus we have that

$$E(\widehat{u_i'^2}) = \overline{u_i'^2} + \sigma_N^2 \quad (21)$$

or, the estimate of velocity variance is biased from its true value by an amount equal to the noise variance σ_N^2 .

Substituting (21) into (18) and (19), we get the following expressions for the expected values of our estimators:

$$E(\widehat{u'w'}) = \frac{1}{4 \cos \theta \sin \theta} [\overline{u_3'^2} - \overline{u_4'^2}] = \overline{u'w'} \quad (22)$$

$$\begin{aligned} E(\widehat{d_u^2}) &= \frac{1}{2} [\overline{u_3'^2} + \overline{u_4'^2} + 2\sigma_N^2] \\ &= \overline{u'^2} \sin^2 \theta + \overline{w'^2} \cos^2 \theta + \sigma_N^2 \end{aligned} \quad (23)$$

Thus the estimator of the Reynolds stress is unbiased by Doppler noise, but estimators which are related to the kinetic energy (d_u^2 , d_v^2) are biased by an amount equal to the variance of the noise (σ_N^2). Returning to Figure 3, we note that although the method of normalizing the Reynolds stresses constrains the profiles to approach 1 at the bed, the surface values are not constrained. The fact that the near surface values ap-

proach zero (within the expected noise level, see below) is consistent with the Reynolds stress estimator being unbiased by Doppler noise. To estimate the bias in the kinetic energy variables (d_u^2 , d_v^2), we examine histograms of values of d_u^2 and d_v^2 (Figure 5). There are no values of d_u^2 or d_v^2 which are less than $28.8 \text{ cm}^2/\text{s}^2$ and only two values less than $34 \text{ cm}^2/\text{s}^2$. We therefore select a value of $34 \text{ cm}^2/\text{s}^2$ to be representative of the noise variance, σ_N^2 . This is equivalent to a per-ping error of $5.8 \frac{\text{cm}}{\text{s}}$, which is consistent with the value reported by *RD Instruments* [1995].

As an additional check on this bias estimate, we note that this bias has been removed in calculating the turbulent kinetic energy as displayed in Figures 4a and 4b. Comparing these profiles with theory show no persistent bias (other than that due to boat motion, which is discussed below), suggesting that we have correctly removed the bias due to instrument noise. Additionally, the consistency between the distributions of d_u^2 and d_v^2 provides evidence that all four beams of the instrument have similar noise characteristics, as we would expect.

4.1.2. Calculation of spread. In analyzing the bias of the estimators we took the expected values of (16) and (17). Analogously, the spread in the estimators will be defined by the variance of those same equations. The errors in using (16) and (17) to estimate the turbulence quantities will be defined by the standard deviation (or variance) of those equations. From (16) and (17) we have:

$$\begin{aligned} \text{Var}(\widehat{u'w'}) &= \frac{1}{16 \sin^2 \theta \cos^2 \theta} [\text{Var}(\widehat{u_3'^2}) \\ &\quad + \text{Var}(\widehat{u_4'^2}) \\ &\quad - 2 \text{Cov}(\widehat{u_3'^2}, \widehat{u_4'^2})] \end{aligned} \quad (24)$$

$$\begin{aligned} \text{Var}(\widehat{d_u^2}) &= \frac{1}{4} [\text{Var}(\widehat{u_3'^2}) + \text{Var}(\widehat{u_4'^2}) \\ &\quad + 2 \text{Cov}(\widehat{u_3'^2}, \widehat{u_4'^2})] \end{aligned} \quad (25)$$

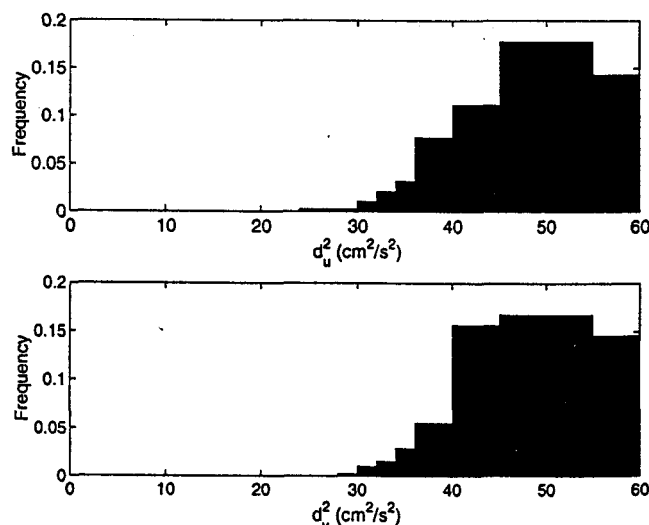


Figure 5. Histogram of frequency of values of d_u^2 and d_v^2 .

In all of these expressions, two fundamental quantities appear: the variance and the covariance of the estimated along-beam velocities squared. Following *Tropea* [1981], we will assume that the instantaneous velocity measurements from a beam are independent of those of another beam. This assumption is equivalent to assuming that the beams are, at a given time, sampling from different eddies, or, that the characteristic size of the eddies is less than the beam spread (see section 4.2). From the Three Mile Slough data set we calculated the covariance between the velocity signals in opposite beams and found that this assumption was justified: although nonzero, the covariance between beams was more than an order of magnitude smaller than the variance of the individual beams. The assumption of independence allows us to eliminate the covariances from the above expressions, leaving only the variances of the measured along beam velocities squared as a source of error.

A second assumption due to *Tropea* [1981] is that the error in the along-beam measurements is the same for all beams. This assumption is equivalent to saying that all beams have the same noise characteristics and similar turbulence statistics. Again, the Three Mile Slough data demonstrate the validity of this assumption, as was shown in the calculation of the noise variance in the previous section. There the noise variance calculated using beams 3 and 4 was equivalent to that calculated using beams 1 and 2, demonstrating the consistent nature of the noise between beams. Incorporating this assumption allows us to consider the variances along a selected beam x_i instead of along all four beams independently.

With these assumptions the above expressions for the variance in the estimators become

$$\text{Var}(\widehat{u'w'}) = \frac{1}{8 \sin^2 \theta \cos^2 \theta} \text{Var}(\widehat{u_i^2}) \quad (26)$$

$$\text{Var}(\widehat{d_u^2}) = \frac{1}{2} \text{Var}(\widehat{u_i^2}) \quad (27)$$

We now need to define the variance of the estimator of the along-beam velocity fluctuations $\widehat{u_i^2}$. Returning to the definition of $\widehat{u_i^2}$ (15), we see that we can write

$$\text{Var}(\widehat{u_i^2}) = \text{Var}\left(\frac{1}{M} \sum_{m=0}^M x_i^2(m)\right) = \frac{1}{M} \text{Var}(x_i^2) \quad (28)$$

The calculations to define the variance of the along-beam velocity measurements squared (i.e., $\text{Var}(x_i^2)$) are given in the appendix. The result (given in terms of the kurtoses of the variables u_i and N , K_{u_i} and K_N , respectively) is

$$\text{Var}(x_i^2) = (K_{u_i} - 1)(\overline{u_i^2})^2 + 4\overline{u_i^2}\sigma_N^2 + (K_N - 1)\sigma_N^4 \quad (29)$$

Substituting this expression into (28), we have

$$\text{Var}(\widehat{u_i^2}) = \frac{1}{M} [(K_{u_i} - 1)(\overline{u_i^2})^2 + 4\overline{u_i^2}\sigma_N^2 + (K_N - 1)\sigma_N^4] \quad (30)$$

Up to this point we have assumed that each measurement of the along-beam velocity has been independent of every other measurement. However, as we will see below (section 5), there is a correlation between one measurement of the velocity and the measurements at later times (as defined by (43)). The effects of this correlation on the error in a sample mean estimator have been analyzed by *Heathershaw and Simpson* [1978], who found that the sample mean of M measurements of a variable x , with variance $\text{Var}(x)$ is defined as

$$\sigma_x = \sqrt{\frac{\text{Var}(x)}{M} [1 + 2 \sum_{m=1}^M \rho_x(m)]^{1/2}} \quad (31)$$

where $\rho_x(m)$ is the correlation of the series of measurements with itself shifted by m measurements. This calculation has been done using the auto-correlation functions discussed in section 5 (and displayed below in Figure 11) and the sum of the autocorrelations was found to have a mean value of about 1. Using (31), this would suggest a multiplicative factor on our estimates of the standard errors in $\widehat{u_i^2}$, and hence $\widehat{u'w'}$ and $\widehat{d_u^2}$, of 1.7 (or a factor of 3 in the variance).

This factor, along with (30), (26), and (27) completely define the random errors present in the calculation of the turbulence characteristics. For completeness, they are

$$\text{Var}(\widehat{u'w'}) = \frac{3}{8M \sin^2 \theta \cos^2 \theta} [(K_{u_i} - 1)(\overline{u_i^2})^2 + 4\overline{u_i^2}\sigma_N^2 + (K_N - 1)\sigma_N^4] \quad (32)$$

$$\text{Var}(\widehat{d_u^2}) = \frac{3}{2M} [(K_{u_i} - 1)(\overline{u_i^2})^2 + 4\overline{u_i^2}\sigma_N^2 + (K_N - 1)\sigma_N^4] \quad (33)$$

Assuming that both the velocity and noise distributions have a "peakedness" similar to a Gaussian distribution, we can choose the kurtoses to be the Gaussian value ($K_{u_i} = K_N = 3$). Using $\sigma_N^2 = 34 \text{cm}^2/\text{s}^2$ as determined from the bias in $\widehat{d_u^2}$ and $\widehat{d_v^2}$, the configuration used to collect the Three Mile Slough data would be expected to have a standard error in the Reynolds stress of approximately $5.3 \text{cm}^2/\text{s}^2$.

The theoretical profile of the Reynolds stress is the linear function given by (8). As discussed further below, we calculated u_* based on the Reynolds stress profiles. The residuals of these data are then given by

$$\overline{u'w'}_{\text{res}} = \overline{u'w'} - u_*^2 \left(1 - \frac{z}{H}\right) \quad (34)$$

where H is assumed to be 9 m. A histogram of these residuals is displayed in Figure 6. The distribution is seen to be biased slightly to the negative, with a mean value of $-0.27 \text{cm}^2/\text{s}^2$. The standard deviation of the distribution, however, is $5.04 \text{cm}^2/\text{s}^2$, so the population is consistent with one drawn from a zero mean popula-

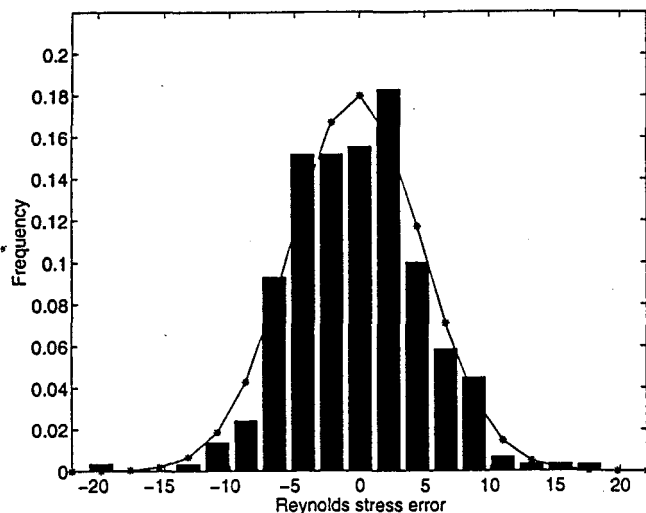


Figure 6. Histogram of errors in Reynolds stress estimates (difference between measured value and theoretical value), for Three Mile Slough data set. Solid line (with asterisks) is equivalent Gaussian (see text for discussion).

tion. The equivalent Gaussian distribution (with mean of $-0.27\text{cm}^2/\text{s}^2$ and standard deviation of $5.04\text{cm}^2/\text{s}^2$) is also displayed and a slight negative skewness of the errors might be evident, but more samples would be necessary to fully analyze these higher moments. The standard deviation of the error distribution is also consistent with the value predicted by error analysis of $5.3\text{cm}^2/\text{s}^2$, which suggests that our understanding of the noise characteristics of the Reynolds stress estimates is correct.

4.2. Other Sources of Error

Inherent in the calculations of the turbulent statistics is the assumption of temporal stationarity over the 10-min window used to define the velocity variances. Violation of this assumption would provide an additional source of error. The condition of stationarity requires that both the mean and the variance of the along-beam velocities be stationary for the period of time used in calculating the turbulent quantities (i.e., the flow is not evolving on a timescale shorter than that used to calculate the turbulence information).

In order to test the temporal assumption, the mean and variance of the along-beam velocities were calculated every 30 s. Then, following *Soulsby* [1980], both a run test and a reversals test were applied to the series of 30 s moments. The run test for stationarity counts the number of sequences (or runs) within a series which remain on one side of the median value (or, alternatively, it counts the number of times the series crosses the median value) [*Shanmugan and Breipohl*, 1988]. The number of runs (along with the sequence length) determines the statistical certainty with which you can accept or reject the hypothesis of stationarity. For our analysis, a 95% confidence threshold was used. The reverse arrangements test counts the number of times that an en-

try in the sequence is less than an earlier entry [*Bendat and Piersol*, 1986]. Again, this quantity is used to accept or reject the hypothesis of stationarity at the 95% confidence level. Applying this analysis to this data set [see *Stacey* 1996] shows that the flow being examined is statistically stationary at the 20-30 min timescale to the 95% confidence level in all four beams. Therefore over a 10-min window the assumption of temporal stationarity is justified.

A second assumption is the spatial homogeneity of the flow. Spatial homogeneity requires that, at a given depth, opposite beams are sampling turbulence fields that have the same statistics (because we use opposite beams to resolve a single turbulence statistic). The maximum spread of the beams x_b is given by

$$x_b = 2H \sin \theta \quad (35)$$

where H is the depth of the water column (or range of data collection if less than the depth). For the Three Mile Slough data set, this results in a maximum beam spread of 6.2 m. The lengthscale for variations in the mean velocity fields, and hence turbulence statistics, is set by bathymetric variations. At this site, this lengthscale is of the order of hundreds of meters, providing sufficient homogeneity for our calculations.

An additional bias may be due to the vertical resolution of the ADCP. The ADCP performs some spatial averaging within its bins, thus limiting the range of eddies for which it will retain information. In the case of 25 cm bins, eddies of size smaller than (approximately) 50 cm will not be well resolved by the ADCP and the variance in the velocity field will be reduced. As a result, the turbulence quantities, both the turbulent kinetic energy (TKE) and the Reynolds stress, will be biased downwards by an amount equal to the sub-depth-cell contributions to these quantities. Because both of these quantities (the TKE and the Reynolds stress) are dominated by the large eddies, this error is believed to be small. However, this bias may be a factor in the downwards bias of the friction velocity based on the Reynolds stresses in the first portion of the current data set; as a result, this bias is an area of on-going research.

Finally, the profiles which lie above the expected curve in Figure 4 represent additional noise introduced into the system by the motion of the boat on which the ADCP was mounted. This additional variance was not represented in the Reynolds stress profiles because the estimator of the Reynolds stress is unbiased (however, it coincided with the profiles for which low values of the friction velocity were estimated). The variance which is induced by the rocking of the boat is quantifiable given information about the motion of the boat. For the Three Mile Slough data set, we have time series of pitch, roll, and heading from the ADCP itself.

The effects of nonzero pitch and roll on this type of analysis were analyzed by *van Haren et al.* [1994]. They considered the biases that are induced if pitch and roll

were off from the expected angles slightly. However, when calculating the variance of the velocity variances, it is also important to examine the variance induced by the motions of the sensor. These variances will be produced by two types of motion: rotation and translation.

The variance induced by rotation of the vessel will be dependent on the time derivative of the pitch angle ($\dot{\phi}_p$) and roll angle ($\dot{\phi}_r$) and on the moment arms of the instrument from the centers of pitch and roll (l_p and l_r , respectively). With these variables, the induced variance will be proportional to the quantity

$$\overline{u_{\text{ind}}^2} \approx l_p^2 \overline{(\dot{\phi}_p)^2} + l_r^2 \overline{(\dot{\phi}_r)^2} \quad (36)$$

For the Three Mile Slough deployment, the time series of $\overline{(\dot{\phi}_p)^2}$ and $\overline{(\dot{\phi}_r)^2}$ are displayed in Figure 7. The primary motion is clearly in the roll component of the motion, which had a small (but unmeasured) moment arm. In fact, the TKE profile from hour 2 was not one of the ones exhibiting bias, as would be expected based on the pitch and roll measurements. The two most biased profiles in Figures 4a and 4b are at times 0.3333 hour and 0.6667 hour into the deployment, a period in which there is very small variability in the pitch and roll.

These data indicate that translational motion due to vertical movements of the boat could be more important in setting the level of bias in the velocity measurements. Such motion will bias all four beams equally, which is consistent with what is seen in the data at hours 0.3333 and 0.6667. It is also possible that such motion (contaminating all beams equally) would overwhelm the variance due to turbulent motions and lead to a downwards bias in the Reynolds stresses (because the stresses are based on the differences in the variances), which is consistent with the low values of the friction velocity discussed below. Vertical translation of the boat is a difficult quantity to measure, as it does

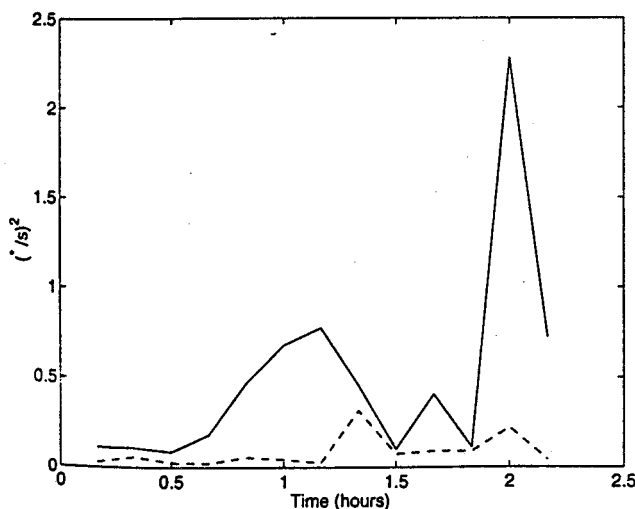


Figure 7. Time series of variance in time derivatives of pitch (dashed line) and roll (solid line).

not necessarily correlate with either pitch or roll. As will be discussed further below (section 6), the quantification of these motions could be critical in future studies using ship-mounted ADCPs.

5. Analysis of Turbulence

As discussed above, the fundamental velocity scale in open channel flow is the friction velocity u_* . With the data from Three Mile Slough, we were able to estimate this quantity in two independent ways: the first using the mean velocities, the second using the Reynolds stresses. The expression for mean velocity in an unstratified open channel follows the well-known logarithmic law:

$$U(z, t) = \frac{u_*(t)}{\kappa} \ln \left(\frac{z - z_{\text{off}}}{z_r} \right) \quad (37)$$

where $u_*(t)$ is the time series of the friction velocity, z_{off} is an offset in the vertical position and z_r is the roughness lengthscale. We have assumed that the roughness lengthscale is independent of time, which is valid for the timescales under consideration.

The parameters $u_*(t)$, z_{off} and z_r were adjusted to provide the best fit to the data in the least squares sense; most of the resulting profiles are displayed in Figure 8. A similar approach was applied by *Lueck and Lu* [1997] to a tidal channel in near Vancouver Island. They fit the logarithmic profile to the bottom portion of the water column, where the water column was unstratified. Because the Three Mile Slough water column was unstratified, we used the entire profile to fit the log profile.

The values of u_* will be discussed below; the roughness lengthscale (z_r) converged to a value of 8.2 cm and the vertical offset placed the first measurement 1.1 m above the bed, consistent with our expectations. The high value of z_r may indicate the presence of some small sand waves.

In order to quantify the errors on u_* , z_{off} and z_r , we applied parametric bootstrapping to the data set [*Efron and Tibshirani*, 1993, pp. 53-56]. The mean velocity data were resampled with the addition of noise, which was assumed to be Gaussian, and the curve fit was repeated, resulting in distributions of each of the fitted parameters. The standard deviation of the Gaussian noise was determined from (31), using $\sigma_N^2 = 34 \text{ cm}^2/\text{s}^2$, $M = 480$ samples and, as discussed above, a factor of 1.7 to account for the autocorrelation of the velocity measurements. The result of this calculation was a standard error in the mean velocities of $\sigma_U = 0.46 \text{ cm/s}$. The resulting distributions of u_* are shown on the graph of friction velocity (discussed below; Figure 9). The standard errors of the offset (z_{off}) and the roughness length (z_r) were 3 and 0.4 cm, respectively.

The Reynolds stresses were used to estimate u_* by extrapolating linearly to the bottom (using the best linear fit in the least squares sense). This is a fundamentally different approach to using logarithmic fits

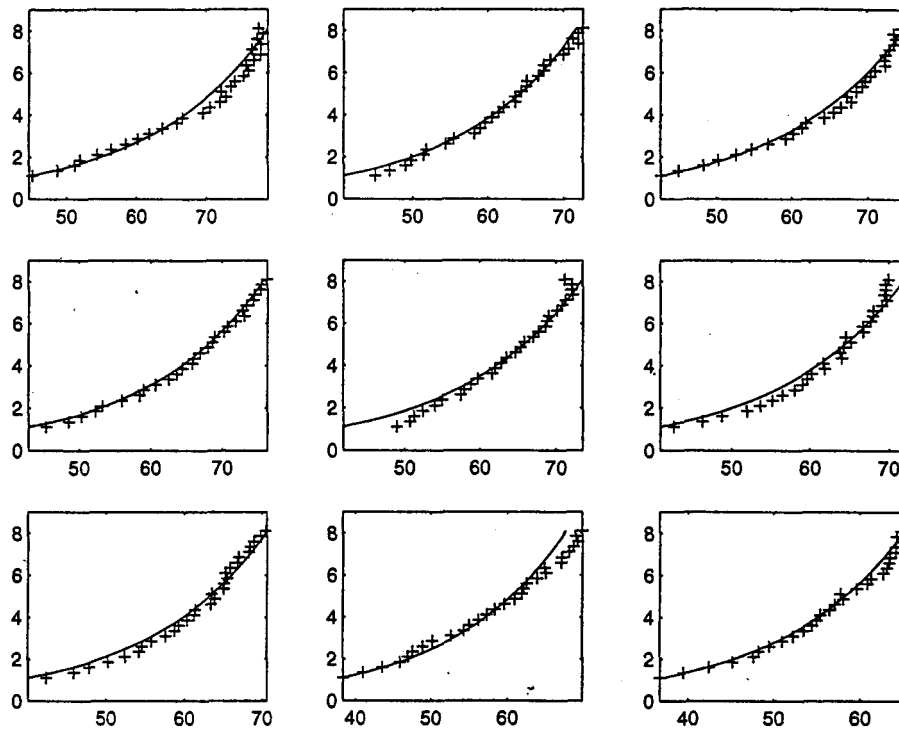


Figure 8. Samples of log fits to nine profiles. All horizontal axes are cm/s; all vertical axes are height above bottom in meters.

to mean velocity profiles to get u_* . Here we rely only on the shear stress itself. In addition to the value of u_* , the curve fit also adjusts the location of the surface, or "zero point" of the Reynolds stress profiles. Once again, to quantify the errors on each of these estimates, parametric bootstrapping was applied to the data. For the Reynolds stresses, a standard error of $5.3 \text{ cm}^2/\text{s}^2$ was chosen, consistent with the above error analysis.

The values of the friction velocity are discussed below; the estimates of the surface position had a mean value of 8.33 m with a standard error of 0.67 m. This result is consistent with the estimate of water column depth from the depth sounder of 9 m (there is no error estimate for the depth sounder measurement). Only 3 of the 13 profiles resulted in curve fits which were nonphysical due to negative values of either the friction velocity squared or the position of the surface. These profiles (numbers 1, 6, and 8) have been discarded for the continued discussion.

Each estimate of the time development of the friction velocity is shown in Figure 9. The time development of the estimates based on the mean velocities shows the effects of the slight deceleration, as the friction velocities also decrease slightly. The values based on the Reynolds stress profiles show more scatter than the ones based on mean velocities, but the magnitude is similar. The increase in friction velocity (based on the Reynolds stress profiles) over the 2 hour period may be an effect of the deceleration of the flow, which has been seen to be associated with an increase in Reynolds stress activity [Gross and Nowell 1985].

During the first hour of the study, several of the values of u_* predicted by the Reynolds stress profiles fall below those from the mean velocities. Even accounting for the variation in these estimates (denoted by the triangles in Figure 9), the two estimates of the friction velocity appear to be somewhat inconsistent. Over the first hour of the deployment, in particular at times 0.333 and 0.667, the friction velocity based on the Reynolds stresses appears to be biased low relative to the values

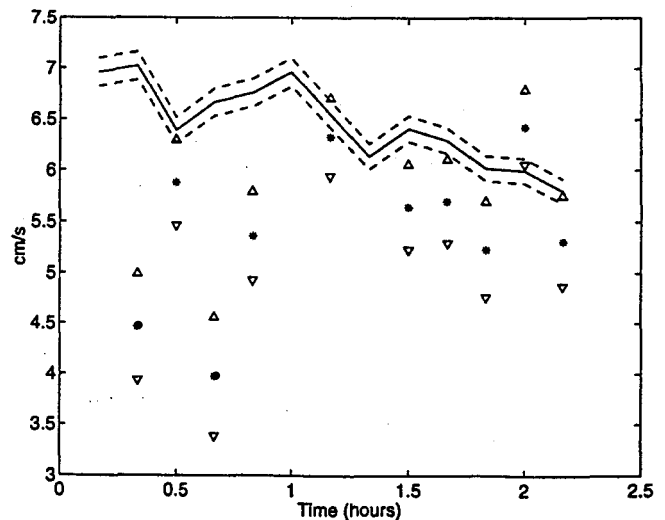


Figure 9. Time series of u_* based on Reynolds stress profiles: mean values (asterisks) and plus and minus 2σ (triangles) based on bootstrapping; and logarithmic fits to mean velocity measurements (solid line), plus and minus 2σ limits (dashed line) based on bootstrapping.

based on the mean profile, which may indicate an unknown source of bias, perhaps due to boat motion (see additional discussion below). For the last hour and a half of the deployment, however, the agreement between the two values is encouraging.

Another way of looking at the friction velocity uses the drag coefficient C_d which is defined as the square of the ratio of the friction velocity to the depth-averaged mean velocity:

$$C_d = \left(\frac{u_*}{\langle \bar{u} \rangle} \right)^2 \quad (38)$$

where angle brackets indicate a depth average. The values of C_d calculated using each of the estimates of u_* are shown in Figure 10. Just as above, bootstrapping has been applied to estimate the error in each estimate of C_d .

Apparently, the values of u_* calculated are slightly higher than would be expected in steady channel flow, and, as a result, C_d is significantly above the expected value of 0.0025 for both estimates of u_* . This is due to the high value of the roughness length; as mentioned above, this roughness scale indicates the presence of bed forms, which would elevate the value of C_d required. The relationship between the values of C_d calculated using the two methods of defining u_* is, by necessity, the same as was discussed with regards to u_* itself.

Finally, the measurements allow us to examine some of the structures of the turbulence. A turbulent quantity of particular interest in modeling turbulent flows is a characteristic lengthscale, or the typical size of a turbulent overturn. Frequently, in unstratified channels

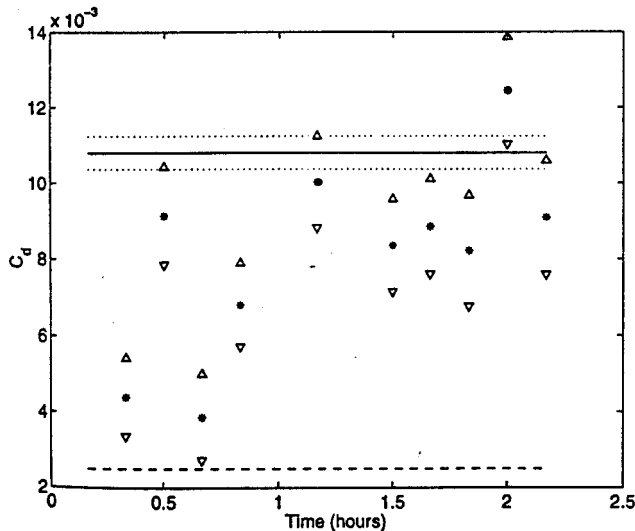


Figure 10. Drag coefficient C_d calculated as the square of the ratio of the friction velocity u_* and the depth-averaged mean velocity. Displayed are estimates using u_* based on the Reynolds stress profiles: mean values (asterisks) and plus and minus 2σ (triangles) based on bootstrapping; u_* based on the mean velocities (solid line) (plus and minus 2σ from bootstrapping (dotted line)); and the canonical value, $C_d = 0.0025$ (dashed line).

this lengthscale is considered to vary parabolically and be proportional to the total depth H :

$$l_0 = \kappa \frac{z}{H} \left(1 - \frac{z}{H} \right) H \quad (39)$$

which has a maximum of $\kappa H/4$ at $z = H/2$. Note that for the Three Mile Slough data set, $H \approx 9$ m, so the lengthscale would be expected to have a maximum of about 1 m at mid-depth ($z = 4.5$ m).

Dillon [1982] discussed different lengthscales of the turbulence but compared them all to what we will refer to as the mixing lengthscale:

$$l_m = \left(\frac{\overline{u'w'}}{S^2} \right)^{1/2} \quad (40)$$

where S^2 is the mean shear of the flow $(\partial U/\partial z)^2$. Notice that if we define the shear production $P = \overline{u'w'}S$, then the mixing lengthscale becomes:

$$l_m = \left(\frac{P}{S^3} \right)^{1/2} \quad (41)$$

A related lengthscale in stratified flows would be (see Stacey et al., [1999] for further discussion)

$$l_N = \left(\frac{P}{N^3} \right)^{1/2} \quad (42)$$

which is analogous to the Ozmidov lengthscale (which substitutes the dissipation for the shear production). For the Three Mile Slough data set the flow was unstratified, so the lengthscale of interest was the mixing lengthscale, defined by (40). Figure 12 (see below) displays the mixing lengthscale and the parabolic lengthscale defined by (39) (the other lengthscales in Figure 12 are discussed below). The agreement between these two lengthscales is quite good, suggesting that in an unstratified tidal flow, (39) is an appropriate model for the mixing length l_m .

Another common lengthscale is the integral lengthscale, which is typically defined as the spatial integral of the autocorrelation function of a velocity component [Tennekes and Lumley, 1972]. Using the ADCP data, we have defined the temporal autocorrelation function for each depth cell and each beam. The definition of this function for the i th beam is

$$R_i(z, \tau) = \frac{\overline{u'_i(z, t)u'_i(z, t + \tau)}}{u'^2_i(z)} \quad (43)$$

This function is displayed in Figure 11 for each of the four beams and for all depths. In general, we can see that the velocities are correlated over longer periods near the bed than at the surface. Further, asymmetries are seen between beams, particularly between beams 3 and 4, which will be exploited below to calculate a lengthscales associated with the Reynolds stresses.

To calculate the integral lengthscale, we must integrate the autocorrelation function over time and then

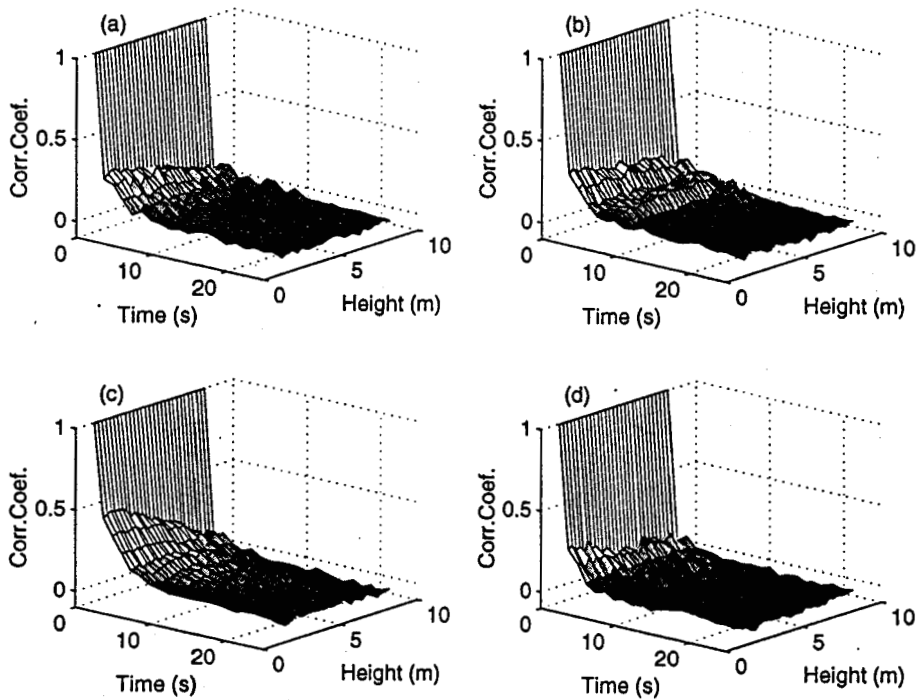


Figure 11. Autocorrelation function for each of the four beams. Each plot shows contours of the autocorrelation at a given depth and lag time. Calculations based on measurements from (a) beam 1, (b) beam 2, (c) beam 3 and (d) beam 4.

apply Taylor's hypothesis of frozen turbulence [Kundu, 1990] to define the integral lengthscale in the x direction as

$$\lambda_{i,x}(z) = U(z) \int_0^{\infty} R_i(z, \tau) d\tau \quad (44)$$

where i indicates the beam being used, and the subscript x is used to specify that it is the lengthscale in the x direction.

In Figure 12, all of the above described lengthscales are displayed. All four integral lengthscales increase away from the bed as expected, and, as mentioned above, the mixing lengthscale (l_m) matches the parabolic lengthscale (l_0) remarkably well. In general, we can say that all four integral lengthscales are larger in magnitude than either the mixing lengthscale or the parabolic lengthscale. This is not unexpected, however, based on direct numerical simulation of sheared turbulence. The database of Holt *et al.*, [1992] has been used to quantify both an average integral lengthscale and the mixing lengthscale calculated using (40). These data were created in unstratified conditions and also demonstrates an integral lengthscale which is larger than the mixing lengthscale. In the Holt data, the ratio of the average integral lengthscale to the mixing lengthscale is about 2, very similar to the ratio seen in the Three Mile Slough data set.

Examining the definition of the autocorrelation of the along-beam velocities (equation (43)), we can write for beam 3,

$$\lambda_{3,x}(z) = \frac{U(z)}{u_3^2(z)} \int_0^{\infty} \overline{u_3'(z, t)u_3'(z, t + \tau)} d\tau \quad (45)$$

Expanding u_3' using (2a) gives

$$\begin{aligned} \overline{u_3'(t)u_3'(t + \tau)} = & \overline{a^2 u'(t)u'(t + \tau)} \\ & + \overline{b^2 w'(t)w'(t + \tau)} \\ & + \overline{abu'(t)w'(t + \tau)} \\ & + \overline{abw'(t)u'(t + \tau)} \end{aligned} \quad (46)$$

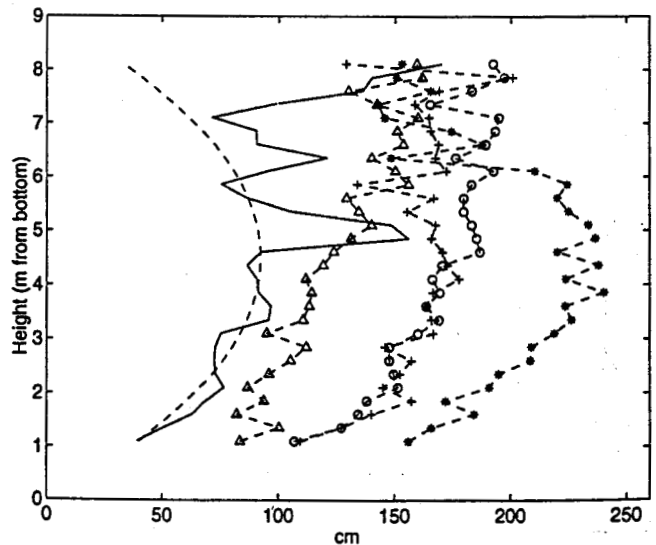


Figure 12. Profiles of lengthscales based on entire data set, mixing lengthscale, $l_m = (-\overline{u'w'}/S^2)^{1/2}$ (solid line), Integral lengthscales based on integration of autocorrelation function of beam 1 (line with pluses), beam 2 (line with circles), beam 3 (line with asterisks), beam 4 (line with triangles), and the traditional parabolic profile, $l_0 = \kappa H z/H(1 - z/H)$ (dashed line).

where $a = \sin \theta$ and $b = \cos \theta$, and we have dropped the z from the argument of each velocity component for brevity.

The symmetry of the correlation function requires that

$$\overline{u'(t)w'(t+\tau)} = \overline{u'(t+\tau)w'(t)} \quad (47)$$

which allows us to rewrite (46) as

$$\begin{aligned} \overline{u'_3(t)u'_3(t+\tau)} &= \overline{a^2 u'(t)u'(t+\tau)} \\ &+ \overline{b^2 w'(t)w'(t+\tau)} \\ &+ 2abu'(t)w'(t+\tau) \end{aligned} \quad (48)$$

Performing the same analysis on the velocity fluctuations along beam 4 results in

$$\begin{aligned} \overline{u'_4(t)u'_4(t+\tau)} &= \overline{a^2 u'(t)u'(t+\tau)} \\ &+ \overline{b^2 w'(t)w'(t+\tau)} \\ &- 2abu'(t)w'(t+\tau) \end{aligned} \quad (49)$$

where, just as when calculating the Reynolds stresses, the difference between these two expressions is in the sign on the cross-correlation term.

Because these autocorrelations are statistical quantities, we can again invoke the spatial homogeneity of the turbulence statistics and combine these two quantities to isolate the cross correlation:

$$\frac{\overline{u'_3(t)u'_3(t+\tau)} - \overline{u'_4(t)u'_4(t+\tau)}}{4abu'(t)w'(t+\tau)} = \quad (50)$$

Returning now to the definition of the integral lengthscales $\lambda_{3,x}$ and $\lambda_{4,x}$ (equation (44)) and the definition of the autocorrelation function (equation (43)), we can write

$$\begin{aligned} \overline{u_3'^2} \lambda_{3,x} - \overline{u_4'^2} \lambda_{4,x} &= \\ U \int_0^\infty (\overline{u'_3(t)u'_3(t+\tau)} - \overline{u'_4(t)u'_4(t+\tau)}) d\tau \end{aligned} \quad (51)$$

The integrand in this expression can be replaced using (50) to give

$$\overline{u_3'^2} \lambda_{3,x} - \overline{u_4'^2} \lambda_{4,x} = 4abU \int_0^\infty \overline{u'(t)w'(t+\tau)} d\tau \quad (52)$$

We can now define a new lengthscale based on this cross correlation, which will represent the longitudinal scale of the eddies which dominate the Reynolds stresses. This lengthscale will be defined by

$$\lambda_{uw,x} = \frac{U}{\overline{u'w'}} \int_0^\infty \overline{u'(t)w'(t+\tau)} d\tau \quad (53)$$

which can be substituted into (52) to give

$$\overline{u_3'^2} \lambda_{3,x} - \overline{u_4'^2} \lambda_{4,x} = 4abu'w' \lambda_{uw,x} \quad (54)$$

Finally, we rearrange (54) to completely define this new lengthscale in terms of known quantities:

$$\lambda_{uw,x} = \frac{\overline{u_3'^2} \lambda_{3,x} - \overline{u_4'^2} \lambda_{4,x}}{4 \sin \theta \cos \theta \overline{u'w'}} \quad (55)$$

where we have substituted $\sin \theta$ and $\cos \theta$ for a and b , respectively.

This lengthscale ($\lambda_{uw,x}$) represents the longitudinal extent of the eddies which contribute to the Reynolds stresses. The variation of this lengthscale with depth is shown in Figure 13, along with the profiles of the mixing length and the parabolic lengthscale. It is clear from this figure (note that the x axis is on a logarithmic scale) that the integral lengthscale of the Reynolds stresses is much larger than the mixing lengthscale. It is also slightly larger than the integral lengthscale calculated using beam 3 (the largest of the integral lengthscales). In fact, the magnitude of this lengthscale indicates there should be a nonzero covariance between beams 3 and 4 due to the presence of at least some eddies which have a streamwise scale comparable to the beam spread. As discussed earlier, there is a nonzero (but small) covariance, which is consistent with this estimate of a streamwise eddy size.

Referring to the definition of u'_3 , we can see that this result is not entirely unexpected. The integral lengthscale calculated from beam 3, if broken into components on the Cartesian grid, will have contributions from $\lambda_{uu,x}$, $\lambda_{ww,x}$, and $\lambda_{uw,x}$. Because the ADCP beams are preferentially measuring the vertical components of the velocity (i.e., $\cos 20^\circ > \sin 20^\circ$), the integral lengthscale of the velocity along each beam will be preferentially weighted towards $\lambda_{ww,x}$.

The behavior of the correlation functions which these lengthscales are based on has been examined by Townsend [1976] (note that Townsend's notation is slightly different; $R_{ij}(\tau, 0, 0)$ is the correlation of veloc-

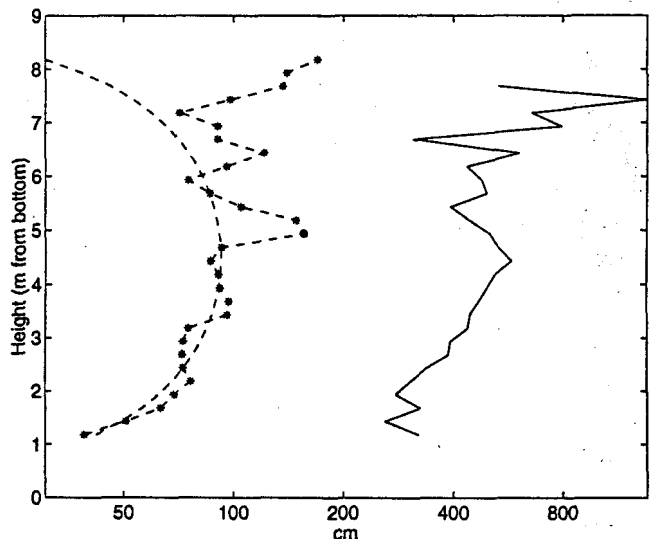


Figure 13. Profiles of lengthscales from Three Mile Slough data set. Mixing lengthscale, l_m (line with asterisks); theoretical parabolic profile (dashed line); lengthscale of Reynolds stresses, λ_{uw} (solid line). See text for definitions.

ity component i with component j in the \mathbf{x} direction). For channel flow, Townsend tabulates the distance at which the correlation is reduced to 0.05; this distance is 6.8 times larger for $R_{11}(\mathbf{r}, 0, 0)$ than for $R_{33}(\mathbf{r}, 0, 0)$ (proportional to $\lambda_{w,w,z}$ in our notation). Therefore the longitudinal scales of velocity components which involve w' (such as $\lambda_{w,w,z}$) would be expected to be larger than those measured from the individual beams (such as $\lambda_{3,z}$).

These lengthscales have been estimated in a tidal boundary layer by Gross and Nowell [1985]. Using current meter triplets, estimates of Reynolds stress cospectra showed peaks at a wavelength of approximately 10 m, which are of the same order as the scales estimated here. Gross and Nowell also noted that this scale exceeded the depth of their flow and argued that this suggested that the eddies which dominated the Reynolds stress measurements were "flattened out," again consistent with the conclusions here.

We also note that the assumption that the largest turbulent eddies dominate the transfer of momentum is also supported by this distribution of the Reynolds stress length scale. Additionally, because the mixing lengthscale is meant to represent the vertical extent of the mixing events, the two scales together define the anisotropy of the eddies which dominate the vertical transport of momentum. From Figure 13, we estimate the horizontal scale to be 5-6 times larger than the vertical scale for the large eddies which are actively mixing the flow.

6. Application to Other Conditions

The technique applied to this unstratified flow is a powerful one, which holds a great deal of promise in the exploration of turbulent mixing in a variety of estuarine and coastal flows. However, in moving from an unstratified environment to a stratified one, additional factors need to be considered.

First of all, in stratified conditions, the stratification limits the size of the eddies to be less than the Ozmidov scale ($l_{oz} = (\epsilon/N^3)^{1/2}$), which is analogous to the stratification lengthscale defined above (equation (42)). This places a limit on the ability of the ADCP to resolve turbulent motions due to the inherent averaging within each depth cell. Therefore an important consideration is the limit on the size of the eddies (which can be estimated by either the Ozmidov or stratification lengthscale) relative to the spatial average of the ADCP within each bin.

In addition to stratification imposing a limit on eddy size, presence of a boundary, such as the bed or the surface, will also constrain the size of the turbulent eddies. The effects of the ADCP bin size relative to this constraint may be evident in the near-surface region of the Three Mile Slough data set. In the upper meter of the water column, several quantities (particularly the lengthscales) exhibit large variations in their mag-

nitudes. This is also the region where the lengthscale limitation of the surface would approach the bin size of the ADCP, resulting in a reduction in the ability to resolve the actual turbulent motions. In practice therefore the ability of an ADCP to resolve turbulent motions may be reduced in the near-bed and near-surface regions, depending on the bin size being used.

An additional effect of stratification is an increase in the anisotropy of the turbulence. Using the anisotropy values assumed here (equations (12a) and (12b), which are the unstratified values) will therefore introduce a bias into the calculation of q^2 . Because the ADCP measures a larger component of the vertical velocity than the horizontal, and stratification reduces the vertical motions, as anisotropy increases, our estimate of q^2 will be below the actual value. The maximum error in these calculations has been estimated using the Holt et al., [1992] database and was found to be about 30% (see Stacey et al., [1999] for further discussion).

Finally, as was discussed above, boat motion, and the associated motion of the ADCP induces a variance in the data which could overwhelm the quantities being measured. The data from Three Mile Slough indicates that the bias in the turbulent kinetic energy may be most influenced by vertical translation of the sensor, rather than typical pitch and roll motions. However, pitch and roll motions will also induce variance into the measurements, in a manner proportional to equation (36). Therefore, it will be critical in making these measurements that a stable platform be used which will minimize the effects of pitch, roll and vertical translation. Other successful ship-mounted measurements have been performed in estuarine flows (see Stacey et al., 1999); in those cases, boat motion was completely negligible.

7. Conclusions

Acoustic Doppler current profilers are beginning to be used more and more widely in estuarine hydrodynamic studies. As a result, the technique presented here could be a valuable method for increasing the availability of information on turbulent mixing in stratified tidal flows.

The results from Three Mile Slough demonstrate that the noise levels associated with the ADCP are not prohibitively high when trying to resolve profiles of turbulence statistics. In fact, the agreement with theory was quite good in both the Reynolds stresses and the turbulent kinetic energy. Further, the errors that were seen in that data set, both in the biases and spreads, were consistent with those predicted by the application of statistical error analysis. The one exception was a potential downwards bias in the estimates of friction velocity based on the Reynolds stress profiles during the first hour of the deployment. This bias could have been due to either boat motion or the spatial averaging inherent in the ADCP system. If the downwards bias were due to the averaging of the ADCP, then during

the second hour of the data set, the friction velocity based on the Reynolds stresses would actually exceed that based on the mean velocities, a result which would be consistent with an increase in Reynolds stress activity during the deceleration phase of the tide.

The measurements in Three Mile Slough show a friction velocity which is somewhat higher than would be expected in steady channel flow. However, high values of u_* resulted from estimates using both logarithmic fits to the mean data and linear fits to the turbulence data. The logarithmic fits resulted in a roughness lengthscale of about 8.2 cm, which is suggestive of bed forms such as sand waves oriented across the channel. These bed forms would explain both the high values of u_* and C_d which were seen in the data.

The mixing lengthscale based on the Reynolds stress measurements and mean shear was parabolically distributed, increasing away from the bed with very similar magnitude to the typically assumed parabolic profile. The distributions of additional integral lengthscales were also consistent with the mixing length, with magnitude approximately 2 times larger. This factor was consistent with the value from direct numerical simulation (DNS) of unstratified, sheared turbulence. Finally, the longitudinal lengthscale of the eddies which dominate the Reynolds stress measurements was found to be somewhat larger than the other lengthscales of the flow. This finding reinforces the assumption that the largest eddies dominate the transport of momentum.

Based on these results in an unstratified channel, the variance technique presented here seems to hold promise in quantifying turbulent mixing over the entire water column of tidal flows. In stratified conditions [Stacey *et al.*, 1999] this technique allows for the simultaneous measurement of shear, stratification and turbulent mixing throughout the water column and tidal cycle. Such data sets will permit a much more in-depth examination of the balance of forces that determine the evolution of an estuarine water column.

Appendix: Error Analysis Calculations

In section 4.1.2 an important calculation was the variance of the quantity $x_i'^2$, the square of the fluctuating component of the along-beam velocity measurements. In this appendix we give the details of the analysis which defines (29).

First of all, the definition of variance allows us to write

$$\begin{aligned} \text{Var}(x_i'^2) &= E[(x_i'^2 - E(x_i'^2))^2] \\ &= E[x_i'^4 - 2x_i'^2 E(x_i'^2) \\ &\quad + (E(x_i'^2))^2] \end{aligned} \quad (\text{A1})$$

Now we note that $x_i' = u_i' + N$ which implies that $x_i'^2 = u_i'^2 + 2u_i'N + N^2$. Taking the expected value of this expression gives us

$$E(x_i'^2) = \overline{u_i'^2} + \sigma_N^2 \quad (\text{A2})$$

which can be substituted into (A1) to give

$$\text{Var}(x_i'^2) = \frac{E[(u_i' + N)^4 - 2x_i'^2(\overline{u_i'^2} + \sigma_N^2)]}{(\overline{u_i'^2} + \sigma_N^2)^2} \quad (\text{A3})$$

Evaluating the second and third terms gives

$$\text{Var}(x_i'^2) = E[(u_i' + N)^4] - (\overline{u_i'^2} + \sigma_N^2)^2 \quad (\text{A4})$$

The first term can be expanded to give

$$\begin{aligned} E[(u_i' + N)^4] &= E[u_i'^4 + 4u_i'^3N + 6u_i'^2N^2 \\ &\quad + 4u_i'N^3 + N^4] \end{aligned} \quad (\text{A5})$$

We now note that the expected values of an odd power of a zero-mean Gaussian are zero, which requires $E(N) = E(N^3) = 0$. Further, the fact that the noise is independent of the velocities requires that $E[u_i'^2N^2] = E(u_i'^2)E(N^2) = \overline{u_i'^2}\sigma_N^2$ so we can simplify (A5) to define

$$E[(u_i' + N)^4] = E(u_i'^4) + 6\overline{u_i'^2}\sigma_N^2 + E(N^4) \quad (\text{A6})$$

Finally, if we define the kurtosis of the variables u_i and N ,

$$K_{u_i} = \frac{E(u_i'^4)}{(\overline{u_i'^2})^2} \quad (\text{A7a})$$

$$K_N = \frac{E(N^4)}{\sigma_N^4} \quad (\text{A7b})$$

we can substitute back into (A4) to give the variance of the measured velocities squared as

$$\begin{aligned} \text{Var}(x_i'^2) &= K_{u_i}(\overline{u_i'^2})^2 + 6\overline{u_i'^2}\sigma_N^2 + K_N\sigma_N^4 \\ &\quad - (\overline{u_i'^2})^2 - 2\overline{u_i'^2}\sigma_N^2 - \sigma_N^4 \end{aligned} \quad (\text{A8})$$

This expression can be further simplified to

$$\begin{aligned} \text{Var}(x_i'^2) &= (K_{u_i} - 1)(\overline{u_i'^2})^2 \\ &\quad + 4\overline{u_i'^2}\sigma_N^2 + (K_N - 1)\sigma_N^4 \end{aligned} \quad (\text{A9})$$

which defines the quantity needed in the analysis of the variance in the turbulence estimates.

Notation

- u total along-channel velocity.
- w total vertical velocity.
- u_i total velocity along ADCP beam i .
- θ angle ADCP beam makes with the vertical.
- N white noise random variable.
- u_* friction velocity.
- C_d coefficient of drag.
- H total depth of the water column.
- z vertical coordinate.

\bar{x}	Reynolds average (expected value) of variable x .
x'	fluctuations of variable x ($x - \bar{x}$).
\hat{x}	estimator of the mean of variable x .
$\langle x \rangle$	depth average of variable x .
σ_x	standard deviation of variable x .
K_x	kurtosis of variable x .

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