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Corporate Environmental Programs General Electric Company 100 Woodlawn Ave., Pittsfield, MA 01201

Transmitted Via Facsimile & Federal Express

March 3, 2000

Dean Tagliaferro On Scene Coordinator Site Evaluation and Response Section (HBR) U.S. Environmental Protection Agency One Congress Street, Suite 1100 Boston, MA 02203-2211 Bryan Olson Project Coordinator Office of Site Remediation and Restoration U.S. Environmental Protection Agency One Congress Street, Suite 1100 Boston, MA 02114-2023

Re: GE-Pittsfield/Housatonic Site

Upper ¹/₂-Mile Reach Removal Action: Results of DNAPL Investigation and Proposal to Address Presence of DNAPL

Dear Mr. Tagliaferro and Mr. Olson:

Enclosed as an attachment to this letter are the results of the recent investigation for further delineation of dense non-aqueous phase liquid (DNAPL), encountered during sediment removal activities in Cell C as part of the Upper ½-Mile Reach Removal Action. Additionally included in the attachment, is a proposal to address the presence of DNAPL that was delineated by the investigation. The proposal involves additional excavation of the DNAPL-impacted materials and potential modification to the restoration system in this area. The proposal will be implemented following the United States Environmental Protection Agency (USEPA) approval of this plan.

Very truly yours,

andrew J. Silfer /dmn

Andrew T. Silfer, P.E. GE Project Coordinator

SDM/plh

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cc: T. Conway, EPA H. Inglis, EPA R. Goff, USACE K.C. Mitkevicius, USACE R. Bell, Esq., DEP J.L. Cutler, DEP S. Steenstrup, DEP A. Weinberg, DEP Field Supervisor, USFW T. La Rosa, EOEA J. Milkey, MA AG C. Fredette, CT DEP K. Finkelstein, NOAA R. Nasman, Berkshire Gas Mayor G.S. Doyle J.R. Bieke, Shea & Gardner M. Carroll, GE A. Thomas, GE S. Gutter, Sidley & Austin Public Information Repositories ECL I-P-IV(A) (1) GE Internal Repositories

GENERAL ELECTRIC COMPANY - PITTSFIELD, MASSACHUSETTS UPPER ½-MILE REMOVAL ACTION OF HOUSATONIC RIVER

RESULTS OF DNAPL INVESTIGATION AND PROPOSAL TO ADDRESS PRESENCE OF DNAPL

I. INTRODUCTION

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On February 7, 2000, GE submitted an *Investigation Work Plan - Occurrence of Dense Non-Aqueous Phase Liquid (DNAPL)* (the Work Plan) to the United States Environmental Protection Agency (USEPA), with a copy to the Massachusetts Department of Environmental Protection (MDEP). The Work Plan included a proposal for the implementation of a DNAPL investigation in response to the observation of a coal-tar-based DNAPL within the Cell C sediment removal area for the Upper ½-Mile Reach Removal Action within the Housatonic River (Figure 1). The proposed investigation included a phased soil boring program within Cells C and D to delineate the horizontal and vertical extent of DNAPL present in this area as well as the initiation of groundwater pumping of two existing wells [RW-3(X) and E2SC-9] located on the north bank. The USEPA issued a conditional approval letter on February 11, 2000 and the investigation program was initiated on February 14, 2000. Installation of groundwater pumping equipment at the two wells has been performed and groundwater pumping began in well E2SC-9 on February 29, 2000. Following the initiation of groundwater at well RW-3(X) since sediment excavation/restoration activities were nearly completed in the upstream portion of Cell C. GE received verbal approval from the USEPA and pumping at well RW-3(X) has not been initiated.

This document, which has been prepared by Blasland, Bouck & Lee, Inc. (BBL) on behalf of GE, presents the results of the investigation program and provides a proposal for excavation of the DNAPL-containing materials and a potential modification of the restoration system in this area. The results of the investigation and the proposal to address the presence of DNAPL are presented in Sections II and III, respectively; the proposed schedule is presented in Section IV.

II. SUMMARY OF DNAPL INVESTIGATION AND RESULTS

The phased investigation program was performed between February 14, 2000 and February 21, 2000 by GE with oversight from USEPA representatives. Implementation of the program resulted in the installation of 15 soil borings in a grid-like pattern at approximately 15-foot spacing, using manual AMS probe sampling techniques. The surveyed soil boring locations are shown on Figure 1. During advancement of the core barrel, the recovered soils were continuously logged and boring logs were developed and are included as Attachment A to this document. The soil samples were characterized with regard to the potential presence of DNAPL based on visual descriptions, photoionization detector (PID) readings, and soil-water shake tests. Following completion of the borings, each borehole was filled with bentonite grout and/or bentonite chips. Separate-phase DNAPL was only observed in the soil cores at three borings (HRSC-3, HRSC-5, and HRSC-6), representing an area of approximately 400 square feet. The approximate horizontal and vertical extent of DNAPL is limited to a 1 to 3 foot thick lens of sediment extending to a maximum depth of elevation 962 feet. Below this elevation a finer sand layer was consistently encountered. The estimated volume of impacted material within this area is 40 cubic yards.

Following completion of several of the boring locations, an intermediate cut-off sheetpile wall was installed within Cell C (see Figure 1) to further isolate the DNAPL area.

Following completion of the soil boring investigation, five of the soil boring locations (HRSC-3, HRSC-5, HRSC-6, HRSC-8, and HRSC-9, were selected for installation of well points (piezometers). Table 1 summarizes the well point construction details. The well points were installed on February 21, 2000, and monitored on February 24 and 25, 2000 for the presence of DNAPL. The results are presented in Table 2.

As indicated in Table 2 DNAPL was only found in one well point (PZ-4-HRSC-3) at a thickness ranging from approximately 1.5 to 1.7 feet.

III. PROPOSED EXCAVATION AND RESTORATION ACTIVITIES

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As determined during the boring program, the horizontal and vertical limits of the DNAPL are confined to an isolated "pocket" of granular sediment present within the western portion of Cell C above a fine sand layer. Based on a review of options for DNAPL removal and/or containment, GE proposes to excavate the sediment containing DNAPL to the boundaries defined by the borings, as depicted in Figures 1 through 4. Maxymillian Technologies, Inc. (MTI) has performed a structural evaluation of the current sheetpile configuration with the proposed removal depth (i.e., to approximately elevation 962 feet) and has determined that it will be necessary to install deeper sheeting (i.e., to approximately elevation 952 feet) along the southern and western boundaries in order to reach the identified removal limits. As a result, additional sheeting will be installed at the locations shown on Figure 1.

Prior to initiation of excavation activities, the well points will be decommissioned by removing the well point and filling the borehole with grout. Following completion of the excavation, a determination will be made as to whether additional DNAPL-impacted materials are present (based on visual observations) and whether it is possible to excavate additional materials based on the location and extent of DNAPL-impacted materials and excavation stability concerns. If it is determined that additional excavation can safely and reasonably be performed, additional excavation will be performed until it is determined, based on visual observations, that the DNAPL-impacted materials have been removed or until the maximum excavation limits have been reached.

Following completion of the excavation, the area will be immediately restored. GE has developed two restoration scenarios for the DNAPL excavation area. The restoration option used will depend on whether DNAPL is observed following completion of excavation activities. Each scenario is further described below.

Scenario 1 - No DNAPL Observed Following Excavation

If the DNAPL-impacted materials have been completely removed, then the area will be restored consistent with the requirements for the remainder of the Upper ¹/₂-Mile Reach (i.e., geotextile in the base of the excavation, followed by a variable depth of isolation layer material, geotextile, geogrid, and a 12-inch thick layer of 9-inch armor stone).

Scenario 2 - DNAPL Observed Following Excavation

master If DNAPL-impacted materials remain following completion of excavation activities, then an impermeable 20 mil high density polyethylene (HDPE) liner will initially be placed over the area to provide physical separation so that the area may be restored consistent with the requirements for the remainder of the Upper 1/2-Mile Reach. The intent of the HDPE liner is to minimize contact with the remainder of the restoration layers during construction (i.e., to prevent mixing). Modification to the restoration system above the HDPE liner would not be necessary (even if some DNAPL-impacted materials remain at depth) since, to a large extent, the DNAPL zone will have been removed. In addition, BBL on behalf of GE, performed calculations which indicate that the average observed vertical hydraulic gradient of well pair E2SC-3S/E2SC-3I (which are located on the river bank in the vicinity of the DNAPL area) is 0.0173 (with maximum value of 0.021). This hydraulic gradient is well below the critical vertical hydraulic gradient of 0.059 theoretically required to cause upward movement of DNAPL (based on the specific gravity of 1.059 measured for the DNAPL). Further, any remaining DNAPL would be present at a much deeper elevation than existed previously, and the potential for upward movement caused by dewatering of the excavation will be eliminated following restoration. It should also be noted that as discussed in the investigation Work Plan submitted to USEPA, under previously existing conditions (i.e., prior to any sediment and DNAPL removal) Appendix IX+3 sediment data in this vicinity did not contain elevated levels of the constituents reported in the DNAPL sample. Also, the surface water data showed no

statistical difference between constituent concentrations collected from surface water upstream and downstream of the DNAPL area.

IV. SCHEDULE AND ADDITIONAL ACTIVITIES

The proposed excavation and restoration activities outlined herein will be implemented following USEPA's approval of this proposal. It is anticipated that following USEPA approval of this proposal, excavation and restoration activities will be completed within a 2 week time frame. Until such time, GE will continue to conduct the following activities:

- Monitor the well points three times per week until the additional excavation activities are initiated;
- Pump groundwater from well E2SC-9 until the DNAPL excavation area is restored and excavation dewatering activities are discontinued (at that time GE will re-assess the need for continued groundwater pumping of this well);
- Recover DNAPL from the excavation area on a daily basis, as practicable; and
- Maintain oil absorbent booms and pads as needed.

Finally, following completion of restoration activities in the upstream portion of Cell C, GE will begin installation of the automated DNAPL collection system in well RW-3(X).

Tables

BLASLAND, BOUCK & LEE, INC. engineers & scientists

TABLE 1

GENERAL ELECTRIC COMPANY - PITTSFIELD, MASSACHUSETTS UPPER ½-MILE REMOVAL ACTION OF HOUSATONIC RIVER

WELL POINT CONSTRUCTION DETAILS

ID	GROUND ELEVATION (Feet AMSL)	HEIGHT OF RISER ABOVE GRADE (Feet)	RISER LENGTH (Feet)	SCREEN LENGTH (Feet)	SUMP LENGTH (Feet)	BASE OF SCREEN ELEVATION (Feet AMSL)	DEPTH TO BOTTOM (Feet)
PZ-1- HRSC-5	966.40	4.10	5.63	2.00	0.00	962.87	7.63
PZ-2- HRSC-6	966.09	4.38	5.63	2.00	0.00	962.84	7.63
PZ-3- HRSC-9	968.44	2.77	5.63	3.97	0.56	961.61	10.16
PZ-4- HRSC-3	965.71	4.39	5.63	2.00	0.00	962.47	7.63
PZ-5- HRSC-8	969.92	3.05	5.63	3.97	0.56	963.37	10.16

Notes:

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1. Materials used for well installation consisted of PVC screens and solid PVC risers.

TABLE 2

GENERAL ELECTRIC COMPANY - PITTSFIELD, MASSACHUSETTS UPPER ¹/₂-MILE REMOVAL ACTION OF HOUSATONIC RIVER

WELL POINT MONITORING PROGRAM

 DATE:
 2/24/00

 TIME:
 0905

 TEMP:
 35°F

 WEATHER:
 Sunny

 METHOD:
 I.P.

 OPERATOR:
 Steve Lewitt

** ALL MEASUREMENTS ARE IN FEET

ID	DEPTH TO WATER	DEPTH TO LNAPL	LNAPL THICKNESS	DEPTH ТО ВОТТОМ	DEPTH TO DNAPL	DNAPL THICKNESS	HEIGHT OF RISER ABOVE GRADE
PZ-1-HRSC-5	2.70		****==	7.30			4.10
PZ-2-HRSC-6	3.22			7.54			4.38
PZ-3-HRSC-9	4.05			8.31			2.77
PZ-4-HRSC-3	2.81			7.55	6.09	1.46	4.39
PZ-5-HRSC-8	3.80			9.47			3.05

 DATE:
 2/25/00

 TIME:
 1430

 TEMP:
 45°F

 WEATHER:
 Overcast

 METHOD:
 I.P.

 OPERATOR:
 Steve Lewitt

** ALL MEASUREMENTS ARE IN FEET

ID	DEPTH TO WATER	DEPTH TO LNAPL	LNAPL THICKNESS	DEPTH TO BOTTOM	DEPTH TO DNAPL	DNAPL THICKNESS	HEIGHT OF RISER ABOVE GRADE
PZ-1-HRSC-5	2.35			7.32			4.10
PZ-2-HRSC-6	2.57			7.54			4.38
PZ-3-HRSC-9	3.41			8.24		****	2.77
PZ-4-HRSC-3	2.60			7.55	5.84	1.71	4.39
PZ-5-HRSC-8	3.02			9.45			3.05

Figures

BLASLAND, BOUCK & LEE, INC. engineers & scientists

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L: ON =* OFF=*REF*,SURV*,*SED-* ON=*SED-POLY P: STD-PCP/BL 2/24/00 SYR-54-NES AK NES PGL 20197070/20197B07.DWG





L: ON =* OFF=*REF*,SURV*,*SED-* ON=*SED-POLY P: STD-PCP/BL 2/25/00 SYR-54-NES AK NES PGL 20197070/20197806.DWG

Attachments

BLASLAND, BOUCK & LEE, INC.

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engineers & scientists

Attachment A

BLASLAND, BOUCK & LEE, INC. engineers & scientists

Soil Boring Logs

Page	1	of	ł	
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DA DA DR DR BIT RIC	TE STARTH TE FINISHI ILLING CO ILLING ME `SIZE: 1.5 I G TYPE: Jac	ED: 2/2/20 ED:2/2/200 MPANY: THOD: E nch X 4 Fe khammer	000 00 BBL Direct Pus eet	h	BOR DESC NOR EAST GRO	EHOL CRIPTI THINC TING: UND E	E DEPTH: 8.0 Feet ONS BY: Jim Hassett 5: 533415.74 133339.40 LEVATION: 965.71 BORING ID: HR-3 CLIENT: General Pittsfield SITE: Housatonic I	SC-3 Electric Company I, MA River
DEPTH (ft)	ELEVATION (ft)	SAMPLE BEPTH INTERVAL (f)	RECOVERY (ft)	SCREENING DEPTH INTERVAL (A)	PID IIEADSPACE (ppm)	SILAKE TEST	STRATIGRAPHIC DESCRIPTION	
0	965.71	0-4	2.0				Dark grav black fine coarse SAND, some fine medium	Crovel NADI soturated
1	964.71						Dark gray-black me-coarse SAND, some me-meanin	
2	963.71							
3	962.71						Note: Entire core barrel coated with NAPL to 4 feet, bu feet did not exhibit presence of NAPL within center of	it gray sand in lower 0.4 core.
4	961.71	4-8	3.75				Gray very fine-fine SAND.	3.6 (962.11')
5	960.71							
6	959.71							
7	958.71							7 75! (057 06!)
8	957.71	<u></u>					Gray very fine-fine SAND, some fine-medium Gravel.	
							Boring terminated at 8.0 feet (957.71 feet).	
9			ļ					
10			-					
REN Bori	IARKS: ing backfilled	with bent	onite.		LI	I		

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FE STAR FE FINISI LLING C LLING N SIZE: 1.5 TYPE: Ja	TED: 2/17 HED:2/17, COMPANY IETHOD: 5 Inch X 4 ackhamme	/2000 /2000 Y: BBL Direct F Feet r	Push	BORE DESC NORT EASTI GROU	HOLE RIPTI HING ING: 1 IND EI	DEPTH: 14.0 FeetBORING ID: HR-SC-4ONS BY: Nicholas A. SmithCLIENT: General Electric Company Pittsfield, MA: 533418.25SITE: Housatonic RiverJEVATION: 969.32SITE: Housatonic River
ELEVATION (ft)	SAMPLE DEPTH INTERVAL (ft)	RECOVERY (ft)	SCREENING DEPTH INTERVAL (I)	PID ILEADSPACE (ppm)	SIJAKE TEST	STRATIGRAPHIC DESCRIPTION
969.32	0-4	2.6	0-1	3.1	-	Gray fine SAND, trace fine Gravel.
968.32			1-2	5.5	TS	- -
967.32 966.32			2-4	4.8	-	1.9' (967 Dark grav fine-coarse SAND, some fine Gravel. 2.1' (967.2 Gray fine SAND, trace fine Gravel. 2.6' (966.7) Dark grav fine-medium SAND, little-some fine-medium Gravel
965.32	4-8	2.6	4-5.6	1.2	-	Black staining from 3.0 to 3.1 feet.
964.32			5.6-6.2		-	
963.32			6.2-8	1.0	-	Gray fine-medium SAND and fine-medium GRAVEL.
962.32						7.01/0/2.2
961.32	8-10	1.8	8-10	2.0	-	Gray fine SAND. Gray fine SAND, 50me fine Gravel.
960.32						8.2' (961.1 Dark gray fine-medium GRAVEL, some fine-medium Sand.
	10-14	2.1	10-11	3.2		
	FE STAR FE FINISI LLING C LLING M SIZE: 1.3 TYPE: Ja 969.32 968.32 966.32 966.32 965.32 964.32 963.32 963.32 963.32 961.32 960.32	FE STARTED: 2/17 FINISHED:2/17. LLING COMPANY LLING METHOD: SIZE: 1.5 Inch X 4 TYPE: Jackhamme (i) 969.32 969.32 966.32 965.32 965.32 966.32 964.32 961.32 8-10 960.32	FE STARTED: 2/17/2000 FE FINISHED:2/17/2000 LLING COMPANY: BBL LLING METHOD: Direct I SIZE: 1.5 Inch X 4 Feet TYPE: Jackhammer 969.32 0-4 968.32 966.32 966.32 964.32 963.32 963.32 963.32 961.32 8-10 960.32	FE STARTED: 2/17/2000 FE FINISHED:2/17/2000 LLING COMPANY: BBL LLING METHOD: Direct Push SIZE: 1.5 Inch X 4 Feet TYPE: Jackhammer (i) (ii) (iii) (iiii) (iiiii) (iiiiiii) (iiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiii	Image: Constraint of the sector of the se	Image: Constraint of the second se

Shake test: "-" denotes no sheen or NAPL observed; N denotes NAPL observed; S denotes sheen observed; T = trace.

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DAT DAT DRII DRII BIT RIG	E STARTE E FINISHE LLING COI LLING ME SIZE: 1.5 Ir TYPE: Jack	D: 2/17/200 D: 2/17/200 MPANY: E THOD: Din thich X 4 Fee thammer	00)0 BBL rect Push t	1	BOREHOLE DEPTH: 14.0 Feet DESCRIPTIONS BY: Nicholas A. Smith NORTHING: 533418.25 EASTING: 133318.24 GROUND ELEVATION: 969.32			BORING ID: HR-SC-4 CLIENT: General Electric Company Pittsfield, MA SITE: Housatonic River	
DEPTH (A)	ELEVATION (f)	SAMPLE DEPTH INTERVAL (f)	RECOVERV (ft)	SCREENING DEPTH INTERVAL (ft)	STRATIGRAPHIC DESCRIPTION				
11	958.32	10-14	2.1	11-14	4.5	-	Dark gray fine-medium GRAVEL	., some fine-medium Sand.	
12	957.32								
13	956.32							12.9' (956.42'	
14	955.32						Dark gray-black fine-medium SA	ND. some fine-medium Gravel and Pebbles.	
15							Boring terminated at 14.0 feet (95	5.32 feet).	
16									
17			· · · · · · · · · · · · · · · · · · ·						
18									
19									
20									
21									
22									

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DA DA DR DR BIT RIC	TE START TE FINISH ILLING CO ILLING MI SIZE: 1.5 G TYPE: Jac	ED: 2/14// ED:2/14/2 OMPANY ETHOD: 1 Inch X 4 F :khammer	2000 2000 : BBL Direct P Sect	ush	BO DE: NO EA: GR	REHO SCRIP RTHII STING OUND	CLE DEPTH: 10.0 Feet TIONS BY: Nicholas A. Smith NG: 533429.11 S: 133328.76 DELEVATION: 966.40	BORING ID: HR-SC-5 CLIENT: General Electric Company Pittsfield, MA SITE: Housatonic River
DEPTH (A)	ELEVATION (A)	SAMPLE DEPTH INTERVAL (II)	RECOVERV (ft)	SCREENING DEPTII INTERVAL (fi)	PID IIEADSPACE (ppm)	SIIAKE TEST	STI D	RATIGRAPHIC ESCRIPTION
0	966.40	0-1.8	0.8	0-1.8	42	S	Black fine-medium SAND, trace O	rganics, sheen.
			1				-	
1	965.40							
2	964.40	1.8-6	3.8	1.8-3	280	N		
							Black fine-medium SAND Little fir	2.2' (964.20')
3	963.40			3-4	175	N	Black Inte-Interfaint SAIVD, Inter In	
								3.4' (963.00')
4	962.40			4-6	168	TN	Gray fine-medium SAND, multico	lor sheen to 3.6 feet.
						S		
5	961.40					ļ	Dark gray fine SAND, some mediu	4.9' (961.50') m Gravel, compact.
			ļ			ļ		
6	960.40	6-8	1:4	6-8	86	-		
								6.8' (959.60')
/	959.40						Gray fine SAND, trace sheen (may	be from above). 7 1' (959 30')
							Dark gray fine SAND, some mediur Isolated specks of sheen noted on to	n Gravel, compact.
8	958.40	8-10	1.7	8-10	7.4	-	isolated speeks of sheel hoted of 10	oseneu graver preces (may be irom above).
9	957.40							
10	956.40							
							Boring terminated at 10.0 feet (956.	40 feet).
REN Bori	IARKS: ng installed	through an	oproxima	ntely foot	of stand	ling wa	ter containing NAPL.	

Shake test: "-" denotes no sheen or NAPL observed; N denotes NAPL observed; S denotes sheen observed; T= Trace.

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DA' DA' DRI DRI BIT RIG	TE STARTI TE FINISH ILLING CC ILLING MH SIZE: 1.5 I STYPE: Jac	ED: 2/15/ ED:2/15/2 OMPANY ETHOD: Inch X 4 F khammer	2000 2000 ': BBL Direct Pr Feet	ush	BOR DESC NOR EAST GRO	EHOI CRIPT THIN TING: UND	JE DEPTH: 10.0 Feet FIONS BY: Nicholas A. Smith G: 533432.30 133344.55 ELEVATION: 966.09	BORING ID: HR-SC-6 CLIENT: General Electric Comp Pittsfield, MA SITE: Housatonic River	bany	
DEPTH (A)	ELEVATION (6)	SAMPLE DEPTII INTERVAL (f)	RECOVERY (ft)	SCREENING DEPTH INTERVAL (ft)	PID IIEADSPACE (ppm)	STRATIGRAPHIC JESCRIPTION UP AND				
0	966.09	0-3	2.2	0-0.7	178	N				
							Black fine-medium SAND, some	e fine-medium Gravel, contains NAI	°L.	
1	965.09			0.7-1.2	75	N				
				1.2-3	26	N			5' (964-50')	
2	964.09						Gray fine SAND, moderate sheer	1 from 1.5 to 2.0 feet.	<u>5 (904.59)</u>	
								2.	<u>4' (963.69')</u>	
3	963.09	3-7	4.0	3-4	24	-	- Gray fine-medium SAND, trace s Note: Core liner is coated with N	sheen. APL		
							ALTERNATING SEQUENCE:	3	.0' (963.09')	
4	962.09			4-5	22	-	LAYER 1 - Gray fine SAND, la LAYER 2 - Gray fine-medium	ayers 2" to 6" thick: SAND, layers 2" to 6" thick:		
5	961.09			5-7	28	-				
6	960.09									
7	959.09	7-10	3.0	7-8	17.5	-		7.0)' (959.09')	
							ALTERNATING SEQUENCE: LAYER 1 - Grav fine SAND Ia	ivers 1" to 2" thick		
8	958.09			8-10	17	-	LAYER 2 - Gray fine-medium S	SAND, layers 2" to 6" thick;		
9	957.09									
10	956.09									
							Boring terminated at 10.0 feet (95	6.09 feet).		
REM. Borin Borin Shake	ARKS: ig installed th ig backfilled e test: "-" den	hrough ap to 6 inche notes no s	proximates with behavior 1	tely 1 foot o entonite gro NAPL obser	f standing ut, and to ved; N de	g wate) surfa	r containing NAPL. ce with bentonite chips. NAPL observed.			

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DAT DAT DRI DRI BIT RIG	E STARTE E FINISHE LLING COL LLING ME SIZE: 1.5 lr TYPE: Jack	D: 2/17/2 D:2/17/2 MPANY: THOD: E nch X 4 Fe chammer	000 000 BBL Direct Pu eet	sh	BOR DESC NOR EAST GRO	EHOI CRIP THIN FING: UND	LE DEPTH: 10.0 Feet BC FIONS BY: Nicholas A. Smith CI G: 533404.97 133316.96 ELEVATION: 969.31 SI	ORING ID: HR-SC-7 LIENT: General Electric Company Pittsfield, MA TE: Housatonic River
DEPTH (A)	ELEVATION (f)	SAMPLE DEPTH INTERVAL (G)	RECOVERY (ft)	SCREENING DEPTH INTERVAL (ft)	PID HEADSPACE (ppm)	SIIAKE TEST	STRAT DESC	TIGRAPHIC CRIPTION
0	969.31	0-3	2.0	0-1	2.0			
							Brown fine-medium SAND and fine-m	nedium GRAVEL.
1	968.31			1-3	0.5	-		
						1		
2	967.31		1			1		1.8' (967.51')
			1		1	1	Brown-gray fine SAND, little fine-med	lium Gravel, loose.
3	966 31	3-7	40	3-5	0.4	<u> </u>		
	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,							3.0' (966.31')
	965 31				1	+	Oray very line-line SAND, loose.	
	905.51							
5	964.31			5-7	0.5	-		
						_		
6	963.31	ļ					Gray fine SAND.	5.7' (963.61')
					ļ	ļ		
7	962.31	7-10	2.6	7-8.5	0.4	ļ.,		- 11/0/1 - 11
			ļ				Gray fine SAND, loose, trace orange in	
8	961.31							
				8.5-10	0.0	-		
9	960.31						-	8.9' (960.41')
							Dark gray fine-medium SAND, some f	īne Gravel.
10	959.31							
							Boring terminated at 10.0 feet (959.31)	feet).
REM Boring	ARKS: g installed th	rough app	proximate	ely 1 foot of	f standin	g wate	r.	

Boring backfilled to 6 inches with bentonite grout, and to surface with bentonite chips. Shake test: "-" denotes no sheen or NAPL observed; N denotes NAPL observed; S denotes sheen observed; T = trace.

				****				Page 1 of 1
DAT DAT DRI DRI BIT RIG	E STARTE E FINISHE LLING CO LLING ME SIZE: 1.5 Ir TYPE: Jack	2D: 2/17/2 2D:2/17/20 MPANY: THOD: E nch X 4 Fe khammer	000 000 BBL Direct Pu: eet	sh	BOR DES NOR EAS GRO	REHO CRIP RTHIN TING DUND	LE DEPTH: 10.0 Feet TIONS BY: Nicholas A. Smith RG: 533415.50 : 133327.43 ELEVATION: 969.92	BORING ID: HR-SC-8 CLIENT: General Electric Company Pittsfield, MA SITE: Housatonic River
DEPTH (A)	ELEVATION (ft)	SAMPLE DEPTH INTERVAL (A)	RECOVERY (f)	SCREENING DEPTH INTERVAL (ft)	PID IIEADSPACE (ppm)	SHAKE TEST	ST	RATIGRAPHIC DESCRIPTION
0	969.92	0-2	1.0	0-2	4.8	S	Brown fine-medium SAND, little	fine Gravel.
l	968.92					<u> </u>		1.21/0/0.72
			1				Black fine SAND, little Silt, fine-r	medium Gravel, and Wood.
2	967.92	2-6	2.5	2-3.5	5.2	-	Black staining from 1.6 to 2.0 feet	L.
3	966.92							
				3.5-6	1.6	-		3 5' (966 42'
4	965.92						Dark gray-black fine SAND, trace	medium Gravel.
5	964.92							5 1' (964 82')
							Black fine-medium SAND and fine	e-medium GRAVEL, some staining.
6	963.92	6-10	3.8	6-7	1.3	-		
	0(2.02			7.0			Dark gray fine-medium SAND and 6.2 to 6.7 feet.	6.2' (963.72') d fine-medium GRAVEL, slight staining from
1	962.92			/-9	1.0	-		7 2' (962 72)
							Brown-gray very fine-fine SAND,	trace-little Pebbles, loose.
8	961.92							8.0' (961.92')
							Brown fine SAND, occasional Peb	bles, firm.
9	960.92			9-10	1.2	-		
10	959.92							
							Boring terminated at 10.0 feet (959	0.92 feet).

Shake test: "-" denotes no sheen or NAPL observed; N denotes NAPL observed; S denotes sheen observed; T = trace.

DA DA DR DR BIT RIC	TE START TE FINISH ILLING CC ILLING MI SIZE: 1.5 G TYPE: Jac	ED: 2/14/ ED:2/14/2 OMPANY ETHOD: Inch X 4 F	2000 2000 ': BBL Direct P Reet	ush	B D D E A G	OREHO ESCRIP ORTHII ASTING ROUND	DLE DEPTH: 10.0 Feet BORING ID: HR-SC-9 PTIONS BY: Nicholas A. Smith CLIENT: General Electric Company Pittsfield, MA ING: 533420.22 SITE: Housatonic River G: 133349.07 SITE: Housatonic River
DEPTH ((I)	ELEVATION (f)	SAMPLE DEPTH INTERVAL (ft)	RECOVERY (f)	SCREENING DEPTH INTERVAL (ft)	PID IIEADSPACE (ppnt)	SHAKE TEST	STRATIGRAPHIC DESCRIPTION
0	968.44	0-2	2.0	0-1	12	TS	Dark gray fine-medium SAND, little fine-medium Gravel, trace Organics
			ļ		ļ		Brown fine-medium SAND, little fine-medium Gravel
	967.44		<u> </u>	1-2	6		Grav fine SAND_trace Organics and fine Gravel_compact
	0((1)	26	<u> </u>	2.1			
2	966.44	2-6	3.0	2-4	4		-
3	965.44		<u> </u>			-	
	,						
4	964-44			4-6	0.5	-	4.0' (964.14')
							Brown fine SAND, little medium Gravel.
5	963.44						
6	962.44	6-10	3.0	6-8	1.8	-	
7	961.44						7.0' (961.44')
			ļ			ļ	7.5' (960.94')
8	960.44			8-10	0.0	-	Dark gray-black fine-medium SAND and medium GRAVEL 8.0' (960.44')
							Light brown-black fine-medium SAND 8.2' (960.24')
9	959.44						
10	958.44						
							Boring terminated at 10.0 feet (958.44 feet).
REN Bori	AARKS:	d to 6 incl	nes with	bentonite	grout, a	nd to sur	rface with bentonite chips.

Shake test: "-" denotes no sheen or NAPL observed: TS denotes trace sheen observed.

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DATE STARTED: 2/14/2000 DATE FINISHED:2/14/2000 DRILLING COMPANY: BBL DRILLING METHOD: Direct Push BIT SIZE: 1.5 Inch X 4 Feet RIG TYPE: Jackhammer						REHO SCRIP RTHIN STINC DUND	DLE DEPTH: 10.0 Feet TIONS BY: Nicholas A. Smith NG: 533426.78 : 133366.83 DELEVATION: 968.86	BORING ID: HR-SC-10 CLIENT: General Electric Company Pittsfield, MA SITE: Housatonic River
DEPTH (A)	ELEVATION (ft)	SAMPLE DEPTH INTERVAL (I)	RECOVERY (ft)	SCREENING DEPTH INTERVAL (f)	PID READSPACE (ppm)	SHAKE TEST	ST	RATIGRAPHIC DESCRIPTION
0	968.86	0-2	2.0	0-2	2.4	-	Dark brown fine SAND, little fine	Gravel.
1	967.86						Wood fragment	<u>1.0' (967.86'</u> 1.3' (967.56'
2	966.86	2-6	2.5	2-3	0.8	-	Dark brown fine SAND, trace wo	ody Organics, very compact.
3	965.86			3-6	0.2	-		
4	964.86						Dark brown-gray fine-medium SA	3.5' (965.36' ND, trace-little fine Gravel and Organics.
5	963.86						Trace black coloring in organics fr	om 4.5 to 4.8 leet. 4.8' (964.06'
	0.40.04						Gray fine-medium SAND, trace fir	ne Gravel.
6	962.86	6-10	.3.0	6-8	0.0	-		
7	961.86						Light begung grou fine SAND	7.0' (961.86)
8	960.86			8-10	0.0	-	Light brown-gray fine SAND.	
9	959.86							
10	958.86						Fine-medium SAND seam from 8.5	9 TO 9.1 feet.
10 1		L	ļ				Boring terminated at 10.0 feet (958	(86 feet)

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DATE STARTED: 2/14/2000 DATE FINISHED:2/14/2000 DRILLING COMPANY: BBL DRILLING METHOD: Direct Push BIT SIZE: 1.5 Inch X 4 Feet RIG TYPE: Jackhammer					BORE DESCI NORT EASTI GROU	HOL RIPT HINC NG: ND E	E DEPTH: 9.25 Feet BORING ID: HR-SC-11 IONS BY: Nicholas A. Smith CLIENT: General Electric Company Pittsfield, MA G: 533401.49 SITE: Housatonic River I33341.53 SITE: Housatonic River
DEPTH (A)	ELEVATION (ft)	SAMPLE DEPTH INTERVAL (A)	RECOVERY (ft)	SCREENING DEPTH INTERVAL (ft)	PID HEADSPACE (ppm)	SIIAKE TEST	STRATIGRAPHIC DESCRIPTION
0	966.79	0-1.25	0.5	0-1.25	0.0	-	Brown fine-medium SAND, trace-little fine Gravel.
	0(5.70						
1	965.79	1.25-5.25	1.7	1.25-5.25	0.0	-	
2	964.79						
							Brown fine-medium SAND, little-some fine Gravel.
3	963.79						
4	962.79						
5	961.79						
		5.25-9.25	2.3	5.25-9.25	0.0	-	
6	960.79						
7	959.79						
8	958.79						
9	957.79						8.95' (957.84') Brown fine SAND, little fine medium Groupl
							Boring terminated at 0.25 feet (057.54 feet)
10	956.79						Doring terminated at 7.23 leet (937.34 leet).
				L			

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DATE STARTED: 2/14/2000 DATE FINISHED:2/14/2000 DRILLING COMPANY: BBL DRILLING METHOD: Direct Push BIT SIZE: 1.5 Inch X 4 Feet RIG TYPE: Jackhammer						REHO CRIP RTHIN TING JUND	LE DEPTH: 10.0 Feet TIONS BY: Nicholas A. Smith NG: 533407.25 : 133355.57 ELEVATION: 968.26	BORING ID: HR-SC-12 CLIENT: General Electric Company Pittsfield, MA SITE: Housatonic River
DEPTH (A)	ELEVATION (ft)	SAMPLE DEPTH INTERVAL (ft)	RECOVERY (f)	SCREENING DEPTH INTERVAL (ft)	PID ILEADSPACE (ppm)	SHAKE TEST	STF D	RATIGRAPHIC ESCRIPTION
0	968.26	0-2	1	0-2	0.0	-	Dark brown fine medium SAND to	man Organias and fine Gravel
			<u> </u>		ļ	ļ	Dark brown mie-medium SAND, u	ace organics and the oraver.
1	967.26							1 21 (067.061)
	0// 2/	26	2.0				Brown fine-medium SAND trace fi	ine Gravel loose
2	966.26	2-6	2.8	2-4	0.0	-		
3	965.26							
								· · · ·
4	964.26			4-6	0.0	-		3.9' (964.36')
							Brown fine-coarse SAND, some fin	e Gravel.
5	963.26							5.2' (963.06')
							Brown-dark brown fine-coarse SAN	ID, little fine-medium Gravel, trace Organics.
6	962.26	6-10	4.0	6-7	0.0	-	Dark gray fine-medium SAND, little	e medium Gravel, trace Organics.
							Gray fine SAND, trace medium Gra	vel (black shale, rounded)
7	961.26			7-8	0.0	-	an in the second sec	7.0' (961.26')
							Light gray fine SAND, trace mediur	n Sand.
8	960.26			8-10	0.0	-		
							Boring terminated at 10.0 feet (958.	26).
9	959.26							
10	958.26							
REM/ Borin Shake	ARKS: g backfilled : test: "-" der	to 6 inche	s with be	entonite gr	out, and	to sur	face with bentonite chips.	

DATE STARTED: 2/15/2000 DATE FINISHED:2/15/2000 DRILLING COMPANY: BBL DRILLING METHOD: Direct Push BIT SIZE: 1.5 Inch X 4 Feet RIG TYPE: Jackhammer						EHO CRIP THIN FING	LE DEPTH: 10.0 Feet FIONS BY: Nicholas A. Smith IG: 533432.02 : 133377.18 ELEVATION: 969.10	BORING ID: HR-SC-13 CLIENT: General Electric Company Pittsfield, MA SITE: Housatonic River
DEPTII (fi)	ELEVATION (ft)	SAMPLE DEPTH INTERVAL (ft)	RECOVERY (ft)	SCREENING DEPTH INTERVAL (ft)	PID IIEADSPACE (ppm)	SIIAKE TEST	ST	TRATIGRAPHIC DESCRIPTION
0	9 69.10	0-2	2.0	0-1.2	0.0	-	Brown fine-medium SAND som	e fine-medium Gravel
						<u> </u>	brown mechedian Strivb, som	
1	968.10					_	Dark brown-black fine SAND so	1.2' (967.90')
				1.2-2	0.0	-	Note: black color is from wood d	ecay.
2	967.10	2-6	3.7	2-3.8	1.1	-		
	0((10					<u> </u>		
	966.10		<u> </u>			<u> </u>		3 5' (965 60')
4	265.10			3.8-6	0.0		Light gray fine SAND, layered w	ith dark brown fine SAND, some silt and
							Grev medium coarce SAND_trac	4.1' (965.00')
5	964.10	L					Gray mediam-coarse shave, the	
						1		
6	963.10	6-10	4.0	6-8	0.0	-		
7	962.10					ļ		
					ļ	<u> </u>		7.4' (961.70')
8	961.10			8-10	0.0	-	Pebble at 7.9 feet.	7.9' (961.20')
9	960.10					<u> </u>	Light gray fine SAND, trace med	ium Sand lenses.
	0.50.57							
10	959.10				 	<u> </u>	Boring terminated at 10.0 feet (95	59.10 feet).
DEM		I	l	<u> </u>	1	<u> </u>		

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DATE STARTED: 2/15/2000 DATE FINISHED:2/15/2000 DRILLING COMPANY: BBL DRILLING METHOD: Direct Push BIT SIZE: 1.5 Inch X 4 Feet RIG TYPE: Jackhammer						REHO CRIP RTHIN TING DUND	LE DEPTH: 10.0 Feet TIONS BY: Nicholas A. Smith NG: 533419.60 : 133377.01 ELEVATION: 968.75	BORING ID: HR-SC-14 CLIENT: General Electric Company Pittsfield, MA SITE: Housatonic River
DEPTIL (A)	ELEVATION (ft)	SAMPLE DEPTH INTERVAL (f)	RECOVERY (ft)	SCREENING DEPTH INTERVAL (ft)	PID HEADSPACE (ppm)	SHAKE TEST	ST	RATIGRAPHIC DESCRIPTION
0	968.75	0-2	1.9	0-1	0.9	-	Brown fine-medium SAND, little	fine Gravel.
			1					
1	967.75			1-2	2.2	-	Dark brown-black fine SAND, sor	ne Silt and Woody Organics, compact.
	044.75	2.6	10	2.4	0.2	<u> </u>		
	900.73	2-0	4.0	2-4	0.5		Dark brown fine SAND, some Silt	2.0' (966.75') trace Woody Organics.
3	965.75				1		Brown-gray fine-medium SAND	2.4' (966.35') trace-little fine Gravel coarsening downward
	705.75				1		Brown gray mie mearan or nob,	
4	964.75			4-6	0.0	-		
			1					
5	963.75							
6	962.75	6-10	3.3	6-9	0.0	-		
7	961.75				ļ	ļ		
		ļ			<u> </u>			
8	960.75		ļ					
					<u> </u>			
9	959.75			9-10	0.0	-	Trace pebbles from 8.7 to 9.1 feet.	9.1' (959.65')
10	958.75				<u> </u>		Brown fine SAND.	
							Boring terminated at 10.0 feet (958	2.75 feet).
REM. Borin Shake	ARKS: ng backfilled	to 6 inche	s with be	entonite g	rout, and	to su	face with bentonite chips.	

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*DATE STARTED: 2/15/2000 DATE FINISHED:2/15/2000 DRILLING COMPANY: BBL DRILLING METHOD: Direct Push BIT SIZE: 1.5 Inch X 4 Feet RIG TYPE: Jackhammer						OREH ESCRI ORTH ASTIN ROUN	OLE DEPTH: 8.0 Feet PTIONS BY: Nicholas A. Smith ING: 533449.39 G: 133373.39 D ELEVATION: 965.72	BORING ID: HR-SC-15 CLIENT: General Electric Company Pittsfield, MA SITE: Housatonic River
DEPTII (A)	ELEVATION (ft)	SAMPLE DEPTH INTERVAL (ft)	RECOVERY (ft)	SCREENING DEPTH INTERVAL (f)	PID HEADSPACE (ppm)	SHAKE TEST	STR DF	ATIGRAPHIC ESCRIPTION
0	965.72	0-4	2.5	0-1	22	N	Brown fine-medium SAND, little-so	me fine Gravel, sheen.
1	964.72			1-4	24	-	(Sheen may be from overlying water)	
2	963.72							1.9' (963.82')
							Gray fine SAND, loose, scattered she	eens from 1.9 to 2.9 feet.
3	962.72						(Sneen may be from overlying water)
						ļ		:
4	961.72	4-8	4.0	4-6	24	-	Grav fine SAND, trace medium Sand	4.0' (961.72')
								. .
5	960.72						-	
6	959.72		· ·	6-8	16	-		
7	958.72							
8	957.72							-
			ļ				Boring terminated at 8.0 feet (957.72	2 feet).
9	956.72					<u> </u>		
						<u> </u>		
10	955.72						-	
REM Borin Borin Shake	ARKS: g installed th g backfilled e test: "-" der	rough ap to 6 inch notes no	proxima les with t sheen or	tely 6 inc pentonite NAPL 0	hes of : grout, bserved	standing and to s I: N der	g water containing sheen and NAPL. surface with bentonite chips. notes NAPL observed.	

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'DAT DAT DRII DRII BIT RIG	E STARTE E FINISHE LLING COI LLING ME SIZE: 1.5 Ir TYPE: Jack	2/15/ CD:2/15/ MPANY THOD: nch X 4 1 chammer	2000 2000 2: BBL Direct F Feet	Push	B D N E G	OREHOL ESCRIPT ORTHINC ASTING: ROUND E	E DEPTH: 8.0 Feet IONS BY: Nicholas A. Smith 5: 533440.43 133361.22 LLEVATION: 965.46	BORING ID: HR-SC-16 CLIENT: General Electric Pittsfield, MA SITE: Housatonic River	c Company
DEPTH (A)	ELEVATION (ft)	SAMPLE DEPTH INTERVAL (f)	RECOVERY (ft)	SCREENING DEPTH INTERVAL (f)	PID HEADSPACE (ppm)	SUAKE TEST	ST	TRATIGRAPHIC DESCRIPTION	
0	965.46	0-4	3.4	0-1	3.8	S TN	Gray fine-medium SAND, trace	fine Gravel, trace sheen.	
1	964.46			1-2	1.2	-			
2	963.46			2-4	12.5				1.7' (963.76')
							Light gray fine SAND.		
3	962.46								• •
4	961.46	4-8	4.0	4-6	32	-	Light grav fine SAND_trace mer	lium Sand	4.0' (961.46')
	960.46								
6	959.46		·	6-8	20	-			
7	958.46								
8	957.46								
							Boring terminated at 8.0 feet (95	7.46 feet)	
9									
10									
REM Boring Borin Shake	ARKS: g installed th g backfilled e test: "-" de	to 6 incl notes no	pproxim hes with sheen o	ately 8 ir bentonit r NAPL	nches of s e grout, a observed	standing wa and to surfa : N denotes	ter containing sheen and NAPL. ce with bentonite chips. NAPL observed: S denotes sheen	observed; T = trace.	

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DATE STARTED: 2/17/2000 DATE FINISHED:2/17/2000 DRULLING COMPANY: BBL						OREHC	DLE DEPTH: 8.0 Feet	BORING ID: HR-SC-17
DRI BIT RIG	LLING ME SIZE: 1.5 I TYPE: Jac	THOD: nch X 4 khamme	Direct I Feet	Push	D N E. G	ORTHI ASTINC ROUNE	NG: 533401.26 5: 133284.29 • ELEVATION: 972.40	Pittsfield, MA SITE: Housatonic River
DEPTH (ft)	ELEVATION (ft)	SAMPLE DEPTH INTERVAL (ft)	RECOVERY (ft)	SCREENING DEPTH INTERVAL (f)	PID HEADSPACE (ppm)	SHAKE TEST	STI D	RATIGRAPHIC ESCRIPTION
0	972.40	0-4	1.6	0-0.7	0.7	TS	Brown fine SAND and SILT, trace	Organics, compact.
1	971.40			0.7-4	5.2	TS		
2	970.40							
3	969.40							3.1' (969.30')
		_					Brown fine-medium SAND. Black staining from 3.3 to 3.8 feet.	
4	968.40	4-8	1.7	4-5	2.0		Brown block fine medium SAND	3.8' (968.60')
						<u> </u>	Brown-black Inte-incolum SAND,	nue me-neurum Graver.
5	967.40			5-8	2.0	-		
6	966.40		· · ·			1		
				<u> </u>		1		6.3' (966.10')
7	965.40					1	Brown-gray fine-medium SAND an	nd fine-medium GRAVEL.
								7 3' (965 10')
8	964.40						Light brown-gray fine SAND, little	-some Pebbles-Cobbles.
							Boring terminated at 8.0 feet (964.4	10 feet)
9	963.40							
				I				
10	962.40							
				I				
REN	ARKS:	d to 6 in	obac wit	h hontonite	arout	and to a	for with hostonity shine	

Shake test: "-" denotes no sheen or NAPL observed; N denotes NAPL observed; S denotes sheen observed; T = trace.