SHEAR-WAVE VELOCITY COMPILATION FOR NORTHRIDGE STRONG-MOTION RECORDING SITES

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ABSTRACT

Borehole and other geotechnical information collected at the strong-motion recording sites of the Northridge earthquake of January 17,1994 provide an important new basis for the characterization of local site conditions. These geotechnical data, when combined with analysis of strong-motion recordings, provide an empirical basis to evaluate site coefficients used in current versions of US building codes. Shear-wave-velocity estimates to a depth of 30 meters are derived for 176 strong-motion recording sites. The estimates are based on borehole shear-velocity logs, physical property logs, correlations with physical properties and digital geologic maps. Surface-wave velocity measurements and standard penetration data are compiled as additional constraints. These data as compiled from a variety of databases are presented via GIS maps and corresponding tables to facilitate use by other investigators.

INTRODUCTION

Ground shaking generated by the Northridge earthquake of January 17,1994 was recorded by strong-motion instruments at more than 200 sites located out to distances of 250 km. Horizontal ground acceleration at the surface exceeded 0.9 g at seven of these sites within 22 km of the epicenter and 0.4 g at 31 sites within 40 km of the epicenter. As the largest nearsource, strong-motion data set yet collected in the US, these recordings provide an important basis for empirical estimates of site-specific amplification factors F_a and F_v as specified in current US building codes at input ground motion levels greater than 0.1g (Borcherdt, 2002a and 2002b).

Detailed geotechnical data collected over a period of several years both before and since the Northridge earthquake provides the necessary basis to characterize the local site conditions quantitatively at the recording sites. These data are needed to quantify the site conditions using shear velocity to 30 m depth and in turn classify the sites according to siteclass definitions used in present versions of building codes. This report provides a compilation of shear-wave velocity estimates to a depth of 30 m for 176 strong-motion sites. Estimates are derived from borehole seismic and physical property logs, correlations with physical properties, digital geologic maps as displayed using a GIS, and additional information as guidelines.

Site characterizations herein are based on information assembled from databases kindly provided by a number of investigators (Boore, pers. commun. 1995; Silva, pers. commun. 1995; Vucetic, pers. commun. 1996, Wills and Silva, pers. commun. 1998; Nigbor, and Bardet; ROSRINE, 1999; Gibbs et al., 1999; King, pers. commun. 1995). These data compiled in GIS as reviewed with site visits by Fumal are used to provide a relatively complete and up-to-date list of shear-velocity estimates to 30 m for the recording sites.

STRONG-MOTION SITE CHARACTERIZATION IN TERMS OF $\it{V}_{S_{30}}$

Borehole logging efforts to compile shear-wave velocity logs and corresponding physical property logs to depths of at least of 30 m provide the desired database for characterizing the strong-motion sites. The major borehole data sets as collected both before and after the Northridge earthquake by Gibbs et al. (1980; 1996), Fumal et al., (1981; 1982a; 1982b; 1984), and Nigbor et al., (1998) are tabulated in databases of Boore et al. (1995, pers. commun.), Bardet, et al. (1998, [http://rccg03.usc.edu/rosrine/\),](http://rcg03.usc.edu/rosine) Silva, (1998, pers. commun.), Wills and Silva (1998), and Vucetic, et al. (1996, pers. commun.). Many of the boreholes are near the strong-motion instrument sites and hence, provide a measurement that can be used as an accurate estimate of $v_{s_{30}}$. In other cases the boreholes are not near the recording site and some type of extrapolation is needed.

Towards developing estimates of the shear-wave velocity at sites for which no borehole measurements were nearby, geotechnical information was compiled from digital files of mapped surface geology and site-specific digital databases. Surface geology as compiled digitally from Tinsley and Fumal (1985), Yerkes (pers. commun, 1994), and Jennings, et al., (1977) were used to compile data in a geographic information system (GIS). Digital databases compiled by Boore, (pers. commun. 1996), Silva, (pers. commun, 1998), and Vucetic and Doroudian, (1995) with modifications by King (pers. commun. 1996) were compiled in GIS to provide additional site-specific information. The various geologic maps were displayed using GIS at large scales with superimposed locations labeled with estimates of v_s to 30 m as derived from boreholes and surface wave measurements. Working versions of these maps were used to infer the spatial relationships of the various data sets and derive estimates of $v_{s_{30}}$ for the strong-motion recording sites. v_s measurements as inferred from borehole and surface wave measurements are shown in Figure 1 superimposed on the digital geologic database of Tinsley-Fumal (1985) and in Figure 2 as superimposed on the digital geologic database of Jennings, et al., (1977). (Similar maps are available on request using the site condition map of Wills et al. (2001) as a base.)

Shear-wave velocity estimates for the strong-motion recording sites are tabulated in Table 1. Five types of estimates are indicated. "Type 1" estimates are those derived from borehole measurements within 300 m of the location of the strong-motion recording by the USGS (Gibbs, et al., 1980, 1996, 1999, 2000) or Agbabian and associates (Nigbor, 1998) as part of the ROSRINE program. These data are available in databases developed by Boore (1995, pers. commun.), Bardet et al. (1998), Silva (pers. commun. 1998), and Vucetic and Doroudian (1995). "Type 1b" estimates are derived from borehole measurements within 300 m of the site by CDMG, NUREG, or Crandall and Associates as compiled in the databases of Silva and Vucetic. "Type 2" estimates were derived from borehole measurements from one of the sources identified as "Type 1 or 1b", but up to distances of 1500 m from the site. "Type 3" identifies those estimates derived from averages computed from measurements in other boreholes in materials with similar physical properties (Fumal, 1978; Fumal and Tinsley, 1985). Sites used to derive the Type 3 estimates are tabulated. "Type 4" identifies those estimates derived from average velocity estimates derived herein for the corresponding geologic unit. Standard deviations for the samples used to Type 4 estimates are tabulated. "Type 5" refers to measurements inferred using surface wave techniques as summarized in the databases of Silva and Vucetic. Type 1 or 2 estimates are available for nearly all of the sites with base accelerations exceeding 0.3 g near or within the projected rupture surface.

Surface geology classifications are provided for each of the sites in Table 1. Sites were classified using physical property logs if available for Type 1 and 1b sites, onsite inspections, for sites visited, and detailed geologic digital maps if other information was not available. Site class designation was assigned to each site based on the estimate of $v_{s_{30}}$.

A histogram showing the distribution of shear-wave velocity estimates for the strongmotion recording sites is shown in Figure 3a. The corresponding histogram for only sites with borehole measurements (type 1 and 1b sites) is shown in Figure 3b. The histogram for all sites suggests a tri-modal distribution with local maximum at 337 m/s, 437 m/s and 862 m/s. This tri-modal distribution is consistent with the site class definitions used in present building code provisions. The histograms (Figures 3a and 3b) indicate gradual transitions in material characteristics at site class boundaries. They show that materials in site classes B, C, and D underlie the strong-motion recording sites with shear-wave velocities ranging between 200 and 1300 m/s.

COMPARISON OF BOREHOLE 30 *Vs* **AND SURFACE-WAVE ESTIMATES**

Shear-wave velocity measurements inferred using noninvasive surface-wave techniques are less expensive and generally easier to obtain than borehole measurements. However, uncertainties introduced by the need to infer material characteristics at depth using a variety of models and model assumptions have tended to discourage their use. Comparisons of shear velocity estimates derived from surface wave information and those derived from borehole measurements are shown in Figures 4a and 4b. Comparisons are provided for sites with type 1 borehole data as well as those with types 1, 2, 3, and 4 data. Best fitting least squares lines and correlation coefficients are shown for each comparison together with a theoretical line with unity slope and exact correlation.

The comparisons (Figures 4a and 4b) indicate significant variation in some of the surface wave inferences. The correlation between surface wave measurements and those derived from borehole data is better for Type 1 borehole estimates as opposed to all of the Type 1, 2, 3, and 4 data. However, the Type 1 sample with surface wave data is also significantly smaller. With the exception of site RPV for which the surface wave measurement appears quite anomalous and has been eliminated from the plot, the comparisons indicate that the surface wave measurements show a well-defined correlation with those inferred from borehole information. This comparison suggests that the relationships such as those indicated in Figures 4a and 4b together with surface wave measurements would indeed be useful for characterizing site conditions at sites for which borehole measurements are not feasible.

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Table 1. Identification, surface geology, and shear-wave velocity inferred from boreholes and surface-wave measurements for strong-motion stations that recorded the Northridge earthquake.

* type 1 = borehole measurement (USGS/Agbabian) within 300 m of site (note: at some of these sites velocities were measured by both

USGS and Agbabian. For these sites the USGS value is shown in the first column, Agbabian in the second.

type 1b = borehole measurement (CDMG/NUREG/Crandall) within 300 m of site

type 2 = borehole measurement (USGS/Agbabian/CDMG/NUREG/Crandall) within 1500 m of site

type 3 = inferred from average of borehole measurements in similar materials

type 4 = average velocity for geologic map unit

type 5 = surface-wave measurement

¹USGS OFR 76-731

²USGS OFR 77-850

³USGS OFR 80-378

⁴USGS OFR 81-399

⁵USGS OFR 82-407

⁶USGS OFR 82-833

⁷USGS OFR 84-681

⁸USGS OFR 99-446

⁹USGS unpublished data

¹⁰ Agbabian

Figure 1. GIS map showing strong-motion recording sites and inferred shear velocity superimposed on base geologic map of Tinsley and Fumal (1985).

Figure 2. GIS map showing strong-motion recording sites and inferred shear velocity superimposed on base geologic map of Jennings et al. (1977).

Figure 3. Histograms showing empirical distribution of shear-wave velocity estimates as inferred for each of the strong-motion recording sites (a) and for sites with borehole measurements (type 1 and 1b) as tabulated in Table 1. The distribution for all sites (a) suggests a tri-modal distribution centered at 337, 437, and 862 m/s. It shows that materials in site classes D, C, and B with shear velocities ranging between 200 and 1300 m/s underlie the strong-motion recording sites in southern California.

Figure 4. Estimates of shear-wave velocity inferred from surface-wave, shear-wave velocity measurements compared to those inferred from type-1 borehole measurements (a) and those inferred from type- 1, 2, 3, and 4 borehole information (b).