

A METHODOLOGY FOR OBTAINING DESIGN PIER SCOUR DEPTHS

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OUTLINE

- Problem statement
- Local scour at single circular piles
- Methodology for scour at complex piers without physical model tests
- Methodology for scour at complex piers with physical model tests
- Summary

PROBLEM STATEMENT

- Determine the equilibrium local scour depth at a complex structure under design flow conditions

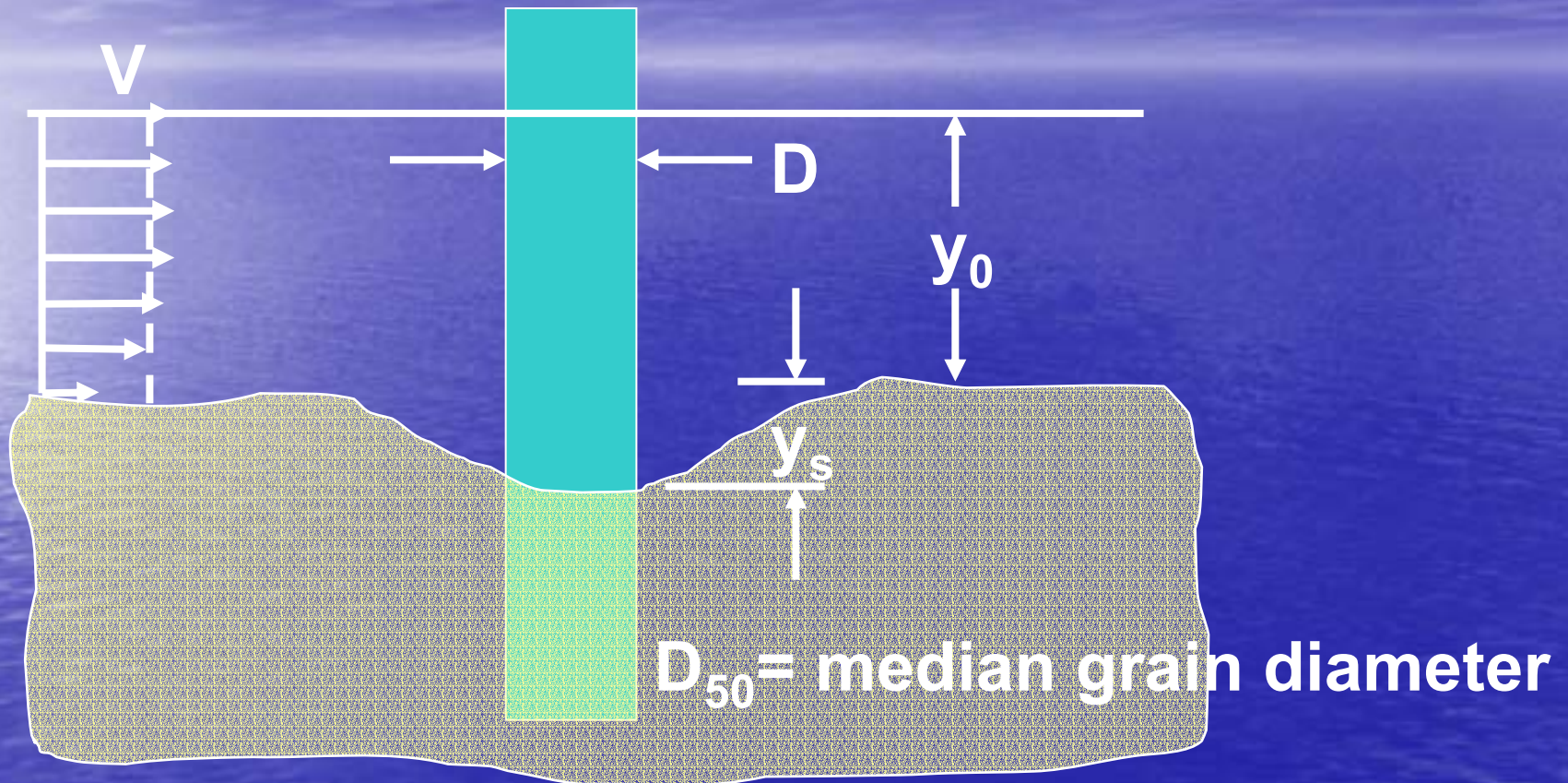
GENERAL APPROACH

- Most local scour information and knowledge is for single circular piles
- Use single pile scour knowledge to predict scour at complex structures

SINGLE PILE PREDICTIVE EQUATIONS

- Many equations in the literature
- Equations developed at University of Florida

Local Scour Definition Sketch



Background

- Research at University of Florida
 - Flume tests
 - Model studies
 - Formulation of predictive equations
- Large scale Clearwater Scour tests
 - USGS Flume in Turners Falls, Mass.
- High Velocity Live Bed Scour tests
 - University of Auckland in Auckland, NZ

Predictive Equations

- Clearwater scour

$$\frac{y_s}{D} = K_s^{2.5} f_1\left(\frac{y_0}{D}\right) f_2\left(\frac{V}{V_c}\right) f_3\left(\frac{D}{D_{50}}\right)$$

- Live Bed scour

$$\frac{y_s}{D} = f\left(\frac{y_0}{D}, \frac{V}{V_c}, \frac{D}{D_{50}}, \frac{V_{lp}}{V_c}\right)$$

Predictive equations

- Clearwater scour $0.45 \leq \frac{V}{V_c} \leq 1$

$$f_1\left(\frac{y_0}{D}\right) = \tanh\left[\left(\frac{y_0}{D}\right)^{0.4}\right],$$

$$f_2\left(\frac{V}{V_c}\right) = 1 + \frac{0.25 \ln(V/V_c)}{(V/V_c)^2},$$

$$f_3\left(\frac{D}{D_{50}}\right) =$$

$$\frac{2.95}{2.5 \exp\left[0.45\left(\log\left(\frac{D}{D_{50}}\right) - 1.64\right)\right] + 0.45 \exp\left[-2.5\left(\log\left(\frac{D}{D_{50}}\right) - 1.64\right)\right]}.$$

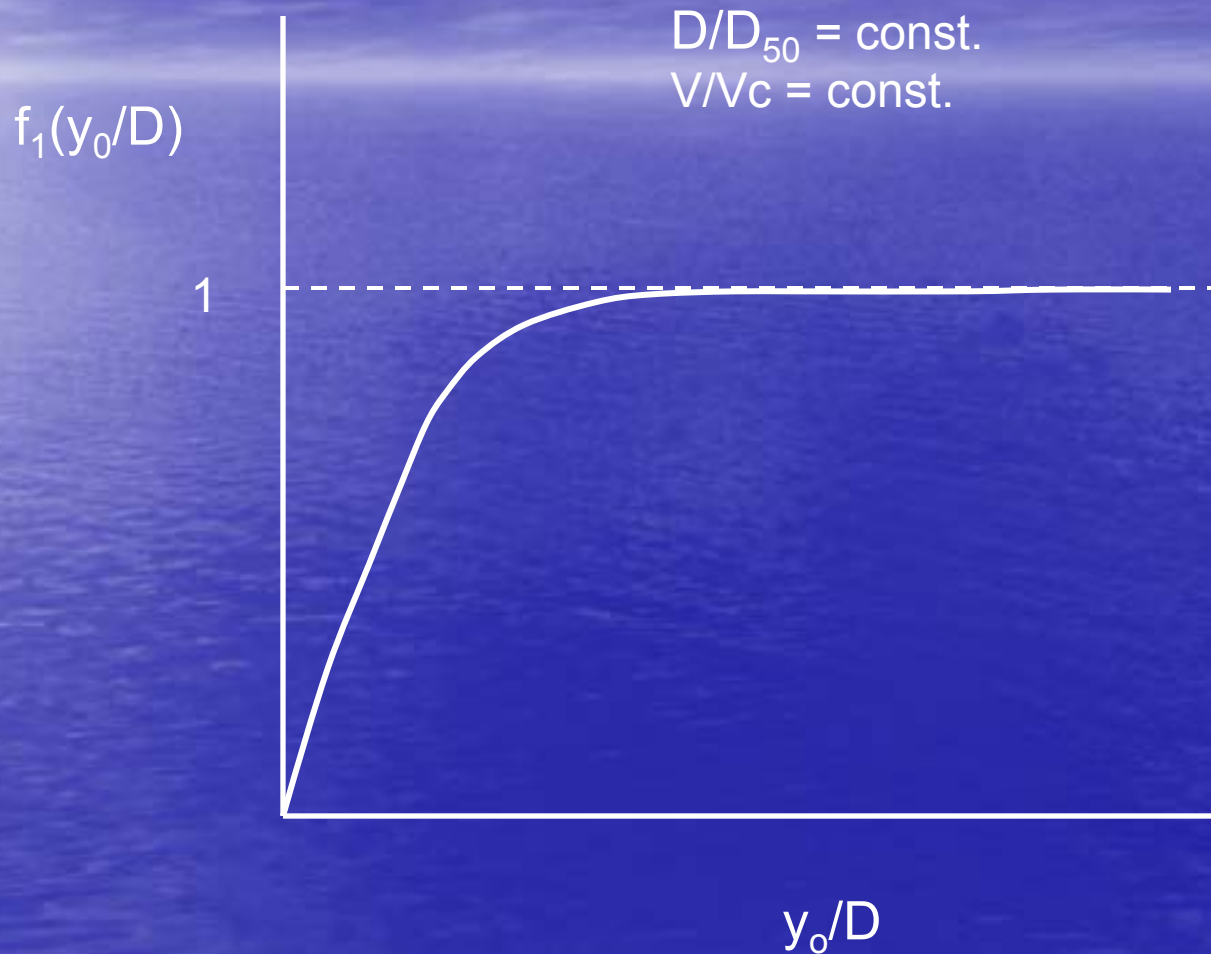
Predictive equations

$$1.0 \leq \frac{V}{V_c} \leq \frac{V_{Ip}}{V_c}$$

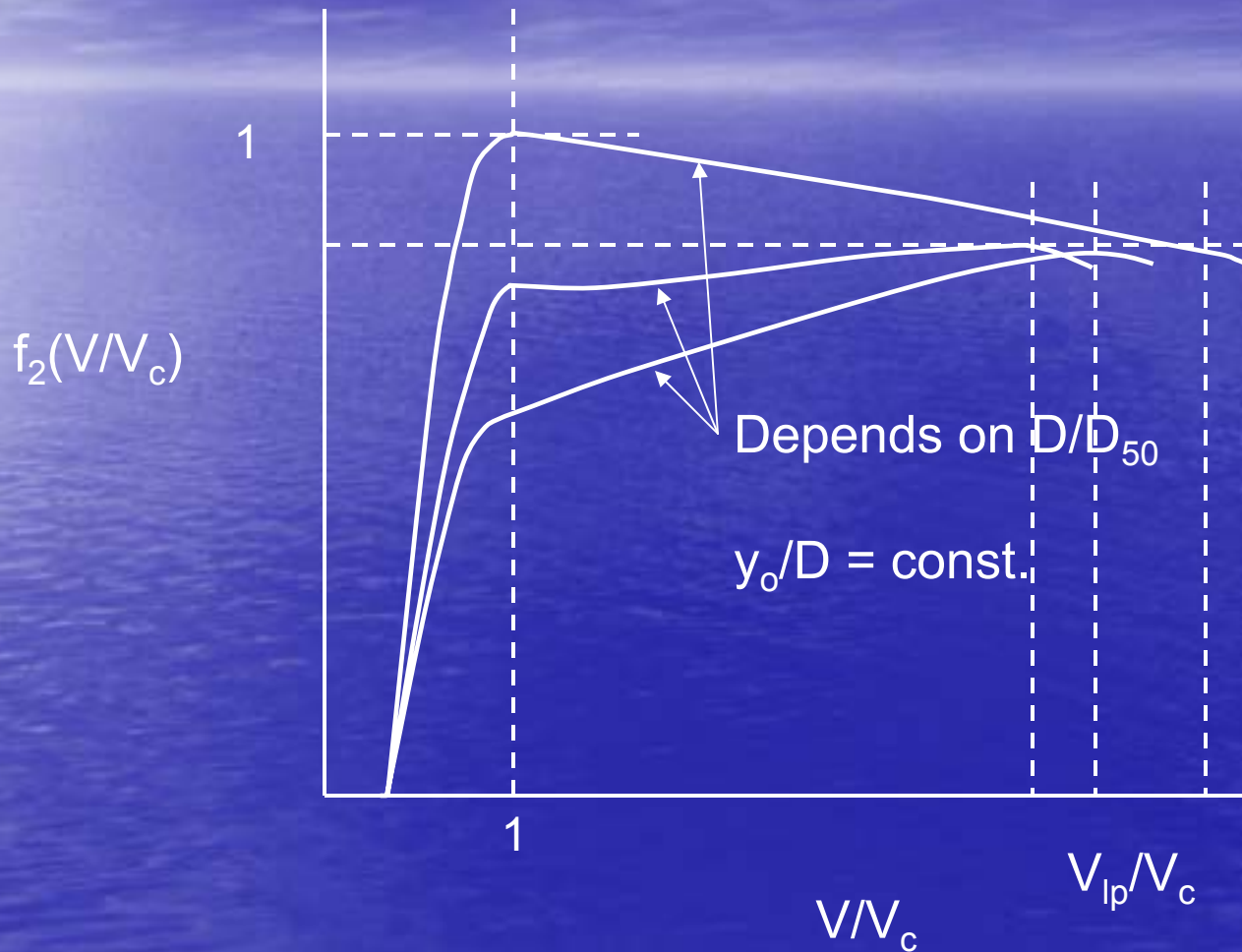
$$\frac{d_{se}}{D} = K_s f_1 \left(\frac{y_0}{D} \right) \left[2.2 \left(\frac{V - V_c}{V_{Ip} - V_c} \right) + 2.5 f_3 \left(\frac{D}{D_{50}} \right) \left(\frac{V_{Ip} - V}{V_{Ip} - V_c} \right) \right]$$

$$\frac{V}{V_c} > \frac{V_{Ip}}{V_c}$$

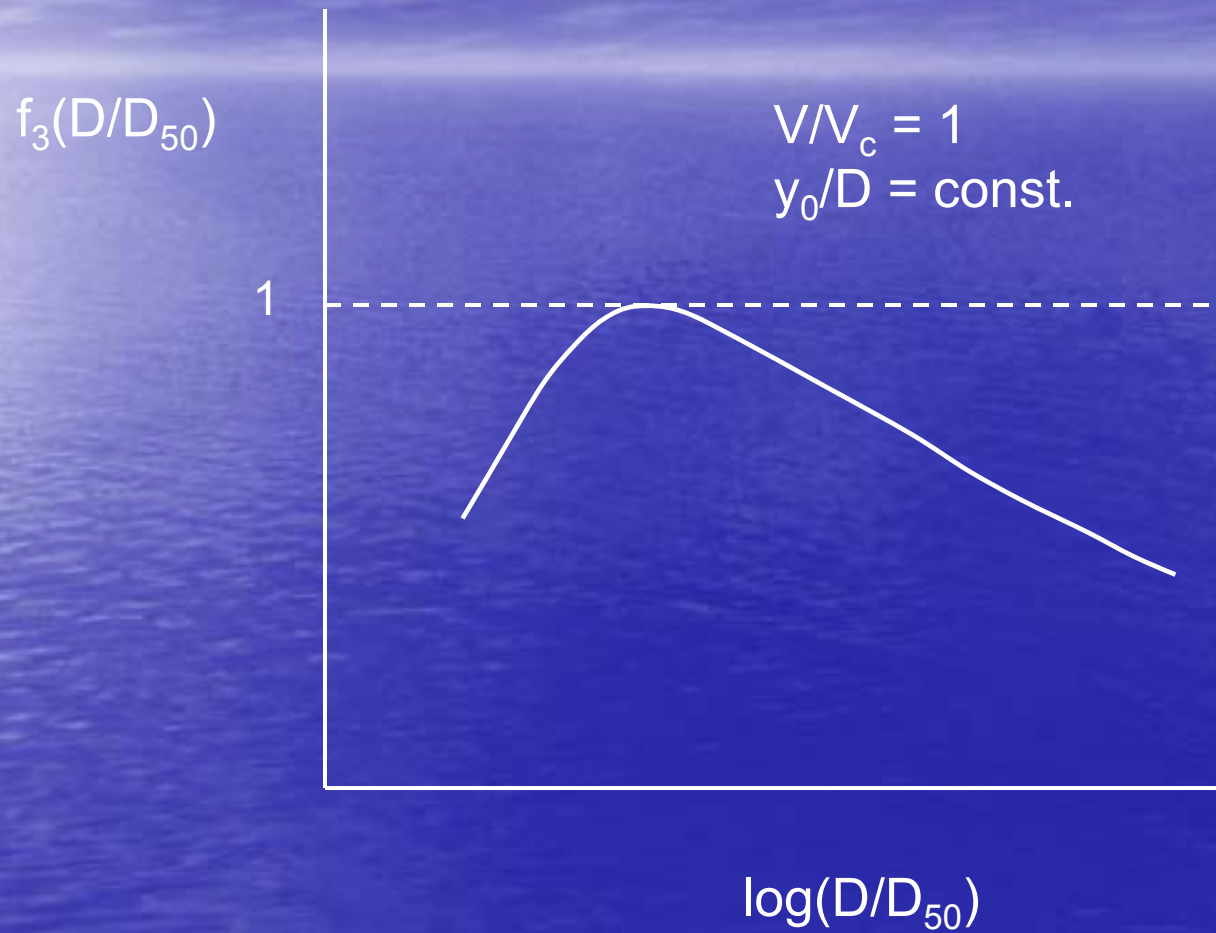
$$\frac{d_{se}}{D} = K_s 2.2 \tanh \left[\left(\frac{y_0}{D} \right)^{0.4} \right]$$



Scour Dependence on V/V_c



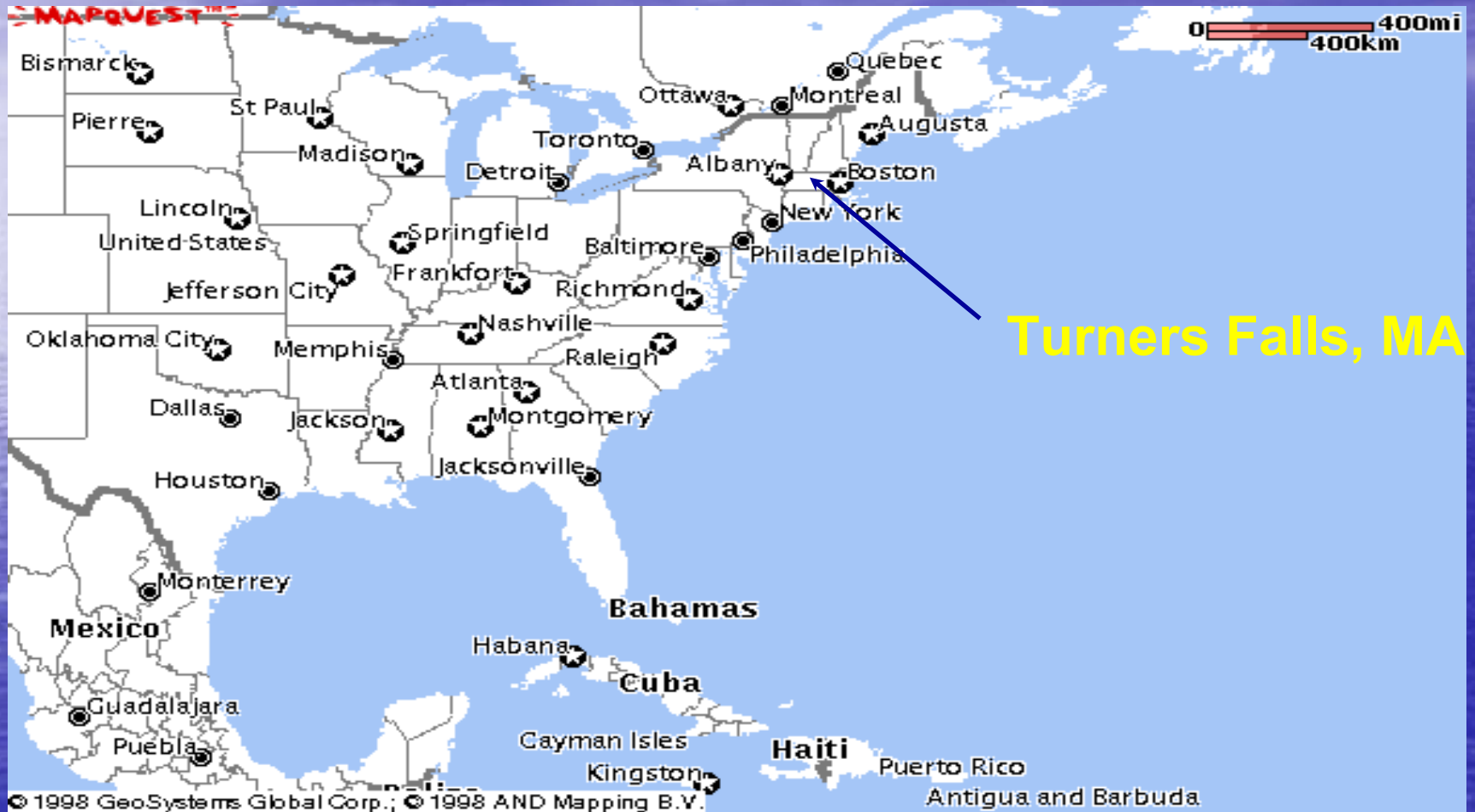
Scour Dependence on D/D_{50}



Large Structure Experiments

- Needed data for large D/D_{50}
- Located large flume at USGS Lab in Turners Falls, Massachusetts
- Performed clearwater tests with
 - 3 pile diameters (0.114 m, 0.305 m, 0.914 m)
 - 3 sediment sizes (0.22 mm, 0.8 mm, 2.9 mm)
- D/D_{50} values up to 4,156

USGS-BRD Laboratory



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Clearwater Tests in USGS Flume



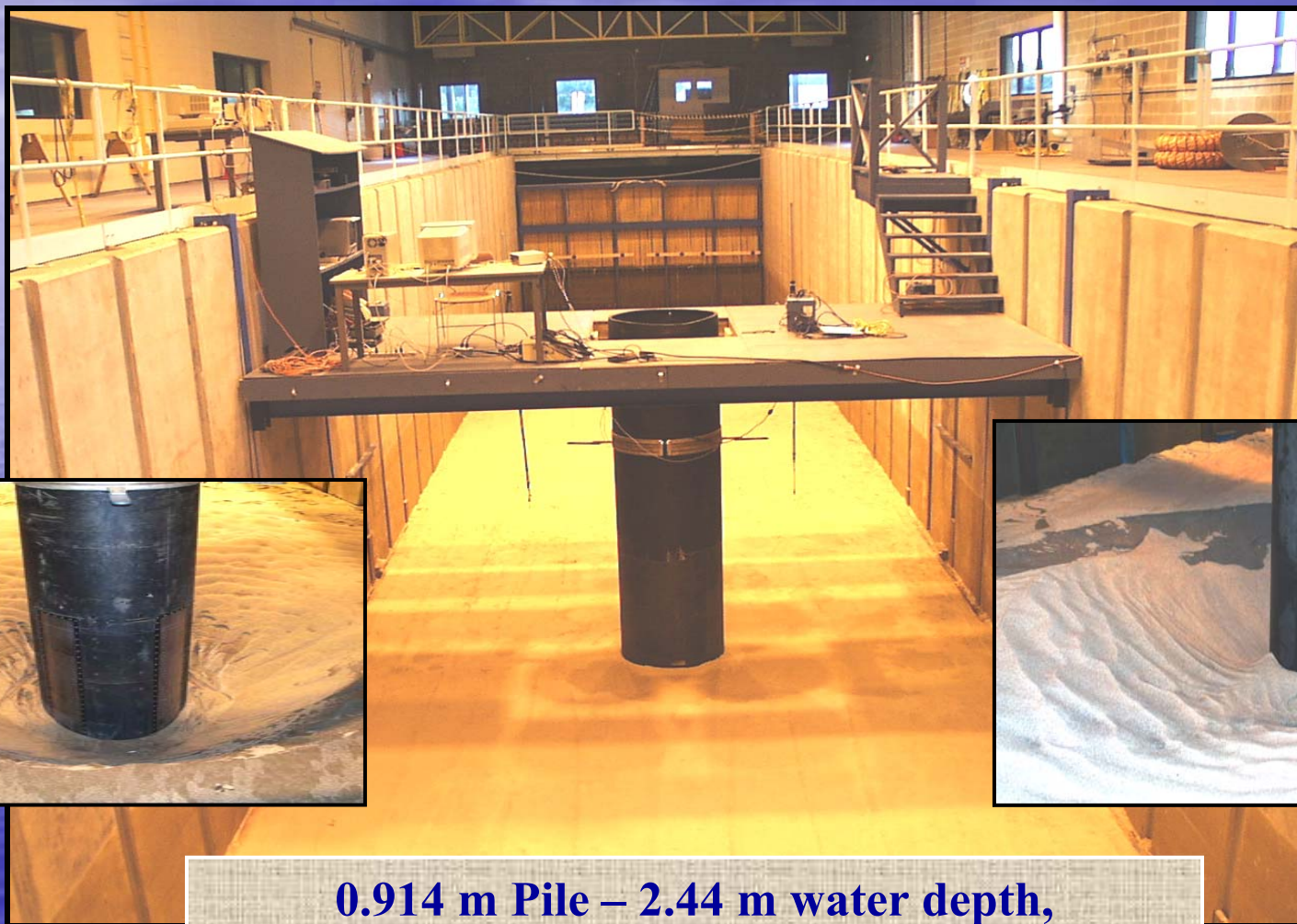
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Clearwater Tests in USGS Flume



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Clearwater Tests in USGS Flume



**0.914 m Pile – 2.44 m water depth,
Clear Water Scour**

Results of Clearwater Tests

- Verified predictive equations in clearwater scour range
- Provided new information regarding scour dependence on:
 - y_0/D
 - Suspended fine sediment

Objectives of Live Bed Scour Study

- Obtain data in the live bed scour range
- Determine if live bed peak exists
- If live bed peak exists, under what conditions

Facilities Needed for Live Bed Scour Tests

- Flume with
 - sediment recirculation capabilities
 - flow capacity to achieve required velocities
- Instrumentation for measuring flow, bedforms and scour depth
- Decided on University of Auckland

Auckland, New Zealand



New Zealand

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Auckland



University of Auckland College of Engineering



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Flume at the University of Auckland

5 ft wide by 4 ft deep by 148 ft long

- Tilting (1%)
- Maximum discharge:
 - Water: 42 cfs (1200 l/s) (60 hp, 30 hp)
 - Sediment: 2.1 cfs (60 l/s)
- Moveable instrument carriage (Non-Powered)

Auckland Flume



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Flume at the University of Auckland



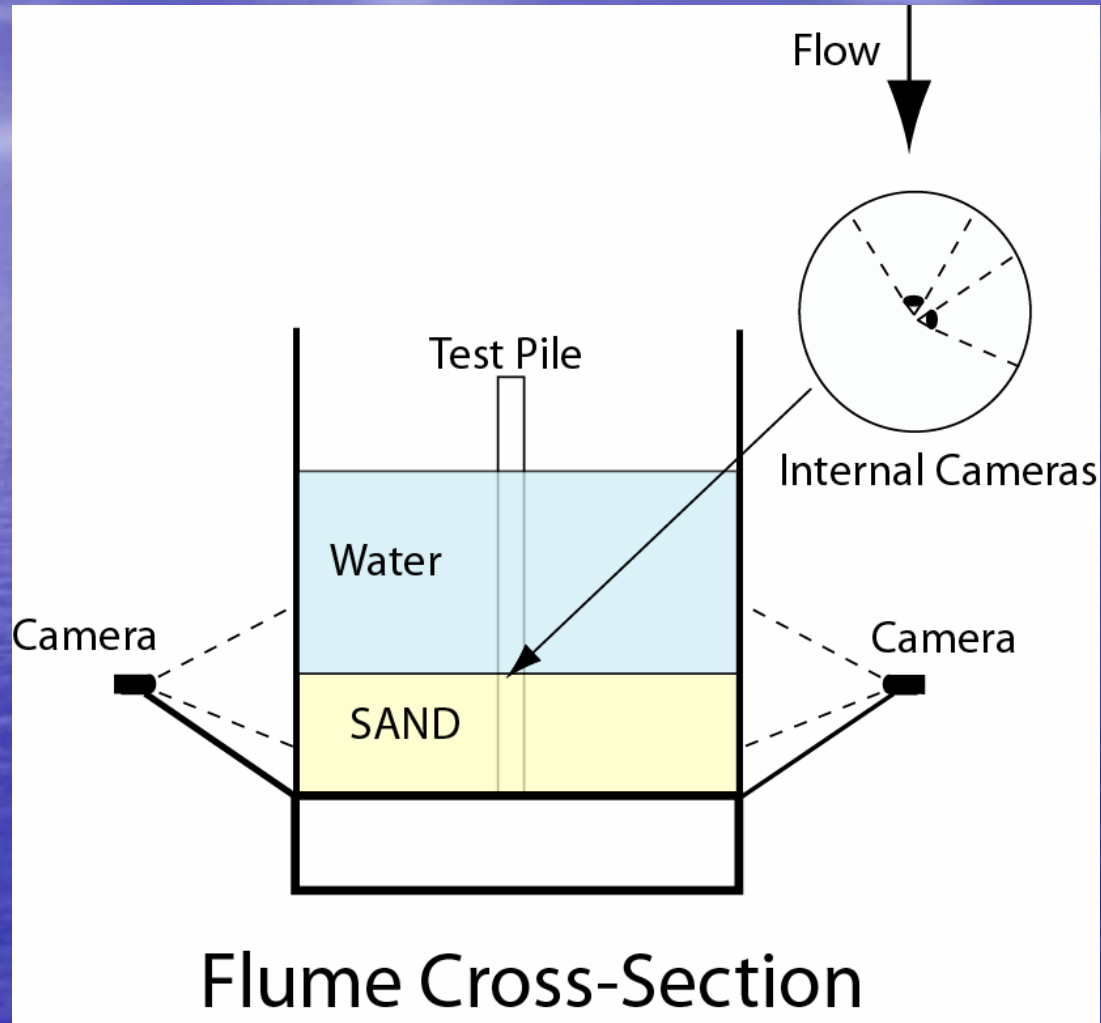
Test Structure



6 in. Diameter Pile, 0.30 mm D50 Sediment

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Instrumentation



Experiments

- Two Sand Sizes
 - $D_{50} = 0.27$ mm,
 - $D_{50} = 0.84$ mm,
- Circular Pile, $D = 6$ in (152 mm)
- 22 Tests

Live Bed Test



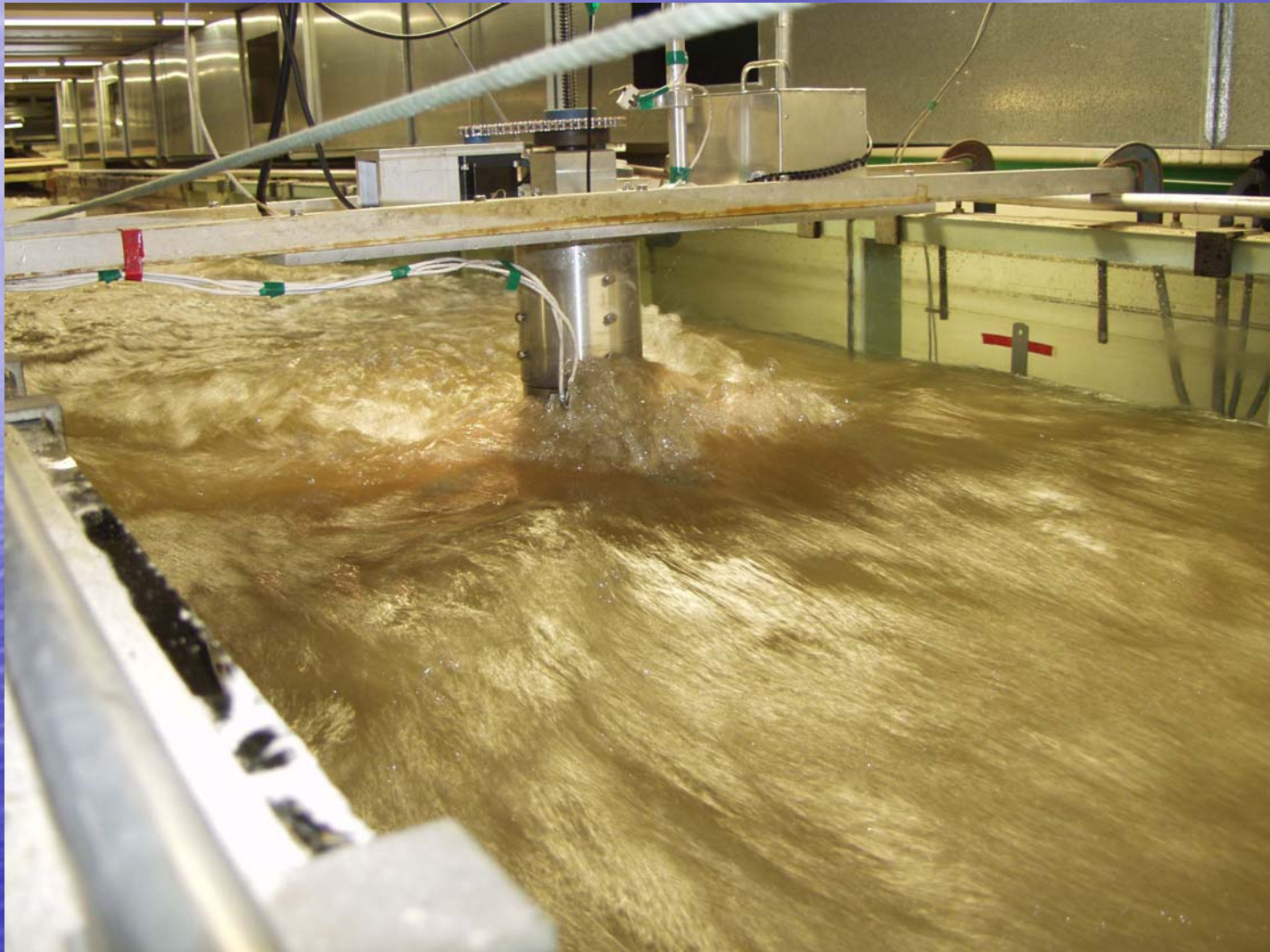
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Live Bed Test



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Live Bed Test



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Live Bed Test

University of Auckland Flume Test 14

$$D = 0.152 \text{ m}$$

$$V_c = 0.41 \text{ m/s}$$

$$D_{50} = 0.84 \text{ mm}$$

$$V_p = 2.1 \text{ m/s}$$

$$Y_0 = 0.38 \text{ m}$$

$$V/V_c = 2.95$$

$$V = 1.21 \text{ m/s}$$

$$V_p/V_c = 5.1$$

Bed Forms



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Live Bed Scour Hole



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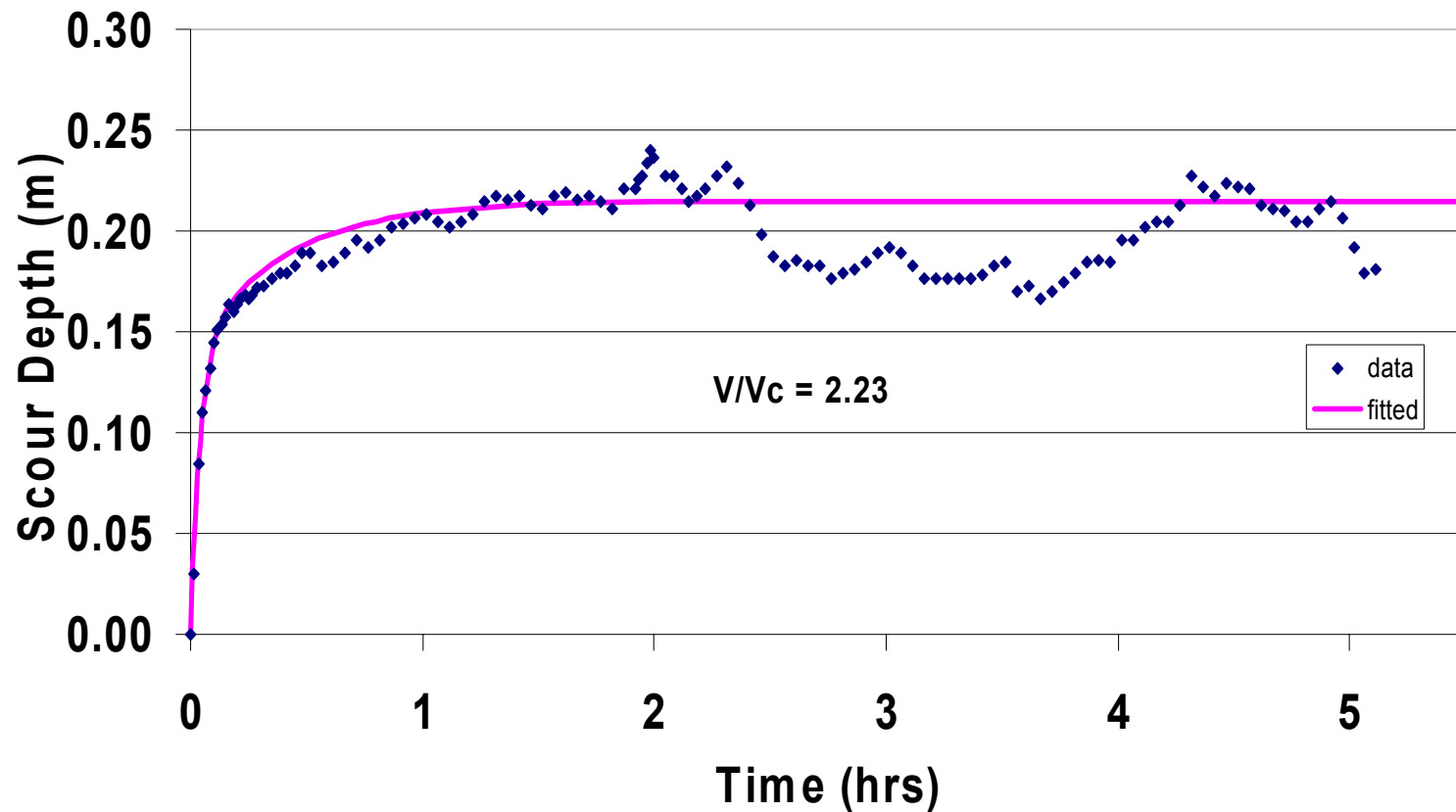
Live Bed Scour Hole



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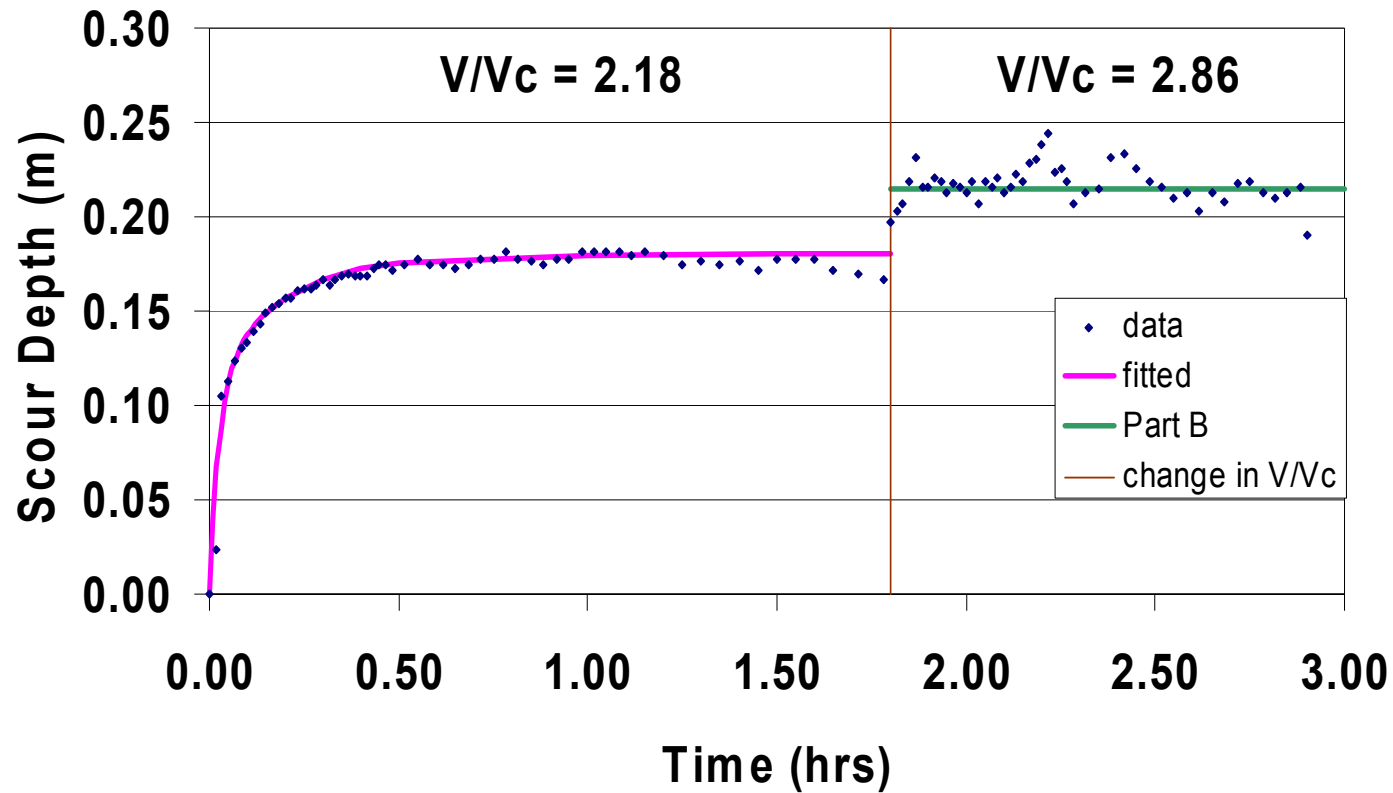
Live Bed Scour Data

Exp 202, video, fit eqn 8146



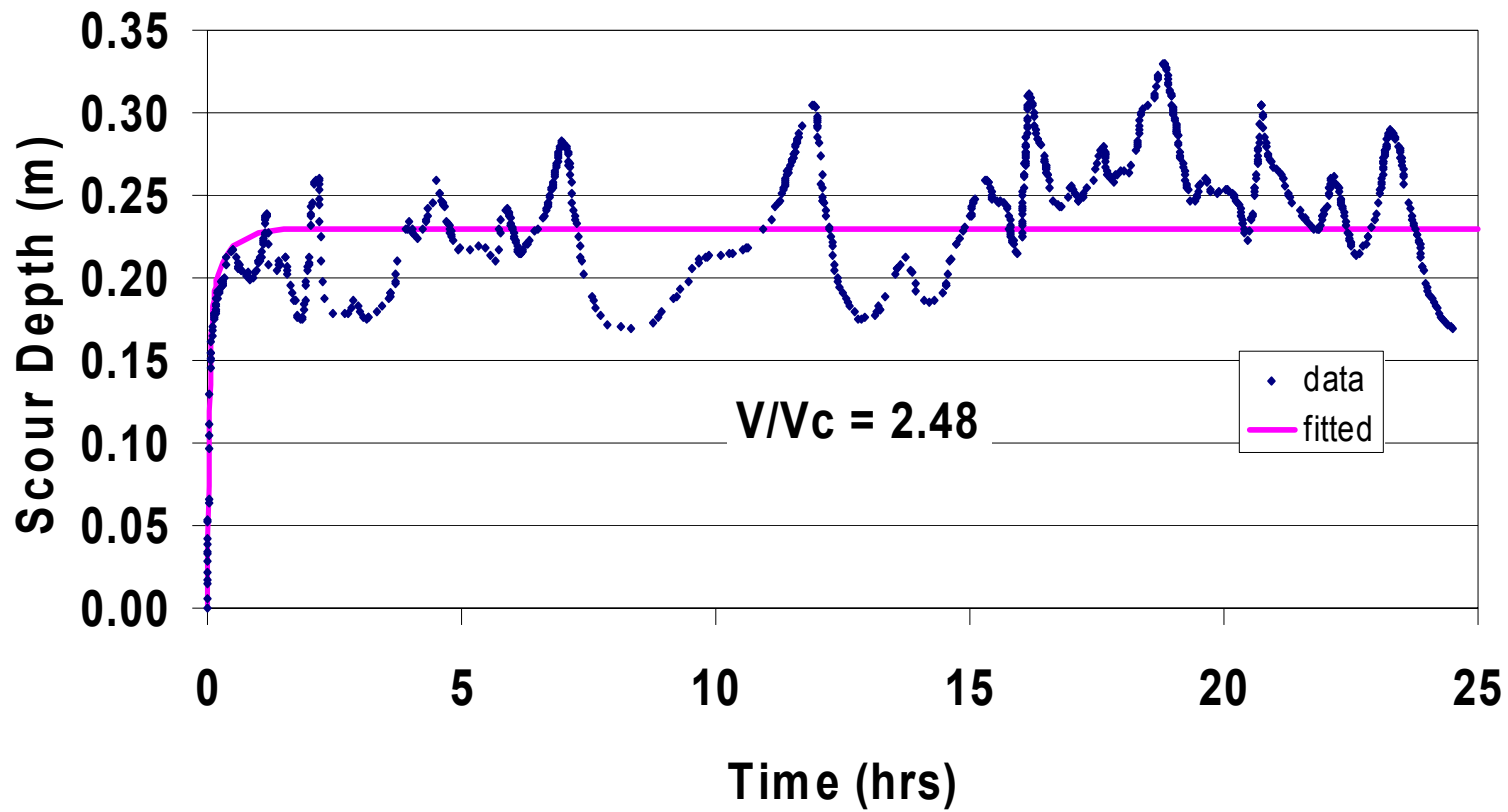
Live Bed Scour Data

Exp 207, video, fit eqn 8146



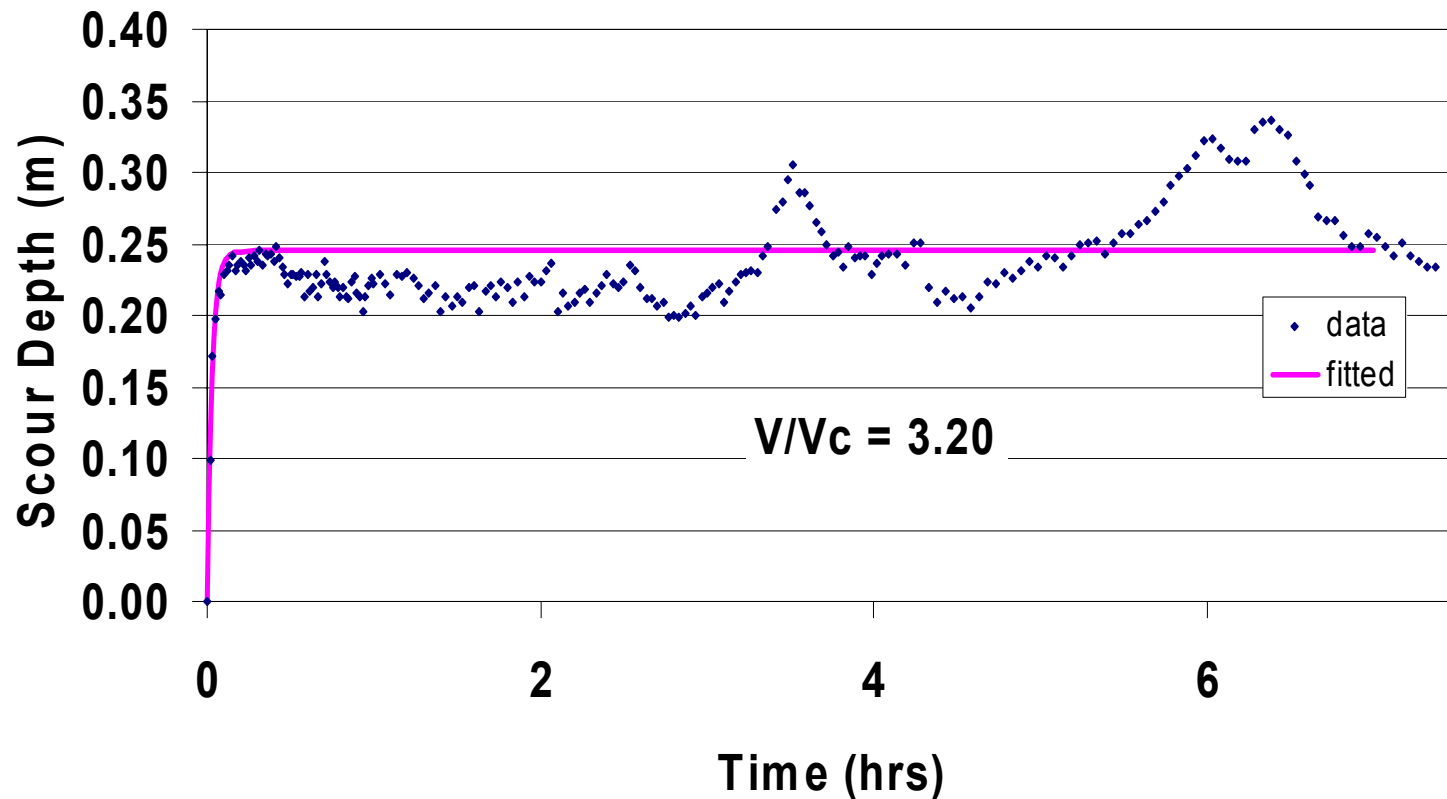
Live Bed Scour Data

Exp 208, pinger 5, fit eqn 8146



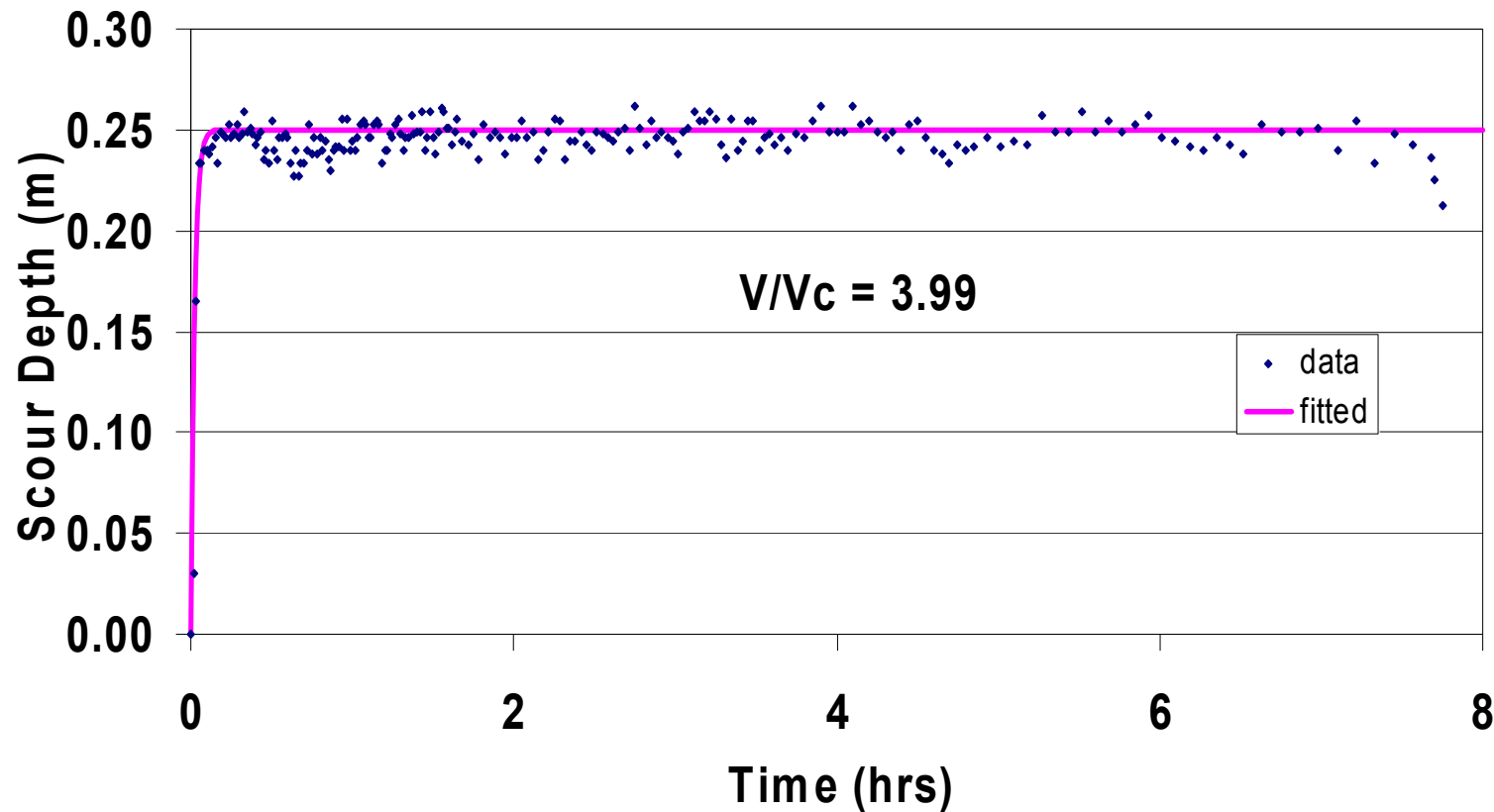
Live Bed Scour Data

Exp 203, video, fit eqn 8146



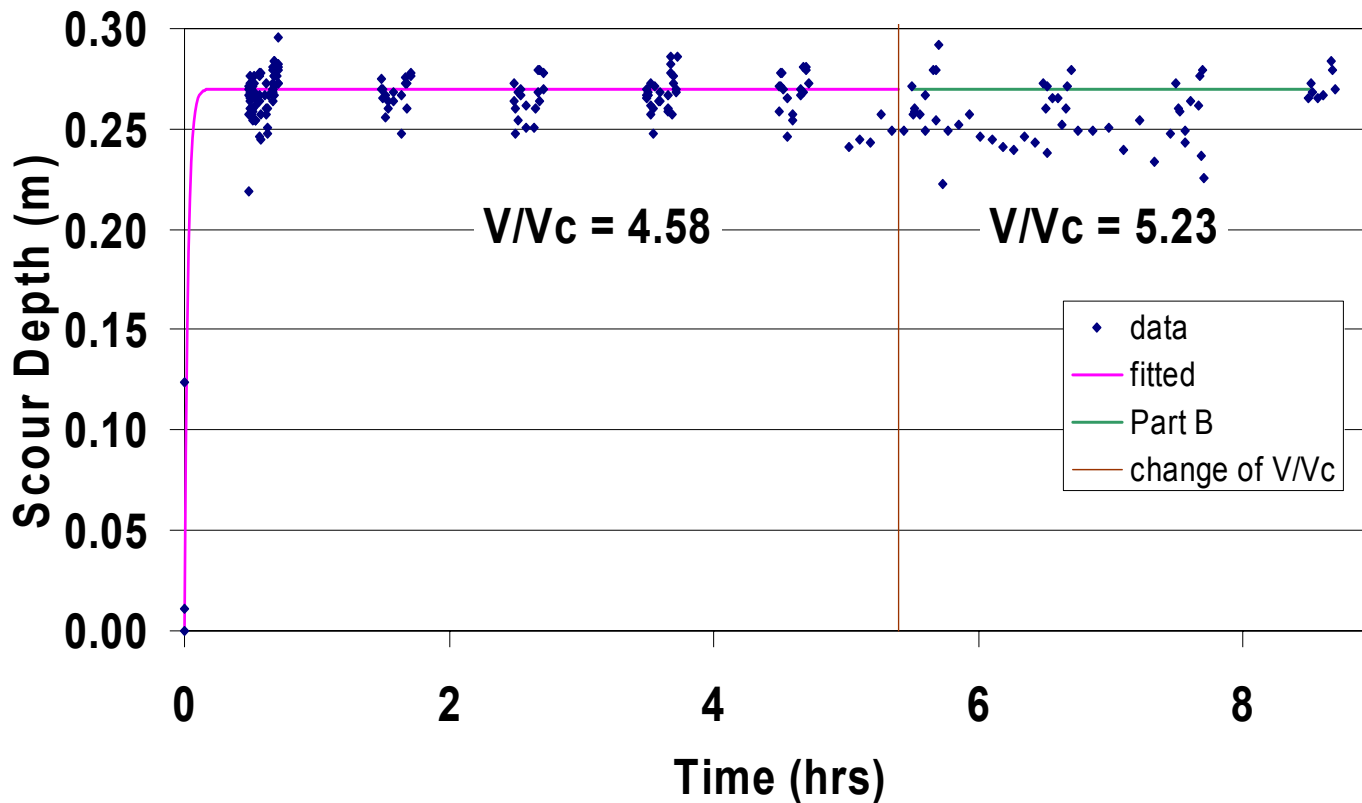
Live Bed Scour Data

Exp 204, video, fit eqn 8146



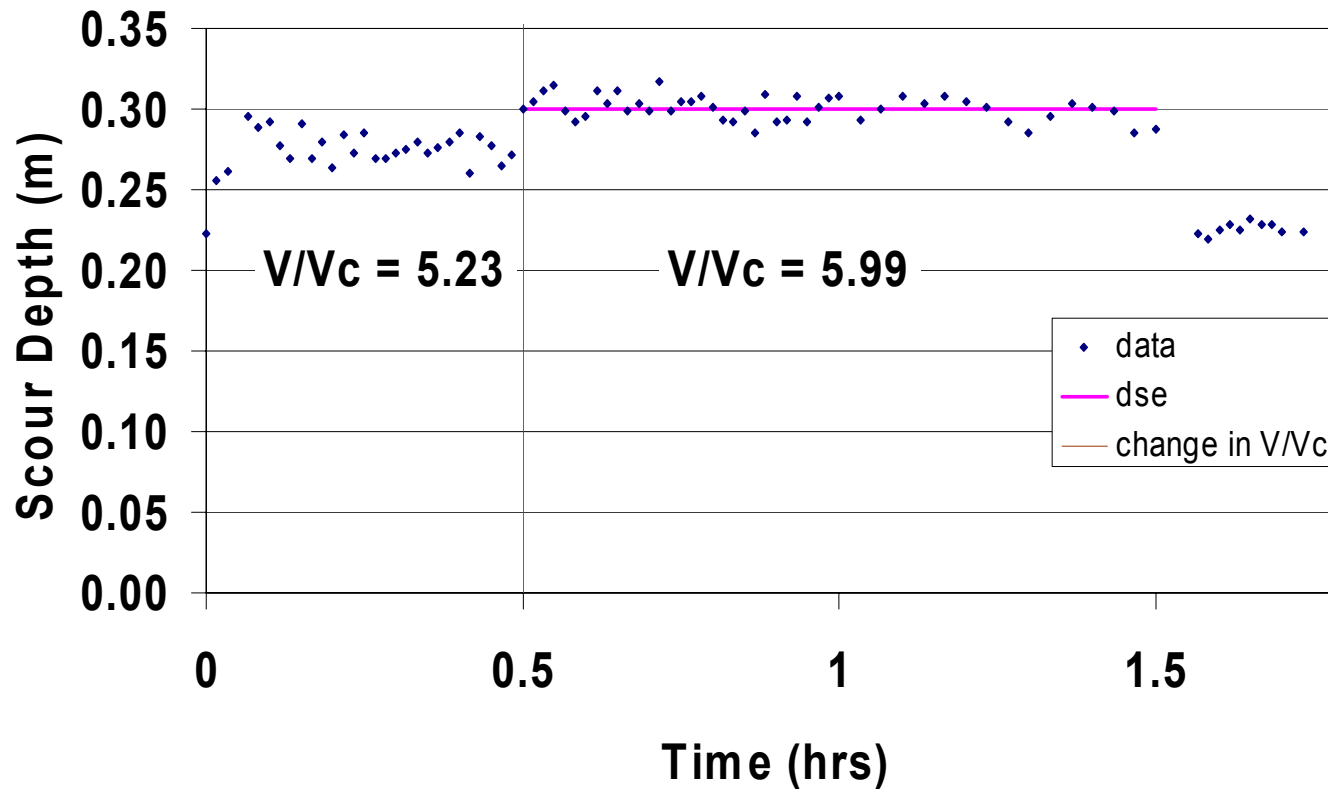
Live Bed Scour Data

Exp 205, video, fit eqn 8146



Live Bed Scour Data

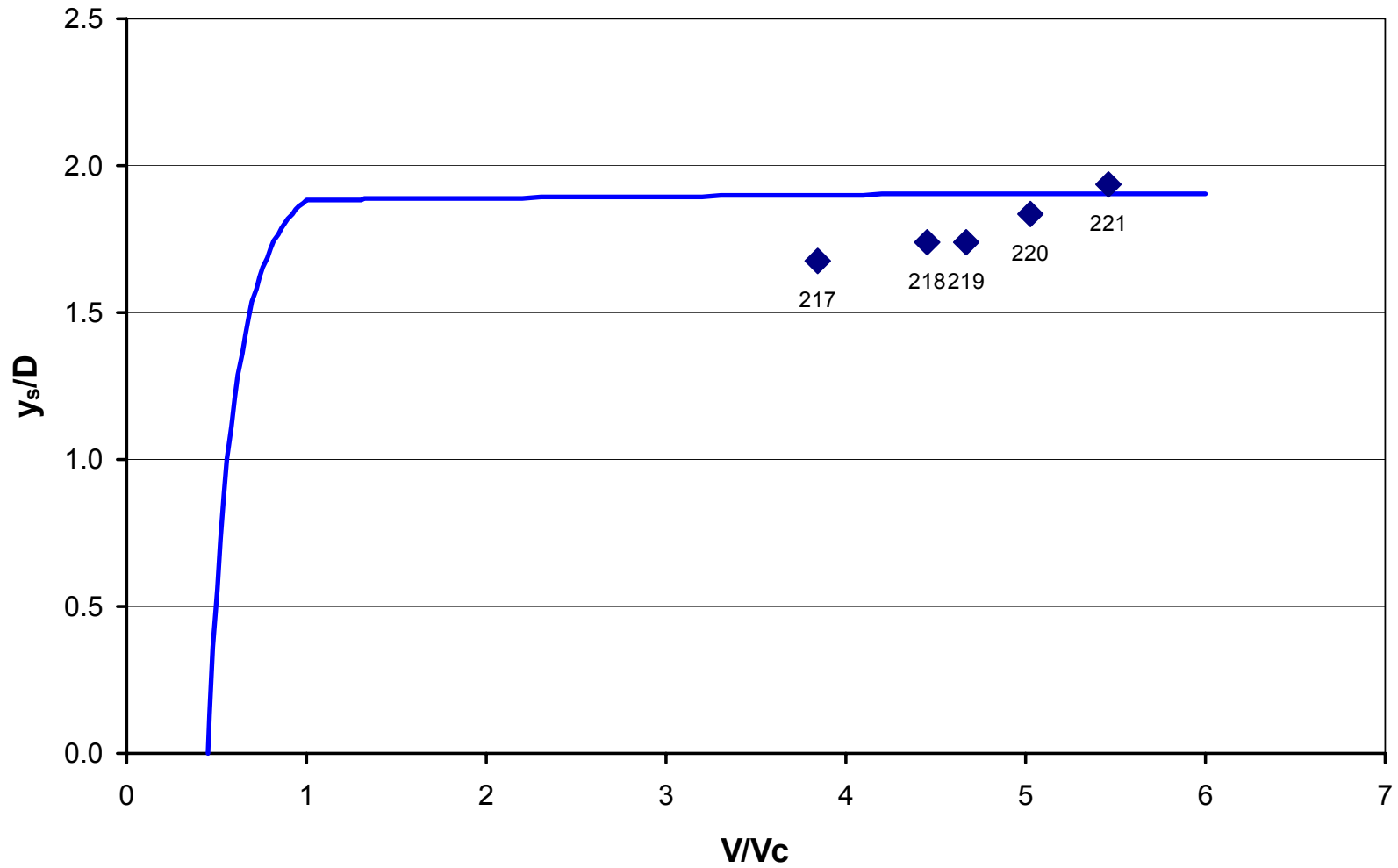
Exp 206, video data



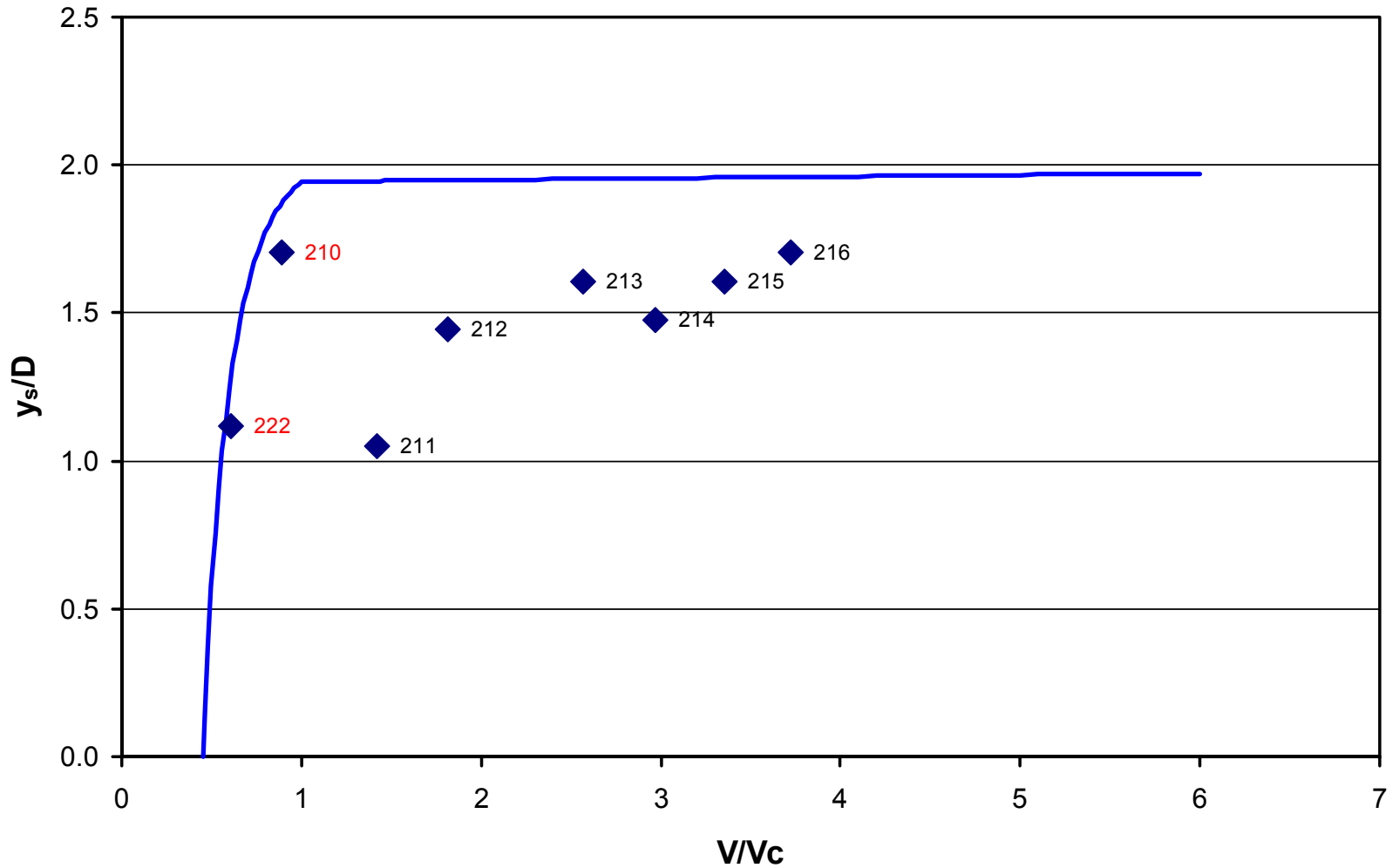
Predicted Vs Measured

- Using equations shown earlier equilibrium scour depths were predicted for
 - Our clearwater tests
 - Our live bed tests
 - Data from Sterling Jones
- Results on following plots

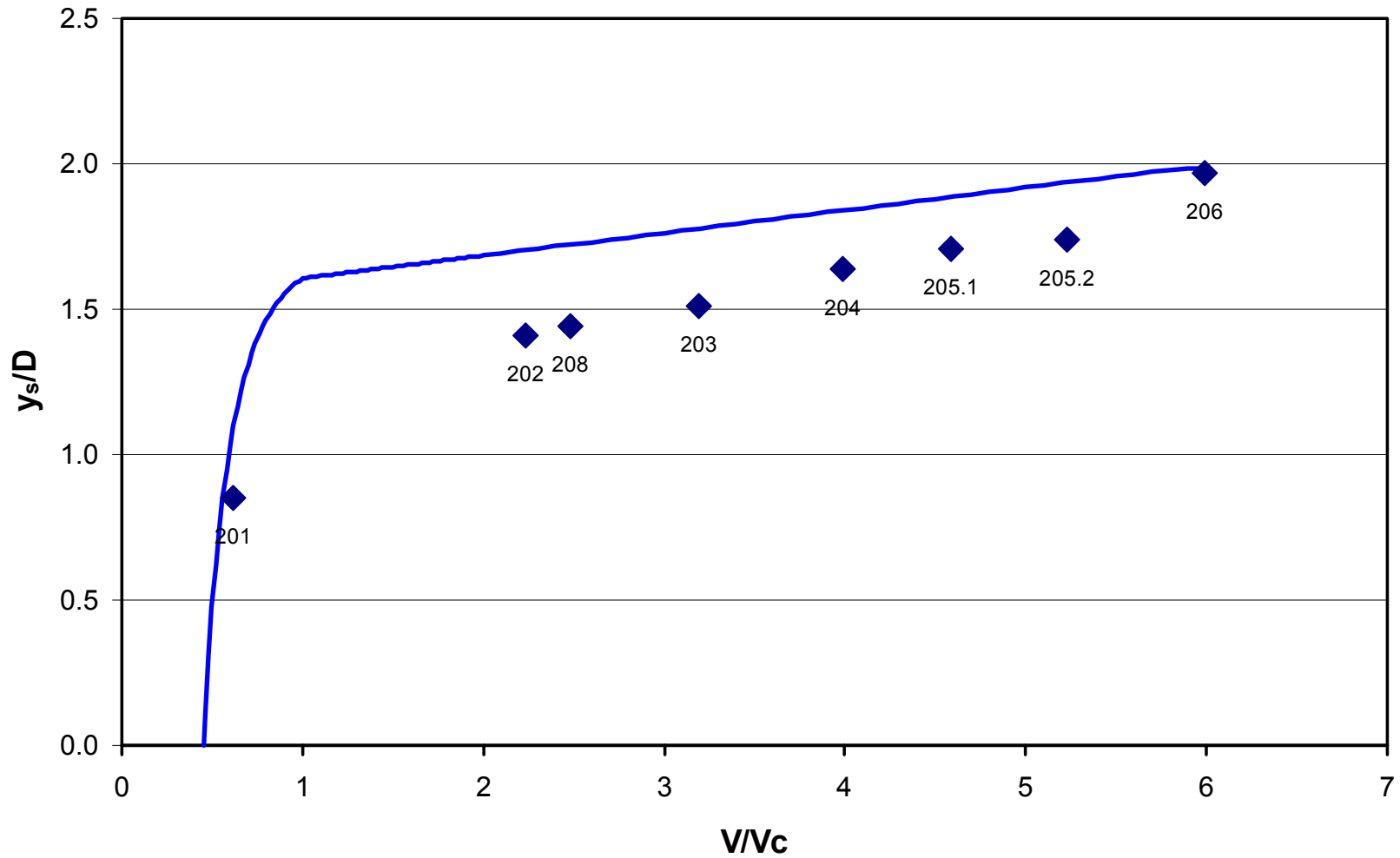
y_s/D vs V/V_c , $D/D_{50} = 181$, $y_0/D = 2$



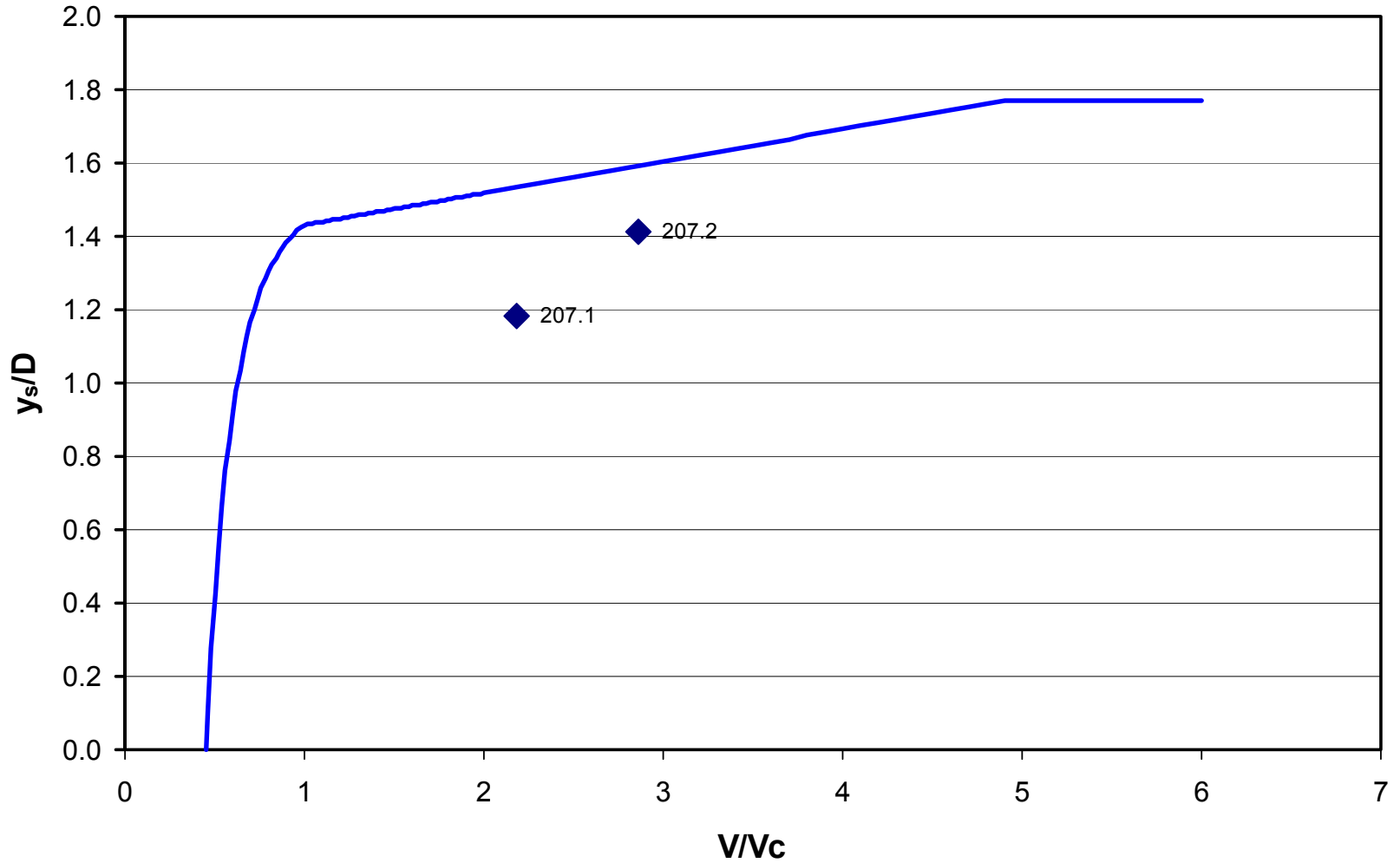
y_s/D vs V/V_c , $D/D_{50} = 181$, $y_0/D = 2.5$



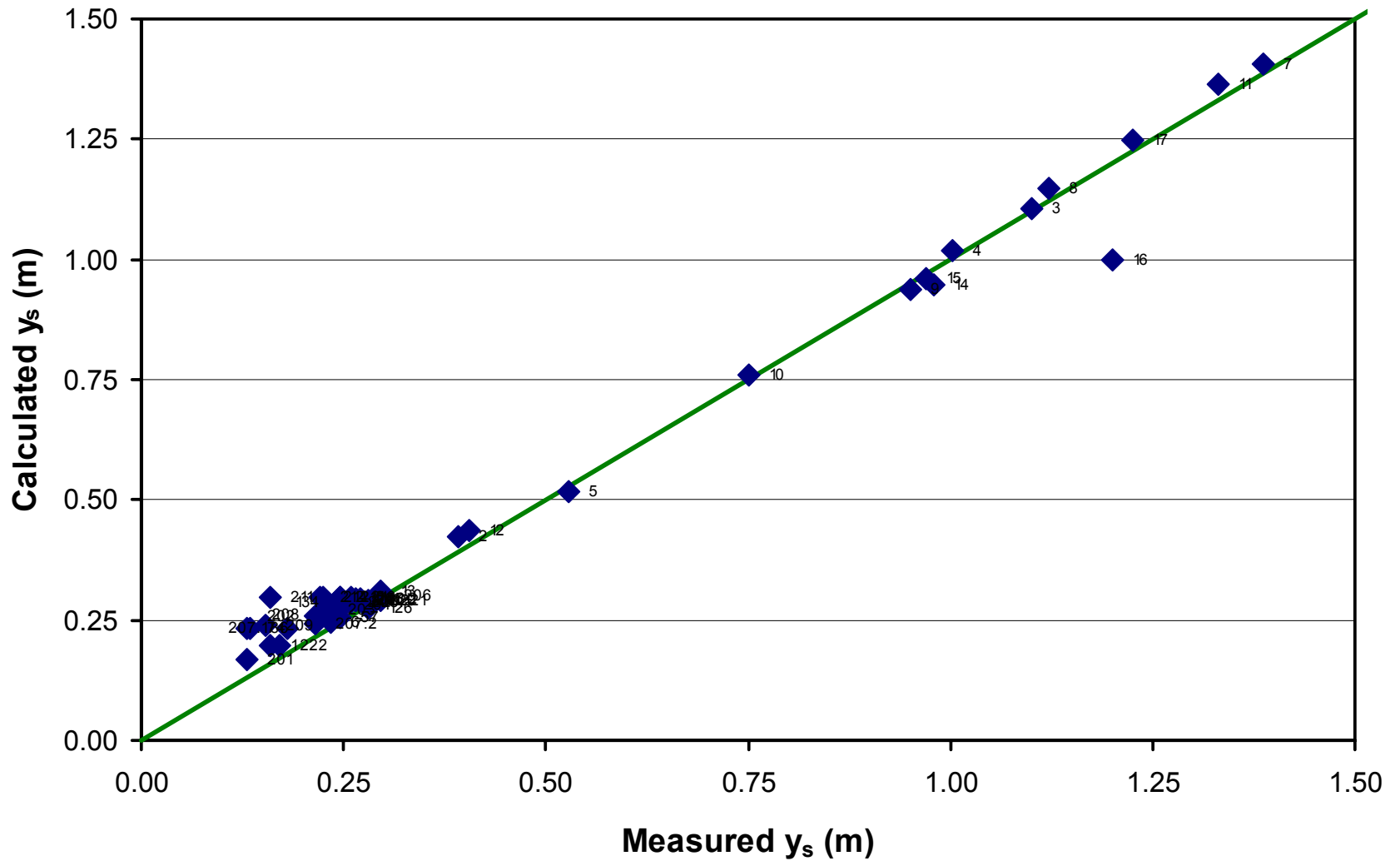
y_s/D vs V/V_c , $D/D_{50} = 564$, $y_0/D = 2.7$



y_s/D vs V/V_c , $D/D_{50} = 564$, $y_0/D = 1.3$



Calculated vs. Measured, y_s

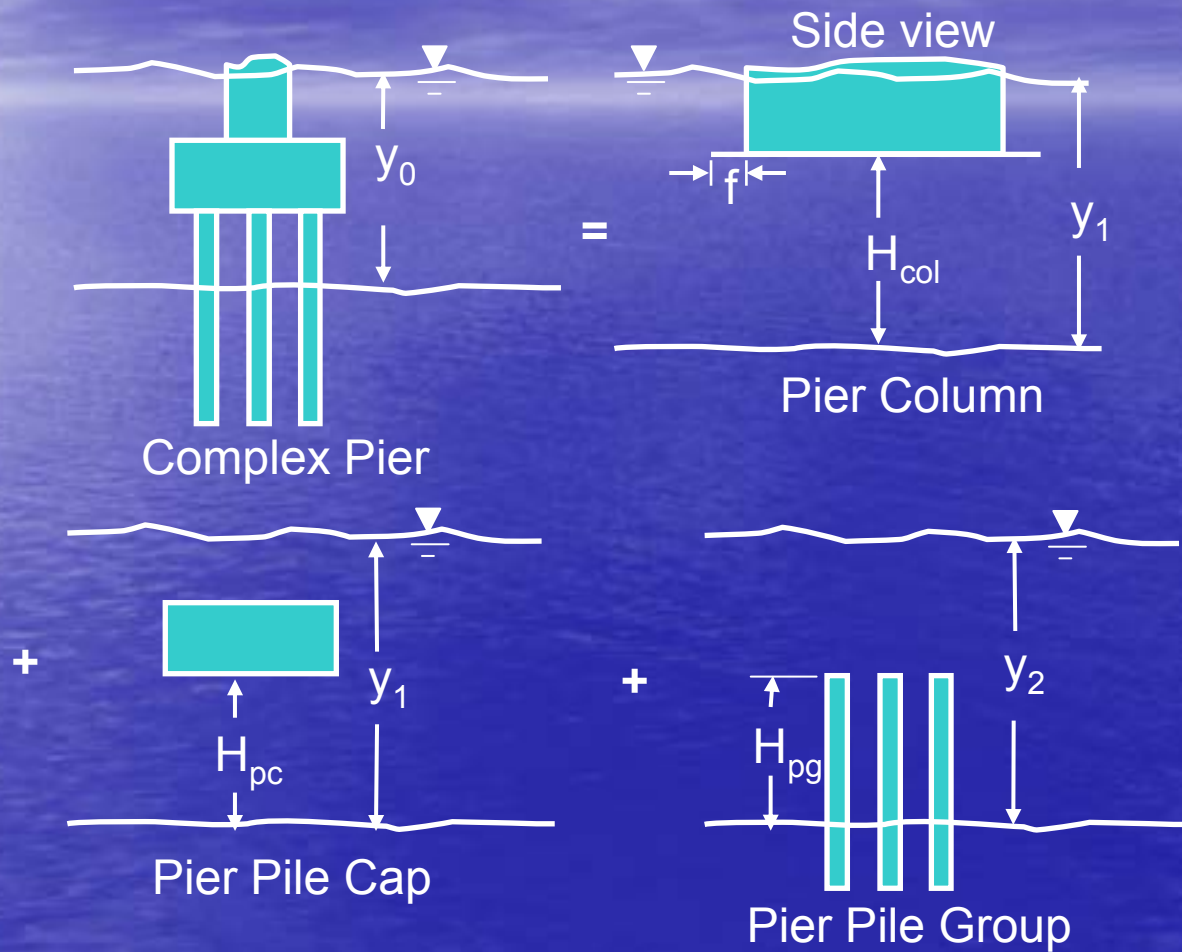


FIELD DATA VERIFICATION

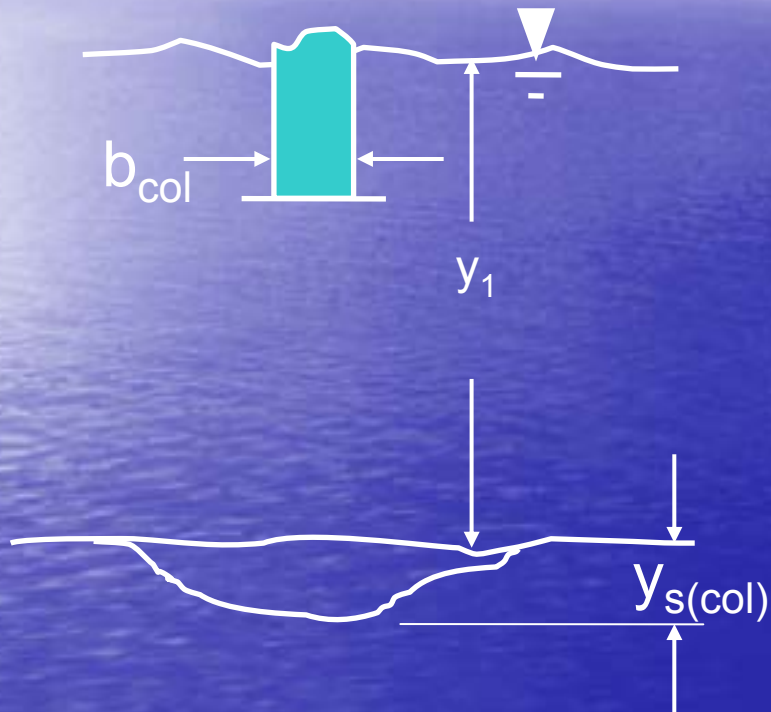
- Application of equations to a prototype structure shows good agreement
- Subject of second presentation

COMPLEX PIERS WITHOUT MODEL TESTS

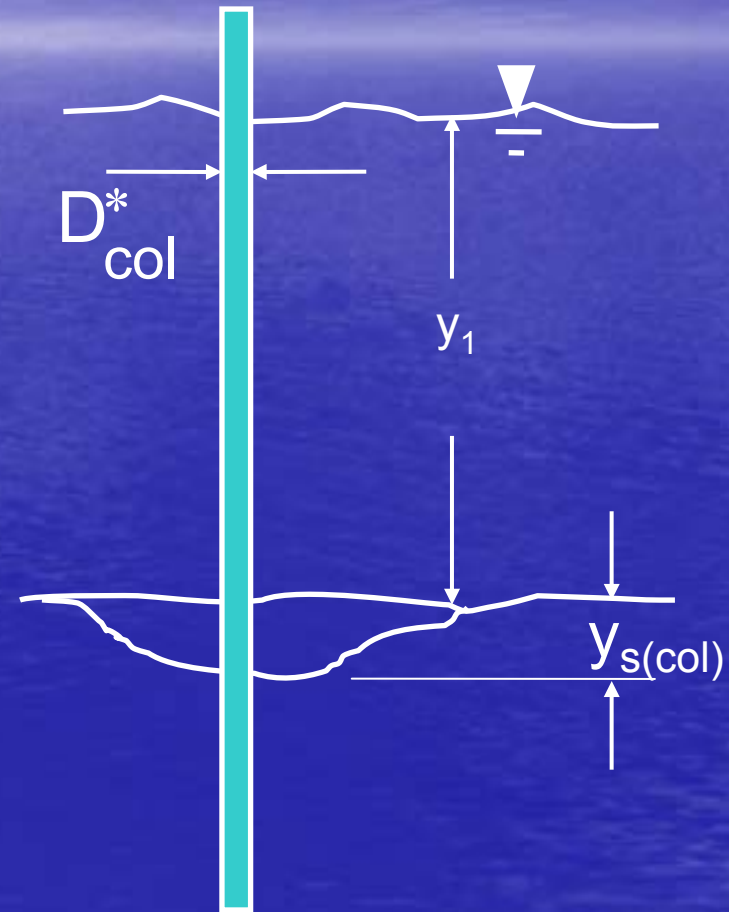
- Decompose pier into its components
- Determine the “effective diameter”, D^* , of each component
- Compute the contribution to the total scour depth by each component
- Sum the component scour depths to obtain the total



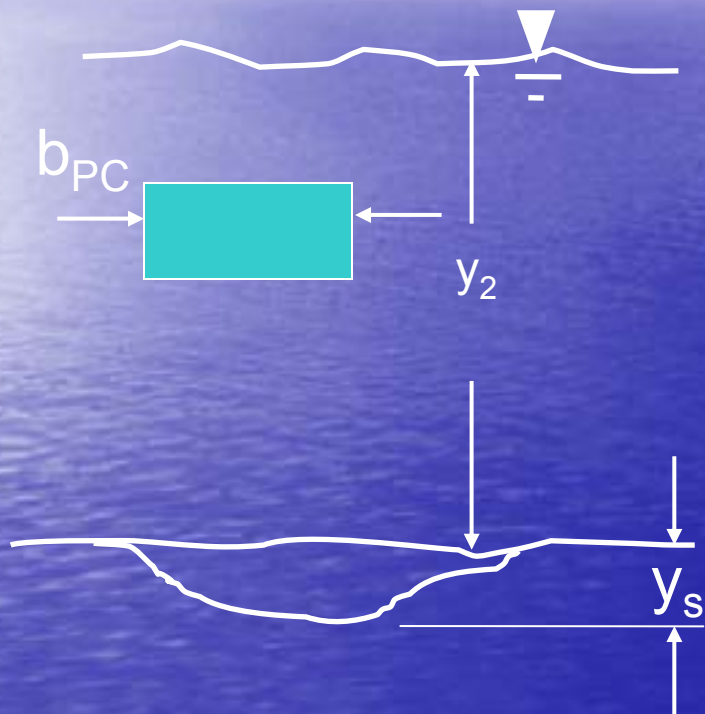
Pier Column



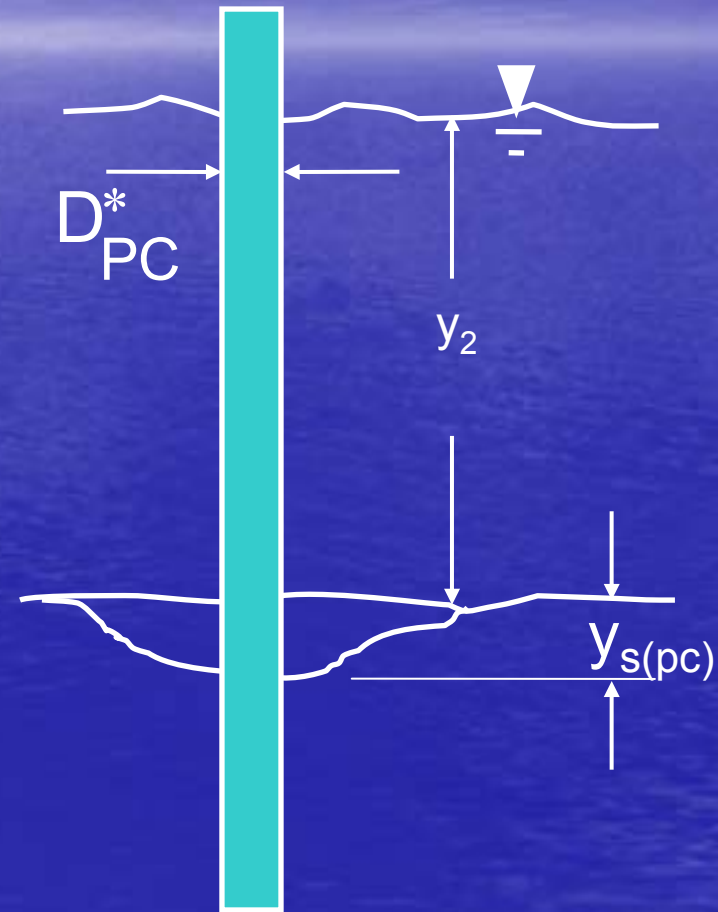
Single Pile that will have same scour depth for same sediment and flow conditions



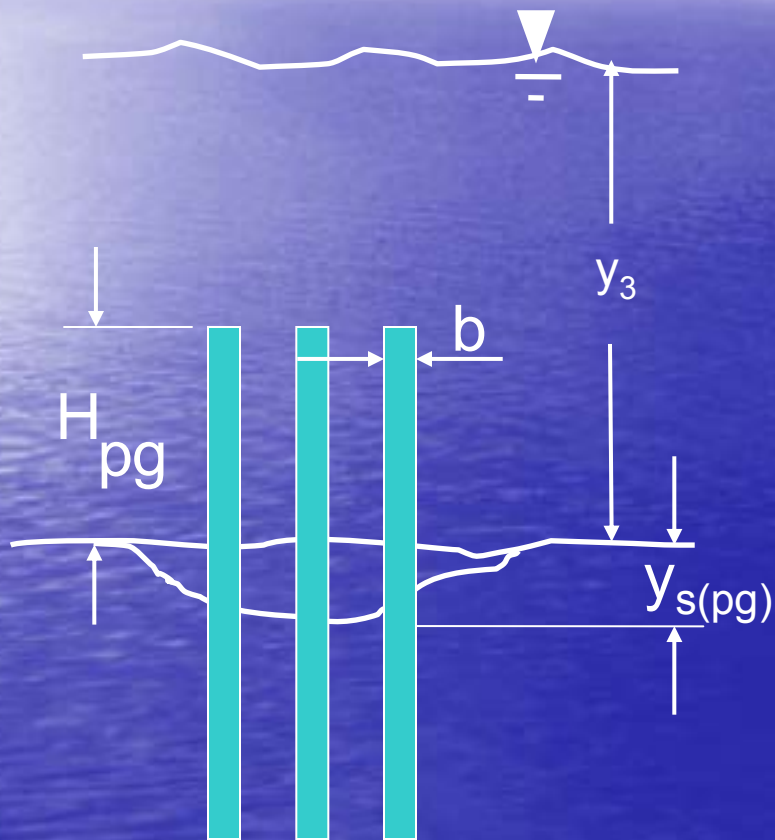
Pile Cap



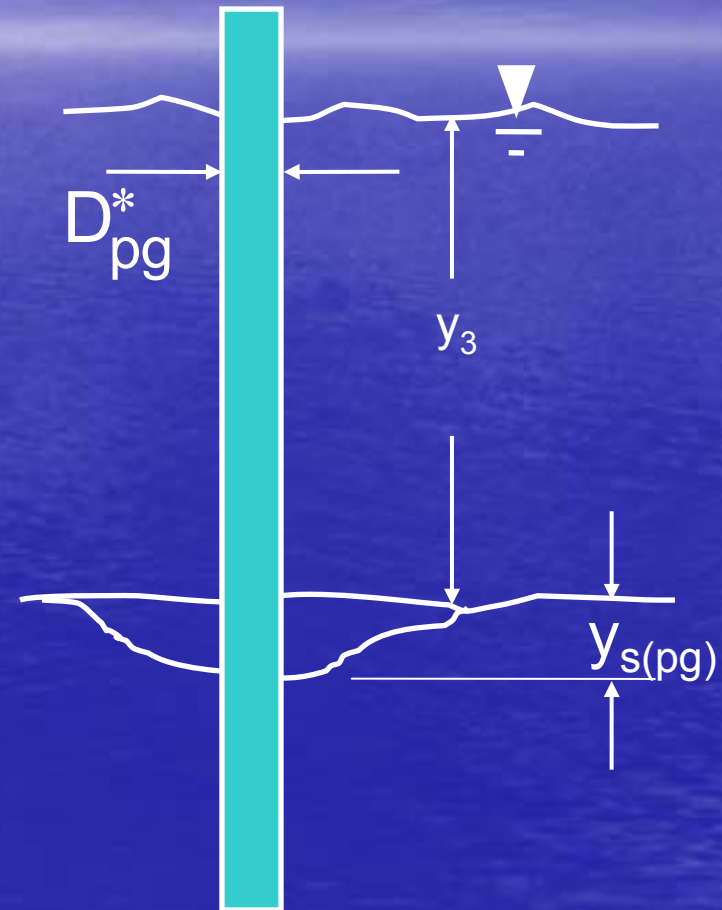
Single Pile that will have same scour depth for same sediment and flow conditions



Pile Group



Single Pile that will have same scour depth for same sediment and flow conditions



Total Scour Depth

$$y_s = y_{s(\text{col})} + y_{s(\text{pc})} + y_{s(\text{pg})}$$

Design Scour Depths with Physical Model Tests

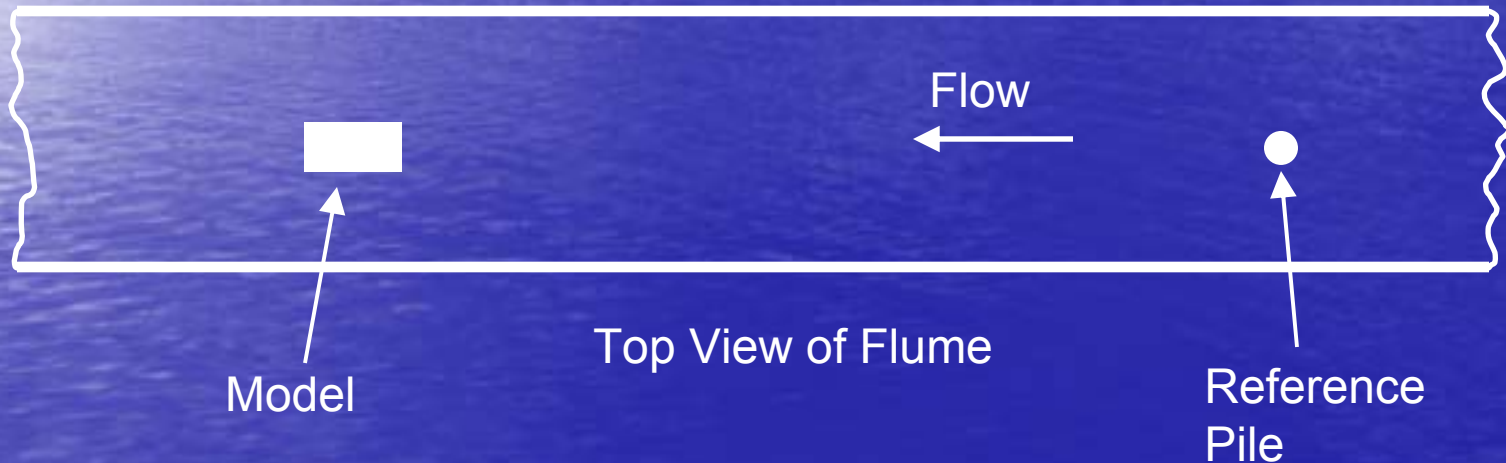
- For pier designs significantly different from the generic shape shown in the previous analysis Physical Model Tests are recommended
- Model test design
- Interpretation of model results

Physical Model Test Design

- Models as large as flume will allow
- $D/D_{50} > \sim 50$
- Sufficient test duration
- Reference pile if possible
- Low suspended sediment in water
- Small distribution of sediment size (small sigma)

Physical Model Test Design

Reference pile



Physical Model Test Design

- Reference pile
 - Use scour at reference pile to correct for:
 - Flume sediment size distribution
 - Suspended fine sediment in water
 - Duration of test less than required to reach equilibrium scour depth

Interpretation of Physical Model Test Results

- Compute equilibrium scour depth for reference pile
- Compute Scour Depth Correction Factor:

$$SDC = \frac{\text{Computed Scour Depth at Reference Pile}}{\text{Measured Scour Depth at Reference Pile}}$$

Interpretation of Physical Model Test Results

Compute model pier equilibrium scour depth:

$$y_{s(\text{model})} = \text{SDC}^*(\text{Measured scour depth at model pier})$$

Interpretation of Physical Model Test Results

- Compute effective diameter of model pier, D_m^* , using single, circular pile equation, i.e. solve for D_m^* in the following equation:

$$\frac{y_s}{D_m^*} = 2.5 K_s \left\{ \tanh \left[\left(\frac{y_0}{D_m^*} \right)^{0.4} \right] \right\} \left\{ 1 + \frac{0.25 \ln(V/V_c)}{(V/V_c)^2} \right\}$$

$$\left\{ \frac{2.95}{2.5 \exp \left[0.45 \left(\log \left(\frac{D_m^*}{D_{50}} \right) - 1.64 \right) \right] + 0.45 \exp \left[-2.5 \left(\log \left(\frac{D_m^*}{D_{50}} \right) - 1.64 \right) \right]} \right\}$$

Interpretation of Physical Model Test Results

- Next compute the effective diameter of the D_m^* prototype pier, D_p^*

$$D_p^* = (\text{Geometric Scale of Model}) D_m^*$$

Interpretation of Physical Model Test Results

- Knowing the prototype effective diameter and the design flow and sediment conditions the prototype scour depth can be computed using the single structure scour equations presented above.

Summary

- Design local scour depths for complex bridge piers can be computed without physical model tests for a large number of pier designs
- Pier designs that differ significantly from the generic shape require physical model tests as part of the analysis

Summary (cont.)

- A good physical model test design and execution is essential
- Care must be taken in the interpretation of the model scour results and their use in arriving at design scour depths for the prototype pier



Questions?
Comments

Required Shear Profile Points

