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VIV Application to Deepwater Risers

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December 2-3, 2003



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Agenda

- Introduction
- Environment
- Riser Response to Currents
- Fatigue
- Tendons
- The FIX
- Analysis Methods and Programs
- Model Tests and Data Sets
- Other Issues
- Areas of R&D
- Industry Experience
- Conclusion



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Thanks to:

- Thanos Moros, BP
- Owen Oakley, Jim Stear, Hugh Thompson, Chevron
- Kim Vandiver, MIT
- Guy Mansour, Atlantia
- T. Sarpkaya, Naval Postgraduate School
- Don Allen/Li Lee, Shell Global Solutions
- Rodney Masters, AIMS



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Introduction

Risers and tendons

- Steel Catenary Riser: SCR
- Top Tensioned Riser: TTR
- Flexible Riser
- Free Standing Riser: FSR
- Highly Compliant Riser: HCR
- Tendons



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Perspective from the industry:

The offshore industry has been made painfully aware of what may go wrong, how VIV may affect operations. The industry is having to be conservative because of uncertainties in environment and in engineering prediction tools.

For water depths to 5000', we can proceed with reasonable solutions. For 10,000', we cannot afford high levels of conservatism and still afford to make money producing oil.



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VIV is a resonant feedback phenomena

Engineering in uncertainty:

Risks not generally in getting design numbers wrong,

but in not designing for new phenomena.

DISASTER!
The Greatest
Camera Scoop
of all time!

CAMERA FILMS



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Risks and Cost Impacts

- Risers and Tendons represent critical structural and oil containment systems
- Uncertain environment – “new” phenomena being discovered
- Uncertain response – highly non-linear, stochastic in nature
- Failure has high cost



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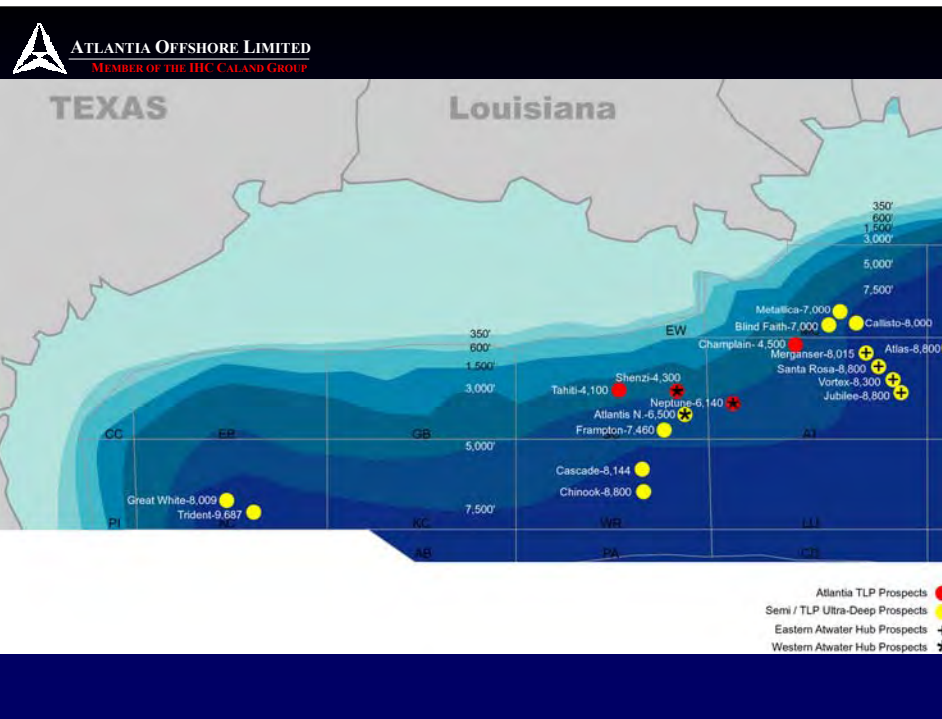
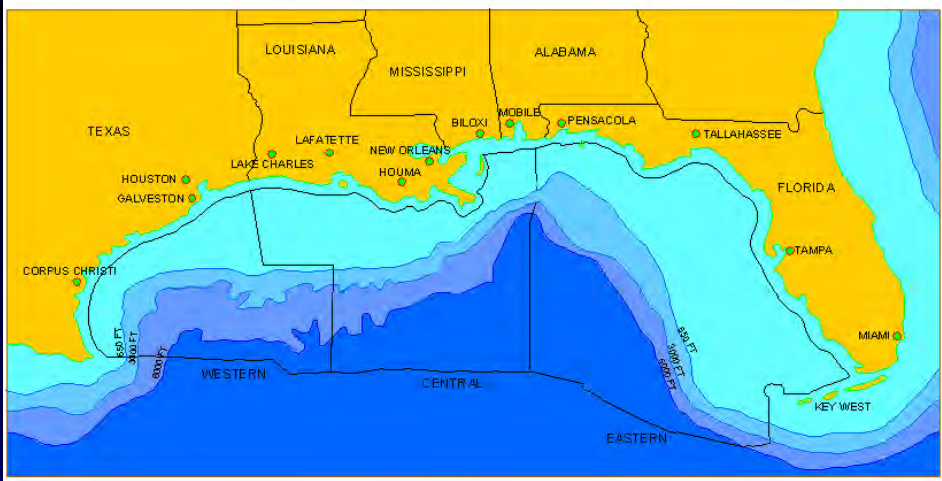
Environment

- Geography
- Mode experiment
- Types of Ocean Currents
- Typical and extreme magnitudes
- Durations
- Uncertainties, unknowns
- Combination with other events
 - Ratio of environmental forces, shallow to deep



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MODE Experiment

- WHOI - 1970's
- Weather in the Oceans
- Fronts, temporal variation
- Large scale turbulence
- Vertical Structure



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Ocean Currents

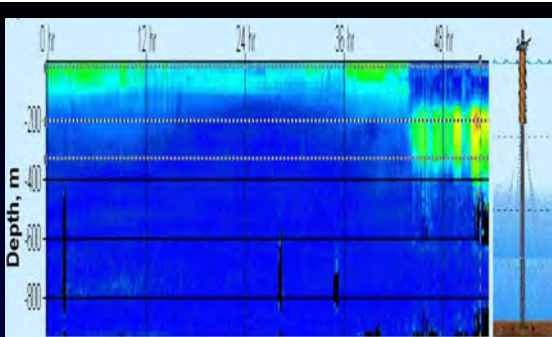
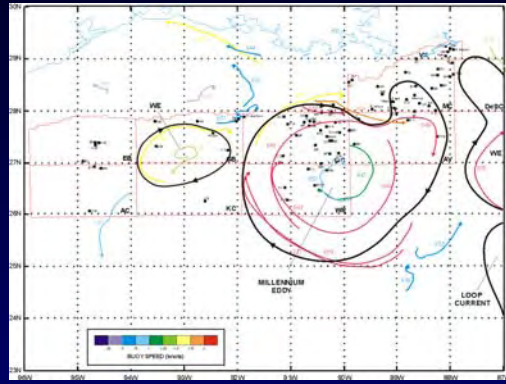
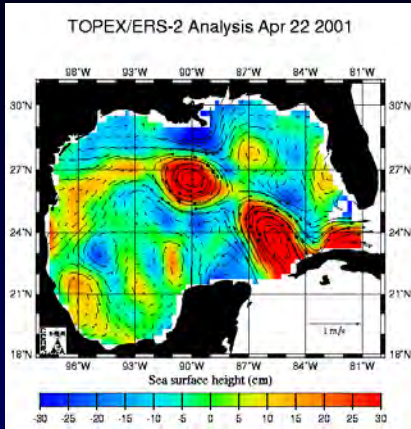
- Tidal Currents (how they exist in deep water)
- Loop
- Eddy
- Storm (wind stress model)
- Inertial
- Slope
- Bottom Boundary Current (Sigsby Escarpment)



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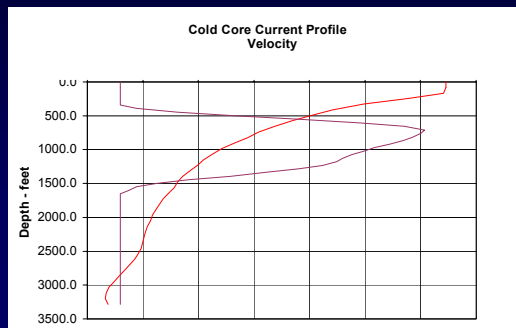
Ocean Currents

loop current / eddy current



Ocean Currents

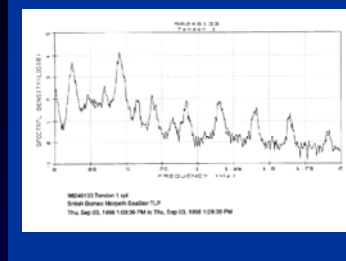
Cold Core Eddy





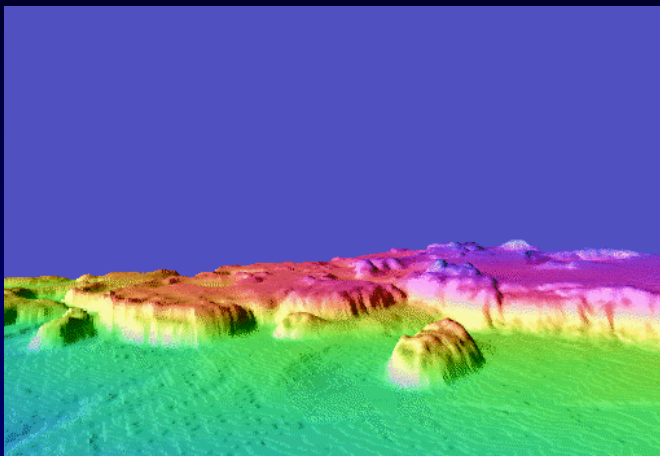
Inertial and Slope currents

- Morpeth
 - Hurricane Earl
- Neptune
 - Hurricane Georges
 - 1.8 – 2.2 kts in 2000'



Sigsbee Escarpment

Currents identified from bottom furrows

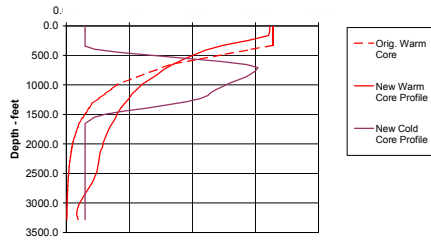




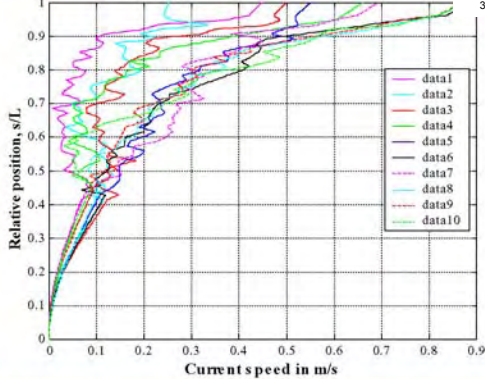
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Current profiles

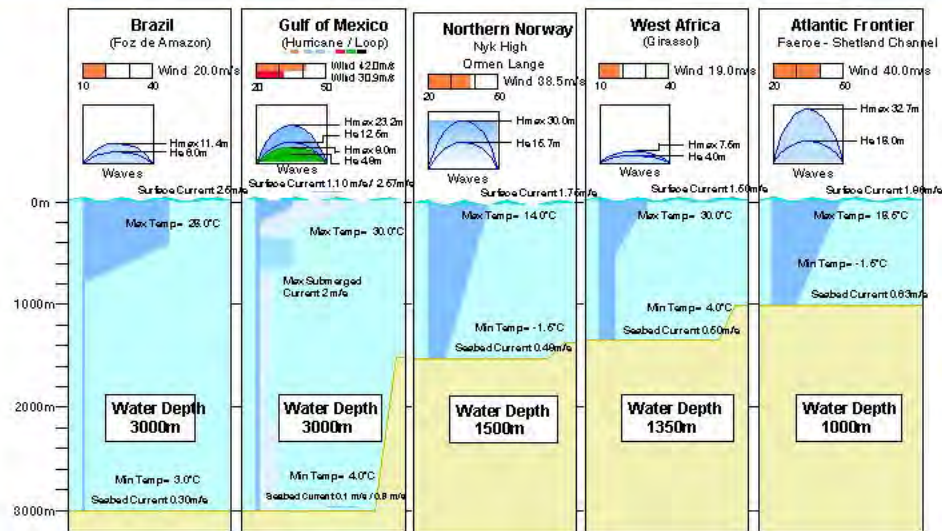
Allegheny Current Profiles



Allegheny Currents with extrapolations



Deepwater Currents



Environment

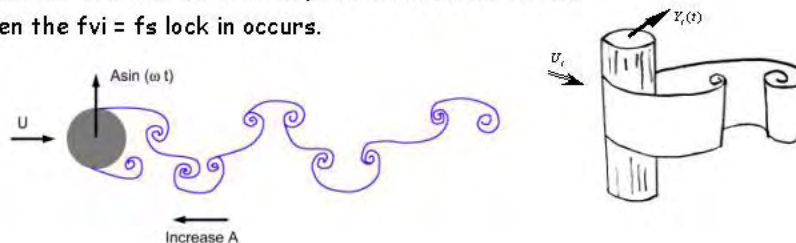
Insert DW Currents presentation

VIV Principles



- When flow passes a structure separates, forming a vortex street.
- The vortices have an alternating shedding pattern and interact with the structure, causing the so called "Vortex Induced Vibrations".
- Inline vibrations generally generated by symmetric vortex shedding, (but twice the f_s) are also important.
- Two main parameters define the behaviour of vortices.

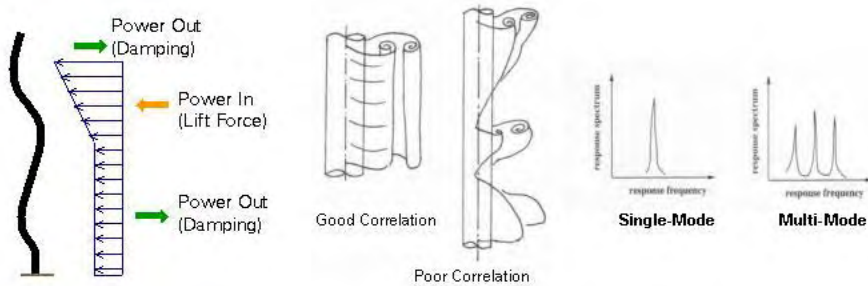
$$Re = UD / \nu \quad f_s = StU / D \quad f_i / f_s$$
- The frequency of vortices depends on the spacing.
- Structure can vibrate with amplitudes in excess of $1D$.
- When the $f_{vi} = f_s$ lock in occurs.



VIV Principles



- In a 3D structure with non uniform currents, the vortex sheet is not uniform along the length of the riser. The vortices along the length are not fully correlated, making the problem more complex to analyse.
- In a shear current the strong velocities near the top will excite the structure, while lower speeds will offer damping.
- Long Structures in a non uniform current tend in general to respond to multi-modes



VIV Principles



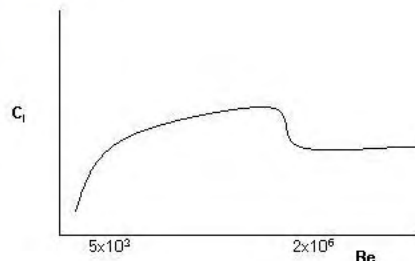
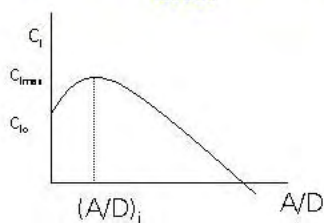
- The resultant vibrating amplitude is a function of both lift & damping

$$A/D = f\left(\frac{\text{Lift Force}}{\text{Damping}}\right)$$

Hydro + Structural
>>

- Lift Depends on vibrating Amplitude and $Re, f_i/f_s$.

Typical Lift Curve used in many VIV codes

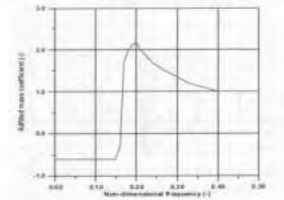


VIV Principles



- Damping = $O(C_D(i) \gamma(i,r) + f(\omega_r, m_t, \zeta_s))$
- Hydrodynamic local C_D is function of Re.
- m_t includes the structural and added mass.
 - Added Mass is a function of local St, $f_n = f_{osc} D/U$

Added Mass Curve.



- ζ_s for most of typical pipes is ~ 0.003 . - For non standard pipes, damping experiments are required.
- Drag amplification of oscillating structure is higher than stationary.

$$C_D(i)/C_{D0} = 1 + 1.043 \left(2 \frac{\gamma_{ms}(i)}{D} \right)^{0.43}$$

VIV Principles

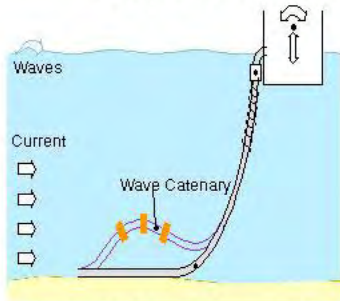


- Other parameters that affect VIV are
 - Roughness of the structure, tends to change the Re Regime, then Cl, Cd & St.
 - Turbulence in the flow. - Similar effects to roughness.
- Little data exist on the behaviour of the above parameters.
- Whatever exists is proprietary to some oil majors.

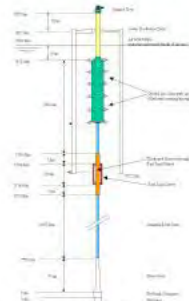
VIV - Why Important to Risers



Compliant Risers



Top Tension & Free Standing



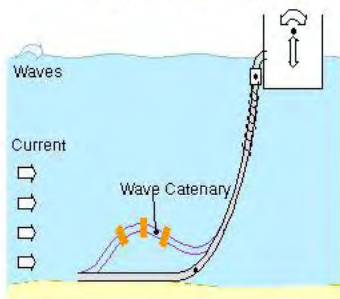
Needs to be understood and catered for in design and operations to avoid failures like

- Over-stressing Joints
- Fatigue of critical components
- Riser Interference.

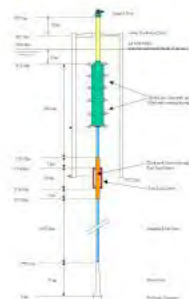
VIV - Why Important to Risers



Compliant Risers



Top Tension & Free Standing



VIV contributes significantly to the fatigue life of the riser.

- For Compliant:
 - Top Articulation
 - TDP
- For TT & Free Standing
 - Keel Joint
 - Well head connection
 - Below buoyancy Tank
 - Seabed Connection



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Riser Response to Currents

Types of Risers:

- TTR - top tensioned riser
- SCR - steel catenary riser
- Flexible riser
- HCR - highly compliant
- FSR - free standing riser



Riser Response to Currents

- Typical dimensions
 - 4” flowline SCR
 - 4” flowline SCR with 9” buoyancy
 - 21” drilling riser
 - 48” drilling riser with buoyancy
 - 18” export SCR
 - 120” aircans
 - 8” tieback risers



Effects of End Conditions

- Taper Joint
- Flexjoint
- Keel joint
- Tensioner/Gimbal
- Guide frames
- Cantilever wellhead





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Drilling Roller frame



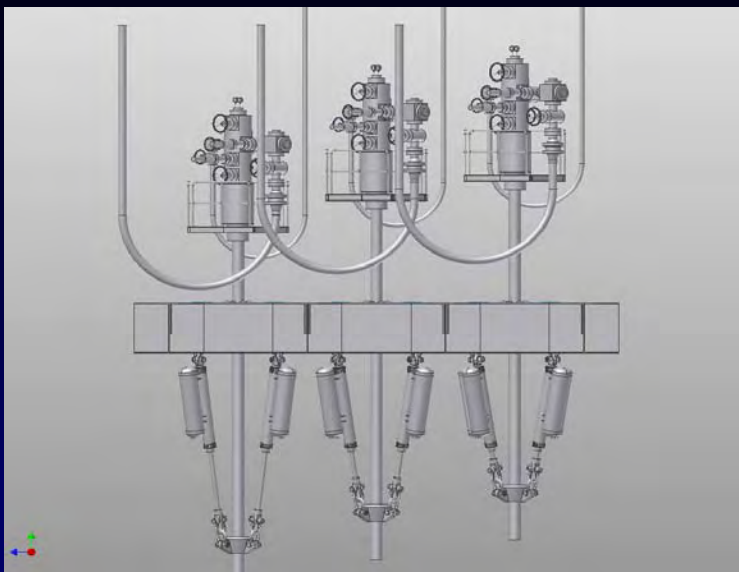
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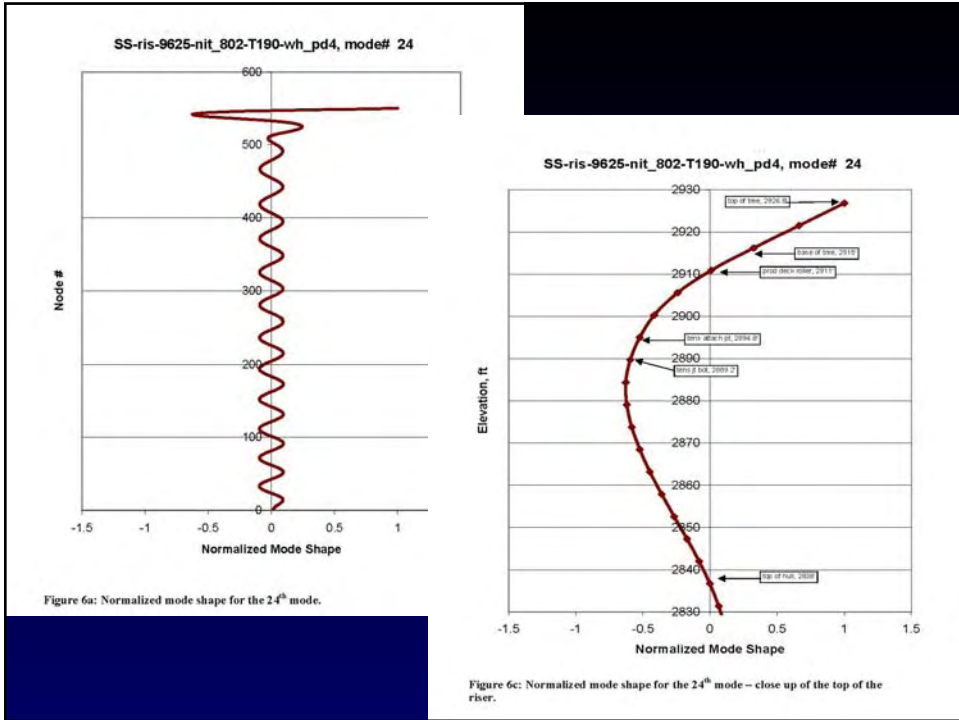
PRT at Full Extend, Null, Full Retract





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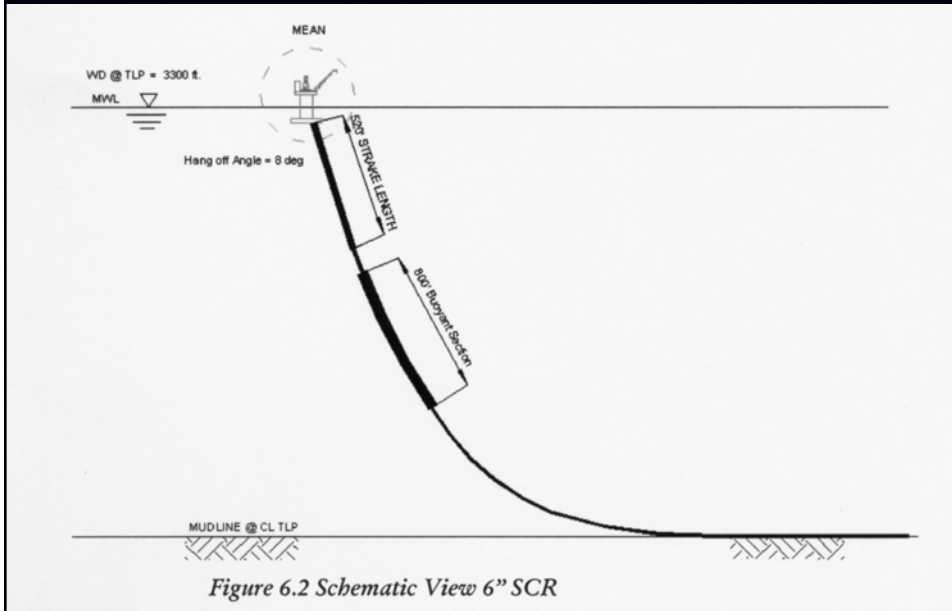


Riser Response to Currents

- Buoyancy, cladding, strakes, etc
- Touchdown point



KingKong riser with Buoyancy



Riser Response to Currents

Static response

- Vessel Offset
- Increased drag due to VIV
- Interference analysis
- Touchdown trench effects



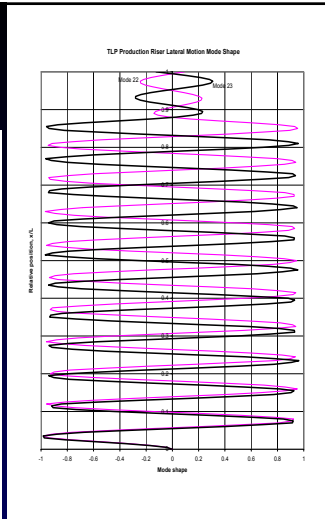
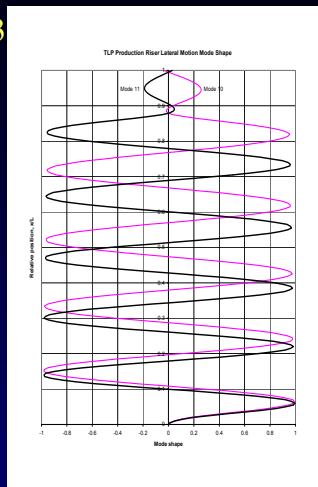
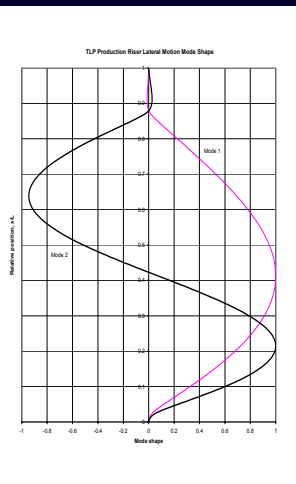
Riser Response to Currents

Dynamic response

- Modal analysis (string, beam models)
- Bending vs axial modes
- Finite versus infinite transmission models



TLP production Riser Modes 1-2,11-12,22-23





Mode No.	Frequency, Hz
1	0.090
2	0.180
3	0.271
4	0.363
5	0.456
6	0.552
7	0.650
8	0.750
9	0.853
10	0.959
11	1.069
12	1.183
13	1.301
14	1.423
15	1.549
16	1.680
17	1.816
18	1.957
19	2.103
20	2.255

Typhoon TLP Tendon Lateral Motion Natural Frequency
(Pretension = 1917 kips)

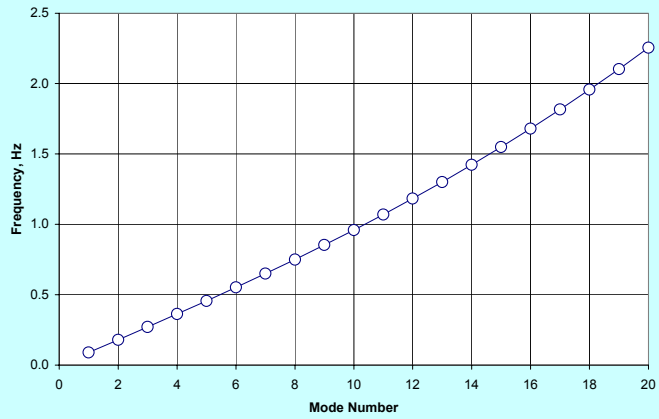
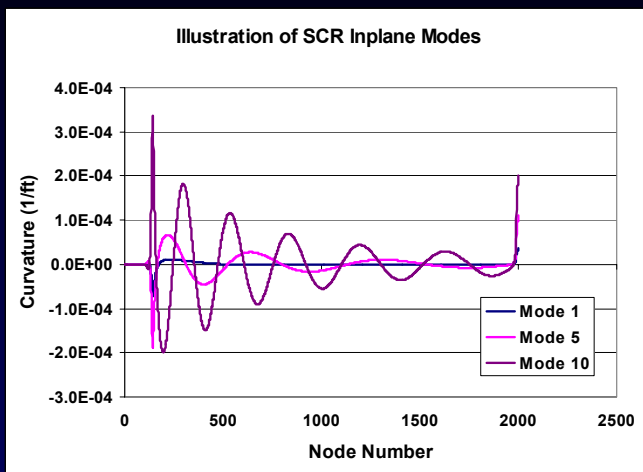
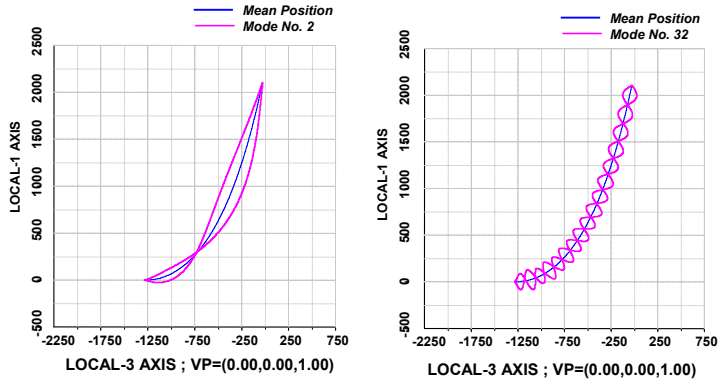


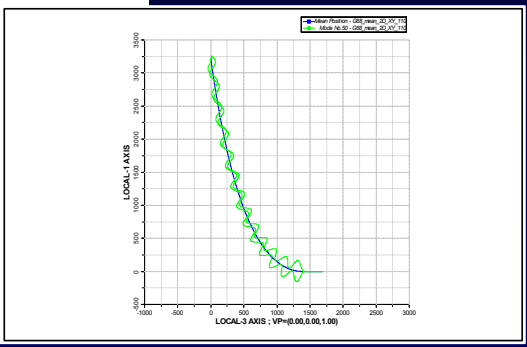
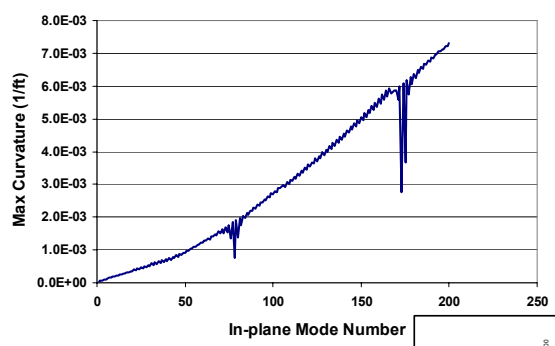
Illustration of SCR Inplane Modes



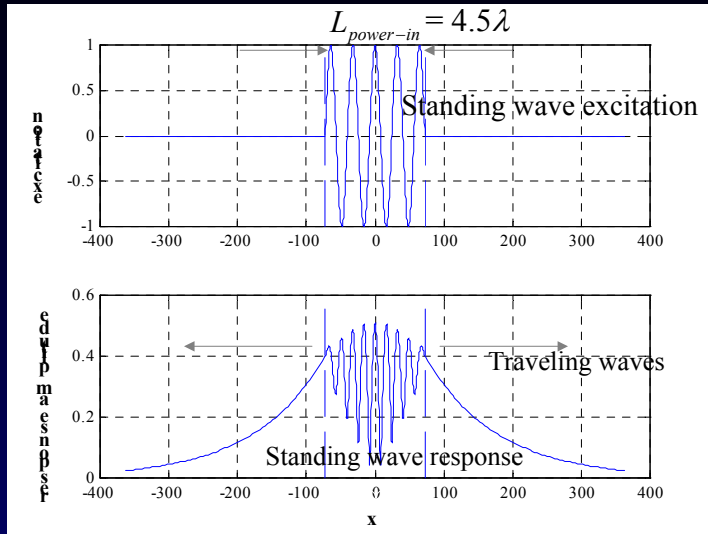


Examples of SCR Mode Shapes

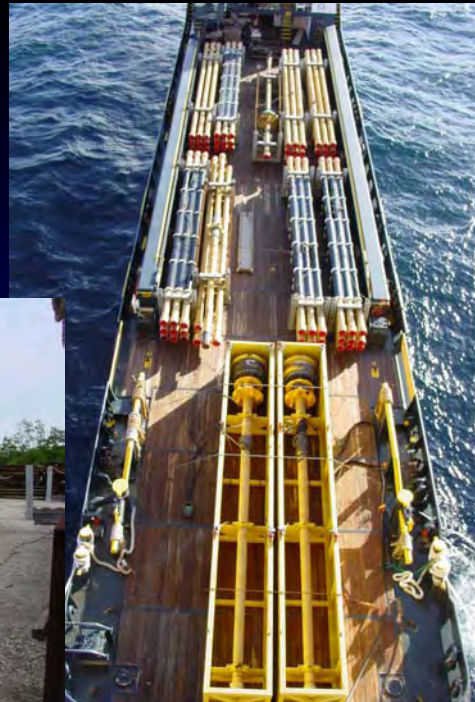
Illustration of Axial Modes Influence



Steady Flow Response for an Infinite Beam with Finite Length Excitation



Matterhorn Production Risers





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Matterhorn production riser from main deck



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Joliet Single Piece Tendon



Steel tendons 24" in diameter.

Provided by
Steve Leverette
Atlantia Offshore Limited



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Riser Response to Currents

- VIV response characterization
 - Strouhal number, Reduced velocity
 - Cl (AOD, Re , V_r)
 - Pre-lockin, Lock-in, lock-out
 - Power in, versus damping and radiation
 - Effects of damping on response
 - Power balance solution



VIV Characteristics

$$St = \frac{f_{st} D}{U}$$

Strouhal Number

$$V_r = \frac{U}{f_{st} D}$$

Reduced Velocity



$St=0.2, V_r=5$

Historical Shear7, C_{lift} Formulation

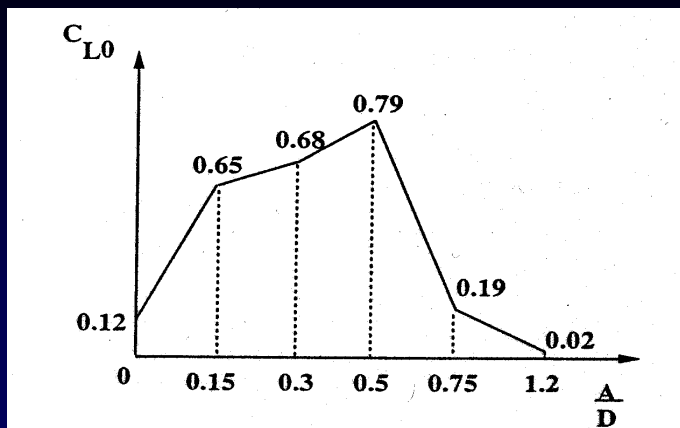
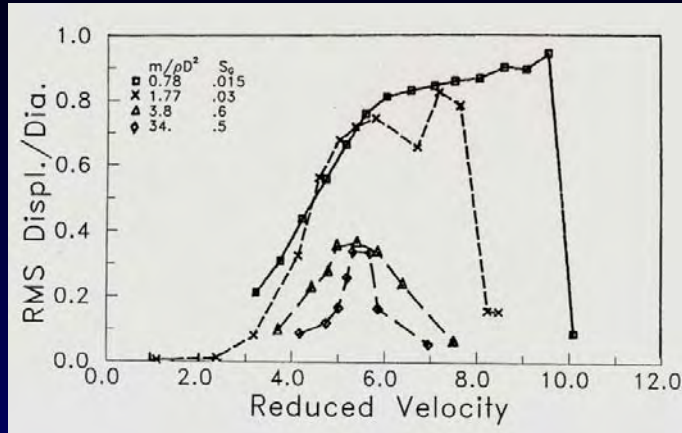


Figure 8: The lift coefficient as a function of $\frac{A}{D}$



Pre-lock-in, Lock-in, Lock-out



Dynamic Response

Power Balance

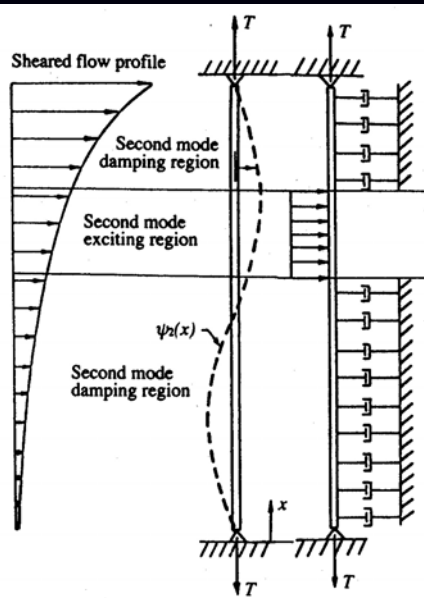


Figure 11. Power flow model for a flexible cylinder.

Power Balance

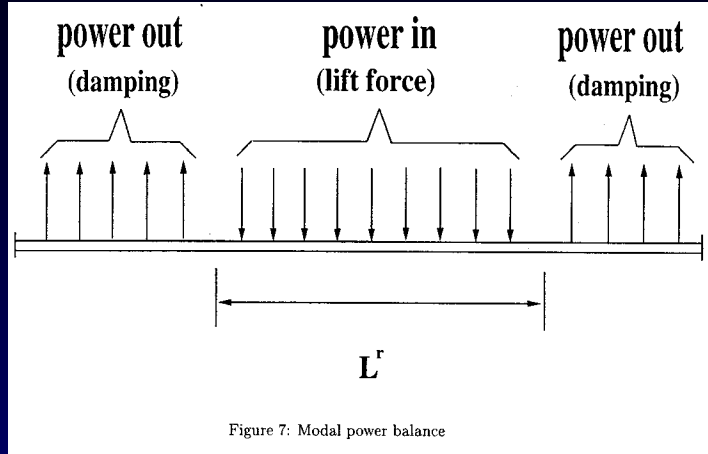


Figure 7: Modal power balance

Riser Response to Currents

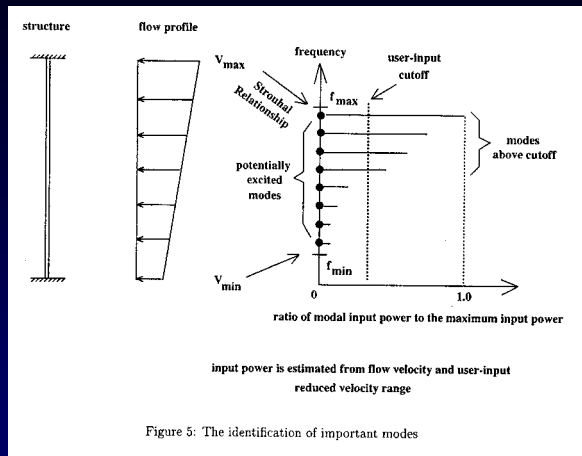
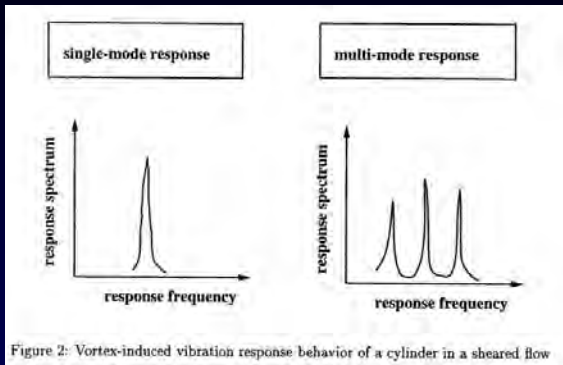


Figure 5: The identification of important modes



Riser Response to Currents



Single-mode vs
Multi-mode

In-line vs
transverse



Riser Response to Currents

- Bending and increased drag effects
- Spectral approach to response
- Uncertainties in response
- Typical hot spots
- Types of currents that are problematic
- Effect of increased tension (decrease mode, reduce bending)



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Fatigue caused by VIV

Fatigue

$$D = \sum_{i=1}^k \frac{n_i}{N_i}$$

Miner's rule

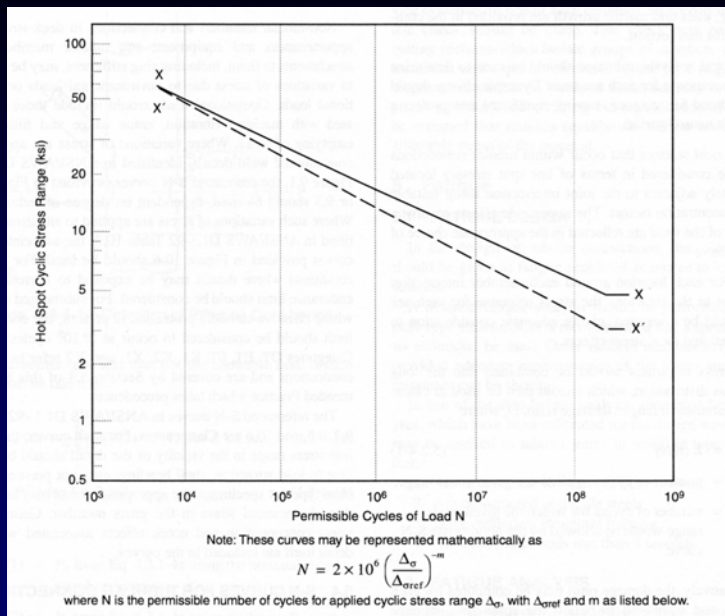
$$\log N = \log a - m \log \Delta \sigma$$

$$\log N = \log a - m \log [\Delta \sigma (t/t_{ref})^k]$$

API, AWS

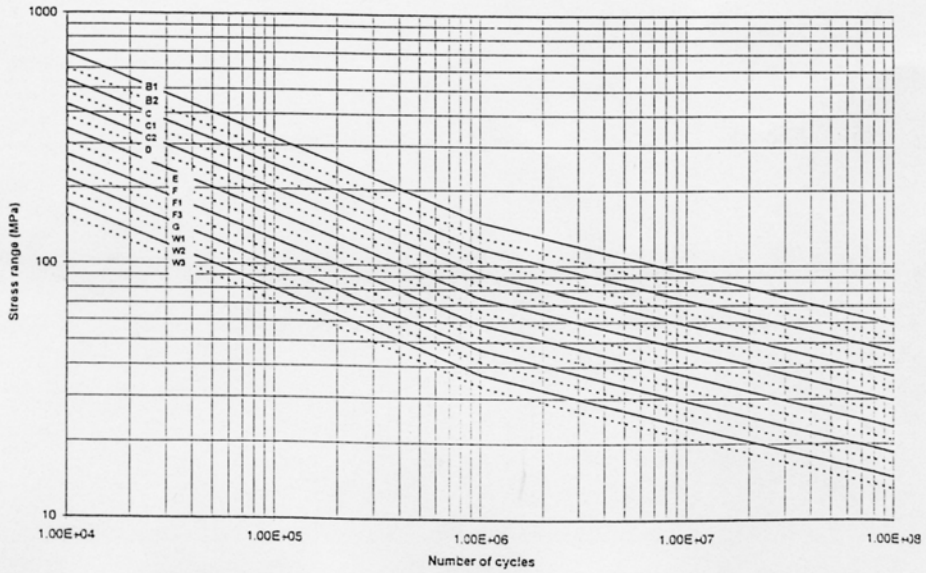
DOE, DnV

Fatigue

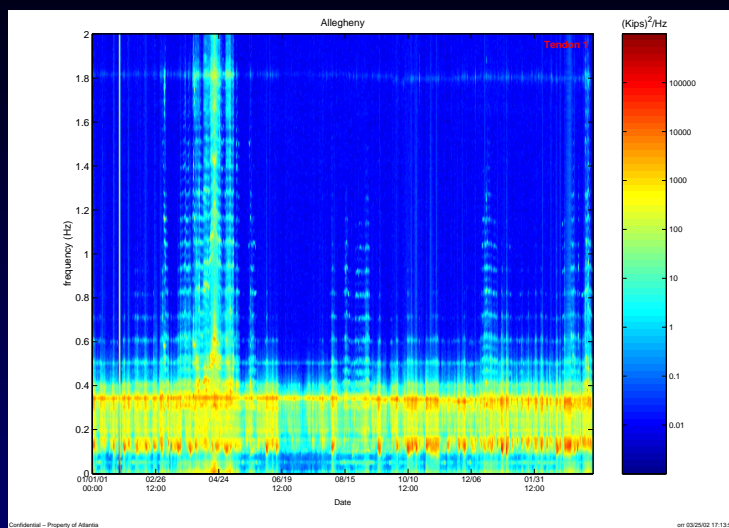




Fatigue



One Year of Tension Spectra





Safety Factors

- Depends on application
- Typically 3 for structures
- Typically 10-20 for risers



Fatigue

- Fracture Mechanics
 - Frequency Sensitivity: mode, curvature, no. cycles
 - Ways to combine response modes
 - Rainflow
 - Spectral approach to fatigue (Wirsching, Dirlik)
- Sensitivity to assumptions



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Tendons in VIV

- OTC Paper
 - Leverette, Rijken, Thompson, Dooley



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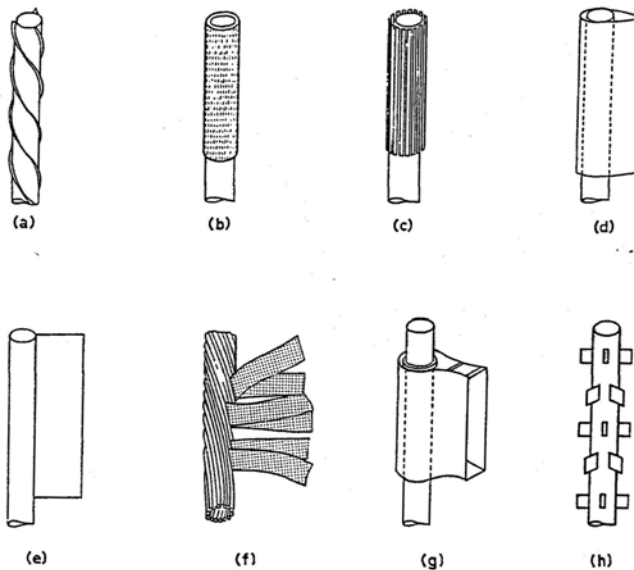
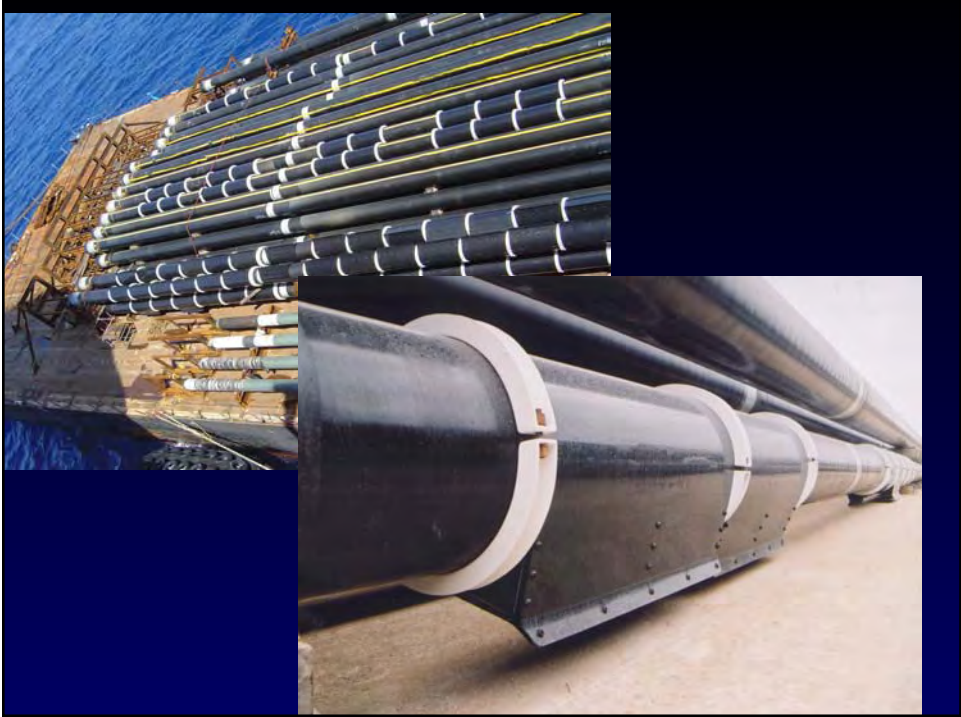


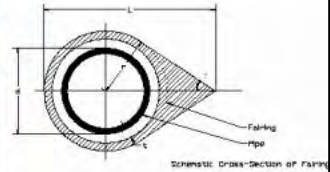
Fig. 3-23 Add-on devices for suppression of vortex-induced vibration of cylinders: (a)



VIV Suppression (cont)



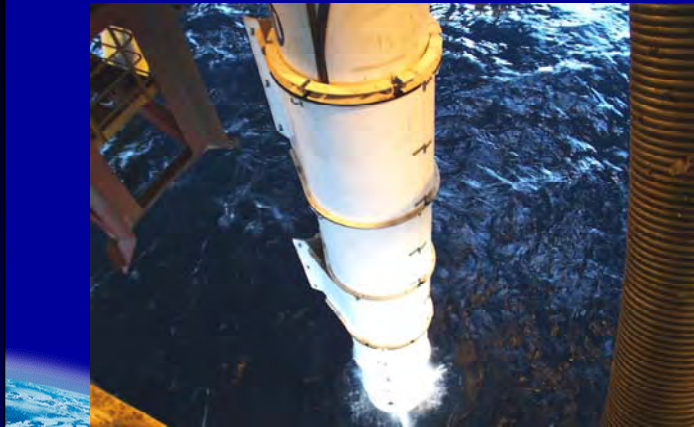
Shell Global Solution Fairing Design





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Shell Global Solutions

Fairings for drilling riser

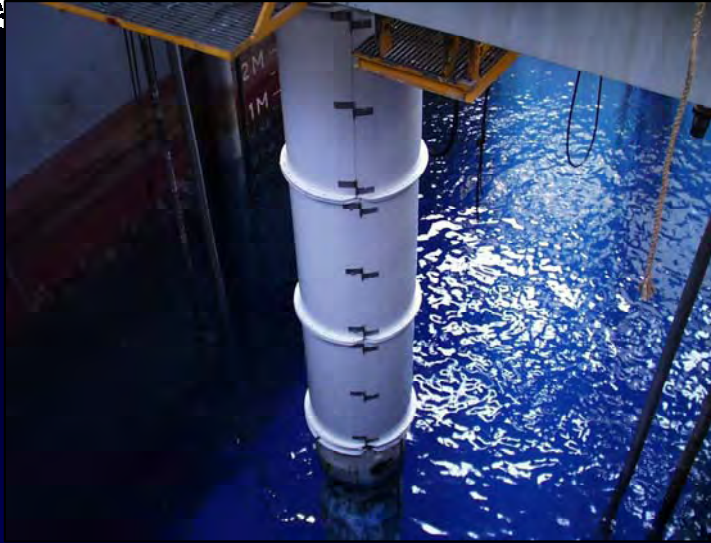


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Fairings for TLP tendon



Fairings in moonpool



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Half strakes for pipelines

VIV Suppression (cont)



- Typical fairings and strakes both offer very good suppression in terms of A/D in the 90s%
- Fairings have lower drag and tend to be more efficient in a downstream riser for riser in arrays.
- For risers in arrays, the strake efficiency of the downstream riser is lower than that in a free stream.
 - Experiments at MIT have indicated downstream riser efficiency of the order of 50% less to the upstream.
- Strakes non susceptible to marine growth need to be selected for near surface applications.
 - Marine growth significantly affects the strake performance and subsequence the riser fatigue life.
 - A 30% reduction in efficiency could result in 70% reduction in fatigue life.

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Tools for Predicting VIV



- There are a number of models in the industry that can be used to analyse risers against VIV behaviour.
- The models can be classified in two main categories:
 - Semi-empirical
 - Shear7
 - VIVA
 - VIVANA
 - IFP model
 - Model from University of Milan
 - The Technip time domain model

The majority of these tools are based on an energy balance, (power in = power-out) and rely on semi-empirical formulations for lift, drag and damping coefficients.

Codes are either frequency or time domain.

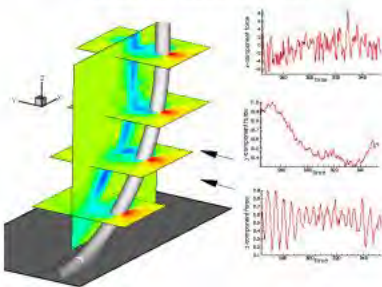
Tools for Predicting VIV (cont)



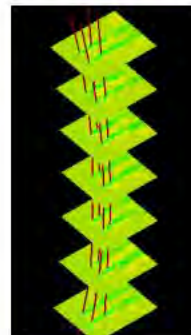
- CFD Models
Based on the fundamentals of fluid mechanics, but not fully advanced yet and take considerable computing time to run. Tend to use a semi-empirical model for the structural coupling.
e.g: Imperial College Model, UT, DHI

CFD application in SCR, IC

Flexible riser pipe in cross-flow



CFD application in TT risers, NH



Tools for Predicting VIV (cont)



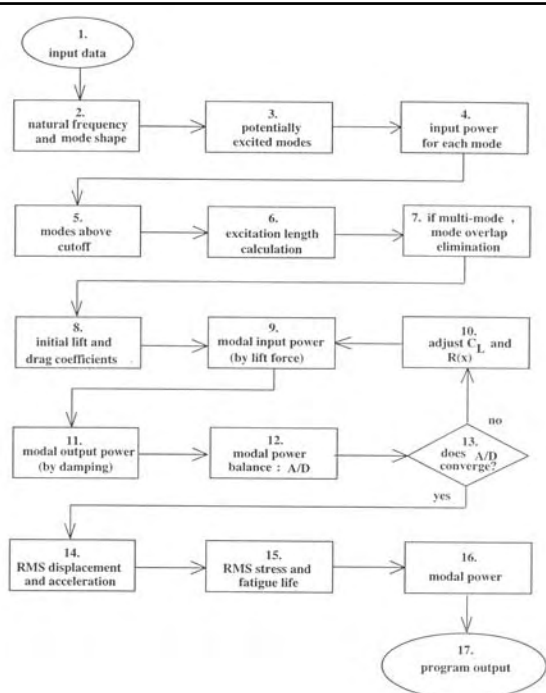
- CFD codes are still slow for use in a project environment, when large number of calculations (number of current profiles can exceed 60) are required.
- CFD today tends to be used on detail studies, i.e understand SPAR TT VIV behavior.
- Industry tends to use more the semi-empirical models for design.
 - Response is very sensitive to the parameters used, i.e C_I , St , Single/Multimode, damping,....
 - The most widely used code is Shear7 although VIVA and VIVANA have been recently used in a number of designs.
 - Most of these codes were “tuned” for cylindrical structures. – New designs do not always fit to this approximation.
 - There have been a number of lab and field initiatives to collect data for different riser types, and calibrate the codes.



Analysis Methods and Programs

- Engineering tools, not simulators
- Summary of how they work
- Comparisons
- Why they vary so much
- CFD
- Wake models and other approaches

Flow diagram of Shear 7 Analysis



Riser Response to Currents

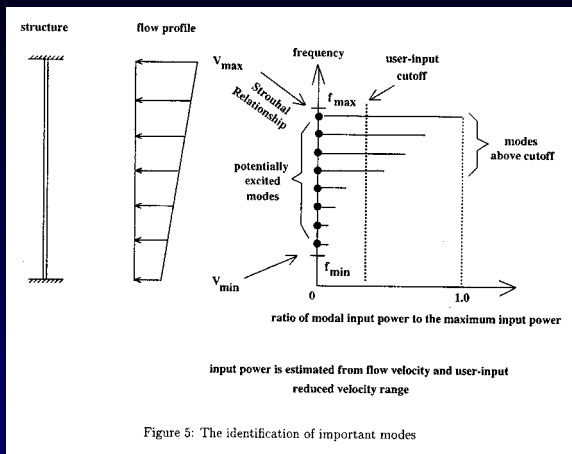
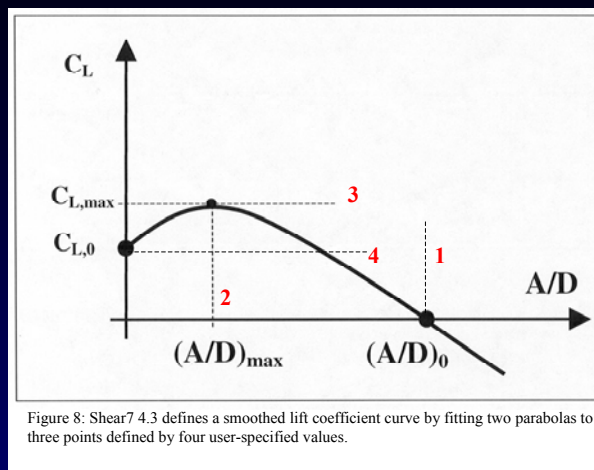
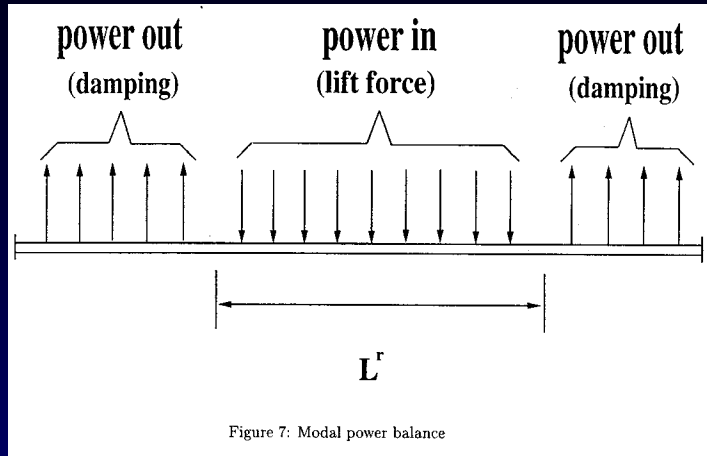


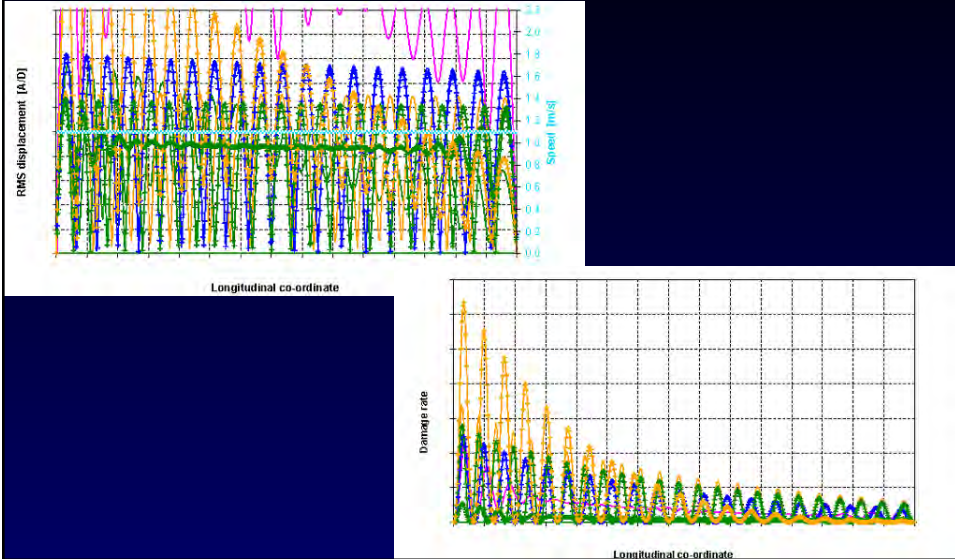
Figure 5: The identification of important modes





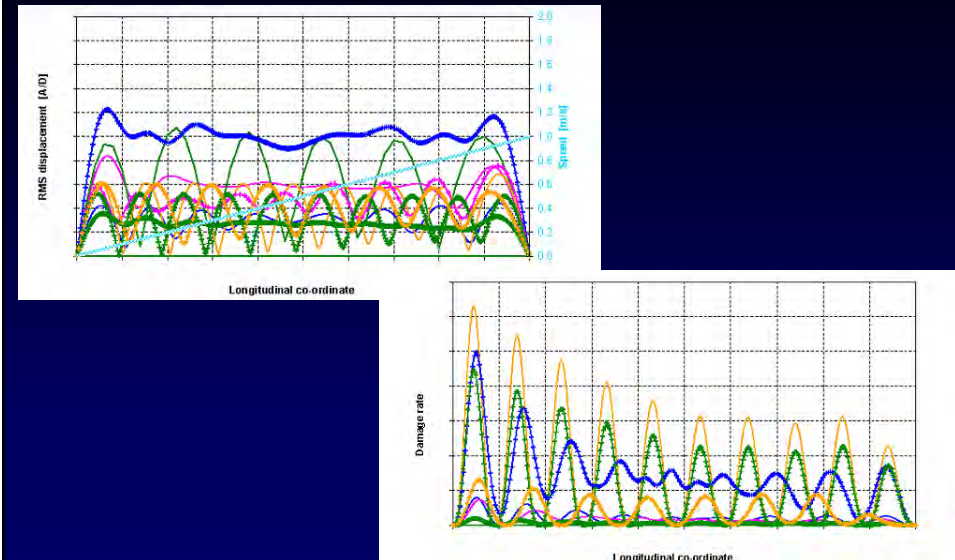
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VIV Program Comparison (Stride JIP - 2H) RMS and Damage



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VIV Program Comparison 2 (Stride JIP - 2H) RMS and Damage





Example Shear7 Analysis

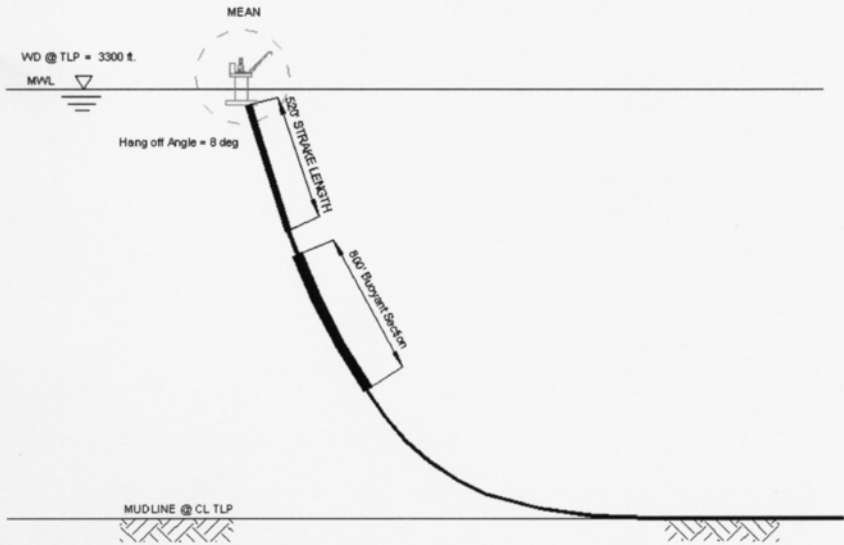
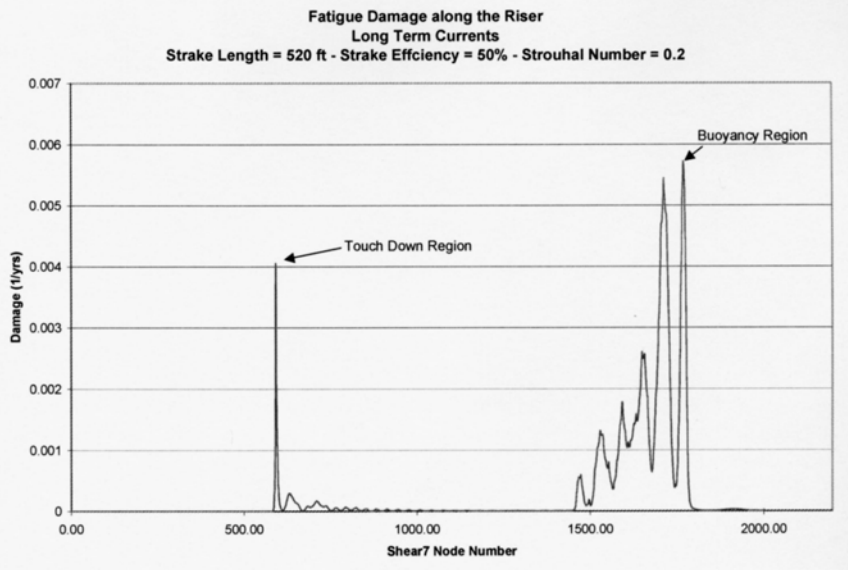
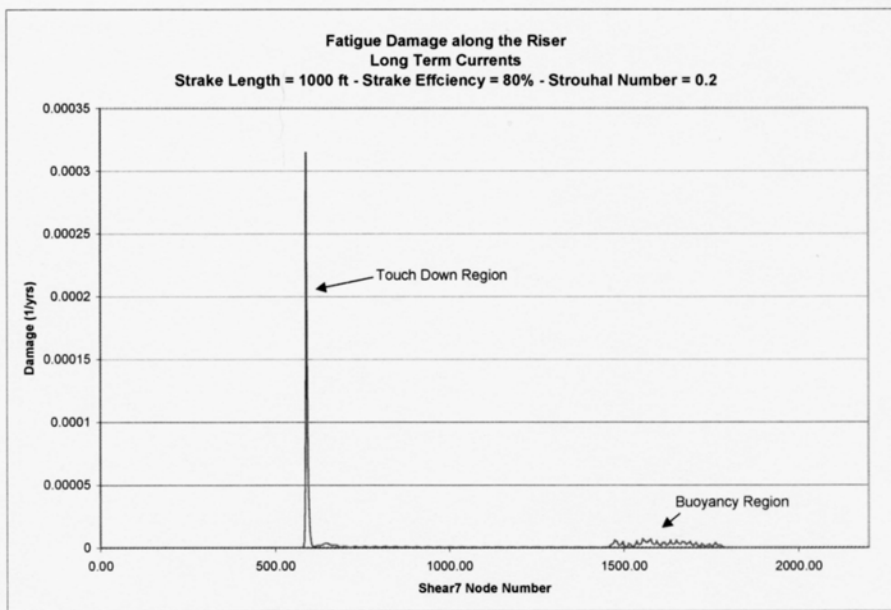


Figure 6.2 Schematic View 6" SCR





Analysis Methods and Programs

- Shear7, Viva, Vivana, Vivarray
- Engineering tools, not simulators
- Summary of how they work
- Comparisons / Example
- Why they vary so much
- CFD
- Wake models and other approaches



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Agenda

- Introduction
- Environment
- Riser Response to Currents
- Fatigue
- Tendons
- The FIX
- Analysis Methods and Programs
- Model Tests and Data Sets**
- Other Issues
- Areas of R&D
- Industry Experience
- Conclusion



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The Literature

Review Papers

- Parkinson (1974, 1989)
- Sarpkaya (1979, 1995)
- Griffin and Ramberg (1982)
- Bearman (1984)
- Pantazopoulos (1994)

Books

- Blevin 1990
- Chen 1987
- Naudascher and Rockwell 1994
- Sumer and Fredsoe 1997
- Au-Yang 2001



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Recent Model Tests and Data Sets

- Deepstar – St Johns, CFD, Lake Seneca
- Stride (2H Allegheny and model tests)
- VIVA
- PMB HCR Lake Pend Oreille
- BP
- Exxon
- Shell



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Other Issues

- Wake effects, riser arrays
- High Re performance of fairings/strakes
- Issue of low mass systems, frequency independence
- Directionality of currents, resulting stress hot spots
- Directionality of response amplitude
- Trenching



Trenching





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- **Areas of R&D**
- Industry Experience
- Conclusion



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Areas of Research and Development

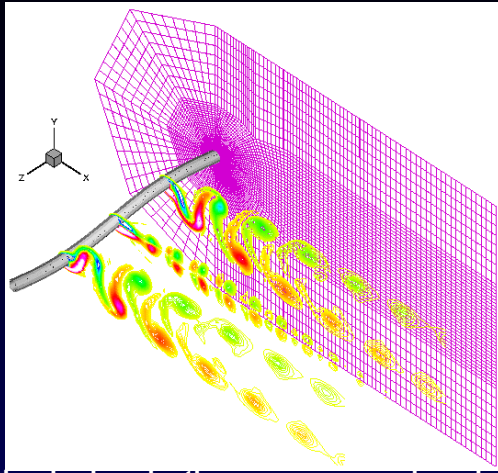
- Empirical
- CFD
- Fairings
- JIP's
 - MIT - VIVA
 - MIT - Vandiver JIP and Shear7
 - Deepstar (ARA CFD, Principia CFD, Lake Seneca, St. Johns)
 - Several CFD proposals out

What is the Industry trying to address.

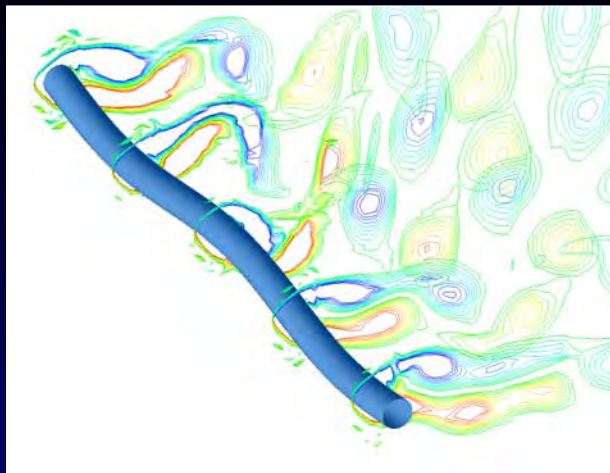


Diameter	Current	Shapes	Depth	Waves	Suppression	Lay Out
8"			1000 to 10,000 feet	Wave Height 2' to 72'		
12"						
16"						
20"						
28"						
36"						
60"						

Slides from Chevron (O. Oakley)

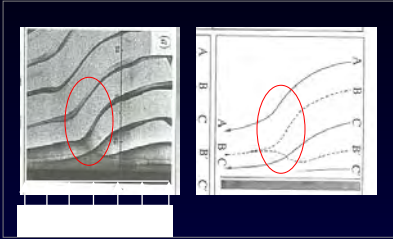


riser undergoing hydro-elastic response; mesh and vorticity contours in the wake are shown; note the difference in the shed wake pattern between the nodes and the anti-nodes of the structure

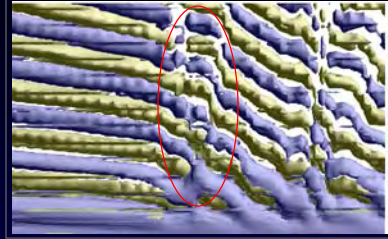


Vortex Dislocations Visualizations

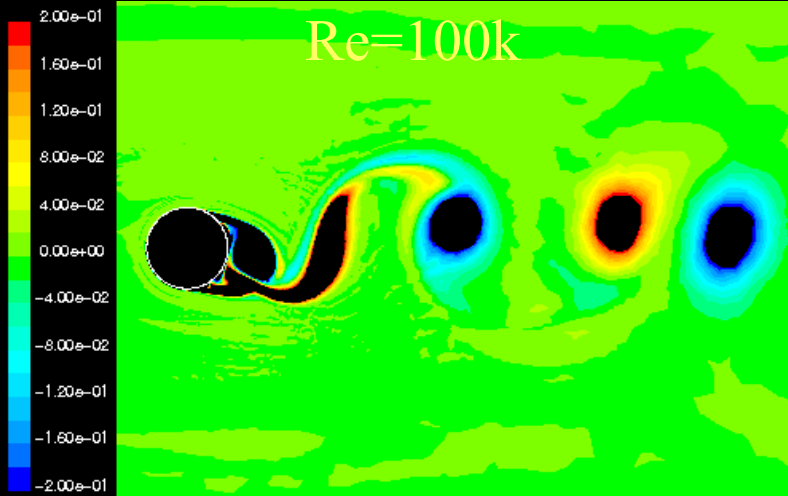
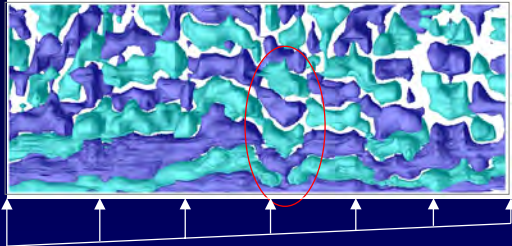
Re=100 Experiments C.H.K. Williamson (1989)



Re_{max}=100 NÉKTÓR-DNS Simulations



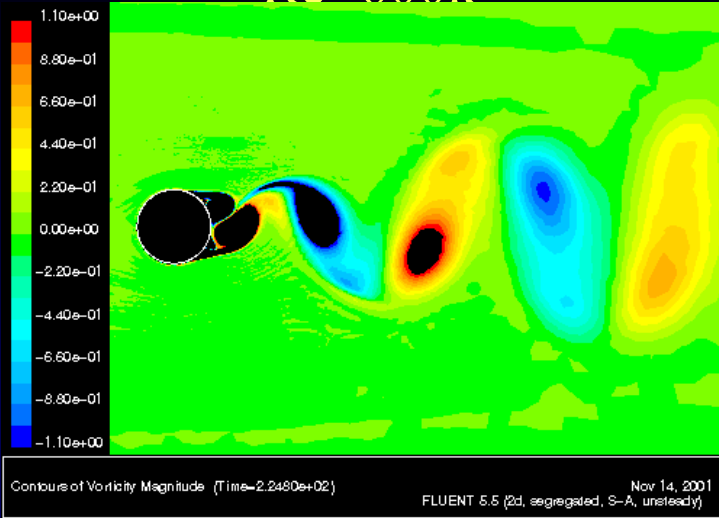
Re_{max}=1000 NÉKTÓR-DNS Simulations



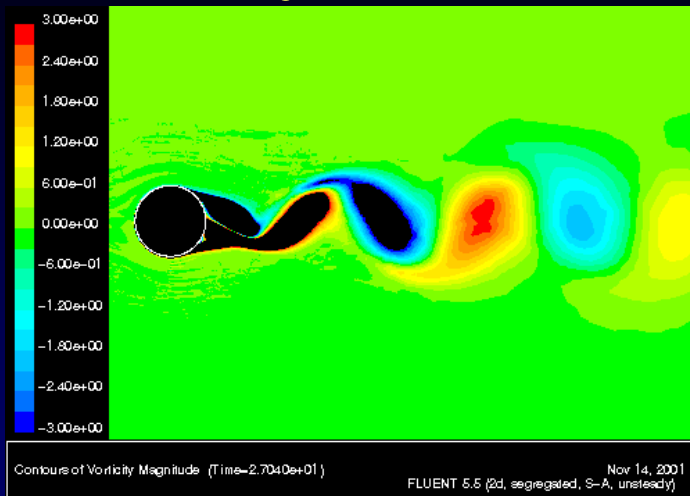
Contours of Vorticity Magnitude (Time=1.0060e+03)

Nov 14, 2001
FLUENT 5.5 (2d, segregated, S-A, unsteady)

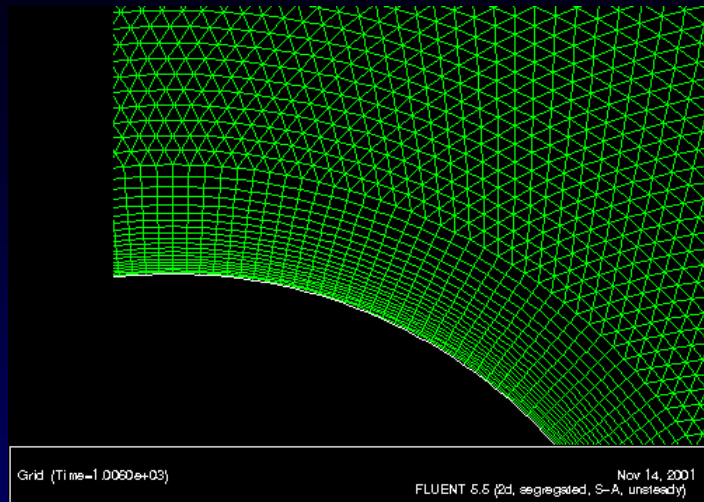
Re=600k



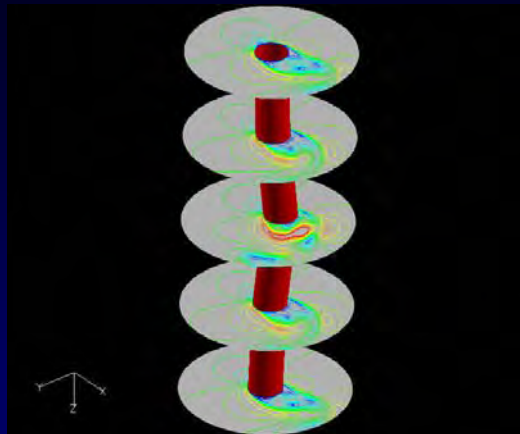
Re=4MM

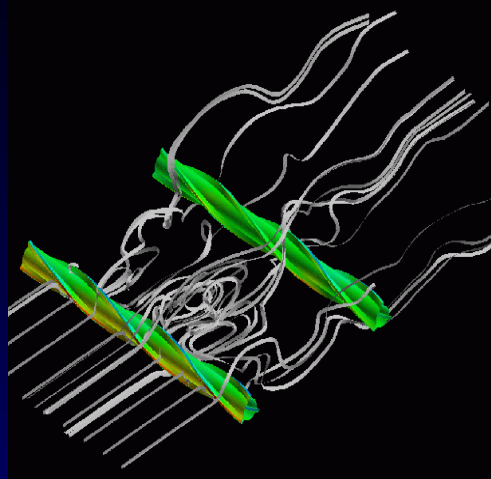


Grid for cylinder

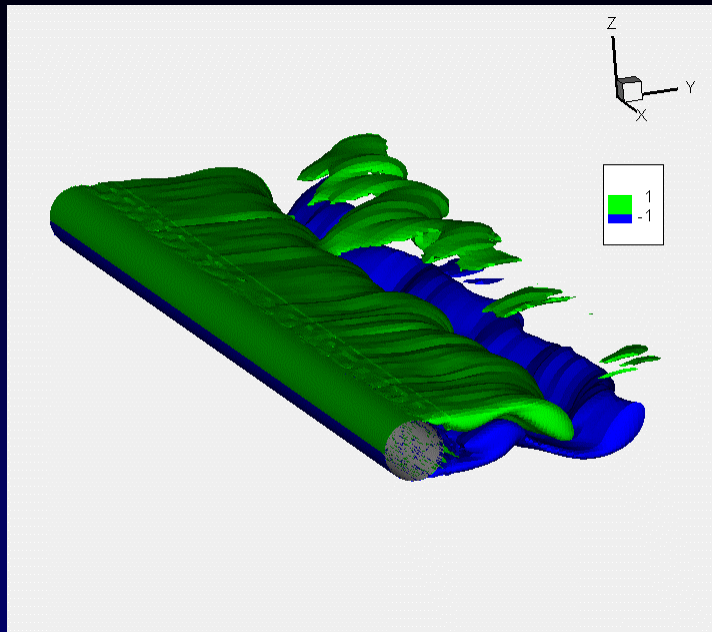


Strip theory approach to riser modeling (J. Kallinderis)





Constant Contours of Spanwise Vorticity (non-dimensional)



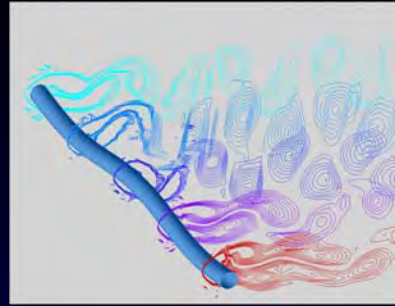
Numerical Modeling

CFD = computational fluid dynamics, solution of Navier-Stokes equations for fluid flow

$$\frac{\partial \hat{U}}{\partial x_i} = 0$$

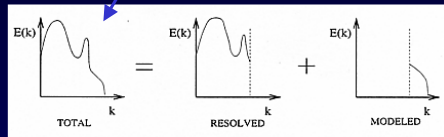
$$U_j \frac{\partial \hat{U}_i}{\partial x_j} = \frac{\partial}{\partial x_j} (v \frac{\partial \hat{U}_i}{\partial x_j}) - \frac{1}{\rho} \frac{\partial \hat{\rho}}{\partial x_i}$$

$$U_j \frac{\partial \hat{\Theta}}{\partial x_j} = \frac{\partial}{\partial x_j} (\Gamma \frac{\partial \hat{\Theta}}{\partial x_j})$$



DNS – direct numerical solver

CFD Approach:	DNS	LES	RANS	DES
Turbulence Model	none	Smag	S-A	S-A
		S-A	k-ε	
		..	k-ω	
			SMC,	

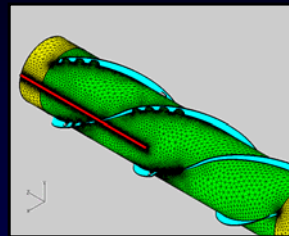


LES turbulence model – spatial filter representation of turbulence fluctuations

CFD – Spars

2dof Modeling – Straked Cylinder

- No chains, pipes or anodes
- S-A turbulence model
- Re = 34MM
- Navier-Stokes Solution:
- URANS - time averaging
- LES – space averaging
- DES – RANS in boundary layer & LES outside

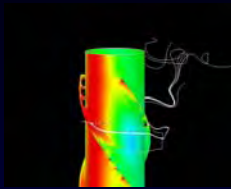


RANS Grid

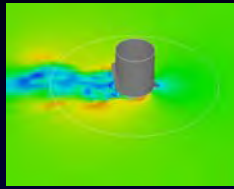


DES Grid

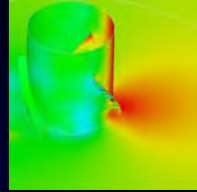
Displacement History Genesis spar, strakes only



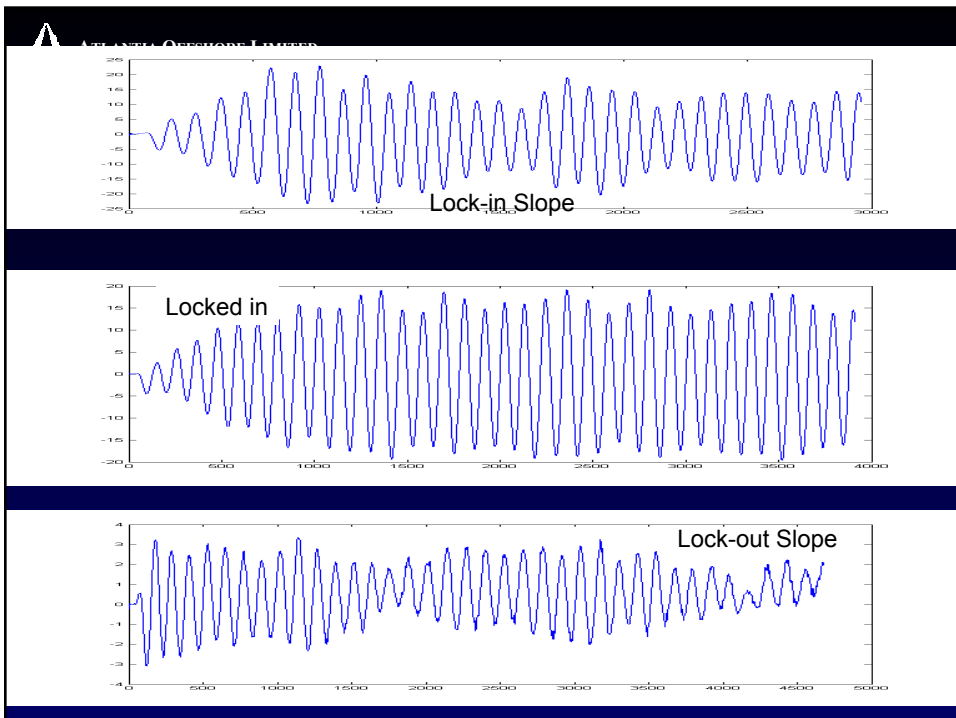
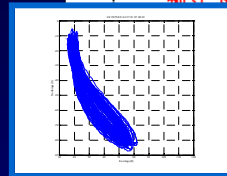
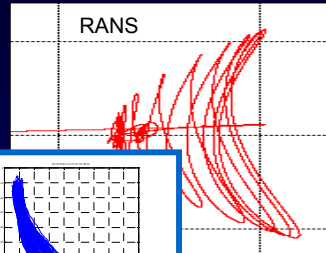
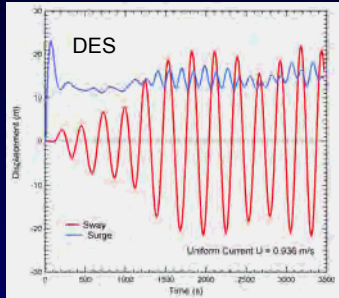
Pressure



Velocity Mag.



Pressure





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Objectives and Challenges for High Mode Number, Flow-Induced Vibration Model Tests in Sheared Flow

Prof. J. Kim Vandiver

MIT

June 3, 2003



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Principal Issues for High Mode Number VIV Response Prediction

- Does lock-in occur at high mode number in uniform and sheared flow?
- What are the statistics of single versus multi-frequency response in sheared flow?
- What are the statistics of in-line and cross-flow response?



Principal Issues for High Mode Number VIV Response Prediction

- Do hydrodynamic damping models need improvement?
- What fairing or strake coverage is required
- What is the effect of Reynolds number on S_D , C_L damping and suppression effectiveness.

Important dimensionless groups

1. $\frac{m}{\rho_f D^2}$ mass ratio 2. $V_R = \frac{U(x)}{f_v D}$, Reduced velocity

3. $\frac{R_n \omega_n}{\rho_f U^2(x) L_{in}}$ Reduced damping

4. What about correlation length?

5. $\zeta_s = \frac{r_s}{2\omega_n m}$, and $n\zeta_t = \frac{2L}{\lambda} \zeta_t$

Modal damping and wave attenuation

6. $R_e = \frac{DU(x)}{\nu}$, Reynolds number

How is lock-in affected by mass ratio versus reduced velocity bandwidth?

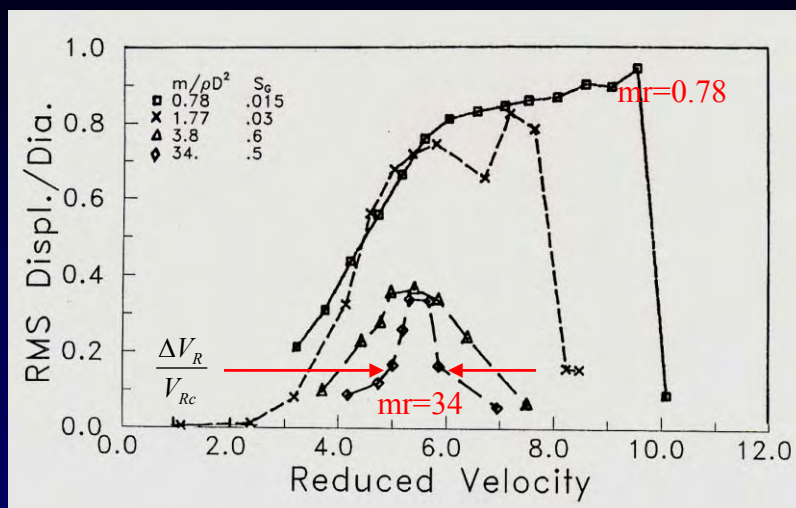
1. $\frac{m}{\rho_f D^2}$ mass ratio

2. $V_R = \frac{U(x)}{f_v D}$, $dV_R = \frac{\Delta V_R}{V_{Rc}} = \text{lock-in bandwidth}$

f_v = the vibration frequency

- Is the response different if the wake adjusts its frequency to match the cylinder or the natural frequency adjusts to match the wake.

A/D versus Reduced velocity (based on a fixed natural frequency)

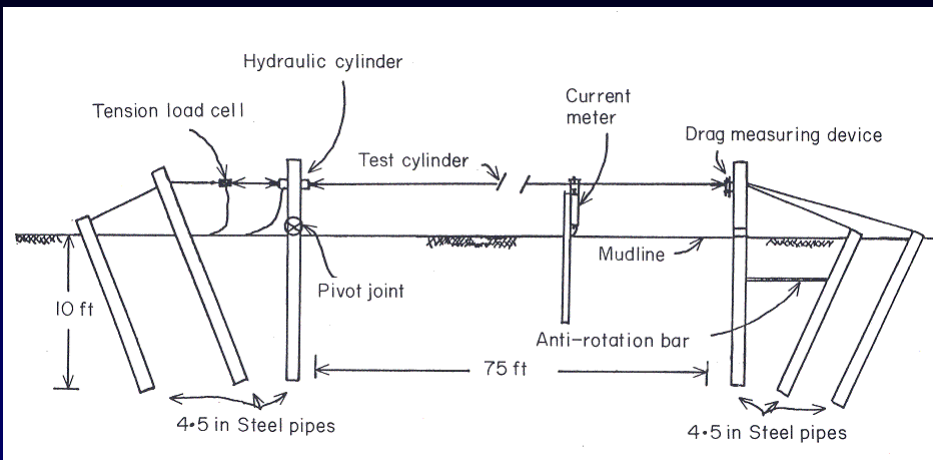


Castine field experiment in uniform flow(1981)

•Uniform flow on short lengths(75 feet)



Site layout



Cable, pipe and wire rope properties

- Cable was PVC plastic tubing with accelerometers, wires, strength members and potting compound, $D=1.25$ inches., $s.g.=1.4$
- Pipe was 1.631 inch steel tube with the cable pulled inside as the measuring instrument. $S.g.=2.4$
- Wire rope was polyethylene coated oceanographic wire, 3x19 construction. $D=.28$ inches, $s.g.=2.5$

Castine pipe response, $L=75$ ft, $D=1.63$ inches, specific gravity = 2.4

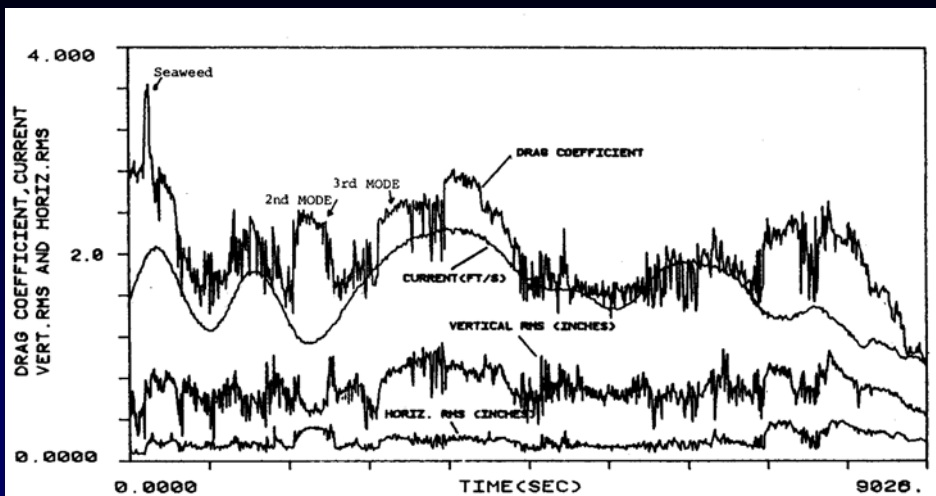
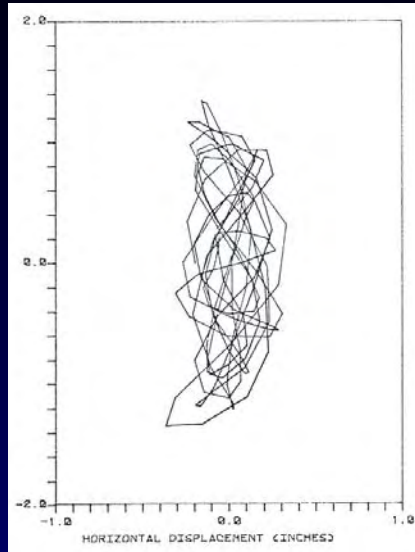
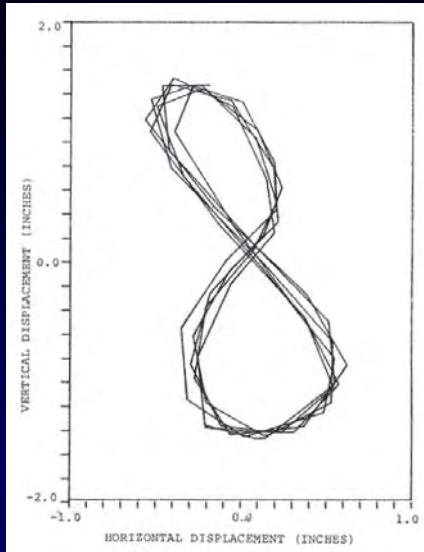
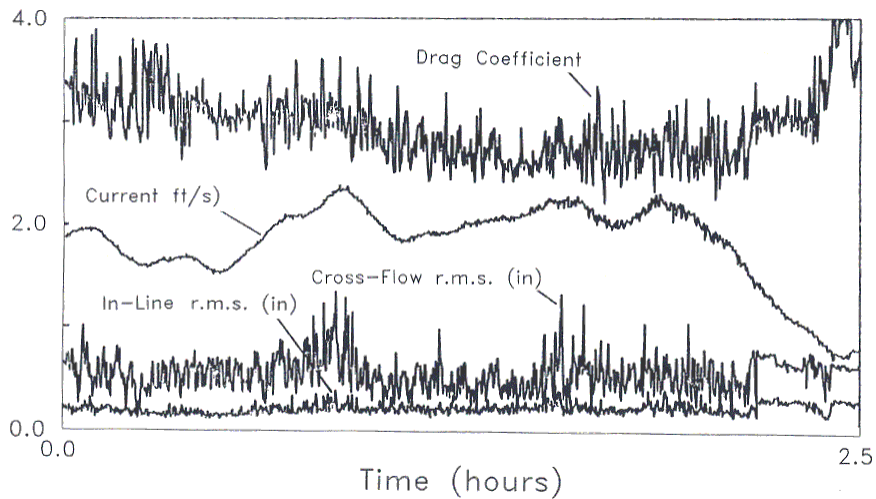


Fig. 4—Drag coefficient, current, and RMS displacement at L/6 for the pipe.

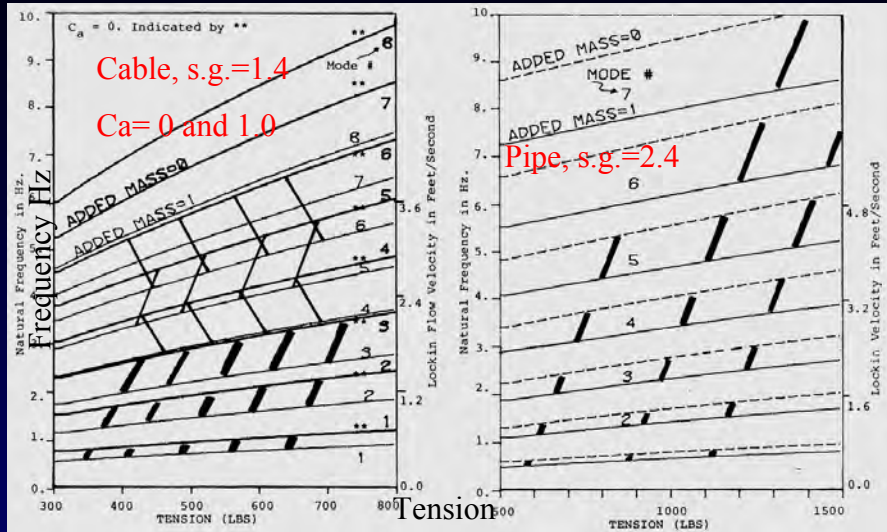
Pipe lock-in and non-lock-in response



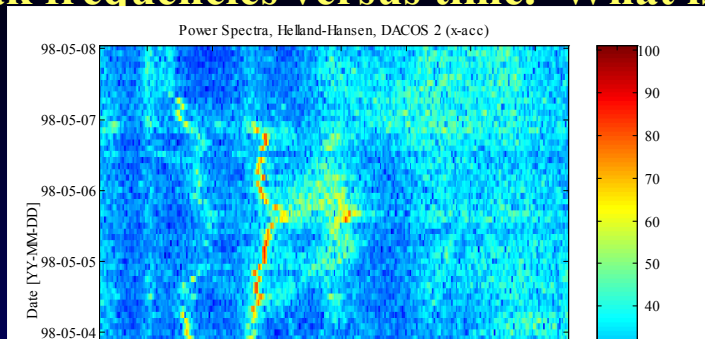
Castine cable cross-flow VIV, $D=1.25$ inches specific gravity = 1.4



Stable lock-in events are aided by isolated natural frequencies with no overlap.

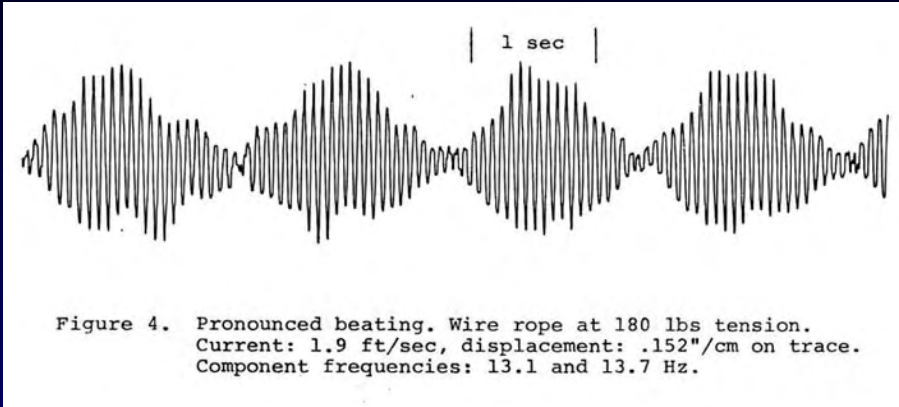


An example from Helland Hansen: Peak frequencies versus time. What is



From Robit Helland Hansen Report

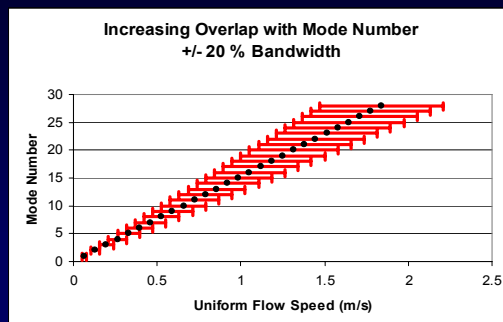
Wire rope: $L=75$ ft, $D=0.25$ inches, specific gravity = 2.6
Steady beating around 6th mode, components 0.6 Hz apart
Reduced velocity bandwidth of $\pm 20\% = \pm 2.6$ Hz



Does lock-in occur at High Mode Number ?

Increase in Uniform flow speed \rightarrow Increasing overlap of competing modes

Reduced velocity bandwidth of $\pm 20\%$ would include many modes.
Can just one respond in a uniform flow? What happens in shear?



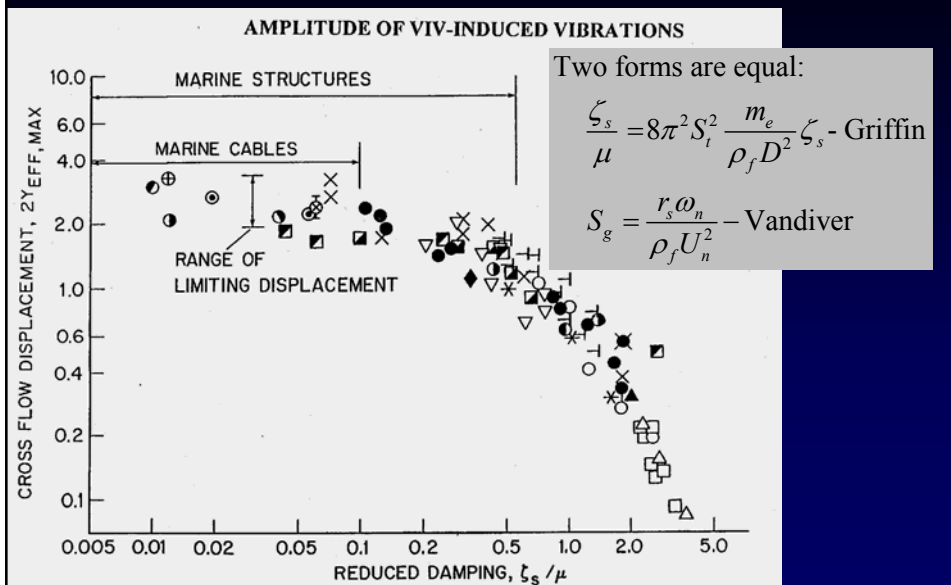
Wire rope: $L=900$ ft, $D=0.280$ inches,
specific gravity = 2.5

- Nearly uniform flow on a 900 foot long sample.
- $\pm 10\%$ flow variation along the length
- Lock-in at 50th mode

Conclusions:

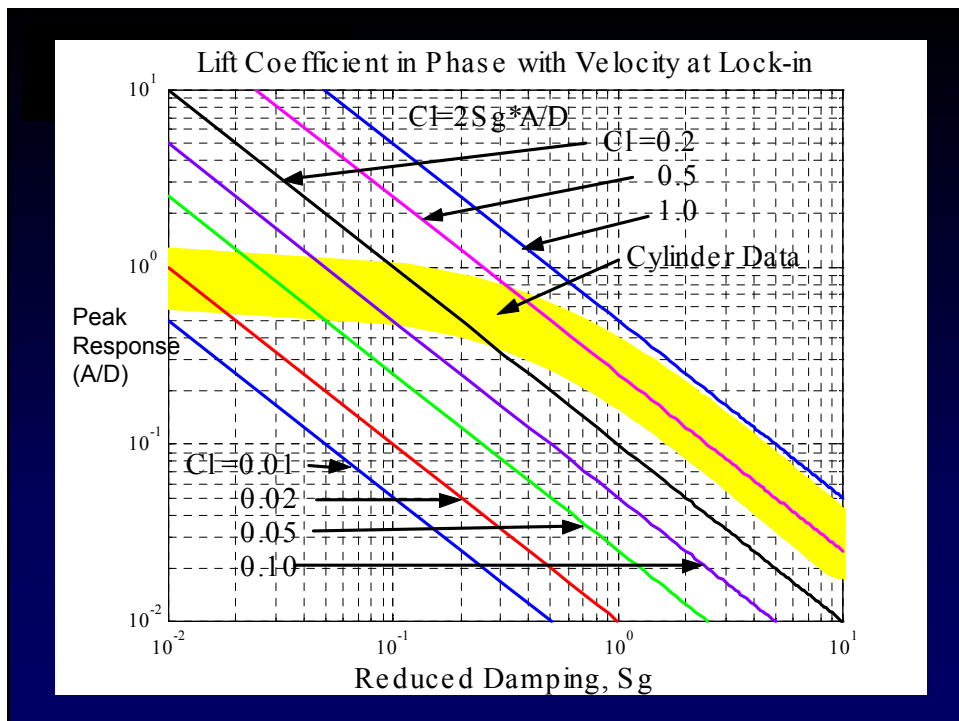
- High mass ratio provides stable sharp resonances.
- V_R bandwidth allows for some spatial tolerance to flow variations. Single frequency dominance is possible in nearly uniform flows, even at high mode number.

The role of damping in uniform flow? The reduced damping is a statement of dynamic equilibrium.



A useful property is that:

$\langle C_{L,n} \rangle = 2S_u \frac{q_n}{D}$, the equation of an hyperbola for lines of constant lift coefficient.



A flexible cylinder in a non-uniform flow with a power-in region of length L_{in} has a resonant modal response given by:

$$\frac{q_n}{D} = \frac{\langle C_{L,n} \rangle}{2} \frac{\rho_w U_n^2 L_{in}}{R_n \omega_n} = \frac{\langle C_{L,n} \rangle}{2 S_u}$$

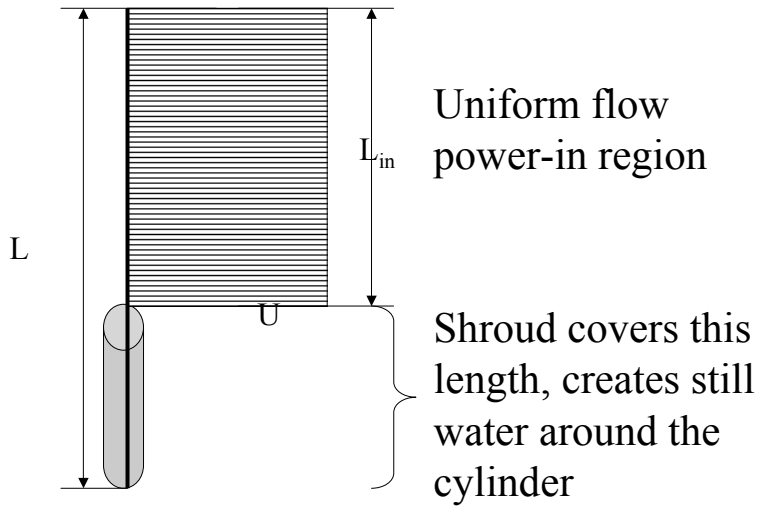
In red are 3 key parameters in prediction programs. They form a dimensionless group S_u and

$\langle C_{L,n} \rangle \equiv$ Average modal lift coefficient

$$S_u = \frac{R_n \omega_n}{\rho_w U^2 L_{in}} = \frac{8\pi^2 S_t^2 M_n \zeta_n}{\rho_w D^2 L_{in}}$$

Consider a simple slab flow problem first to illustrate the example of limited power-in length.

Shroud created a still water damped region



Example Experimental Data for a Slab Flow

- Tsahalis, D. T., *Experimental Study of the Vortex Induced Vibrations of a Long Model Riser Exposed to Uniform and Nonuniform Steady Flow*, Houston, Texas, Westhollow Research Center, 1985.
- Thank you to Shell Global Solutions for allowing me to publish these results.

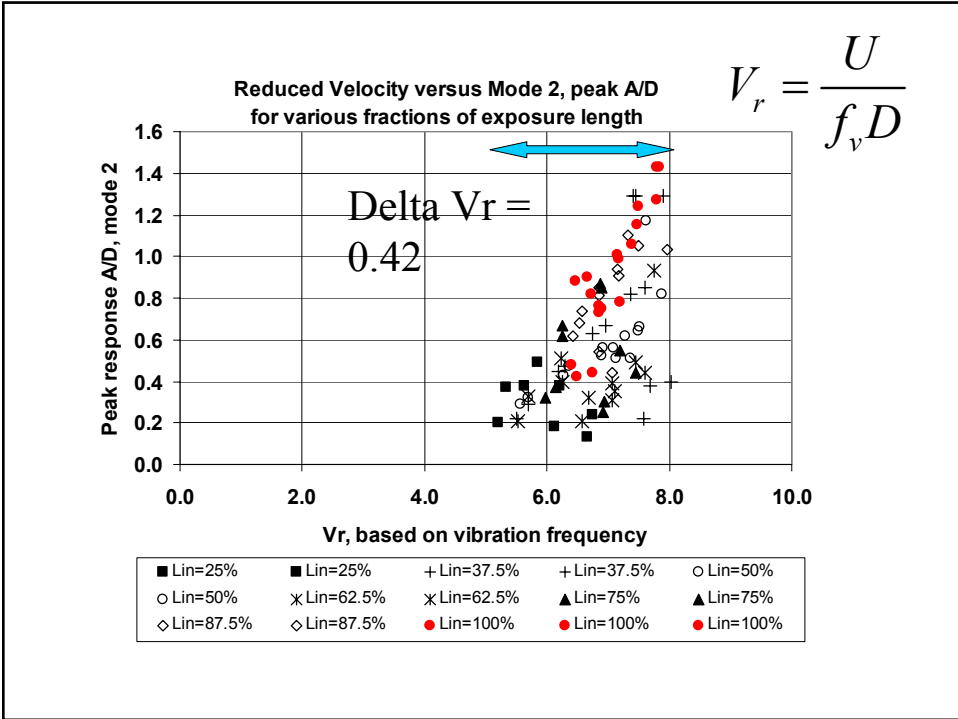
Experiment Details

- Steel pipe, O.D. = 1.5in(3.81cm)
I.D. = 1.334in(3.39cm)
- $U = 0.33$ to 6ft/s(0.1 to 1.83 m/s) in steps of 0.33ft/s(0.1m/s), modes 1 thru 3.
- Biaxial accelerometers at 5 points: $L/8$, $L/4$, $L/2$, $5L/8$, AND $5L/6$.

Shrouds used to simulate slab flow cases.

Exposed length of cylinder in 7 steps

- 100%
- 87.5%
- 75.0%
- 62.5%
- 50.0%
- 37.5%
- 25.0%



Jump to DS 6402 presentation



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Industry Experience (interesting bits of history)

- BNOG West of Shetlands
- BP Scheihellion experiences
- Exxon Brazil
- Andaman Sea
- West Seno Risers and Tendons
- Auger Top Tensioned Sales Riser
- Allegheny SCR clashing
- Allegheny trenching
- Matterhorn (effects of cantilevered wellhead, strakes help installation, cold core re-fit)
- Typhoon SCR design and cold core issue
- Discovery of cold core/submerged current events
- Atlantis (and Thunder Horse) Sigsby Escarpment, high bottom currents, full straked riser – 7200' WD, 12 deg top, ~10,000' for 2 export, 6 production



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VIV Impact on Riser Design



- Fatigue life of SCRs in GOM is short , in many cases matter of months. - Suppression is required.
- TT risers again suffer from VIV - In some cases, depends on the severity of the effect the impact can be minimised by increasing tension.
- Most of the drilling risers have enough tension so the impact can be minimised, also are retrieved and can be inspected more regularly that production risers.
 - Analysis is required before deployment.
- VIV also impacts interference when risers are in arrays.
 - A number of tests have been performed by NDP(BP, Shell, EM, Statoil, NY, CT & TFE) to collect data.
 - Hydrodynamic coefficients in riser arrays are different from those in free stream.
- Workover risers have non cylindrical shape and existing data are not applicable for design.

Approach to Design



- Uncertainties still exist
- Need higher safety factors than the more well understood wave response.
- Parametric studies and robustness checks for riser response to single current events needs to be assessed.
- Use VIV suppression.



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Summary and Conclusions

- VIV is significant design issue in deep water
- Highly non-linear in excitation and in damage
- Instability of wake, not easy to predict
- Industry addressing problem through many studies and investigations
- We can confidently design for it using high safety factors
- As go to ultra deep, cost of high safety factors is prohibitive, additional progress needed.