

JOINT INDUSTRY PROJECT

FINAL REPORT

**DEVELOPMENT OF ATTENUATION EQUATIONS FOR
COMPUTING EARTHQUAKE GROUND MOTIONS AT STIFF SOIL SITES
WITHIN DEEP BASINS**

Submitted To:

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Development of Attenuation Equations for Computing Earthquake Ground Motions at Sites Within Deep Basins

1.0 Summary

Attenuation equations were derived for computing earthquake ground motions at stiff soil sites within deep basins. The equations are applicable to shallow crustal earthquakes with focal depths ranging from 0 to about 20 km, and moment magnitudes ranging from 5.0 to about 7.5. The ground-motion parameters predicted by the equations are peak ground acceleration (PGA) and 5%-damped pseudovelocity (PSV) response spectra at 14 oscillator periods between 0.04 and 4.0 sec; separate sets of equations were developed for the horizontal and vertical components of these parameters.

A total of 16 sets of attenuation equations were derived from nonlinear regression analyses of ground motions recorded at stiff-soil basin sites during (with one exception) California earthquakes. Each set of equations was derived for application to regions with a given amount of information known about the (1) type of earthquake mechanism (strike-slip, normal, reverse, or unknown), (2) type of site geology (stiff soil—site class B; moderately stiff soil—site class C; or, unknown, but not bedrock or soft soil), and (3) depth to basement rock beneath site (known or unknown).

The results clearly show distinct differences between the horizontal and vertical PSV response spectra predicted by the equations. For example, at short distances between the site and earthquake fault rupture, the short-period vertical PSV are similar to or greater than the horizontal PSV, whereas at longer periods greater than about 0.3 sec, the vertical PSV are less than the horizontal PSV by factors of about 2 or more.

Several other characteristics in the results were observed. First, ground motions on moderately stiff soil (site class C) are generally greater than those on stiff soil (site class B). Secondly, ground motions generated by reverse faults are greater than those generated by strike slip faults for periods of 1.0 sec or less. Finally, the results indicate that horizontal ground motions at all periods increase with increasing depth to basement rock beneath a site, whereas, the vertical motions appear to increase at short and long periods.

2.0 Introduction

This report presents the results of a joint industry project (JIP) to develop attenuation equations for computing earthquake ground motions at stiff soil sites within deep basins. The study was funded by private industry (Arco, Exxon, Texaco, Unocal, Shell, Mobil, and CNF) and federal government (Minerals Management Service), and it was motivated by the need for attenuation equations primarily oriented to offshore platform environments. During the course of the project, the Northridge, California, earthquake of moment magnitude 6.7 occurred on January 17, 1994. Because of the wealth of significant ground-

motion data generated by this event, a decision was made to delay the derivation of the attenuation equations until this database became available. Although other earthquakes have occurred since then (e.g., January 17, 1995, Kobe earthquake), the data-collection phase of the project was terminated after the Northridge earthquake data were obtained to avoid further delays in disseminating the results.

The remainder of this report is organized as follows. Section 3.0 discusses the ground-motion database used in the analysis; this database is listed in Appendix A. The approach to derive the attenuation equations is described in Section 4.0, and the results are presented in Section 5.0 and Appendix B. The application of the attenuation equations in practice is discussed in Section 6.0.

3.0 Ground-Motion Database

The database is comprised of strong-motion data from 271 accelerograms recorded during the 25 earthquakes listed in Table 1. The earthquakes span the moment magnitude range $5.2 \leq M_w \leq 7.4$; all are shallow-crustal events, and all but one occurred in California. Only strong-motion data recorded at deep-alluvial sites within basinal environments were admitted in the database. Over half of the data are from post-1981 earthquakes not included in earlier studies of strong-motion at deep-alluvial sites (Crouse and Piazza, 1983; Crouse et al., 1985). The database listing is provided in Appendix A.

Table 1. Earthquake Catalog

Date	Name	M_w	Fault Code	No. of Records	Date	Name	M_w	Fault Code	No. of Records
1933.03.11	Long Beach	6.4	S	2	1980.01.24	Livermore	5.8	S	6
1940.05.19	Imperial Valley	7.0	S	1	1981.04.26	Westmorland	5.9	S	3
1952.07.21	Kern County	7.4	R	4	1983.05.02	Coalinga	6.4	R	20
1966.06.28	Central California	6.1	S	6	1984.04.24	Morgan Hill	6.3	S	13
1966.08.07	Baja	6.3	S	1	1986.01.26	Hollister	5.4	S	1
1968.04.09	Borrego Mountain	6.6	S	7	1986.07.08	Palm Springs	6.1	S	7
1970.09.12	Lytle Creek	5.2	R	3	1987.10.01	Whittier	6.0	R	33
1971.02.09	San Fernando	6.6	R	21	1989.10.17	Loma Prieta	7.0	S	38
1974.11.28	Central California	5.2	S	3	1990.02.28	Upland	5.6	S	2
1978.08.13	Santa Barbara	5.8	R	2	1991.06.28	Sierra Madre	5.6	R	3
1978.09.12	Tabas, Iran	7.4	R	7	1992.06.28	Landers	7.3	S	22
1979.08.06	Coyote Lake	5.8	S	5	1994.01.17	Northridge	6.7	R	37
1979.10.15	Imperial Valley	6.5	S	24					

Note: Fault Code S \Rightarrow strike-slip fault event, R \Rightarrow reverse fault event

Local geotechnical and regional geologic characteristics for each of the recording station sites were included in the database. These data were derived from soil boring logs, shear-wave velocity profiles, geologic maps, other ground-motion studies, and the results of geophysical investigations. The site classification system of Boore et al. (1993) was used to characterize the local geotechnical environment of the station sites as either site class B (soft rock or stiff soil) or site class C (medium-stiff soil). According to Boore et al. (1993), class B sites have an average shear-wave velocity, \bar{v}_s , in the upper 30 m of 366 to 762 m/sec; class C sites range between 183 and 366 m/sec. Only data recorded at sites with average shear-wave velocities falling within the range encompassed by classes B and C were included in the database. Of the 271 records, 112 were recorded at sites identified as class B (class B records), and

132 were recorded at sites identified as class C (class C records). The proper classification of the remaining 27 records could not be established, although they were recorded at sites known to fall within the range spanned by the two classes (class B/C records).

Regional geologic sources were used to determine whether station sites were within a basinal environment, and, when available, the depth to basement rock (alluvial depth) at the site. Only data recorded at sites within basins were included in the database. Alluvial depths were determined at sites recording 146 of the 271 records in the database. Basin boundary information was inadequate to derive other basin and site/basin characteristics that might be correlated with ground motion, such as basin size and site-to-basin edge distance.

Figure 1 shows the distribution of the 271 records in magnitude–distance (M – R) space, where the characteristic magnitude, M , is moment magnitude, M_w , and the characteristic source-to-site distance, R , is the closest distance from the site to the fault rupture surface. Note that the majority of data are within the region, $6.0 \leq M \leq 7.5$ and $10 \leq R \leq 80$ km, and that the data are fairly evenly distributed between strike-slip and reverse events.

4.0 Database Analysis

The ground-motion parameters, Y , investigated were peak ground acceleration ($Y = \text{PGA}$) and pseudo-velocity ($Y = \text{PSV}$), the latter at fourteen periods T of an oscillator with 5%-of-critical damping: $T = 0.04, 0.10, 0.15, 0.20, 0.30, 0.40, 0.50, 0.60, 0.80, 1.00, 1.50, 2.00, 3.00,$ and 4.00 seconds. The 4.00 second upper period bound was determined by the long-period cutoff of the filters used to remove noise in the processing of the accelerograms in the database. Both horizontal and vertical components of the ground-motion parameters were investigated; for the horizontal component investigation, the ground-motion parameter Y of each record was defined as the geometric mean of the record's two horizontal component motions.

The independent variables used in the investigation characterized the earthquake (magnitude, M , defined as the moment magnitude, M_w , and fault-type, F , a binary variable used to distinguish between strike-slip and reverse events), the source-to-site distance (R , the closest distance from the site to the fault rupture surface), the local geotechnical environment (S , a binary variable used to distinguish between class B and class C sites), and the regional basin geology (D , the depth of alluvium at the site).

All of the ground-motion parameters were investigated by means of regression analysis. A number of regression models were investigated to determine appropriate functional relationships between the ground-motion parameter, Y , and the independent variables (M , R , S , F , and D) under a variety of ground-motion modeling scenarios that may be encountered in seismic hazard analysis. The following equation depicts the general form of these models.

$$\ln Y = p_1 + f_1(M) + f_2(R,M) + f_3(F,R) + f_4(S,R,M) + f_5(D) \quad (1)$$

where	p_1	\equiv	constant
	$f_1(M)$	\equiv	magnitude scaling function
	$f_2(R,M)$	\equiv	magnitude-dependent distance attenuation function
	$f_3(F,R)$	\equiv	distance-dependent fault-type scaling function
	$f_4(S,R,M)$	\equiv	distance- and magnitude-dependent site class scaling function
	$f_5(D)$	\equiv	alluvial depth scaling function

Different forms of the model resulted depending upon which, if any, of the fault-type, site class, or alluvial depth scaling functions were included in the model. Furthermore, the regressions of models which include one or more of these terms were necessarily on only those records for which the proper values of the associated variables (F , S , and/or D) were known. For models which were to include the dependence of ground-motion upon the local geotechnical environment, two approaches were adopted. In the first approach, a site-class scaling function $f_4(S,R,M)$ was included in the models, and the regressions were performed on those records in the database for which the class B/class C distinction was known. In the second approach, no site-class scaling function was used in the models, but the regressions were performed on class B records and on class C records separately, resulting in independent models for the two site classes.

As noted, the different models required analysis of different subsets of the database. Table 2 illustrates the eight different database subsets which were used; Figures 2 and 3 show the distribution of the data

Table 2. Database Subsets

Database Subset Number	Criteria for Inclusion in Database Subset						Number of Records in Subset	
	Site Classification			Alluvial Depth			H comp.	V comp.
	B	C	B/C	unknown	known			
1	(♦ or ♦)	♦ or ♦)	♦)	and (♦ or ♦)			271	269
2	(♦ or ♦)	♦ or ♦)	♦)	and (♦ or ♦)			146	146
3	(♦ or ♦)	♦)		and (♦ or ♦)			244	242
4	(♦ or ♦)	♦)		and (♦ or ♦)			139	139
5	♦			and (♦ or ♦)			112	111
6	♦			and (♦ or ♦)			50	50
7		♦		and (♦ or ♦)			132	131
8		♦		and (♦ or ♦)			89	89

within each subset in magnitude–distance (M – R) space. Within each database subset, each record was assigned a weight according to its location in M – R space. Referring to Figures 2 and 3, magnitude and distance intervals were defined, and the weighting scheme gave the recordings in each of the populated magnitude–distance bins the same total weight.

The regressions were performed by the BMDP Statistical Software program 3R (Dixon, 1992). The program provides as output the parameter values of the input ground-motion function that afford the best fit (in the least squares sense) between the function and the (weighted) data. It also calculates the standard

error of the regression, the asymptotic standard deviation of the parameter value estimates, and the asymptotic parameter correlation matrix.

5.0 Regression Results

The regression results (i.e., the resulting ground-motion equations) were analyzed to determine the goodness of fit between the equations and the data, to test the reasonableness of the ground-motion predicted by the equations (especially at close distances where data were less abundant), and to detect trends in the ground-motion data with respect to the independent variables. Based on these analyses, the following general regression model which was judged most appropriate.

$$\ln Y = p_1 + p_2 M + p_3 \ln (R + p_4 \exp \{p_5 M\}) + p_6 S + p_7 F + p_8 D \quad (2)$$

where

Y	\equiv	ground-motion parameter—either PGA or PSV
M	\equiv	moment magnitude, M_w
R	\equiv	closest distance from the site to the fault rupture surface in km
S	\equiv	site classification code: $S = 0$ for site class B, $S = 1$ for site class C
F	\equiv	fault-type code: $F = 0$ for strike-slip, $F = 1$ for reverse
D	\equiv	depth to basement rock (alluvial depth) in km
p_i	\equiv	regression coefficients, $i = 1$ to 8

The equation terms involving the coefficients p_2 through p_5 proved adequate to model the magnitude scaling and the magnitude-dependent distance attenuation that were observed in the data. Simple linear site class, fault-type, and alluvial depth scaling terms are used; more complex, multivariate functions were not found to be warranted by the data.

Because strong motion data were relatively scarce for small distances—large magnitudes, it was anticipated that it might be necessary to impose constraints on some of the coefficients to ensure that the ground-motion parameter Y predicted by the regression model was an increasing function of magnitude M for all distances R . Therefore, two additional coefficients were defined:

$$q_1 \equiv p_2 / p_3 \quad \text{and} \quad q_2 \equiv -(q_1 + p_5) \quad (3)$$

The addition of these dependent regression coefficients allowed the regression equation (2) to be recast in a form more amenable to the imposition of the constraints within the 3R program:

$$\ln Y = p_1 + p_3 \left\{ q_1 M + \ln (R + p_4 \exp \{p_5 M\}) \right\} + p_6 F \quad (4)$$

This equivalent expression was used in the regression analyses, along with the following coefficient constraints.

$$q_1 \leq 0, \quad p_3 \leq 0, \quad p_4 \geq 0, \quad \text{and} \quad q_2 \geq 0 \quad (5)$$

These constraints enforce the physical expectation that Y is an increasing function of M and a decreasing function of R for all positive M and R . Coefficient definitions (3) were used to convert raw regression results to the form of the general regression model, equation (2). All regression results herein are reported in terms of equation (2), unless otherwise indicated.

In application, a number of ground-motion modeling scenarios arise because it is not always possible to assign appropriate values to the site class, fault-type, and alluvial depth variables (S , F , and D). To provide ground-motion equations appropriate for these different modeling scenarios, a total of 16 variations of equation (2) were developed. Table 3 indicates the database subset which was utilized to develop each equation, the modeling scenarios for which it is applicable, and which (if any) of the terms of the general regression model are not present.

Table 3. Ground-Motion Equation Sets

Regression Equation Set No.	Database Subset No.	Applicable Ground-Motion Modeling Scenario								Term Present, Eqn. (2)		
		Site Class (within B-C range)			Fault-Type		Alluvial Depth			$p_6 S$	$p_7 F$	$p_8 D$
		Unknown	Class B	Class C	Unknown	Known	Unknown	Known	n	y	n	y
1	1	◆			and ◆		and ◆			◆		◆
2	2	◆			and ◆		and ◆		◆	◆		◆
3	1	◆			and ◆	◆	and ◆			◆	◆	◆
4	2	◆			and ◆	◆	and ◆		◆	◆		◆
5	3		(◆ or ◆)	◆	and ◆		and ◆			◆	◆	◆
6	4		(◆ or ◆)	◆	and ◆		and ◆		◆	◆		◆
7	3		(◆ or ◆)	◆	and ◆	◆	and ◆			◆	◆	◆
8	4		(◆ or ◆)	◆	and ◆	◆	and ◆		◆	◆		◆
9	5		◆		and ◆		and ◆			◆	◆	◆
10	6		◆		and ◆		and ◆		◆	◆		◆
11	5		◆		and ◆	◆	and ◆			◆	◆	◆
12	6		◆		and ◆	◆	and ◆		◆	◆		◆
13	7			◆	and ◆		and ◆			◆	◆	◆
14	8			◆	and ◆	◆	and ◆		◆	◆		◆
15	7			◆	and ◆	◆	and ◆			◆	◆	◆
16	8			◆	and ◆	◆	and ◆		◆	◆		◆

Note: Database Subset No. refers to designation assigned in Table 2

Each equation set provides horizontal- and vertical-component ground-motion equations for PGA and for PSV at fourteen periods T in the band $0.04 \leq T \leq 4.00$ sec. The values of the parameters p_i in the general equation (2) to be used with each equation set are provided in Appendix B, along with the standard errors of the regressions.

To test the validity of the ground-motion equations' representation of the data, plots of the normalized regression residuals were prepared. The normalized residual of the i^{th} datum of a regression, ϕ_i , is defined as:

$$\phi_i \equiv \frac{(\ln Y_i^{\text{observed}} - \ln Y_i^{\text{predicted}})}{\sigma_{\ln Y}} \quad (6)$$

where Y_i^{observed} is the i^{th} observed value of the ground-motion parameter Y , $Y_i^{\text{predicted}}$ is the corresponding value predicted by the regression equation, and $\sigma_{\ln Y}$ is the standard error of the regression. Figures 4

through 7 provide normalized residual plots for horizontal PGA and horizontal PSV at period $T = 1.00$ second, for ground-motion equation sets 1 and 12. The top frame of each plot shows the residuals plotted versus magnitude, M , and the bottom frame shows the residuals plotted versus distance, R . Ground-motion equation set 1 resulted from regression analyses of database subset number 1, which contains 271 horizontal-component records (see Table 2); ground-motion equation set 12 resulted from regression analyses of database number 6, which contains 50 horizontal-component records. The residuals from both equation sets are uniformly distributed about zero with respect to both magnitude and distance (i.e., the averages of the residuals are close to zero for small and large magnitudes and for short and long distances), which indicates that the equations model the data reasonably well. Note also that the dispersion of the residuals about the mean does not appear to vary between small and large magnitudes; this indicates that the standard error of the regression is not magnitude-dependent, as some researchers have found (Geomatrix, 1992; Idriss, 1993). Similar plots were generated for other periods and different equations, yielding similar results.

A number of plots of horizontal ground-motion attenuation and of response spectra were generated to test the reasonableness and consistency of the ground-motions predicted by the various equation sets. Figure 8 compares median horizontal ground-motion (PGA and PSV at period $T = 1.0$ sec) predicted by equation sets 3 and 8. Equation set 3 was developed using database subset 1, which contains 271 horizontal-component records (see Table 2); as shown in Table 3, the equation does not include a site-class or alluvial depth term, but does include a fault-type term. Equation set 8 was developed using database subset 4, which contains 139 horizontal-component records, approximately half the size of database subset 1; it includes site-class, fault-type, and alluvial depth terms. To compare the predicted ground-motions for similar conditions, the fault-type parameter value $F = 0$ (strike-slip), was used in both equation sets. For equation set 8, the site-class parameter value $S = 0.5$ was used. This is appropriate for comparing equation set 8 to equation set 3, since the latter neglects site class distinction and was derived from a database subset which is approximately evenly split between site classes B and C. For similar reasons, the alluvial depth parameter value $D = 3.0$ km was used in equation set 8, as this was approximately the median depth observed in database subset 4.

The plots in Figure 8 show close agreement between the two equation sets at magnitudes $M = 6.5$ and 7.5 over the distance range spanned by the data. Similar plots in Figures 9 and 10 demonstrate this consistency across database subsets and equation sets for site class B ground-motions (equation sets 8 and 12, using database subsets 4 and 6, respectively) and site class C ground-motions (equation sets 8 and 16, using database subsets 4 and 8, respectively). The consistency demonstrated in Figures 8, 9, and 10 is important because it demonstrates that the results of seismic hazard analyses using the present study are not especially sensitive to the choice of one equation set over another, provided the analysis scenario is consistent with the equation set modelling assumptions. It also indicates that ground-motion trends which are observed in the larger database subsets are present in the smaller subsets as well, permitting greater assurance that the size of the smaller subsets is adequate to develop valid inferences to trends not obtainable from the larger subsets (such as alluvial depth-dependency).

Several horizontal ground-motion trends are observed in Figures 8, 9, and 10. (1) The ground-motion attenuates more slowly for longer periods. However, the most rapid attenuation was observed at periods

between $T = 0.10$ and 0.20 sec, and was slightly slower for shorter periods and for PGA. This is consistent with the findings of Boore et al. (1995), who show the greatest attenuation rate at periods $T = 0.12$ and 0.13 sec. (2) The magnitude scaling increases with increasing period, which is consistent with Boore et al. (1995) and Campbell (1990). (3) The attenuation of ground-motion with distance is slower with increasing magnitude. This trend is contrary to Boore et al. (1994), who found no statistically significant difference in magnitude scaling between data recorded within 10 km of the surface projection of fault rupture and magnitude scaling in the database as a whole. The reduction in attenuation rate with increasing magnitude is more pronounced for PGA and shorter periods, which is in general agreement with Campbell (1990), who predicts complete "saturation" at zero distance of PGA and PSV at periods $T \leq 0.30$ sec. However, only a few of the equation sets in the present study predict zero-distance ground-motion saturation, and then only at periods $T = 0.10$ and 0.15 sec. Saturation is predicted by six of the 16 equation sets for ground-motion at period $T = 0.10$ sec; saturation at $T = 0.15$ sec is predicted by only one the 16 equation sets.

Figures 11 through 14 explore the difference between site class B and class C response spectra, and compare the site class-dependency predicted by different equation sets. The top frame of each figure plots the median site class B and class C horizontal pseudo-spectral acceleration, PSA (by definition, $PSA \equiv PSV \cdot 2\pi/T$) predicted by equation set 8, which assumes a constant ratio of site class B to class C ground-motion at each period. The bottom frame of each figure plots site class B and class C PSA predicted by equation sets 12 and 16, respectively. Equation sets 12 and 16, by contrast, assume that class B and C ground-motions are independent of each other. Together, the four figures plot the equations at magnitudes $M = 6.5$ and 7.5 , and distances $R = 1.0$ and 10.0 km.

It may be observed in Figures 11 through 14 that site class C ground-motions are generally greater than site class B ground-motions. This is consistent with the findings of Boore et al. (1995). The only notable exception appears in the bottom frame of Figure 12; for magnitude $M = 7.5$ and distance $R = 1.0$ km, the site class B PSA at period $T = 0.80$ sec is approximately 40 percent greater than the class C PSA. This result is probably an aberration caused by the extreme scarcity of large-magnitude, short-distance data in the database subsets used to produce equation sets 12 and 16. Comparing this plot with the equation set 8 plot in the top frame, and with the spectral shapes at the other magnitudes and distances, it is seen that at this particular magnitude-distance combination, (a) the class C PSA shows a peak at $T = 0.60$ sec and a dip at $T = 0.80$ sec, relative to other class C PSA, while (b) the class B PSA shows a dip at $T = 0.80$ sec, relative to other class B PSA. As the magnitude-distance combination moves in M - R space towards regions with more data, the PSA plots of Figures 11 through 14 become progressively smoother. Additionally, the consistency improves between equation sets 8 and 12 class B PSA, and between equation sets 8 and 16 class C PSA.

Figures 15 and 16 compare the median horizontal and vertical PSV predicted by equation set 3. It may be noted that the shapes of the horizontal and vertical PSV are significantly different, with the vertical PSV plots showing a shift in frequency content towards shorter periods, relative to the horizontal PSV. Specifically, the greatest vertical spectral accelerations occur at shorter periods ($T \leq 0.20$ sec) than the horizontal spectral acceleration peaks. For magnitudes greater than $M = 6.5$ and distances less than 10 to 30 km, the vertical spectral response at these shorter periods can be significantly greater than the

corresponding horizontal response, this trend increasing with increasing magnitude and decreasing distance. At periods greater than about 0.10 to 0.20 sec however, the vertical response is approximately constant in spectral velocity and, in general, is significantly less than the horizontal response. Below magnitude $M = 6.0$, or beyond distance $R = 30$ km, the vertical spectral response is less than the horizontal response at all periods studied.

Figures 17 through 19 show ground-motion attenuation, and compare the median horizontal ground-motion predicted by the present study with those predicted by other researchers. Figures 17 and 18 compare equation set 7 and Boore et al. (1995) ground-motions for site class B and class C conditions, respectively. Figure 19 compares equation set 4 and Campbell (1990) ground-motions. The forms of the Boore et al. and Campbell equations are as follows.

$$\text{Boore et al. (1995): } \log Y = b_{SS}G_{SS} + b_{RS}G_{RS} + b_2(M-6) + b_3(M-6)^2 + b_4R + b_5 \log R + b_6G_B + b_7G_C \quad (7)$$

where Y \equiv ground-motion parameter—either PGA (in g) or PSV (in cm/sec)
 G_{SS}, G_{RS} \equiv fault-type codes: $G_{SS} = 1$ for strike-slip, $G_{SS} = 0$ otherwise; $G_{RS} = 1$ for reverse, $G_{RS} = 0$ otherwise
 M \equiv moment magnitude, M_w
 R \equiv characteristic distance in km, $R \equiv (d^2 + h^2)^{1/2}$
 d \equiv closest horizontal distance, in km, from the site to the vertical projection of the fault rupture onto the earth's surface
 G_B, G_C \equiv site classification codes: $G_B = 1$ for site class B, $G_B = 0$ otherwise; $G_C = 1$ for site class C, $G_C = 0$ otherwise
 b_i, h, b_{SS}, b_{RS} \equiv regression coefficients, $i = 2$ to 7

$$\text{Campbell (1990): } \ln Y = c_1 + c_2M + c_3 \ln(R + c_4 \exp\{c_5M\}) + c_6F + c_7 \tanh(c_8\{c_9 + M\}) + c_{10} \tanh(c_{11}D) + \sum q_j K_j \quad (8)$$

where Y \equiv ground-motion parameter—either PGA (in g) or PSV (in cm/sec)
 M \equiv magnitude: $M \equiv$ Richter local magnitude, M_L , for magnitudes < 6.0 ; $M \equiv$ surface-wave magnitude, M_S , for magnitudes ≥ 6.0
 R \equiv closest distance from the site to the seismogenic rupture in km
 F \equiv fault-type code: $F = 0$ for strike-slip, $F = 1$ for reverse
 D \equiv depth to basement rock (alluvial depth) in km
 K_j \equiv structure codes characterizing structure which houses the recording instrument, $j = 1$ to 3 ; $K_j = 0$ for typical instrument shelter
 c_i, q_j \equiv regression coefficients, $i = 1$ to 11 , $j = 1$ to 3

Equation sets 7 and 4 were selected for the comparisons since their assumed ground-motion functional dependence on the variables S , F , and D most closely resembled that of the Boore et al. and Campbell models, respectively. Both Boore et al. and equation (2) define the characteristic magnitude, M , as the moment magnitude, M_w . Characteristic magnitudes assigned by Campbell's definition are approximately the same as moment magnitude below a moment magnitude of about 8.0 (Joyner and Boore, 1988), so the characteristic magnitude definitions of the three models may be assumed to be essentially identical.

However, the definitions of the characteristic distance, R , are somewhat different between the three models, that of Boore et al. being the least like Campbell's or the present study. For the purposes of comparison, the approximate equivalence of the distance variable, R , of equation (2), and the variables d and R of equations (7) and (8), respectively, is assumed to be adequate.

Figures 17 and 18 show reasonably close agreement between the ground-motions predicted by equation set 7 and by Boore et al. (1995). Notable differences include (1) the slower attenuation of equation set 7 for shorter distances (especially for PSV at period $T = 1.00$ sec), and the faster attenuation for larger distances (especially for PGA), the transition occurring at approximately 40 to 50 km distance; and (2) the lesser magnitude scaling of equation set 7 at shorter distances and the greater magnitude scaling at larger distances, the transition occurring between approximately 10 and 30 km distance. The agreement between equation set 7 and Boore et al. is greater for magnitude $M = 6.5$ than $M = 7.5$, and for PGA than PSV ($T = 1.00$ sec). Magnitude $M = 7.5$ ground-motion at distances less than 10 km predicted by equation set 7 is less than that of Boore et al., most significantly for site class C PSV ($T = 1.00$ sec), where the difference at 1.0 km is approximately a factor of 2.5.

Figure 19 shows reasonably close agreement between the ground-motions predicted by equation set 4 and by Campbell (1990) for distances $R \leq 50$ km, the maximum record distance admitted in Campbell's database. The attenuation of equation set 4 is slower than that of Campbell, especially for PSV ($T = 1.00$ sec). The difference in magnitude scaling between short and large distances is greater for Campbell, especially of PGA, for which Campbell predicts magnitude saturation at zero distance. The slower attenuation of equation set 4 yields lesser ground-motions than Campbell for distances less than 10 km (especially for magnitude $M = 6.5$), and greater ground-motions for distances larger than 20 km (especially for magnitude $M = 7.5$). Figures 17, 18 and 19 show that equation sets 7 and 4, which display close agreement with each other, consistently predict lesser ground-motion at magnitude $M = 7.5$ than either Boore et al. or Campbell at distances less than 10 km; however, the severity of the difference varies significantly. Apart from this, differences in PGA and PSV ($T = 1.00$ sec) between the present study and the two other researchers do not appear to be consistent across magnitude or distance.

Figures 20 through 23 compare response spectra predicted by equation set 7 with that of Boore et al. for site classes B and C, at magnitudes $M = 6.5$ and 7.5 , and distances $R = 1.0$ and 10.0 km. Close agreement is observed between the two ground-motion models except for magnitude $M = 7.5$ at distance $R = 1.0$ km. As shown in the bottom frames of Figures 20 and 22, the equation set 7 PSA is significantly less than that of Boore et al. for periods greater than $T = 0.30$ sec. This is consistent with the observations made earlier in comparing the attenuation of PSV ($T = 1.00$ sec) of the two models; refer to the bottom frames of Figures 17 and 18.

Figures 24 and 25 compare response spectra predicted by equation set 4 with that of Campbell (1990) at the same magnitude and distance combinations used in Figures 20 through 23. The close agreement between the PGA and PSV ($T = 1.00$ sec) predictions of the two models at distance $R = 10$ km noted earlier in conjunction with Figure 19 is readily apparent in Figure 25 across the entire period range. At magnitude $M = 6.5$, the slower attenuation of all periods of equation set 4 is observed, producing a PSA spectra at distance $R = 1.0$ km uniformly less than Campbell (1990). The ratio of Campbell PSA to

equation set 4 PSA in the top frame of Figure 24 varies between approximately 1.5 and 2.5. However, the agreement in the bottom frame (magnitude $M = 7.5$) is greater; PSA predicted by equation set 4 is still generally less (except for periods between $T = 0.50$ and 0.80 sec) than that predicted by Campbell, but the difference is smaller, and is negligible for periods greater than $T = 2.50$ sec.

6.0 Application to Practice

The attenuation equations in this report were derived from predominantly California strong motion data. Therefore, the question arises whether these equations are applicable in other regions. In the absence of information to the contrary, the equations are recommended for shallow crustal earthquakes in plate-boundary regions, including those where plate subduction is occurring. However, the equations are not recommended for computing ground motions from interplate or intraplate subduction earthquakes; the ground motions from these events are different from those generated by shallow crustal earthquakes. Attenuation equations are available for computing ground motions from subduction earthquakes (e.g., Crouse, 1991a,b; Youngs et al., 1988).

In relatively stable continental interiors, such as eastern North America, the equations in this report also may not be appropriate. Attenuation in the eastern North America is different than in the western United States based on analysis of intensity and ground motion data recorded in both regions.

Assuming the equations in this report are judged to be applicable in a given region, the user must select one or more of the 16 sets of equations that are provided in Appendix B. Tables 2 and 3 were developed to assist in the selection. For example, if no information is known other than the fact the site is comprised of moderately stiff to stiff soil, then equation set 1 would be appropriate. On the other hand, if the site class, fault type, and alluvial depth are known, then one or two equation sets could be selected. For example, if the site class is B, then either equation set 8 or equation set 12 is appropriate; if the site class is C, then equation set 8 or equation set 16 could be used. In fact, when the site class is known, there are two equations that can be selected: one that contains a site term and one that was derived from data recorded at stations in the same site class.

Situations may arise where the fault-type is known to be normal (e.g., in the Basin and Range province of the western United States). The database used to derive the equations does not contain any records from normal-fault earthquakes (see Table 1). For this case, the user could select the set of equations for unknown fault-type, or known fault-type with $F = 0$ or $F = 0.5$. The use of $F = 1$ (reverse faults) is considered too conservative.

Other situations may arise where the locations of a comparable number of reverse and strike-slip faults are known, but the information on their seismic activity is not sufficient to model them as individual sources. In this case, the user may elect to model the tectonic province in which the faults are located as one seismic source and select the set of equations based on the assumption that the fault-type is unknown.

7.0 References

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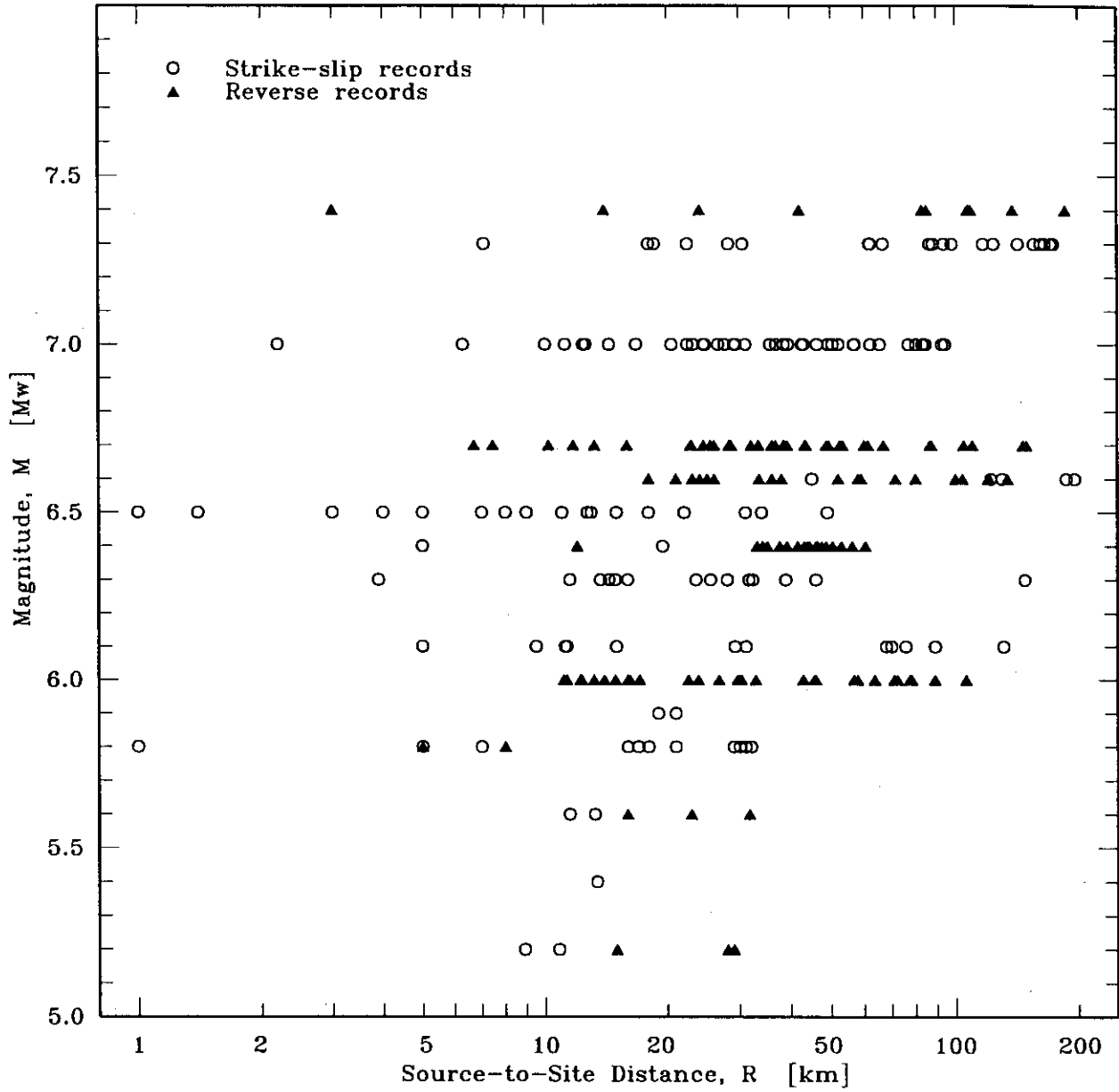


Figure 1. Distribution of database records in Magnitude-Distance ($M-R$) space.

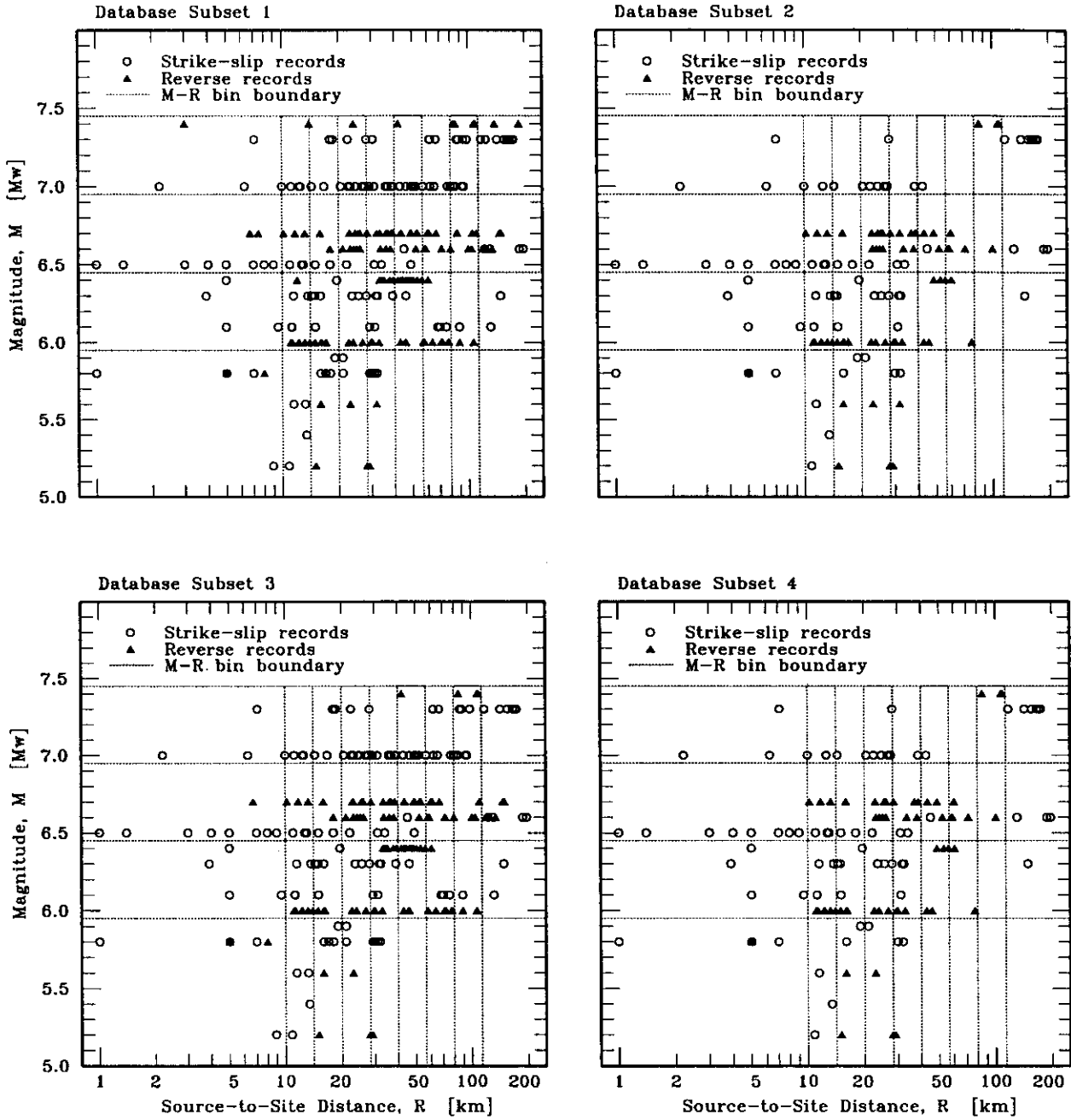


Figure 2. Magnitude–distance distribution of records in database subsets 1 through 4.

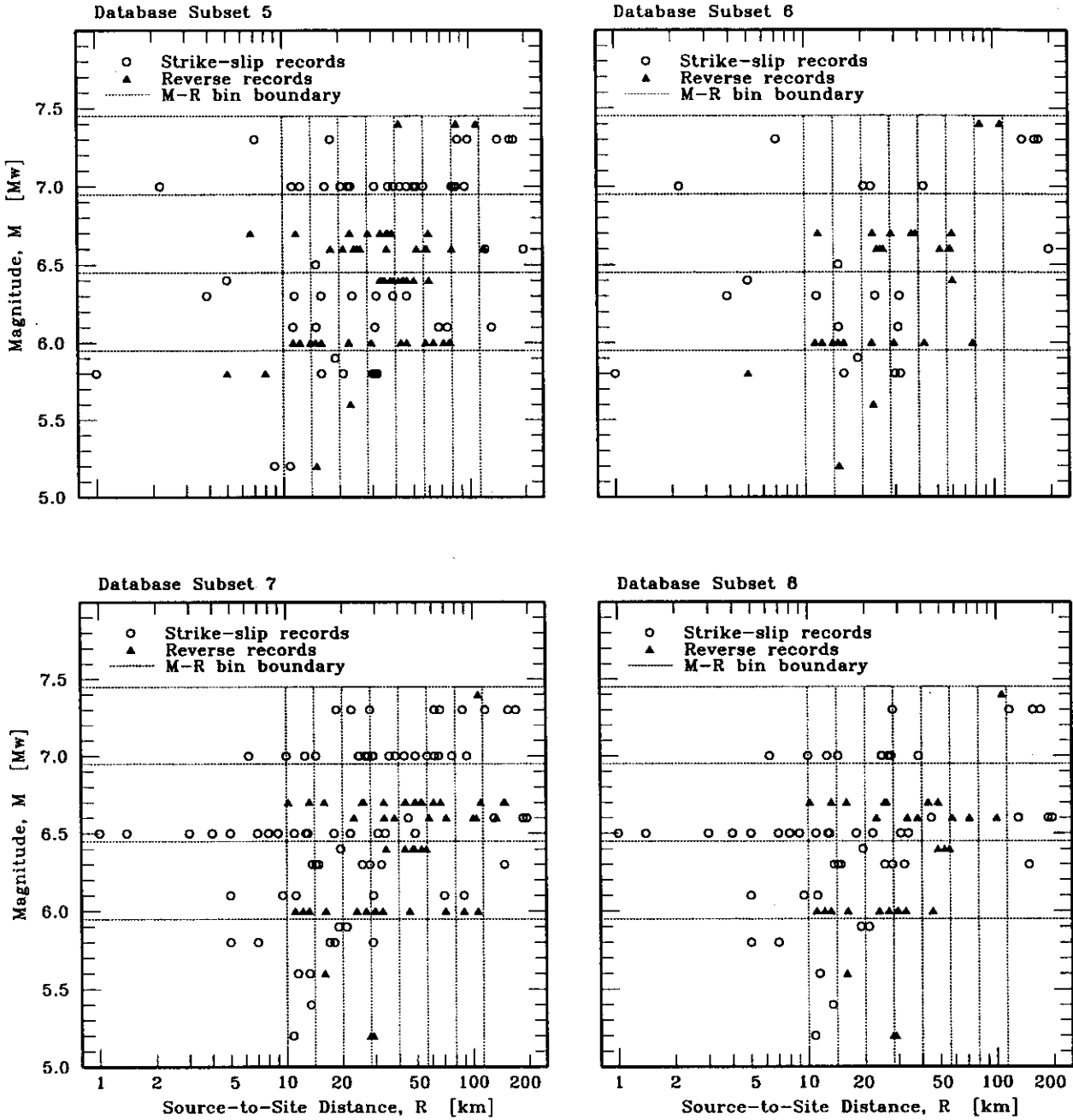


Figure 3. Magnitude–distance distribution of records in database subsets 5 through 8.

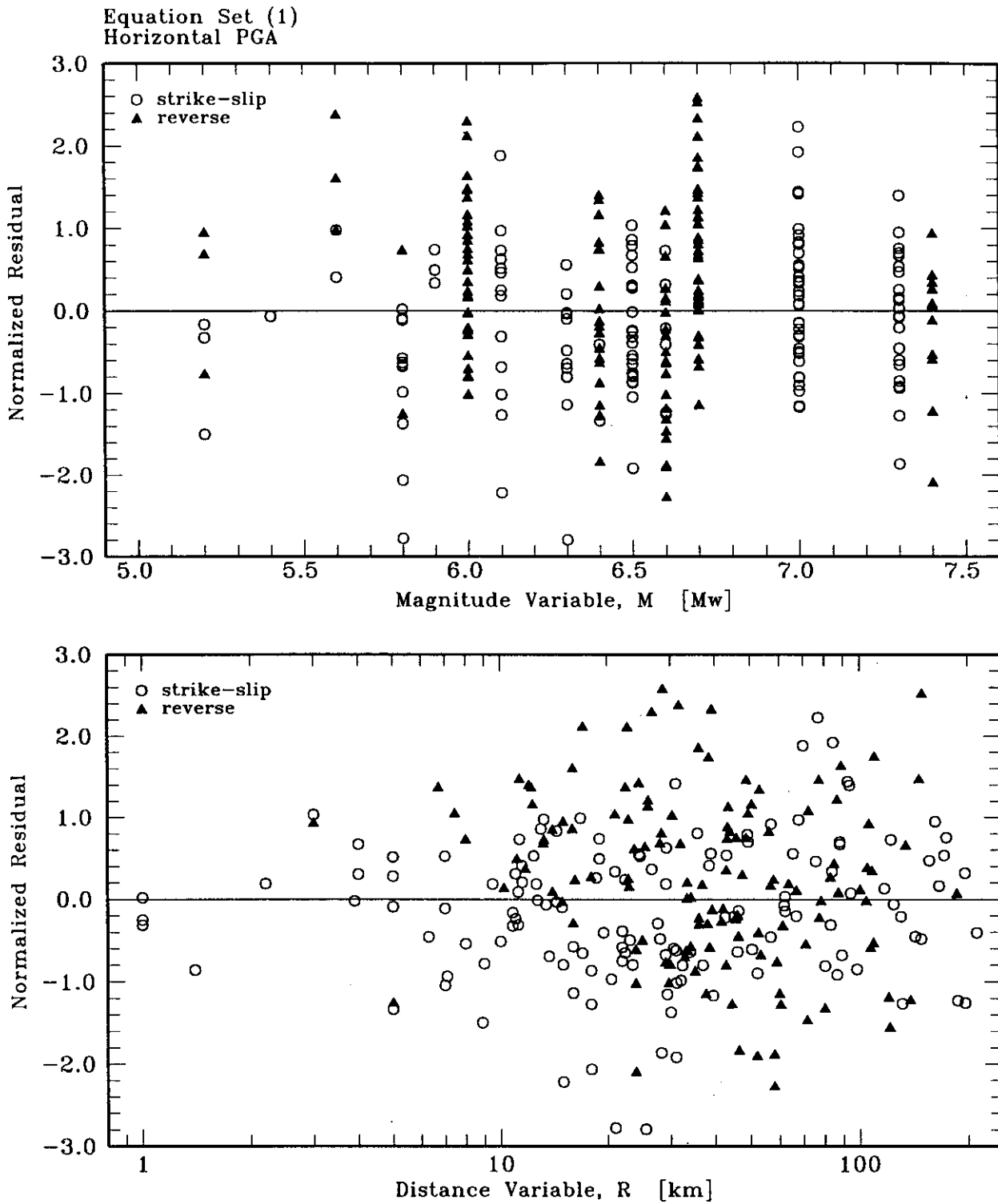


Figure 4. Plots of equation set 1 horizontal PGA residuals versus magnitude and distance.

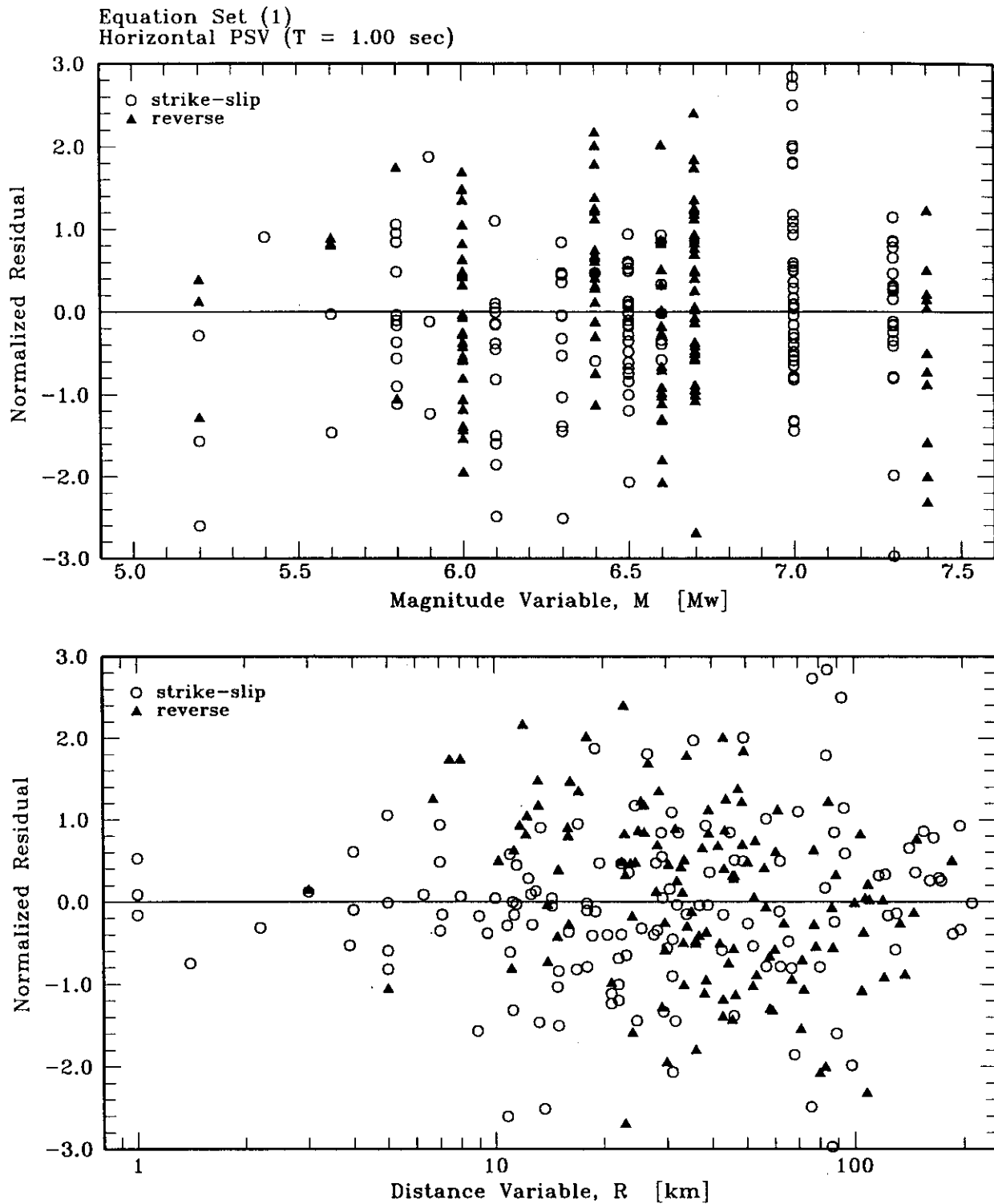


Figure 5. Plots of equation set 1 horizontal PSV (period $T = 1.0$ sec) residuals versus magnitude and distance.

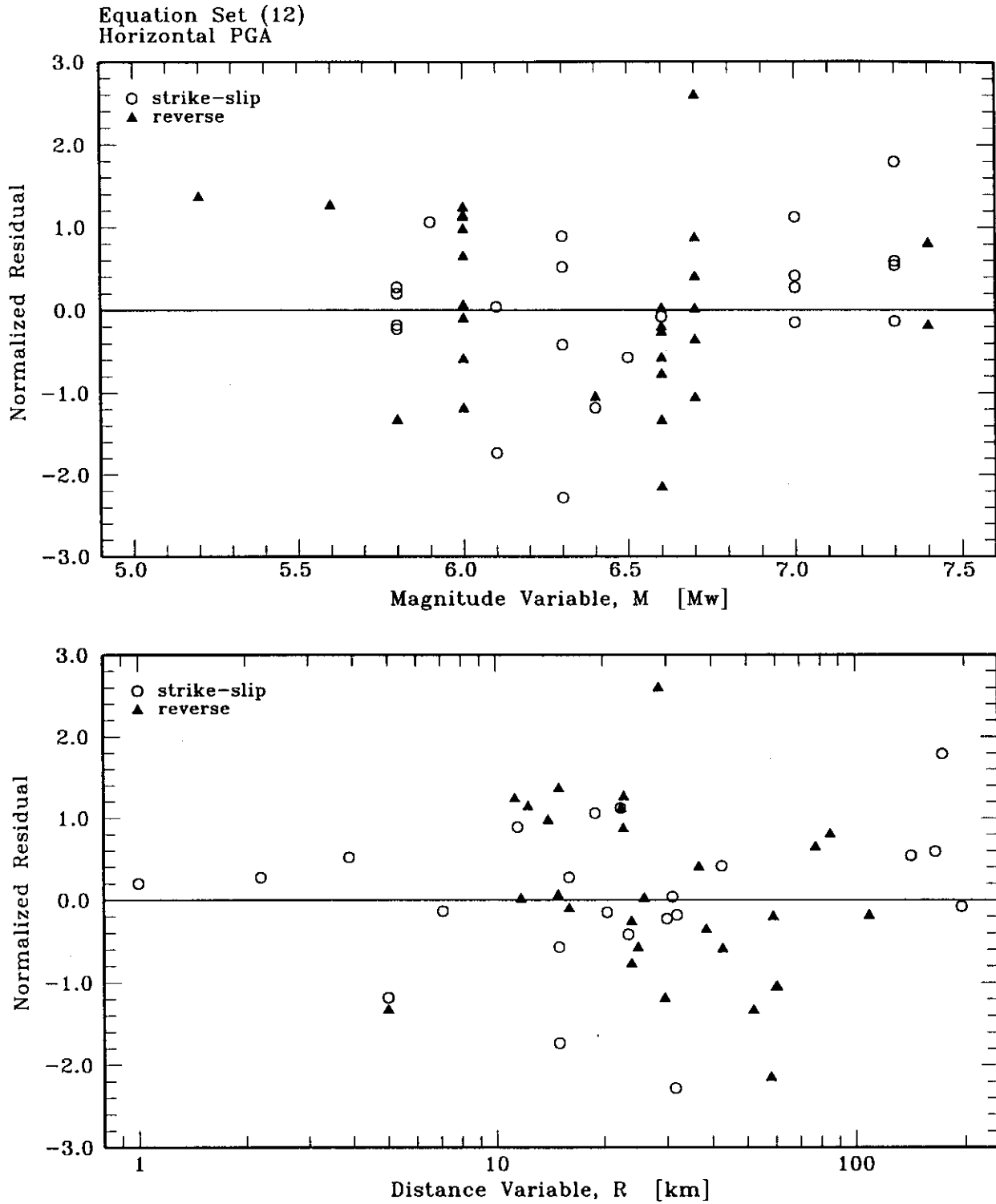


Figure 6. Plots of equation set 12 horizontal PGA residuals versus magnitude and distance.

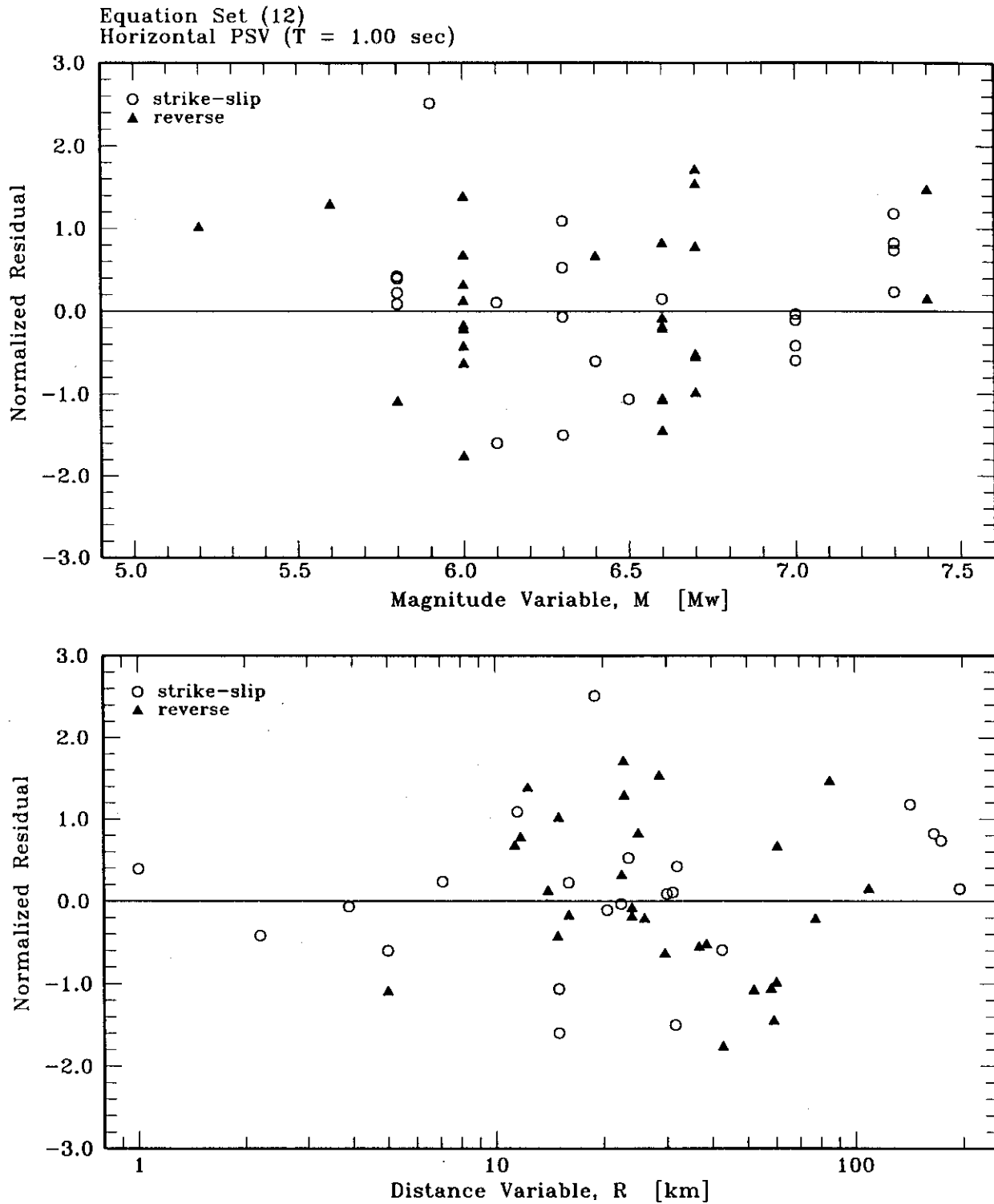


Figure 7. Plots of equation set 12 horizontal PSV (period $T = 1.0$ sec) residuals versus magnitude and distance.

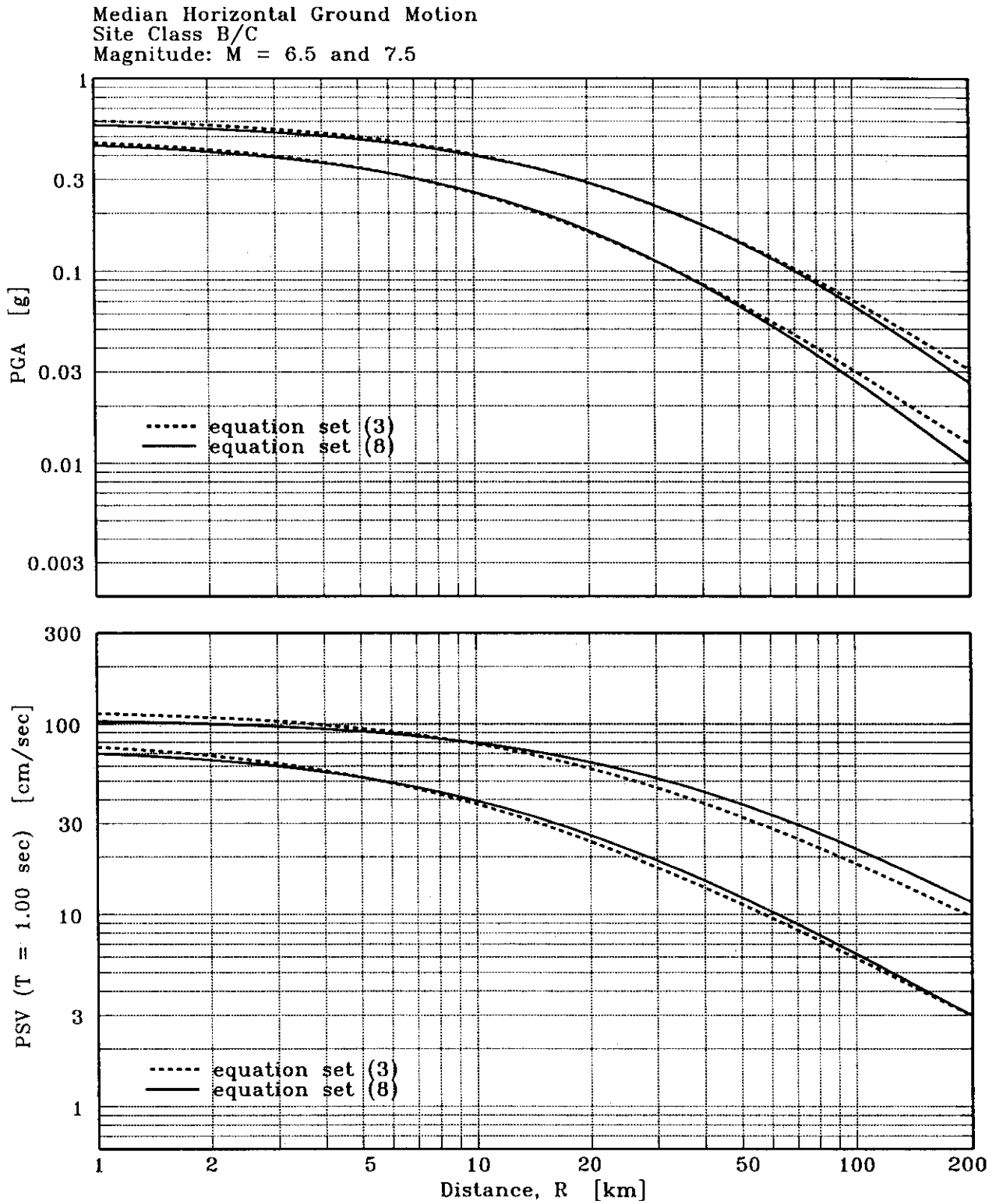


Figure 8. Median horizontal PGA and PSV (period $T = 1.0$ sec) attenuation plots. Plots compare ground-motions at magnitudes $M = 6.5$ and 7.5 predicted by equation sets 3 and 8 for site class B/C, strike-slip conditions. For equation 8, $S = 0.5$ (class B/C) and $D = 3.0$ km (database median depth).

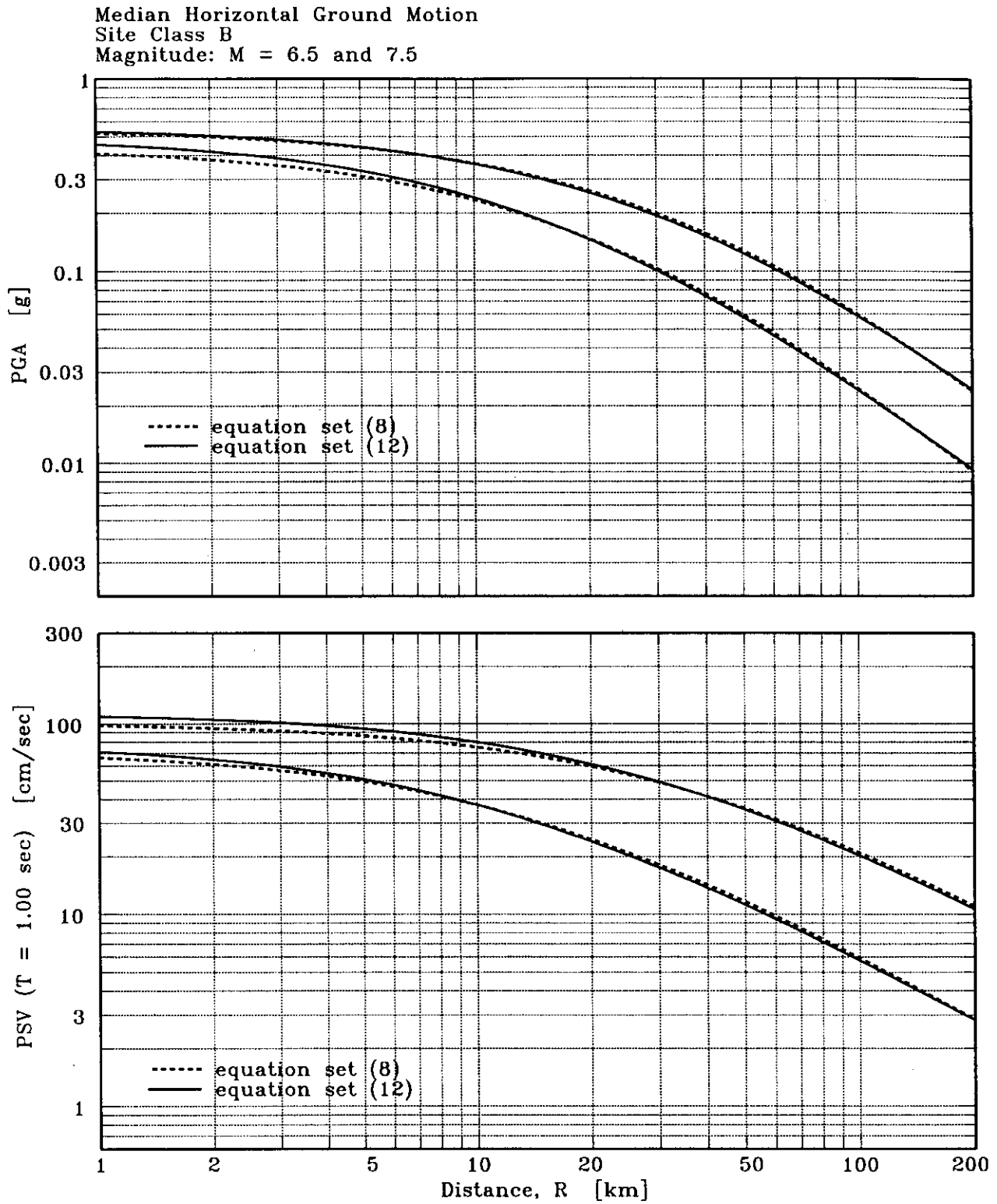


Figure 9. Median horizontal PGA and PSV (period $T = 1.0$ sec) attenuation plots. Plots compare ground-motions at magnitudes $M = 6.5$ and 7.5 predicted by equation sets 8 and 12 for site class B, strike-slip conditions. For both equations, $D = 3.0$ km (database median depth).

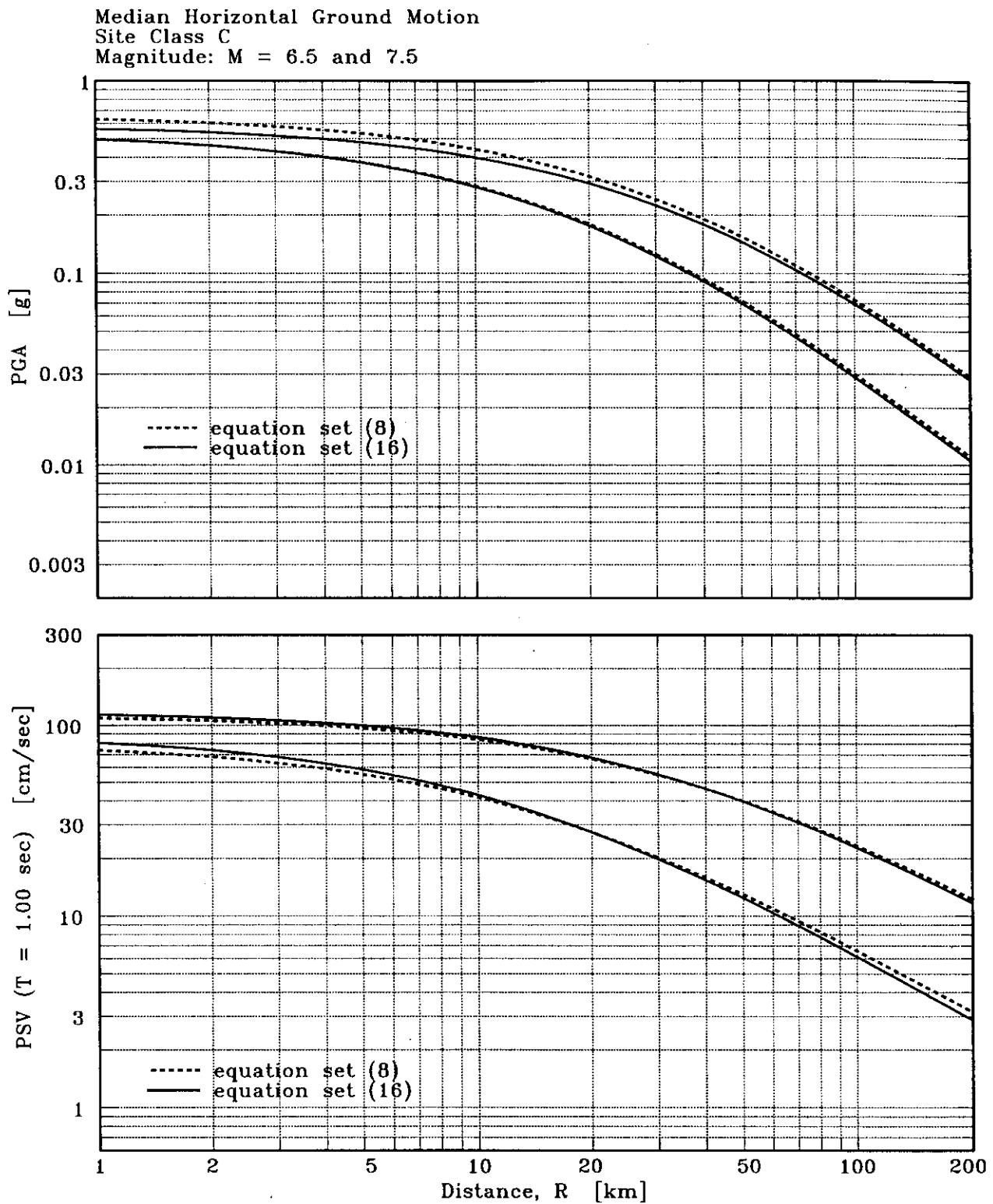


Figure 10. Median horizontal PGA and PSV (period $T = 1.0$ sec) attenuation plots. Plots compare ground-motions at magnitudes $M = 6.5$ and 7.5 predicted by equation sets 8 and 12 for site class C, strike-slip conditions. For both equations, $D = 3.0$ km (database median depth).

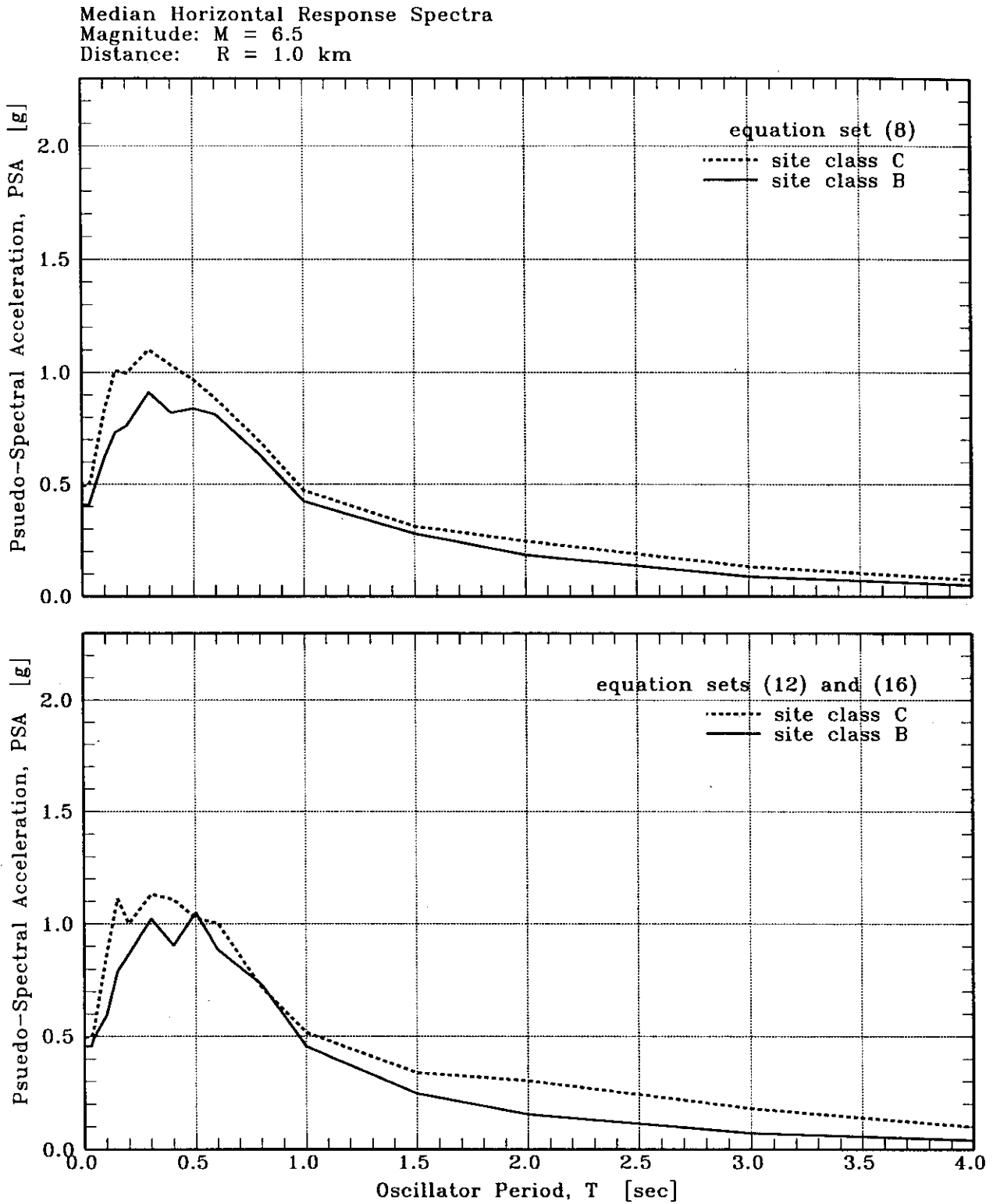


Figure 11. Median horizontal response spectra plots. Plots compare site class B and C response spectra at magnitude $M = 6.5$ and distance $R = 1.0$ km for strike-slip events. The top frame plots equation 8; the bottom frame plots equations 12 and 16. Depth $D = 3.0$ km is used for all equations.

Median Horizontal Response Spectra
 Magnitude: $M = 7.5$
 Distance: $R = 1.0$ km

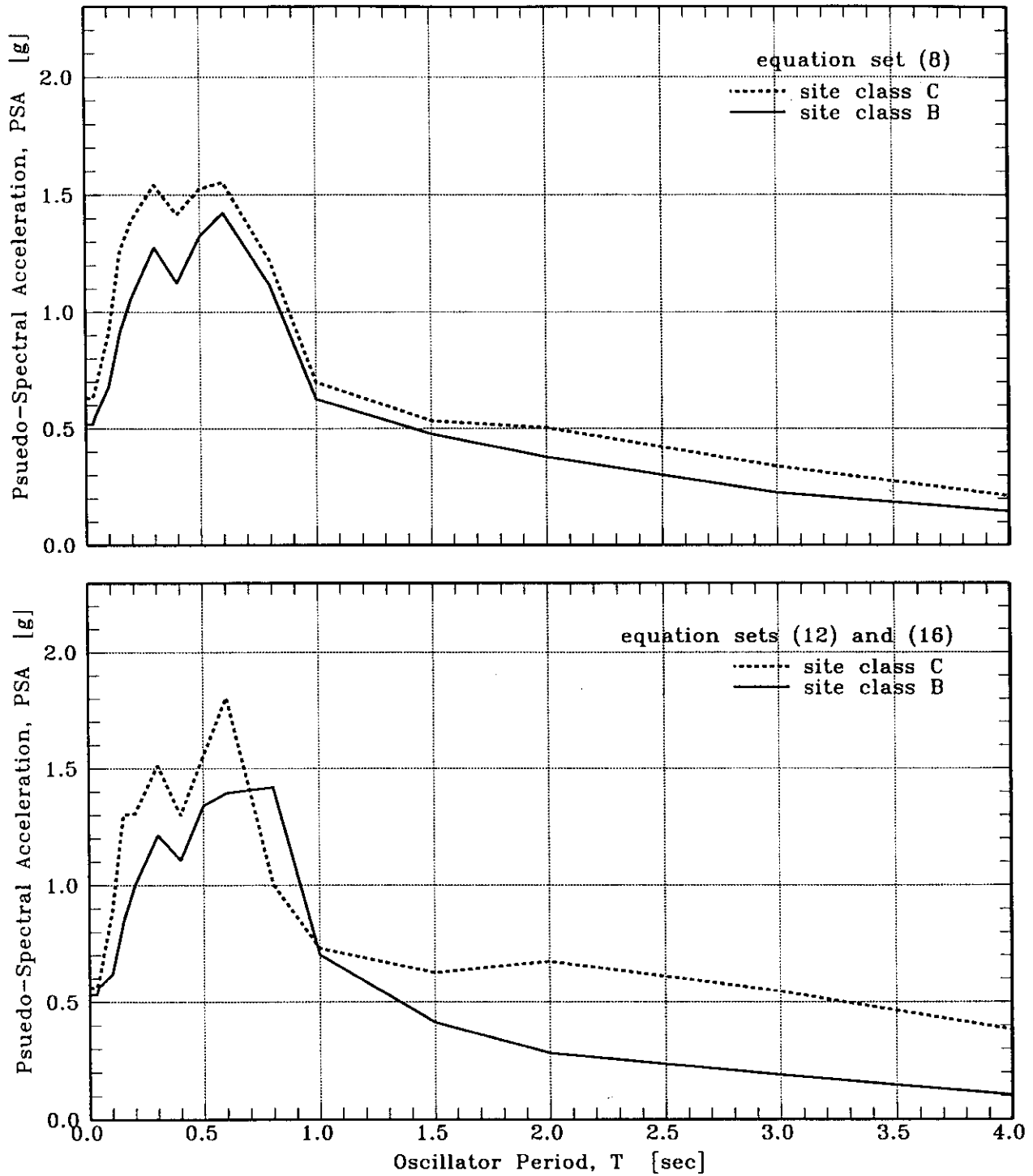


Figure 12. Median horizontal response spectra plots. Plots compare site class B and C response spectra at magnitude $M = 7.5$ and distance $R = 1.0$ km for strike-slip events. The top frame plots equation 8; the bottom frame plots equations 12 and 16. Depth $D = 3.0$ km is used for all equations.

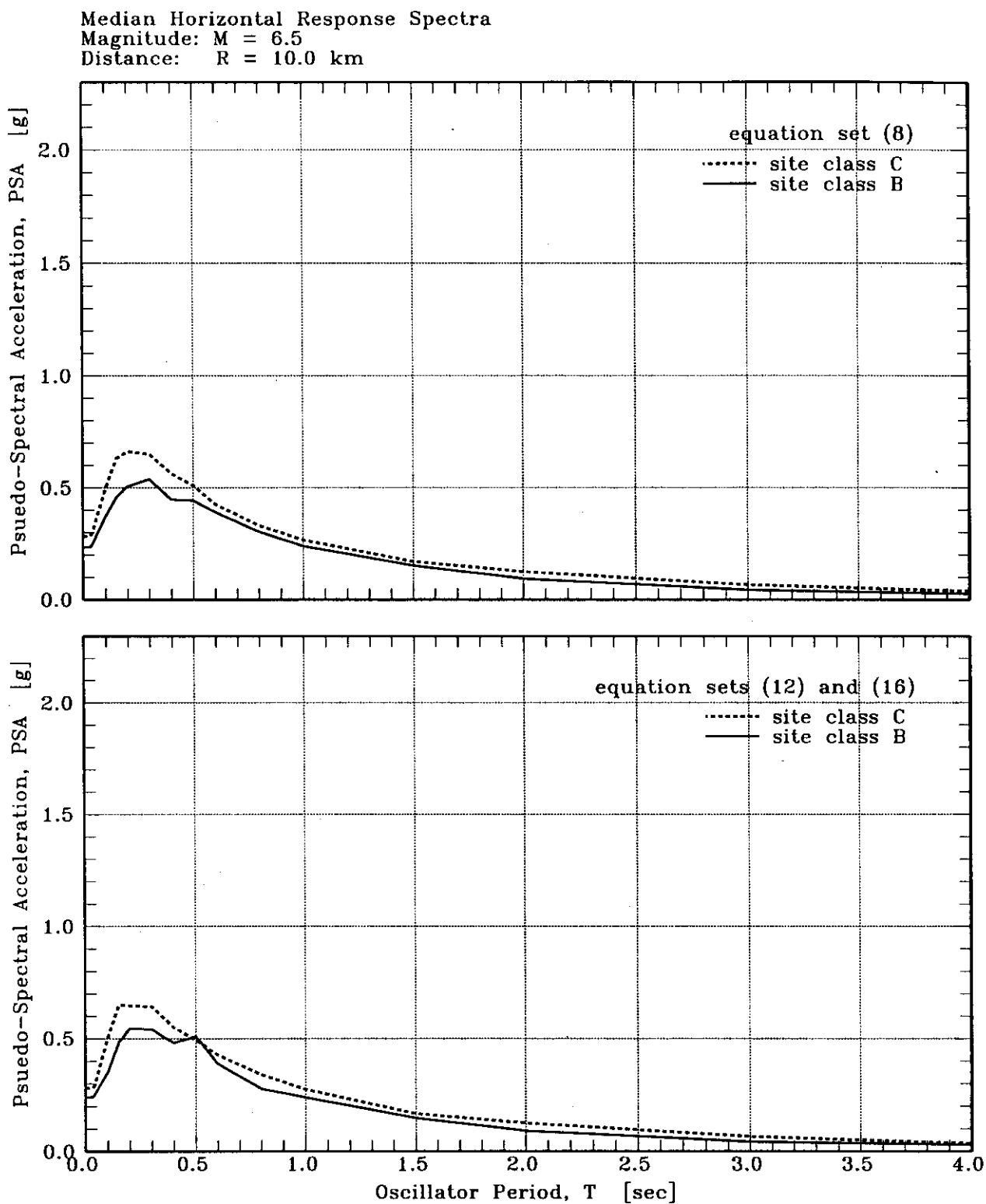


Figure 13. Median horizontal response spectra plots. Plots compare site class B and C response spectra at magnitude $M = 6.5$ and distance $R = 10.0$ km for strike-slip events. The top frame plots equation 8; the bottom frame plots equations 12 and 16. Depth $D = 3.0$ km is used for all equations.

Median Horizontal Response Spectra
 Magnitude: $M = 7.5$
 Distance: $R = 10.0$ km

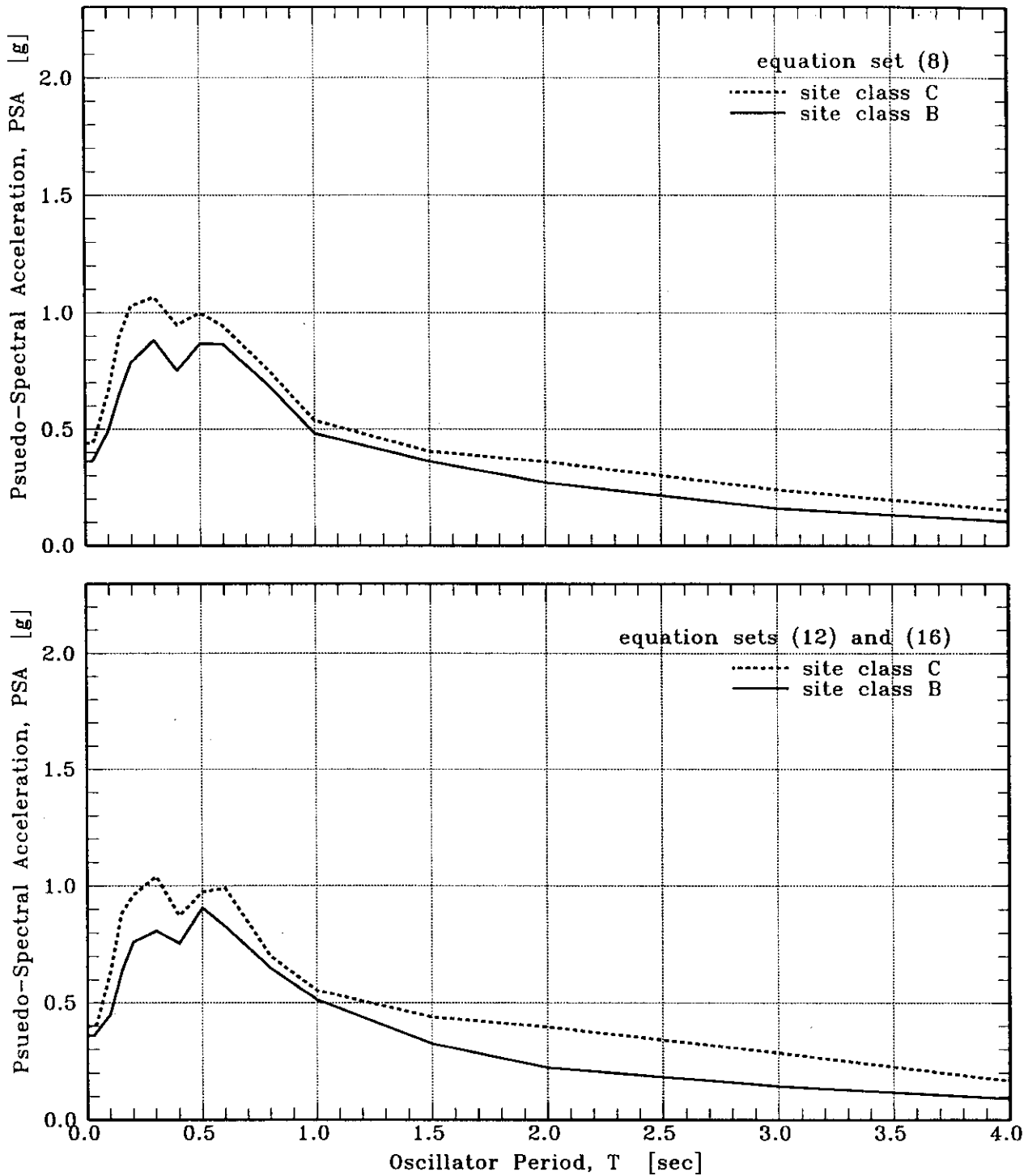


Figure 14. Median horizontal response spectra plots. Plots compare site class B and C response spectra at magnitude $M = 7.5$ and distance $R = 10.0$ km for strike-slip events. The top frame plots equation 8; the bottom frame plots equations 12 and 16. Depth $D = 3.0$ km is used for all equations.

Median Horizontal and Vertical Response Spectra
 Site Class B/C
 Magnitude: $M = 6.5$ and 7.5 ; Distance: $R = 1.0$ km

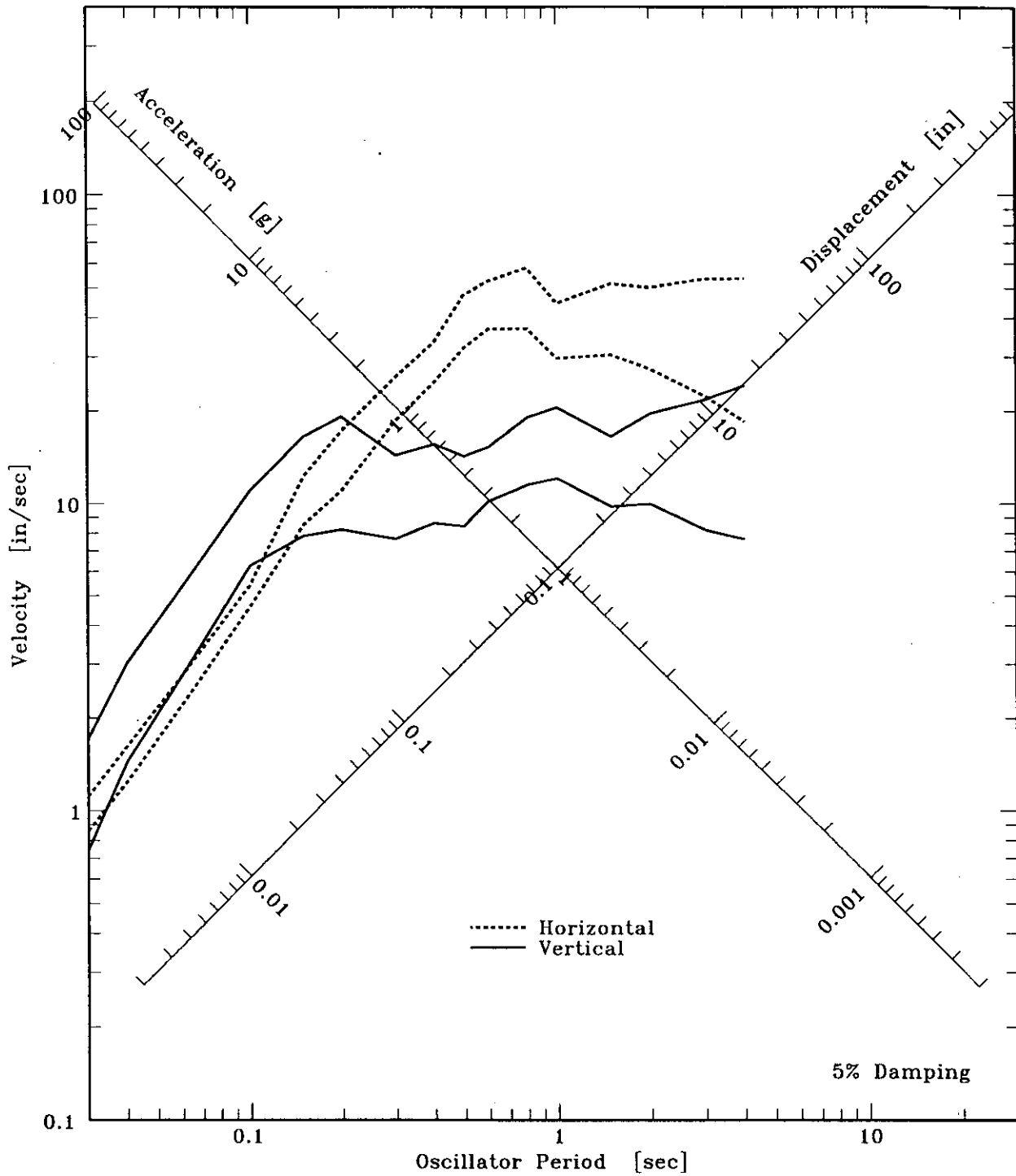


Figure 15. Median horizontal and vertical response spectra plots. Plots compare horizontal and vertical response spectra predicted by equation set 3 for site class B/C, strike-slip conditions, for magnitudes $M = 6.5$ and 7.5 at distance $R = 1.0$ km.

Median Horizontal and Vertical Response Spectra
 Site Class B/C
 Magnitude: $M = 6.5$ and 7.5 ; Distance: $R = 10.0$ km

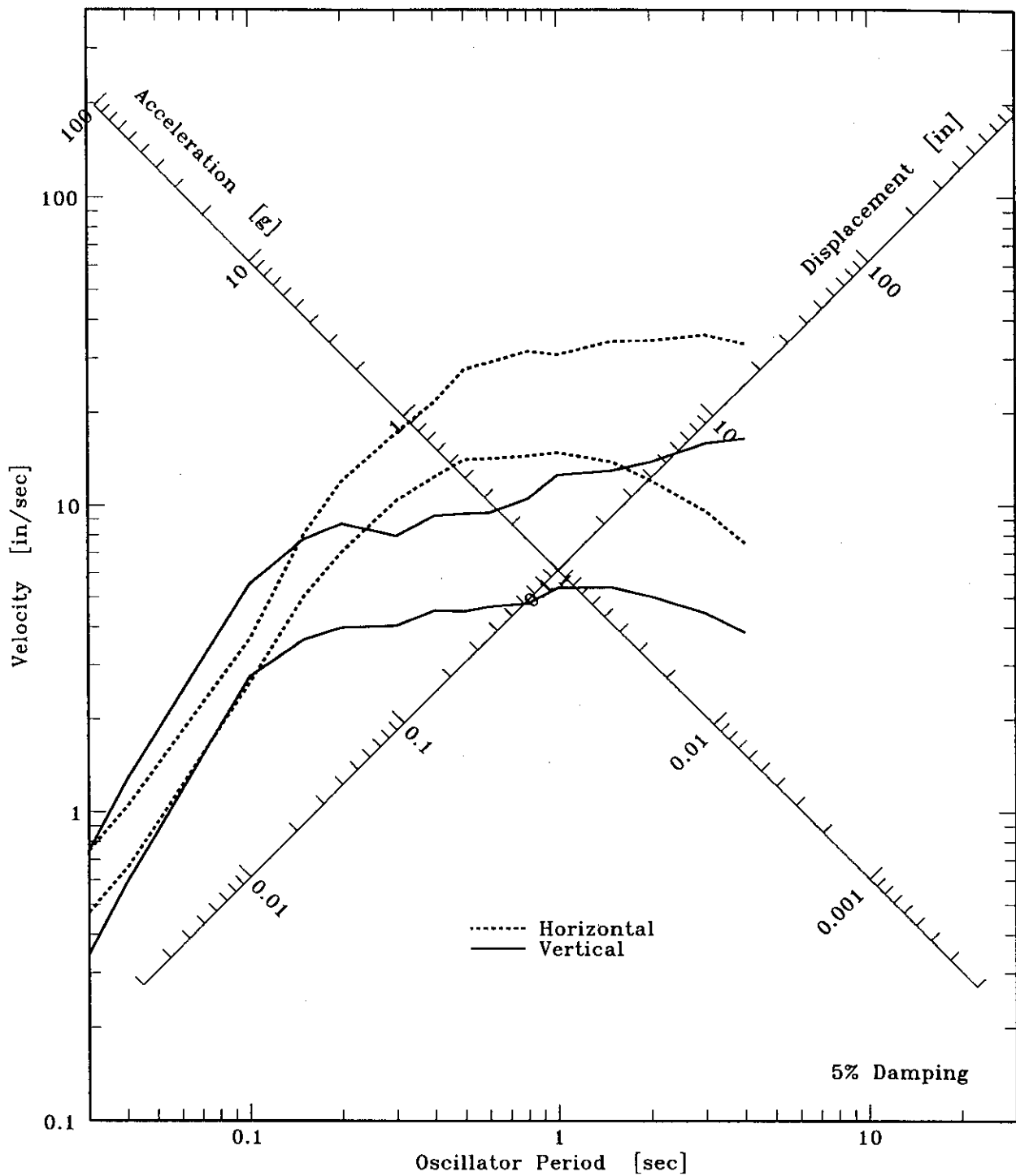


Figure 16. Median horizontal and vertical response spectra plots. Plots compare horizontal and vertical response spectra predicted by equation set 3 for site class B/C, strike-slip conditions, for magnitudes $M = 6.5$ and 7.5 at distance $R = 10.0$ km.

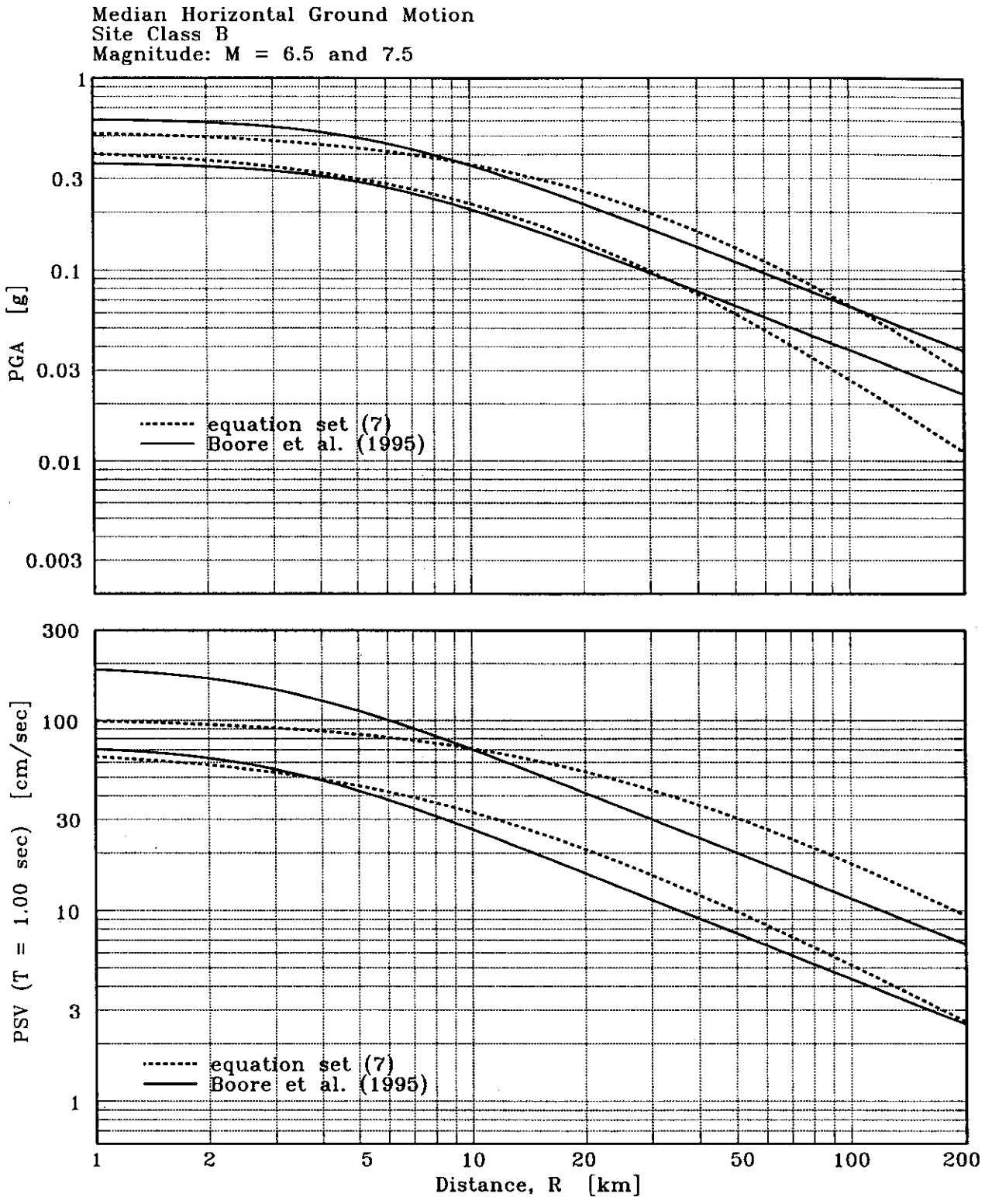


Figure 17. Median horizontal PGA and PSV (period $T = 1.0$ sec) attenuation plots. Plots compare ground-motions at magnitudes $M = 6.5$ and 7.5 predicted by equation set 7 and by Boore et al. (1995) for site class B, strike-slip conditions.

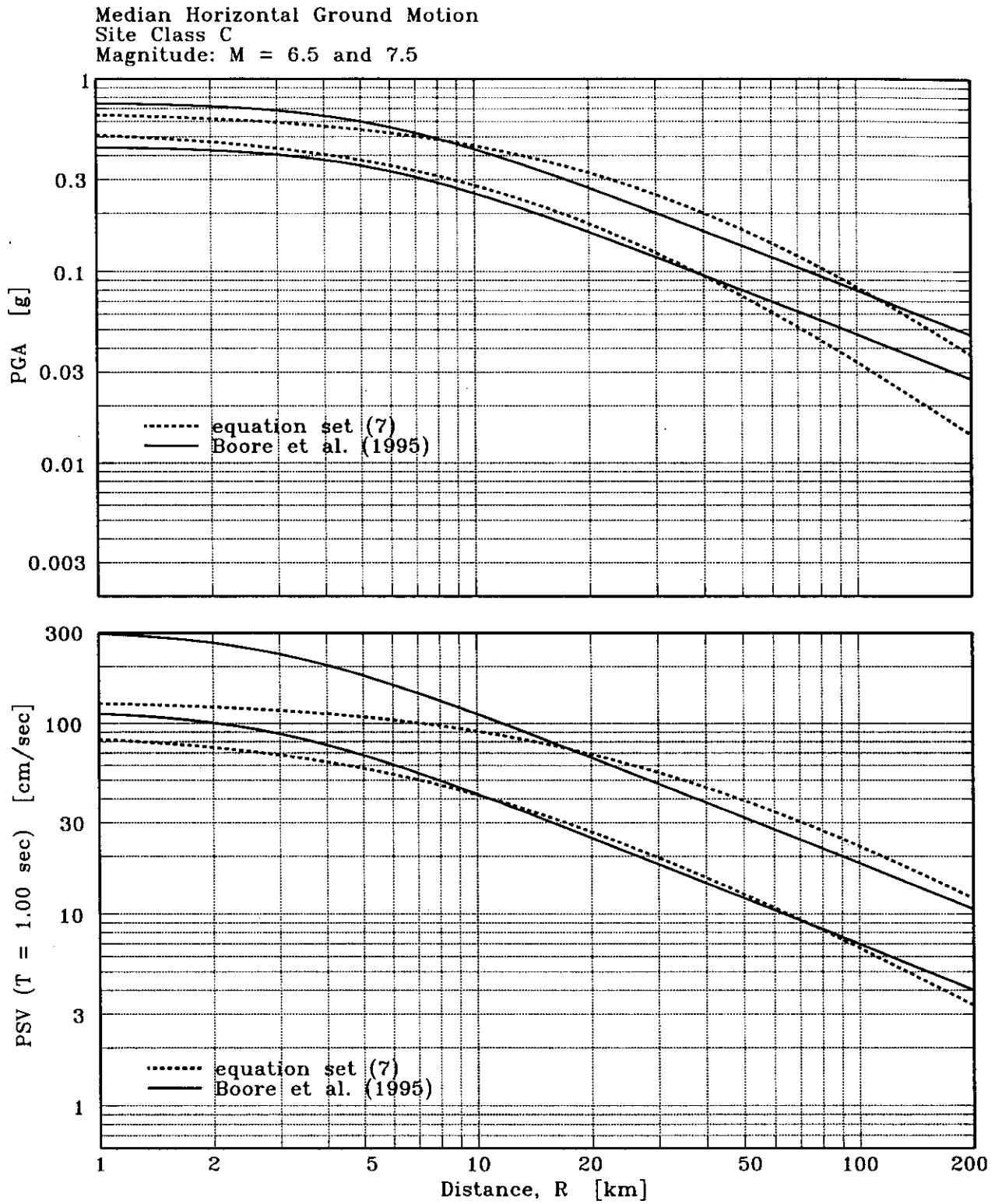


Figure 18. Median horizontal PGA and PSV (period $T = 1.0$ sec) attenuation plots. Plots compare ground-motions at magnitudes $M = 6.5$ and 7.5 predicted by equation set 7 and by Boore et al. (1995) for site class C, strike-slip conditions.

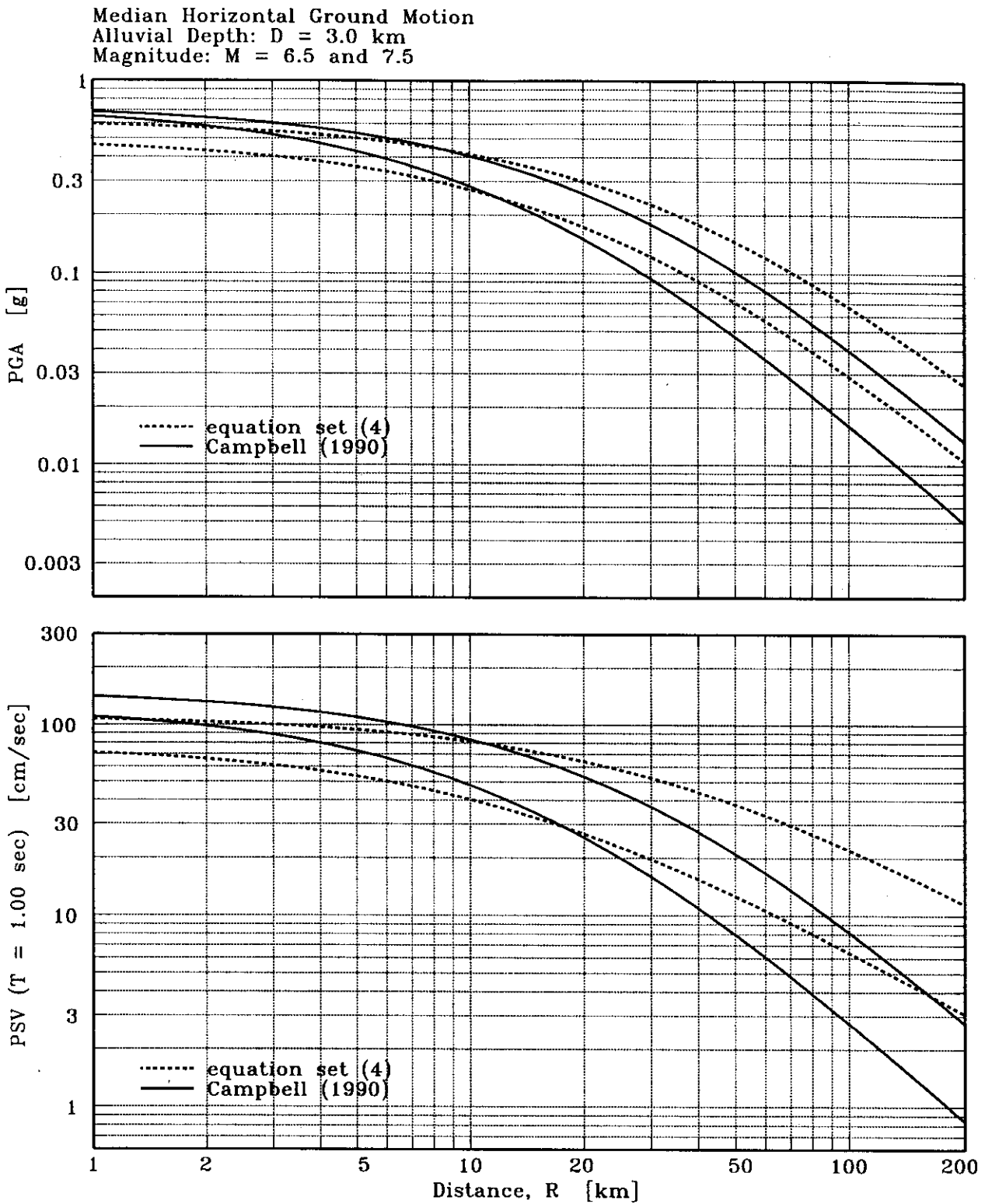


Figure 19. Median horizontal PGA and PSV (period $T = 1.0$ sec) attenuation plots. Plots compare ground-motions at magnitudes $M = 6.5$ and 7.5 predicted by equation set 4 and by Campbell (1990) for strike-slip conditions with alluvial depth $D = 3.0$ km (database median depth).

Median Horizontal Response Spectra
 Site Class B
 Distance: $R = 1.0$ km

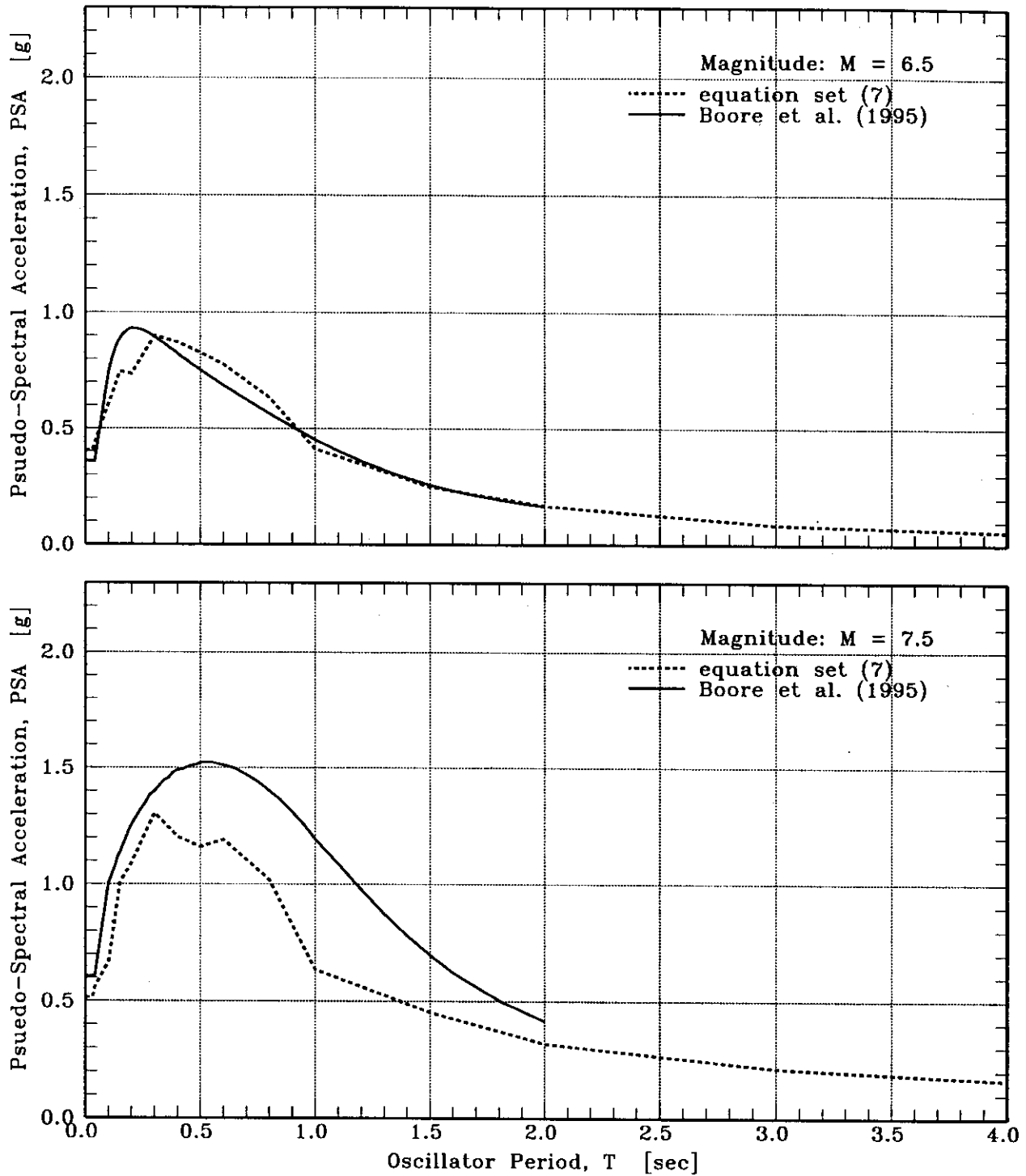


Figure 20. Median horizontal response spectra plots. Plots compare response spectra predicted by equation set 7 and by Boore et al. (1995) for magnitudes $M = 6.5$ and 7.5 at distance $D = 1.0$ km for site class B, strike-slip conditions.

Median Horizontal Response Spectra
 Site Class B
 Distance: $R = 10.0$ km

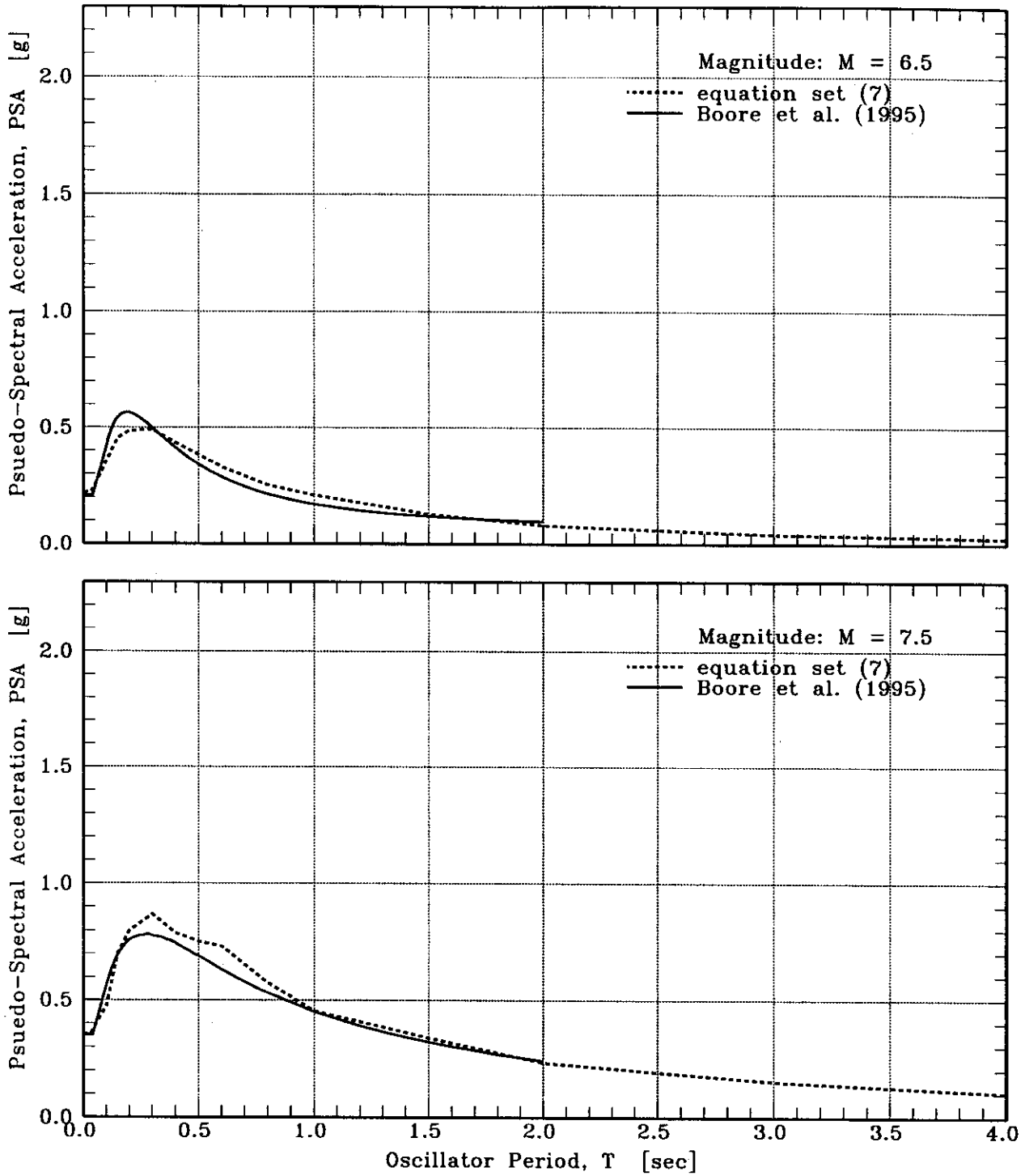


Figure 21. Median horizontal response spectra plots. Plots compare response spectra predicted by equation set 7 and by Boore et al. (1995) for magnitudes $M = 6.5$ and 7.5 at distance $D = 10.0$ km for site class B, strike-slip conditions.

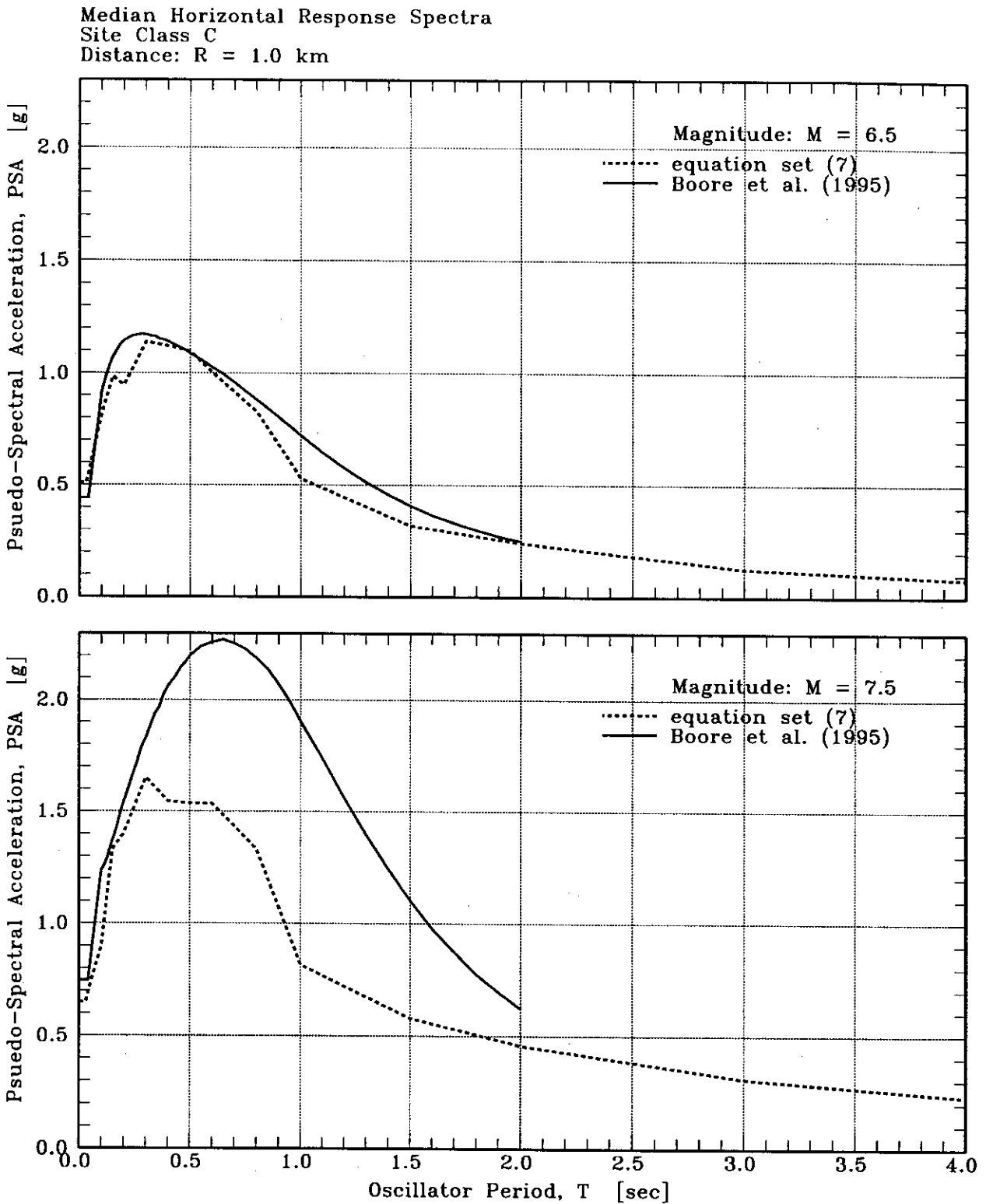


Figure 22. Median horizontal response spectra plots. Plots compare response spectra predicted by equation set 7 and by Boore et al. (1995) for magnitudes $M = 6.5$ and 7.5 at distance $D = 1.0$ km for site class C, strike-slip conditions.

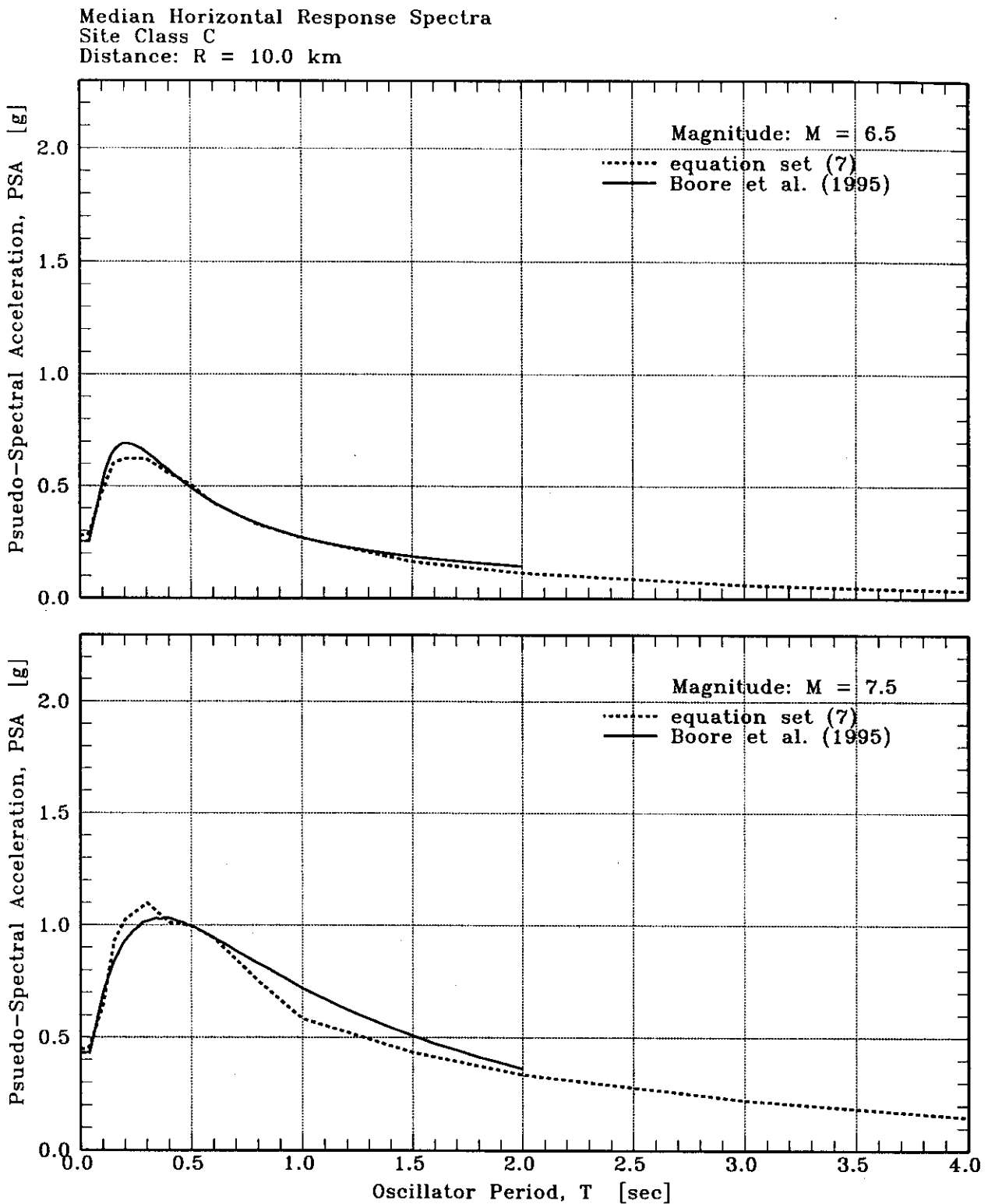


Figure 23. Median horizontal response spectra plots. Plots compare response spectra predicted by equation set 7 and by Boore et al. (1995) for magnitudes $M = 6.5$ and 7.5 at distance $D = 10.0$ km for site class C, strike-slip conditions.

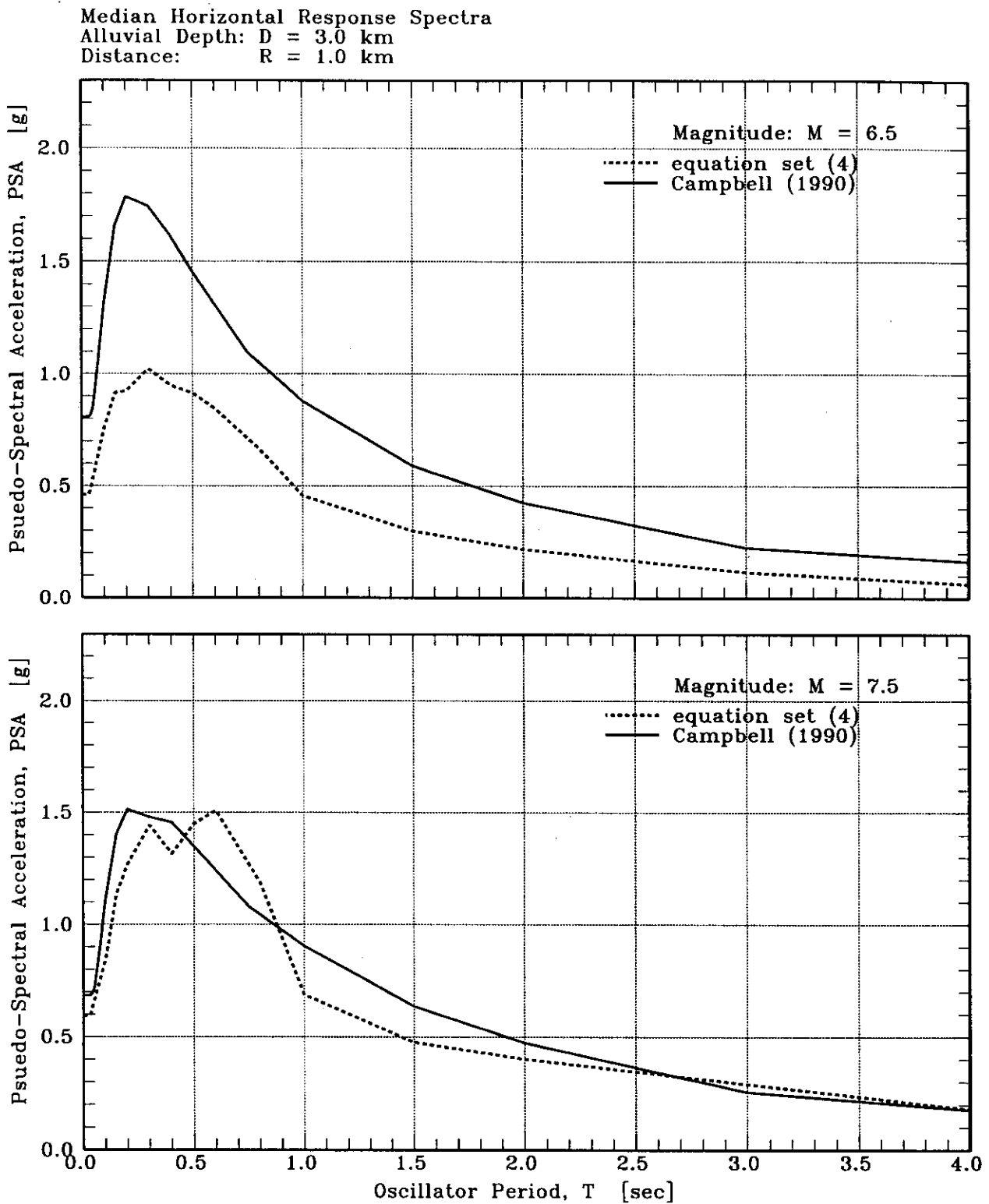


Figure 24. Median horizontal response spectra plots. Plots compare response spectra predicted by equation set 4 and by Campbell (1990) for magnitudes $M = 6.5$ and 7.5 at distance $D = 1.0$ km for strike-slip conditions with alluvial depth $D = 3.0$ km (database median depth).

Median Horizontal Response Spectra
 Alluvial Depth: $D = 3.0$ km
 Distance: $R = 10.0$ km

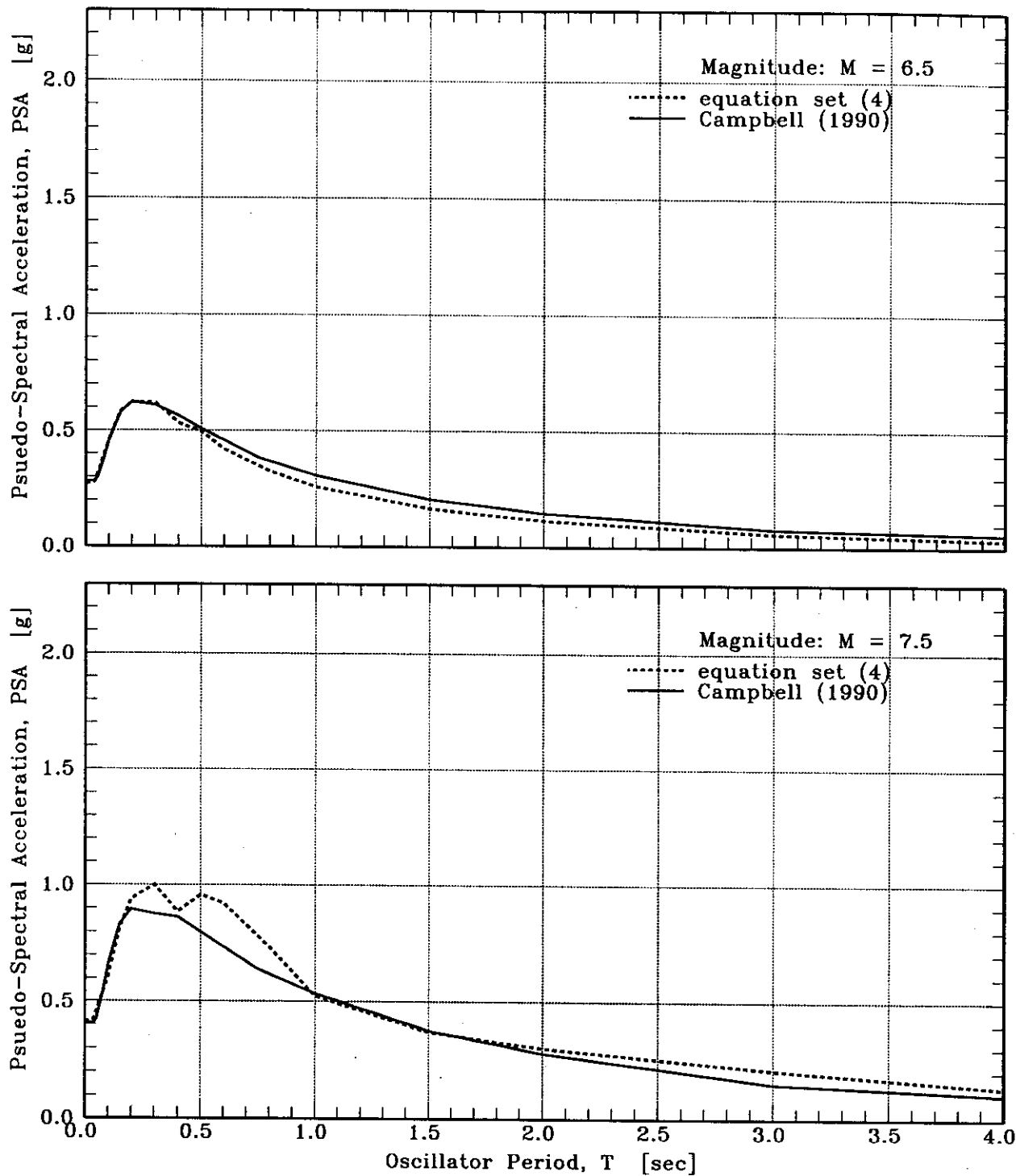


Figure 25. Median horizontal response spectra plots. Plots compare response spectra predicted by equation set 4 and by Campbell (1990) for magnitudes $M = 6.5$ and 7.5 at distance $D = 10.0$ km for strike-slip conditions with alluvial depth $D = 3.0$ km (database median depth).

Appendix A. Ground-Motion Database

Table A-1 on the following pages lists the ground-motion records used in the analysis. The table provides (1) the date, name, moment magnitude, and fault-type of the causative earthquake; (2) the station code, name, structure type, geographic coordinates, site classification, and depth-to-basement; (3) the source-to-site distance of the record; (4) the horizontal and vertical peak ground acceleration values; (5) references for geotechnical/geologic and strong-motion data sources; and (6) a list, for each record, of the databases within which the record is used. The following definitions and notes apply to Table A-1:

<i>M_w</i>	Earthquake moment magnitude
<i>Fault Type</i>	Code indicating the earthquake focal mechanism; <i>S</i> ⇒ strike-slip, <i>R</i> ⇒ reverse-slip
<i>Station Desig.</i>	U.S. Geological Survey station number, California Division of Mines and Geology station number, or an assigned alpha-numeric code
<i>Structure Code</i>	Code describing the structure housing the instrument (e.g. "3/1" refers to a 3-story structure with 1 sub-ground level; "3/" refers to a structure with either no sub-ground levels, or an unknown number of sub-ground levels; no entry indicates an instrument shelter in most cases, but may refer to a small structure)
<i>Site Class</i>	Entry is either "B", "C", or "B/C"; see the report text for the associated site class definitions
<i>Alluvial Depth</i>	Depth to basement rock at the recording station site; depth is reported in km.
<i>Geologic Reference</i>	Refer to the "Geologic References" list that follows Table A-1 for the complete citation corresponding to the given reference number
<i>Closest Approach Distance</i>	Closest distance from the recording station to the earthquake fault rupture surface; distance is reported in km
<i>PGA</i>	Peak ground accelerations recorded for the horizontal (<i>H1</i> and <i>H2</i>) and vertical (<i>V</i>) components; reported in units of <i>g</i>
<i>Strong-Motion Data Source</i>	Refer to the "Strong-Motion Data Sources" list that follows Table A-1 for complete citation corresponding to the given reference number
<i>Database Subsets</i>	A list of the database subsets within which the record is used; the database subsets are identified in Table 2, which is repeated below

Table 2. Database Subsets

Database Subset Number	Criteria for Inclusion in Database Subset						Number of Records in Subset	
	Site Classification			Alluvial Depth			H comp.	V comp.
	B	C	B/C	unknown	known			
1	(♦ or ♦)	and	(♦ or ♦)	271	269			
2	(♦ or ♦)	and	♦	146	146			
3	(♦ or ♦)	and	(♦ or ♦)	244	242			
4	(♦ or ♦)	and	♦	139	139			
5	♦	and	(♦ or ♦)	112	111			
6	♦	and	♦	50	50			
7		and	(♦ or ♦)	132	131			
8		and	♦	89	89			

Table A-1. Strong-Motion Database

Earthquake Yr./Mo./Dy	Location	Mw	Fault Type	Station Desig.	Location	Structure Code	Station Latitude	Station Longitude	Site Class	Alluvial Depth	Geologic Reference	Closest Approach Distance [km]	H1	H2	V	Strong-Motion Data Source	Database Subsets
33.03.11	Long Beach	6.4	S	131	Long Beach, Utility Bldg	3/1	33.770°N	118.196°W	B	3.0	(7),(13),(39)	5.0	0.20	0.16	0.29	(10),(6)	1, 2, 3, 4, 5, 6
33.03.11	Long Beach	6.4	S	288	Vernon, Cmd Terminal	2/0	33.999°N	118.196°W	C	6.4	(7),(10),(4),(39)	19.5	0.13	0.15	0.15	(10),(5)	1, 2, 3, 4, 7, 8
40.05.19	Imperial Valley	7.0	S	117	El Centro, Imperial Val. Irng. Dist.	2/0	32.790°N	115.550°W	B	6.1	(36),(38),(7),(26),(27),(31)	10.0	0.35	0.21	0.10	(10),(5)	1, 2, 3, 4, 7, 8
52.07.21	Kern County	7.4	R	1084	Taft, Lincoln School Tunnel	2/1	35.150°N	119.480°W	B	NA	(36),(7),(28),(11),(31)	42.0	0.16	0.18	0.10	(10),(5)	1, 3, 5
52.07.21	Kern County	7.4	R	283	Santa Barbara Courthouse	2/1	34.450°N	119.700°W	B	0.5	(36),(7),(28),(12),(31),(3)	85.0	0.09	0.13	0.04	(10),(5)	1, 2, 3, 4, 5, 6
52.07.21	Kern County	7.4	R	195	Los Angeles, Hollywood Storage PE Lot	2/1	34.090°N	118.339°W	B	2.3	(7),(10),(30),(31),(39)	107.0	0.06	0.04	0.02	(10),(5)	1, 2, 3, 4, 5, 6
52.07.21	Kern County	7.4	R	475	Pasadena, CIT Athenaeum	2/1	34.139°N	118.121°W	B	0.9	(7),(12),(39)	109.0	0.05	0.05	0.03	(10),(5)	1, 2, 3, 4, 5, 6
66.08.28	Central California	6.1	S	1013	Parkfield, Cholame 2WA	1/0	35.793°N	120.328°W	C	3.0	(34),(13),(3)	0.1	0.49	NA	0.21	(10),(5)	1, 2, 3, 4, 7, 8
66.08.28	Central California	6.1	S	1014	Parkfield, Cholame 5W	1/0	35.671°N	120.359°W	C	1.1	(34),(11),(13),(3)	9.5	0.24	0.27	0.08	(10),(5)	1, 2, 3, 4, 7, 8
66.08.28	Central California	6.1	S	1015	Parkfield, Cholame 8W	1/0	35.671°N	120.359°W	C	1.9	(34),(11),(13),(3)	11.3	0.27	0.35	0.13	(10),(5)	1, 3, 5
66.08.28	Central California	6.1	S	1097	Parkfield, Cholame (Shandon Tombler)	1/0	35.639°N	120.470°W	B	NA	(34),(11),(13),(3)	15.0	0.05	0.06	0.05	(10),(5)	1, 2, 3, 4, 5, 6
66.08.28	Central California	6.1	S	1016	Parkfield, Cholame 12W	2/0	35.150°N	119.480°W	B	2.6	(36),(7),(28),(11),(31)	131.0	0.01	0.01	0.01	(10),(5),(6)	1, 3, 5
66.08.28	Central California	6.1	S	1064	Taft, Lincoln School Tunnel	2/0	32.790°N	115.550°W	B	NA	(36),(38),(7),(26),(27),(31)	148.0	0.01	0.02	0.005	(10),(5),(6)	1, 2, 3, 4, 7, 8
66.08.28	Central California	6.3	S	117	El Centro, Imperial Val. Irng. Dist.	2/0	32.790°N	115.550°W	C	6.1	(36),(38),(7),(26),(27),(31)	45.0	0.13	0.06	0.03	(10),(5)	1, 2, 3, 4, 7, 8
66.08.28	Central California	6.3	S	117	El Centro, Imperial Val. Irng. Dist.	2/0	32.790°N	115.550°W	C	6.1	(36),(38),(7),(26),(27),(31)	122.0	0.04	0.05	0.06	(10),(5)	1, 3, 5
68.04.09	Borrego Mountain	6.6	S	280	San Onofre, SCE Power Plant	1/0	33.370°N	117.580°W	B	NA	(32)	120.0	0.02	0.03	0.02	(10),(5)	1, 2, 3, 4, 7, 8
68.04.09	Borrego Mountain	6.6	S	113	Colton, SCE	1/0	34.060°N	117.320°W	C	0.2	(7),(12),(3)	130.0	0.01	0.01	0.01	(10),(5)	1, 2, 3, 4, 7, 8
68.04.09	Borrego Mountain	6.6	S	190	Long Beach, Terminal Island	1/1	33.770°N	118.230°W	C	2.1	(7),(17),(39)	187.0	0.01	0.01	0.004	(10),(5)	1, 2, 3, 4, 5, 6
68.04.09	Borrego Mountain	6.6	S	475	Pasadena, CIT Athenaeum	2/1	34.139°N	118.121°W	B	0.9	(7),(12),(39)	196.0	0.02	0.02	0.01	(10),(5)	1, 2, 3, 4, 5, 6
68.04.09	Borrego Mountain	6.6	S	288	Vernon, Cmd Terminal	6/1	33.999°N	118.196°W	C	6.4	(7),(10),(4),(39)	211.0	0.01	0.01	0.005	(10),(5)	1, 2, 3, 4, 7, 8
68.04.09	Borrego Mountain	6.6	S	195	Los Angeles, Hollywood Storage PE Lot	2/0	34.090°N	118.339°W	B	2.3	(7),(10),(30),(31),(39)	15.0	0.14	0.20	0.05	(10)	1, 2, 3, 4, 5, 6
68.04.09	Borrego Mountain	6.2	R	290	Wrightwood, 6074 Park Dr.	2/0	34.360°N	117.630°W	B	0.1	(12),(3)	28.0	0.12	0.06	0.05	(10)	1, 2, 3, 4, 7, 8
70.09.12	Lytle Creek	5.2	R	274	San Bernardino, Hall of Records	1/0	34.110°N	117.290°W	C	1.1	(4),(39)	29.0	0.04	0.04	0.03	(10)	1, 2, 3, 4, 7, 8
70.09.12	Lytle Creek	5.2	R	113	Colton, SCE	1/0	34.060°N	117.320°W	C	0.2	(7),(12),(3)	18.0	0.27	0.21	0.13	(10),(5)	1, 3, 5
71.02.09	San Fernando	6.6	R	122	Glendale, 633 E Broadway	3/1	34.150°N	118.250°W	B	NA	(32),(39),(31)	21.0	0.35	0.28	0.11	(10),(5)	1, 2, 3, 4, 5, 6
71.02.09	San Fernando	6.6	R	128	Lake Hughes, Arroy Station 12	1/0	34.670°N	118.560°W	B	NA	(32),(4)	23.0	0.17	0.21	0.09	(10),(5)	1, 2, 3, 4, 5, 6
71.02.09	San Fernando	6.6	R	195	Los Angeles, Hollywood Storage PE Lot	2/0	34.090°N	118.339°W	B	2.3	(7),(10),(30),(31),(39)	24.0	0.11	0.14	0.09	(10),(5)	1, 2, 3, 4, 5, 6
71.02.09	San Fernando	6.6	R	282	Palmdale Fire Station	1/0	34.580°N	118.110°W	B	0.2	(10),(3)	24.0	0.10	0.11	0.09	(10),(5)	1, 2, 3, 4, 5, 6
71.02.09	San Fernando	6.6	R	475	Pasadena, CIT Athenaeum	2/1	34.139°N	118.121°W	B	0.9	(7),(12),(39)	25.0	0.15	0.11	0.09	(10),(5)	1, 2, 3, 4, 5, 6
71.02.09	San Fernando	6.6	R	125	Lake Hughes, Arroy Station 1	1/0	34.680°N	118.440°W	B	3.0	(10),(29),(31),(3)	26.0	0.32	0.27	0.16	(10),(5)	1, 2, 3, 4, 5, 6
71.02.09	San Fernando	6.6	R	110	Castaic, Old Ridge Route	1/0	34.560°N	118.660°W	B	10.0	(12),(32),(31),(3)	33.5	0.11	0.08	0.04	(10),(5)	1, 2, 3, 4, 5, 6
71.02.09	San Fernando	6.6	R	288	Vernon, Cmd Terminal	6/1	33.999°N	118.196°W	C	6.4	(7),(10),(4),(39)	38.0	0.09	0.12	0.05	(10),(5)	1, 3, 5
71.02.09	San Fernando	6.6	R	585	Pasadena Pump Plant	1/0	34.510°N	117.920°W	B	NA	(10)	36.0	0.10	0.10	0.06	(10),(5)	1, 2, 3, 4, 7, 8
71.02.09	San Fernando	6.6	R	289	Whittier Narrows Dam (Upstream)	2/0	34.030°N	118.050°W	C	2.4	(4),(39)	38.0	0.03	0.04	0.02	(10),(5)	1, 2, 3, 4, 5, 6
71.02.09	San Fernando	6.6	R	411	Palos Verdes Estates, 2518 Via Tejon	1/1	33.800°N	118.360°W	B	0.1	(7),(12),(31),(39)	52.0	0.03	0.03	0.02	(10),(5)	1, 2, 3, 4, 7, 8
71.02.09	San Fernando	6.6	R	130	Long Beach, Terminal Island	1/1	33.770°N	118.230°W	C	2.0	(7),(17),(39)	58.0	0.03	0.03	0.02	(10),(5)	1, 2, 3, 4, 7, 8
71.02.09	San Fernando	6.6	R	131	Long Beach, Utility Bldg	3/1	33.770°N	118.190°W	B	3.0	(7),(13),(39)	58.0	0.03	0.02	0.01	(10),(5)	1, 2, 3, 4, 5, 6
71.02.09	San Fernando	6.6	R	290	Wrightwood, 6074 Park Dr.	2/0	34.360°N	117.630°W	B	0.1	(12),(3)	59.0	0.04	0.06	0.02	(10),(5)	1, 2, 3, 4, 5, 6
71.02.09	San Fernando	6.6	R	261	Santa Ana, Engineering Bldg	3/1	33.750°N	117.870°W	C	2.9	(4),(39)	71.5	0.03	0.03	0.02	(10),(5)	1, 2, 3, 4, 7, 8
71.02.09	San Fernando	6.6	R	1102	Wheeler Ridge, Ground Station	6/1	35.030°N	119.010°W	B	NA	(12),(4)	80.0	0.03	0.03	0.01	(10),(5)	1, 3, 5
71.02.09	San Fernando	6.6	R	274	San Bernardino, Hall of Records	1/1	34.110°N	117.290°W	C	1.1	(4),(3)	100.0	0.04	0.04	0.02	(10),(5)	1, 2, 3, 4, 7, 8
71.02.09	San Fernando	6.6	R	465	San Juan Capistrano, City Hall	1/1	34.410°N	119.850°W	B	NA	(32),(31)	104.0	0.04	0.03	0.02	(10),(5)	1, 3, 7
71.02.09	San Fernando	6.6	R	282	Goleta, UCSB Fluid Mech. Lab.	1/1	34.370°N	119.850°W	B	NA	(32)	120.0	0.02	0.02	0.01	(10),(5)	1, 3, 5
71.02.09	San Fernando	6.6	R	280	San Onofre, SCE Power Plant	1/0	33.370°N	118.580°W	B	NA	(13),(4)	121.0	0.01	0.02	0.01	(10),(5)	1, 3, 5
71.02.09	San Fernando	6.6	R	123	Hemet, Station Av Fire Station	1/1	33.728°N	118.979°W	C	NA	(2)	134.0	0.04	0.04	0.03	(10),(5)	1, 3, 5
71.02.09	San Fernando	6.6	R	1377	San Juan Bautista, 24 Polk Street	1/1	36.848°N	121.536°W	B	NA	(8),(31)	8.9	0.04	0.11	0.05	(10)	1, 3, 5
74.11.28	Central California	5.2	S	1250	Gilroy, Gavilan College	2/1	36.973°N	121.568°W	B	NA	(9),(31)	10.8	0.10	0.13	0.03	(10)	1, 2, 3, 4, 7, 8
74.11.28	Central California	5.2	S	1028	Hollister Public Library	2/1	36.850°N	121.400°W	B	1.7	(38),(9),(30),(31),(1)	10.8	0.09	0.16	0.07	(10)	1, 2, 3, 4, 7, 8
78.08.13	Santa Barbara	5.8	R	283	Santa Barbara Courthouse	2/1	34.420°N	119.700°W	B	0.5	(38),(7),(28),(12),(31),(3)	5.0	0.10	0.20	0.08	(10)	1, 2, 3, 4, 5, 6
78.08.13	Santa Barbara	5.8	R	91	Goleta, UCSB Free Field	1/0	34.420°N	119.851°W	B	NA	(2)	8.0	0.35	0.29	0.14	(2)	1, 3, 5
78.09.12	Tabas, Iran	7.4	R	NA	Tabas, Iran	E/B/C	33.800°N	56.920°E	B/C	NA	(20)	3.0	0.88	0.83	0.75	(1)	1
78.09.12	Tabas, Iran	7.4	R	NA	Dezfool, Iran	E/B/C	33.300°N	57.520°E	B/C	NA	(20)	14.0	0.37	0.40	0.19	(1)	1
78.09.12	Tabas, Iran	7.4	R	NA	Bozrooyeh, Iran	E/B/C	33.800°N	57.430°E	B/C	NA	(20)	24.0	0.10	0.09	0.08	(1)	1
78.09.12	Tabas, Iran	7.4	R	NA	Ferdows, Iran	E/B/C	34.030°N	58.173°E	B/C	NA	(20)	83.0	0.10	0.11	0.05	(1)	1
78.09.12	Tabas, Iran	7.4	R	NA	Bajestan, Iran	E/B/C	34.510°N	58.180°E	B/C	NA	(20)	106.0	0.09	0.07	0.03	(1)	1
78.09.12	Tabas, Iran	7.4	R	NA	Sadeh, Iran	E/B/C	33.300°N	59.230°E	B/C	NA	(20)	136.0	0.03	0.03	0.01	(1)	1
78.09.12	Tabas, Iran	7.4	R	NA	Kashmar, Iran	E/B/C	35.300°N	56.450°E	B/C	NA	(20)	168.0	0.03	0.04	0.03	(1)	1
79.06.06	Coyote Lake	5.8	S	57393	Gilroy #6, San Ysidro Microwave Site	1/0	37.026°N	121.484°W	B	0.5	(9),(11),(3)	1.0	0.42	0.32	0.15	(2)	1, 2, 3, 4, 5, 6
79.06.06	Coyote Lake	5.8	S	47381	Gilroy #3, Gilroy Sewage Treat. Plant	1/0	36.987°N	121.556°W	C	0.6	(9),(11),(23)	5.0	0.25	0.26	0.17	(2)	1, 2, 3, 4, 7, 8
79.06.06	Coyote Lake	5.8	S	47380	Gilroy #2, Hwy 101/Boles Road Motel	1/0	36.982°N	121.556°W	C	0.3	(9),(11),(14),(23)	7.0	0.25	0.19	0.14	(2)	1, 2, 3, 4, 7, 8
79.06.06	Coyote																

Table A-1. Strong-Motion Database

Earthquake Yr. Mo. Dy Location	Mw	Fault Type	Station Desig. Location	Structure Code	Station Latitude	Station Longitude	Site Class	Site Depth	Geologic Reference	Closest Approach Distance [km]	PGA [g]	H1	H2	V	Strong-Motion Data Source	Database Subsets
79.10.15 Imperial Valley	6.5	S	5158 El Centro #6, 551 Huston Road	1/	32.839°N	115.487°W	C	6.1	(26),(38)	1.0	0.44	0.38	1.70	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	5028 El Centro #7, Imperial Valley College	1/	32.829°N	115.504°W	C	6.1	(36),(26),(38)	1.0	0.46	0.33	0.51	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	6616 Aeroport	1/	32.850°N	115.330°W	C	6.1	(21),(38)	1.4	0.26	0.33	0.18	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	5054 Bonds Corner	1/	32.893°N	115.338°W	C	6.1	(26),(38)	3.0	0.79	0.59	0.95	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	952 El Centro #8, James Road	1/	32.865°N	115.466°W	C	6.1	(26),(38)	4.0	0.37	0.53	0.44	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	5159 El Centro #8, Cruickshank Road	1/	32.811°N	115.532°W	C	6.1	(26),(38)	4.0	0.47	0.61	0.41	(10)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	5165 El Centro, Dogwood Road	1/	32.800°N	115.540°W	C	6.1	(35),(26),(38)	5.0	0.49	0.35	0.66	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	5060 Brawley Municipal Airport	1/	32.968°N	115.509°W	C	6.1	(26),(38)	7.0	0.22	0.17	0.15	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	955 El Centro #4, Anderson Road	1/	32.860°N	115.430°W	C	6.1	(26),(38)	7.0	0.36	0.49	0.20	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	5055 Holtville Post Office	1/	32.810°N	115.380°W	C	6.1	(26),(38)	8.0	0.22	0.25	0.23	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	412 El Centro #10, Community Hospital	1/	32.780°N	115.587°W	C	6.1	(36),(26),(38)	9.0	0.17	0.23	0.18	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	5053 Calexico Fire Station	1/	32.699°N	115.492°W	C	6.1	(26),(38)	11.0	0.20	0.27	0.10	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	6617 Cucupah	1/	32.550°N	115.230°W	C	6.1	(21),(38)	11.0	0.31	NA	NA	(10),(7)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	5057 El Centro #3, Pine Union School	1/	32.894°N	115.380°W	C	4.6	(26),(38)	12.7	0.22	0.27	0.13	(10),(7)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	5058 El Centro #11, McCabe Union School	1/	32.752°N	115.594°W	C	6.1	(26),(38)	13.0	0.30	0.36	0.14	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	5051 El Centro, Parachute Test Facility	1/	32.950°N	115.630°W	B	4.0	(26),(38)	15.0	0.12	0.14	0.07	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	931 El Centro #12, 907 Brodman Road	1/	32.710°N	115.630°W	C	4.6	(26),(38)	18.0	0.19	0.07	0.15	(10),(7)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	6622 Compuetas	1/	32.573°N	115.663°W	C	2.1	(26),(38)	22.0	0.14	0.14	0.04	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	5059 El Centro #13, Strobel Residence	1/	32.709°N	115.683°W	C	6.1	(26),(38)	22.0	0.19	0.07	0.15	(10),(7)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	5056 El Centro #1, Bonchard Ranch	1/	32.960°N	115.319°W	C	5.2	(21),(38)	31.0	0.08	0.04	0.03	(10),(6)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	5052 Plester City Storehouse	1/	32.790°N	115.850°W	C	1.5	(21),(38)	34.0	0.11	0.07	0.03	(2)	1, 2, 3, 4, 7, 8	
79.10.15 Imperial Valley	6.5	S	11023 Niland Fire Station	1/	33.239°N	115.512°W	C	2.4	(21),(38)	49.0	0.13	0.12	0.04	(10),(7)	1, 3, 7	
80.01.24 Livermore	5.8	S	5066 Coachella Canal #4	1/	33.360°N	115.590°W	C	NA	(2)	17.0	0.05	0.04	0.03	(2)	1, 3, 5	
80.01.24 Livermore	5.8	S	57187 San Ramon, Eastman Kodak	1/	37.729°N	121.928°W	C	NA	(2)	18.0	0.05	0.04	0.02	(2)	1, 3, 5	
80.01.24 Livermore	5.8	S	57134 San Ramon	1/	37.780°N	121.980°W	C	NA	(2)	21.0	0.04	0.02	0.01	(2)	1, 3, 5	
80.01.24 Livermore	5.8	S	67070 Antioch	1/	38.015°N	121.813°W	B	NA	(2)	29.0	0.06	0.08	0.02	(2)	1, 3, 5	
80.01.24 Livermore	5.8	S	57063 Tracy	1/	37.657°N	122.061°W	C	NA	(9),(15)	31.0	0.06	0.06	0.02	(2)	1, 3, 5	
80.01.24 Livermore	5.8	S	57084 Fremont, Mission San Jose	1/	37.530°N	121.919°W	B	2.3	(6),(19),(37),(3)	32.0	0.05	0.05	0.03	(2)	1, 2, 3, 4, 5, 6	
81.04.26 Westmorland	5.9	S	5060 Brawley, Municipal Airport	1/	32.968°N	115.509°W	B	6.0	(26),(38)	19.0	0.18	0.16	0.11	(2)	1, 2, 3, 4, 7, 8	
81.04.26 Westmorland	5.9	S	5051 El Centro, Parachute Test Facility	1/	32.950°N	115.700°W	B	4.0	(26),(38)	19.0	0.23	0.16	0.16	(2)	1, 2, 3, 4, 5, 6	
81.04.26 Westmorland	5.9	S	11023 Niland Fire Station	1/	33.239°N	115.512°W	C	2.4	(21),(38)	21.0	0.19	0.11	0.13	(10)	1, 2, 3, 4, 7, 8	
83.05.02 Coalinga	6.4	R	1182 Pleasant Valley Pump Plant, Switchyard	NA	36.308°N	120.249°W	B/C	NA	(13),(4)	12.0	0.52	0.45	0.38	(10)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36453 Parkfield, Fault Zone 11	1/	36.898°N	120.388°W	B	NA	(13)	33.1	0.09	0.08	0.04	(2)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36413 Parkfield, Gold Hill 3E	1/	35.870°N	120.334°W	B	NA	(13)	34.1	0.07	0.10	0.06	(2)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36138 Parkfield, Fault Zone 12	1/	35.898°N	120.433°W	B	NA	(13)	34.2	0.11	0.11	0.07	(2)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36438 Parkfield, Stone Corral 4E	1/	35.855°N	120.281°W	B	NA	(13)	35.1	0.07	0.07	0.03	(2)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36454 Parkfield, Fault Zone 6	1/	35.858°N	120.420°W	B	NA	(13)	37.6	0.06	0.08	0.03	(2)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36414 Parkfield, Fault Zone 4	1/	35.836°N	120.365°W	B	NA	(13)	39.2	0.12	0.07	0.05	(2)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36416 Parkfield, Gold Hill 2W	1/	35.812°N	120.391°W	B	NA	(13)	41.6	0.08	0.08	0.04	(2)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36413 Parkfield, Fault Zone 2	1/	35.787°N	120.334°W	B	NA	(13)	43.1	0.14	0.12	0.04	(2)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36420 Parkfield, Gold Hill 3W	1/	35.798°N	120.411°W	B	NA	(13)	43.8	0.12	0.14	0.07	(2)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36450 Parkfield, Cholame 3E	1/	35.770°N	120.247°W	B	NA	(13)	44.4	0.05	0.04	0.03	(2)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36453 Parkfield, Gold Hill 4W	1/	35.765°N	120.444°W	B	NA	(13)	46.0	0.10	0.08	0.03	(2)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36441 Parkfield, Vineyard Canyon 6W	1/	35.861°N	120.600°W	B	NA	(13)	46.3	0.08	0.05	0.04	(2)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36230 Parkfield, Cholame 2E (Temblor I)	1/	35.752°N	120.284°W	B	NA	(13)	46.4	0.04	0.03	0.02	(2)	1, 3, 5	
83.05.02 Coalinga	6.4	R	36452 Parkfield, Cholame 1E	1/	35.743°N	120.277°W	C	NA	(34),(27),(31),(13),(3)	47.4	0.09	0.09	0.06	(2)	1, 3, 7	
83.05.02 Coalinga	6.4	R	36228 Parkfield, Cholame 2WA	1/	35.718°N	120.290°W	B	NA	(13)	48.6	0.11	0.11	0.04	(2)	1, 2, 3, 4, 7, 8	
83.05.02 Coalinga	6.4	R	36411 Parkfield, Cholame 4W	1/	35.697°N	120.328°W	B	NA	(34),(13),(3)	50.3	0.14	0.14	0.03	(2)	1, 2, 3, 4, 7, 8	
83.05.02 Coalinga	6.4	R	36227 Parkfield, Cholame 5W	1/	35.671°N	120.328°W	C	1.9	(34),(11),(13),(3)	56.1	0.10	0.10	0.03	(2)	1, 2, 3, 4, 7, 8	
83.05.02 Coalinga	6.4	R	36226 Parkfield, Cholame 8W	1/	35.639°N	120.404°W	B	2.6	(34),(11),(13),(3)	60.4	0.29	0.02	0.21	(10),(3)	1, 2, 3, 4, 5, 6	
83.05.02 Coalinga	6.4	R	36229 Parkfield, Cholame 12W	1/	37.168°N	121.628°W	B	0.0	(9),(13),(3)	3.9	0.29	0.29	0.23	(2),(3)	1, 2, 3, 4, 5, 6	
84.04.24 Morgan Hill	6.3	S	1652 Anderson Dam, Downstream	1/	37.028°N	121.484°W	B	0.5	(13),(16),(3)	11.5	0.11	0.19	0.45	(2),(3)	1, 2, 3, 4, 7, 8	
84.04.24 Morgan Hill	6.3	S	57383 Gilroy #6, San Ysidro Microwave Site	1/	37.093°N	121.434°W	C	0.6	(8),(11),(23)	13.7	0.22	0.17	0.60	(2),(3)	1, 2, 3, 4, 7, 8	
84.04.24 Morgan Hill	6.3	S	57425 Gilroy #7, Mantell Ranch, Jamison Rd	1/	36.987°N	121.536°W	C	0.8	(8),(11),(23)	14.4	0.20	0.20	0.39	(2),(3)	1, 2, 3, 4, 7, 8	
84.04.24 Morgan Hill	6.3	S	47381 Gilroy #3, Gilroy Sewage Treatment Plant	1/	36.982°N	121.568°W	C	0.3	(8),(11),(23)	14.9	0.22	0.10	0.12	(2)	1, 3, 5	
84.04.24 Morgan Hill	6.3	S	47360 Gilroy #2, Hwy 101/Bloss Road Motel	1/	36.973°N	121.568°W	B	NA	(6),(31)	16.0	0.11	0.08	0.05	(2)	1, 3, 5	
84.04.24 Morgan Hill	6.3	S	47006 Gilroy, Gavilan College Phys. Sci. Bldg.	1/	37.048°N	121.803°W	B	3.8	(6),(13),(16),(3)	23.5	0.03	0.03	0.02	(2),(3)	1, 2, 3, 4, 7, 8	
84.04.24 Morgan Hill	6.3	S	57007 Corralitos, Eureka Canyon Road	1/	37.397°N	121.952°W	C	0.4	(6),(13),(37),(3)	25.5	0.11	0.09	0.23	(2),(3)	1, 2, 3, 4, 7, 8	
84.04.24 Morgan Hill	6.3	S	57066 Agnew, Agnew State Hospital	1/	36.888°N	121.413°W	C	1.6	(8),(11)	26.0	0.03	0.02	0.23	(2),(3)	1, 2, 3, 4, 7, 8	
84.04.24 Morgan Hill	6.3	S	1656 Hollister Airport Differential Array	1/	37.530°N	121.919°W	C	2.5	(8),(13),(37),(3)	31.6	0.03	0.02	0.23	(2),(3)	1, 2, 3, 4, 7, 8	
84.04.24 Morgan Hill	6.3	S	57064 Fremont, Mission San Jose	1/	37.530°N	121.919°W	C	2.5	(8),(13),(37),(3)	31.6	0.03	0.02	0.23	(2),(3)	1, 2, 3, 4, 7, 8	
84.04.24 Morgan Hill	6.3	S	1575 Hollister City Hall Annex	1/	36.851°N	121.402°W	C	1.7	(36),(9),(30),(31),(1)	32.3	0.07	0.07	0.54	(10)	1, 2, 3, 4, 7, 8	

Table A-1. Strong-Motion Database

Earthquake Yr./Mo./Dy	Location	Mw	Fault Type	Station Desig.	Station Location	Structure Code	Station Latitude	Station Longitude	Site Class	Alluvial Depth	Geologic Reference	Closest Approach Distance [km]	PGA [g] H1 H2 V	Strong-Motion Data Source	Database Subsets
84.04.24	Morgan Hill	6.3	S	47125	Capitola Fire Station	1/	36.974°N	121.952°W	B	NA	(13),(37)	38.9	0.14 0.10 0.04	(2)	1, 3, 5
84.04.24	Morgan Hill	6.3	S	58135	Santa Cruz, UCSC Lick Lab.	1/	37.001°N	122.060°W	B	NA	(8)	46.0	0.04 0.08 0.03	(2)	1, 3, 5
88.01.26	Hollister	5.4	S	47391	Hollister, Glorieta Warehouse	1/	36.851°N	121.398°W	C	1.7	(38),(8),(30),(31),(4),(1)	13.4	0.14 0.11 0.26	(2)	1, 2, 3, 4, 7, 8
88.07.08	Palm Springs	6.1	S	12025	Palm Springs Airport	1/	33.829°N	116.591°W	C	0.7	(13),(4),(3)	11.2	0.20 0.17 0.19	(2),(3)	1, 2, 3, 4, 7, 8
88.07.08	Palm Springs	6.1	S	12331	Hemet, Sibleon Av Fire Station	1/	33.729°N	116.979°W	C	NA	(13),(4)	29.2	0.14 0.15 0.10	(2)	1, 3, 7
88.07.08	Palm Springs	6.1	S	22170	Joshua Tree Fire Station	1/	34.131°N	116.314°W	B	0.2	(2),(3)	31.9	0.07 0.05 0.04	(2),(3)	1, 2, 3, 4, 5, 6
88.07.08	Palm Springs	6.1	S	12168	Puerta La Cruz, USFS Storage Bldg	1/	33.324°N	116.683°W	B	NA	(2)	87.9	0.08 0.08 0.04	(2)	1, 3, 5
88.07.08	Palm Springs	6.1	S	13172	Temecula, CDF Fire Station	1/	33.496°N	117.446°W	B	NA	(4)	89.9	0.10 0.11 0.03	(2)	1, 3, 7
88.07.08	Palm Springs	6.1	S	13123	Riverside Airport	1/	33.951°N	117.149°W	B	NA	(2)	75.7	0.04 0.05 0.03	(2)	1, 3, 5
88.07.08	Palm Springs	6.1	S	23497	Rancho Cucamonga, Law & Just. Cntr.	1/	34.104°N	117.574°W	C	NA	(13),(4)	89.1	0.02 0.02 NA	(10)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	289	Whittier Narrows Dam (Upstream)	1/	34.050°N	118.050°W	C	2.4	(4),(8)	11.1	0.30 0.23 0.53	(10)	1, 2, 3, 4, 5, 6
87.10.01	Whittier	6.0	R	709	Garvey Reservoir Abutment Bldg	1/	34.050°N	118.110°W	B	2.7	(13),(4),(39)	11.3	0.37 0.48 0.38	(10)	1, 2, 3, 4, 5, 6
87.10.01	Whittier	6.0	R	5129	Bell, Los Angeles Bulk Mail Center	1/	33.990°N	118.160°W	C	5.5	(13),(4),(39)	12.2	0.33 0.45 0.53	(10)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	24481	Alhambra, Fremont School	1/	34.070°N	118.150°W	B	1.5	(13),(4),(39)	12.3	0.40 0.29 0.20	(2),(3)	1, 2, 3, 4, 5, 6
87.10.01	Whittier	6.0	R	268	Vernon, Cnd Terminal	6/1	33.969°N	118.196°W	B	6.4	(7),(10),(4),(39)	13.2	0.27 0.24 0.14	(10)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	5244	Los Angeles, 4407 Jasper Street	1/	34.081°N	118.188°W	B	0.9	(13),(4),(39)	14.0	0.33 0.22 0.12	(10)	1, 2, 3, 4, 5, 6
87.10.01	Whittier	6.0	R	24401	San Marino, Southwestern Academy	1/	34.115°N	118.130°W	B	1.4	(13),(4),(39)	14.9	0.19 0.14 0.14	(2),(3)	1, 2, 3, 4, 5, 6
87.10.01	Whittier	6.0	R	80053	Pasadena, CIT Athenaeum	2/1	34.139°N	118.121°W	B	0.9	(7),(12),(39)	16.0	0.11 0.21 0.16	(2)	1, 2, 3, 4, 5, 6
87.10.01	Whittier	6.0	R	14368	Dorney, County Maintenance Bldg	1/	33.924°N	118.167°W	B	9.1	(13),(4),(39)	16.2	0.18 0.21 0.16	(2),(3)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	24400	Los Angeles, Oregon park	1/	33.905°N	118.178°W	B/C	4.3	(13),(4),(39)	17.1	0.43 0.45 0.14	(2)	1, 2
87.10.01	Whittier	6.0	R	14198	Inglewood, Union Oil Yard	1/	33.907°N	118.279°W	B	4.3	(13),(4),(39)	22.5	0.23 0.26 0.07	(2),(3)	1, 2, 3, 4, 5, 6
87.10.01	Whittier	6.0	R	24303	Los Angeles, Hollywood Storage PE Lot	1/	34.090°N	118.338°W	C	2.3	(7),(10),(30),(31),(39)	23.8	0.12 0.21 0.08	(2),(3)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	14403	Los Angeles, 116th St School	1/	33.929°N	118.280°W	C	6.7	(13),(4),(39)	26.7	0.40 0.28 0.11	(2)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	24390	Century City, LA Country Club South	1/	33.778°N	118.133°W	B	4.8	(13),(4),(39)	29.6	0.06 0.06 0.06	(2)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	14241	Long Beach Recreation park	1/	34.066°N	117.748°W	B	0.0	(13),(4),(39)	29.9	0.06 0.07 0.06	(2)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	23525	Pomona, 4th and Locust FF	1/	34.009°N	118.361°W	B/C	4.9	(13),(4),(39)	30.3	0.16 0.15 0.11	(2)	1, 2
87.10.01	Whittier	6.0	R	24157	Los Angeles, Baldwin Hills	1/	33.754°N	118.200°W	C	2.4	(13),(4),(39)	32.8	0.07 0.05 0.03	(2)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	14395	Long Beach, Harbor Admin. Bldg	1/	33.662°N	117.967°W	B	2.7	(13),(4),(39)	42.8	0.08 0.08 0.05	(2)	1, 3, 5
87.10.01	Whittier	6.0	R	13197	Huntington Beach, Lake St Fire Sta.	1/	33.869°N	117.709°W	B/C	NA	(13),(4)	42.9	0.08 0.08 0.05	(2)	1, 3, 7
87.10.01	Whittier	6.0	R	13122	Featherly Park, Park Maint. Bldg.	NA	34.104°N	117.574°W	C	NA	(13),(4)	45.5	0.06 0.06 0.04	(2)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	23497	Rancho Cucamonga, Law & Just. Cntr.	1/	34.236°N	118.434°W	C	4.3	(13),(21)	45.7	0.09 0.09 0.09	(2)	1, 3, 5
87.10.01	Whittier	6.0	R	24087	Arieta, Northoff Ave. Fire Station	1/	34.326°N	118.444°W	B	NA	(13)	45.9	0.05 0.06 0.04	(2)	1
87.10.01	Whittier	6.0	R	24514	Sylmar, Olive View Medical Center	1/	34.390°N	118.530°W	B/C	NA	(4)	56.7	0.06 0.05 0.03	(2)	1, 3, 5
87.10.01	Whittier	6.0	R	24279	Newhall, LA County Fire Station	1/	34.390°N	118.530°W	B/C	NA	(4)	57.8	0.06 0.05 0.03	(2)	1, 3, 5
87.10.01	Whittier	6.0	R	13123	Riverside Airport	1/	34.013°N	118.800°W	B	NA	(4)	63.6	0.05 0.05 0.03	(2)	1, 3, 5
87.10.01	Whittier	6.0	R	24396	Malibu, Point Dume School	1/	34.013°N	118.800°W	B	NA	(4)	63.6	0.05 0.05 0.03	(2)	1, 3, 5
87.10.01	Whittier	6.0	R	23516	San Bernardino, Sunwest Office Bldg	3/0	34.065°N	117.269°W	C	NA	(4)	70.2	0.03 0.03 0.03	(2)	1, 3, 7
87.10.01	Whittier	6.0	R	24274	Lancaster, Medical Office Bldg FF	1/	34.668°N	118.156°W	C	NA	(4)	72.9	0.06 0.06 0.02	(2)	1, 3, 5
87.10.01	Whittier	6.0	R	24263	Moorepark, Ventura Co. Fire Dept. Garage	1/	34.228°N	118.861°W	B/C	NA	(4)	77.3	0.05 0.02 0.05	(2)	1
87.10.01	Whittier	6.0	R	24278	Castaic, Old Ridge Route	1/	34.584°N	118.642°W	B	10.0	(12),(32),(31),(3)	77.3	0.07 0.07 0.03	(2)	1, 2, 3, 4, 5, 6
87.10.01	Whittier	6.0	R	24271	Lake Hughes, #1-Fire Station #78	1/	34.674°N	118.430°W	B	NA	(10),(29),(31)	78.2	0.03 0.04 NA	(2)	1, 3, 5
87.10.01	Whittier	6.0	R	12331	Hemet, Sibleon Av Fire Station	1/	33.729°N	116.979°W	C	NA	(4)	89.0	0.05 0.08 0.02	(2)	1, 3, 7
87.10.01	Whittier	6.0	R	24274	Rosamond, Goode Ranch	1/	37.046°N	121.853°W	C	NA	(13),(4)	2.2	0.04 0.03 0.03	(2)	1, 2, 3, 4, 5, 6
87.10.01	Whittier	6.0	R	57007	Cornelios, Eureka Canyon Road	4/0	38.909°N	121.758°W	C	3.8	(8),(13),(16),(3)	6.3	0.28 0.39 0.86	(2),(4)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	47459	Watsonville, 4-story commercial Bldg	1/	36.973°N	121.588°W	B	NA	(8),(31)	11.2	0.37 0.33 0.20	(2)	1, 3, 5
87.10.01	Whittier	6.0	R	47006	Gilroy, Gavilan College Phys. Sci. Bldg.	1/	37.118°N	122.031°W	B	NA	(8),(13)	12.4	0.33 0.53 0.41	(2),(4)	1, 3, 5
87.10.01	Whittier	6.0	R	58065	Saratoga, Aloha Ave	1/	36.982°N	121.556°W	C	0.3	(8),(11),(14),(23)	12.6	0.33 0.37 0.90	(2),(4)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	47380	Gilroy #2, Hwy 101/Bolas Road Motel	1/	36.987°N	121.536°W	B	0.6	(8),(11),(23)	14.4	0.47 0.57 0.37	(2)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	58135	Santa Cruz, UCSC Lick Lab.	1/	37.001°N	122.060°W	B	NA	(8)	16.8	0.33 0.46 0.39	(2)	1, 3, 5
87.10.01	Whittier	6.0	R	57383	Gilroy #9, San Ysidro Microwave Site	1/	37.028°N	121.484°W	B	0.5	(8),(11),(3)	20.5	0.17 0.13 0.10	(10),(4)	1, 2, 3, 4, 5, 6
87.10.01	Whittier	6.0	R	1652	Anderson Dam, Downstream	1/	37.168°N	121.628°W	B	0.0	(8),(13),(3)	22.4	0.28 0.25 0.17	(10),(4)	1, 2, 3, 4, 5, 6
87.10.01	Whittier	6.0	R	57504	Coyote Lake Dam, Downstream	1/	37.118°N	121.550°W	B	NA	(8),(1)	23.1	0.18 0.16 0.10	(2),(4)	1, 3, 5
87.10.01	Whittier	6.0	R	1658	Hollister Airport Differential Array	1/	36.888°N	121.413°W	C	1.8	(8),(1)	24.5	0.29 0.27 0.16	(10),(4)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	57425	Gilroy #7, Mantelli Ranch, Jamison Rd	1/	37.033°N	121.454°W	C	0.3	(38),(8),(30),(31),(1)	24.7	0.33 0.23 0.12	(2),(4)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	1575	Hollister City Hall Annex	1/	36.851°N	121.402°W	C	1.7	(8),(13),(37),(3)	26.6	0.25 0.23 0.22	(10),(4)	1, 2, 3, 4, 7, 8
87.10.01	Whittier	6.0	R	57066	Agraves, Agraves State Hospital	1/	37.397°N	121.952°W	C	0.4	(8),(13)	27.6	0.16 0.17 0.10	(2),(4)	1, 3, 7
87.10.01	Whittier	6.0	R	1695	Sunnyvale, Colton Avenue	1/	37.340°N	121.756°W	C	NA	(8),(13)	29.1	0.19 0.22 0.10	(10),(4)	1, 3, 7
87.10.01	Whittier	6.0	R	1227	Palo Alto, VA Hospital, Bldg 1	1/	36.671°N	121.642°W	C	NA	(8),(13)	29.3	0.12 0.09 0.11	(2),(4)	1, 3, 7
87.10.01	Whittier	6.0	R	1601	Stanford University, SLAC Test Lab	1/	37.419°N	122.140°W	C	NA	(8),(13)	31.0	0.38 0.34 0.20	(10),(4)	1, 3, 5
87.10.01	Whittier	6.0	R	1687	Calevaas Array, Calevaas Reservoir	1/	37.452°N	122.205°W	C	NA	(8),(13)	35.7	0.19 0.29 0.10	(10),(4)	1, 3, 7
87.10.01	Whittier	6.0	R	1230	Mento Park VA Hospital, Bldg 137	1/	37.468°N	122.157°W	C	0.2	(13),(37),(19),(16)	38.5	0.13 0.08 0.07	(10),(4)	1, 2, 3, 4, 7, 8

Table A-1. Strong-Motion Database

Earthquake Yr./Mo./Day	Location	Mw	Fault Type	Station Design.	Station Location	Structure Code	Latitude	Longitude	Site Class	Alluvial Depth	Geologic Reference	Closest Approach Distance [km]	PGA [g]	H1	H2	V	Strong-Motion Data Source	Database Subsets
89.10.17	Loma Prieta	7.0	S	58127	Woodside Fire Station	1/	37.429° N	122.258° W	B	NA	(9),(13),(16)	38.4	0.08	0.08	0.05	(2)	1, 3, 5	
89.10.17	Loma Prieta	7.0	S	57084	Fremont, Mission San Jose	1/	37.530° N	121.919° W	B	2.3	(9),(13),(37),(3)	42.6	0.11	0.13	0.09	(2),(4)	1, 2, 3, 4, 5, 6	
89.10.17	Loma Prieta	7.0	S	1686	Fremont, Calaveras Array, Emerson Crt.	1/	37.535° N	121.928° W	C	NA	(9),(13)	43.0	0.15	0.20	0.07	(10),(4)	1, 3, 7	
89.10.17	Loma Prieta	7.0	S	1161	Crystal Springs Reservoir	1/	37.470° N	122.320° W	B	NA	(38),(6),(13)	48.3	0.11	0.12	0.06	(10)	1, 3, 7	
89.10.17	Loma Prieta	7.0	S	1481	Bear Valley Str #12, Williams Ranch	1/	36.658° N	121.249° W	B	NA	(8)	49.2	0.17	0.16	0.10	(10),(4)	1, 3, 5	
89.10.17	Loma Prieta	7.0	S	1688	Calaveras Array, Suncl Fire Station	1/	36.673° N	121.890° W	B	NA	(9),(13)	50.4	0.07	0.10	0.04	(10),(4)	1, 3, 5	
89.10.17	Loma Prieta	7.0	S	1474	Bear Valley Str #5, Callens Ranch	1/	37.657° N	121.195° W	B	NA	(6)	52.1	0.08	0.08	0.05	(2)	1, 3, 5	
89.10.17	Loma Prieta	7.0	S	58219	Hayward, CSUH Stadium Grounds	1/	37.657° N	122.081° W	B	NA	(8),(15)	56.7	0.14	0.18	0.10	(2),(4)	1, 3, 7	
89.10.17	Loma Prieta	7.0	S	58393	Hayward, John Muir School	1/	37.657° N	122.081° W	C	NA	(8)	62.2	0.08	0.08	0.03	(10),(4)	1, 3, 7	
89.10.17	Loma Prieta	7.0	S	1689	Calaveras Array, Dublin Fire Station	1/	37.709° N	121.932° W	C	NA	(8)	65.5	0.10	0.13	0.05	(10),(4)	1, 3, 7	
89.10.17	Loma Prieta	7.0	S	1479	Bear Valley Str #10, Webb Residence	2/	37.806° N	122.287° W	C	NA	(9),(37)	77.0	0.25	0.20	0.15	(2),(4)	1, 3, 7	
89.10.17	Loma Prieta	7.0	S	58224	Oakland 2 story	1/	37.807° N	122.361° W	B	NA	(14)	80.2	0.07	0.03	0.03	(2)	1, 3, 5	
89.10.17	Loma Prieta	7.0	S	58163	Yerba Buena Island	1/	37.870° N	122.240° W	B	NA	(8)	83.9	0.04	0.08	0.02	(10)	1, 3, 5	
89.10.17	Loma Prieta	7.0	S	1005	UC Berkeley, Strawberry Canyon	1/	37.876° N	122.249° W	B	NA	(8)	84.6	0.24	0.25	0.04	(2)	1, 3, 5	
89.10.17	Loma Prieta	7.0	S	58471	Berkeley, Lawrence Berkeley Lab.	1/	37.806° N	122.472° W	B	NA	(8)	89.9	0.11	0.13	0.04	(10),(4)	1, 3, 7	
89.10.17	Loma Prieta	7.0	S	1678	San Francisco, Golden Gate Bridge	1/	37.935° N	122.342° W	B	NA	(8)	92.7	0.07	0.05	0.03	(2)	1, 3, 5	
89.10.17	Loma Prieta	7.0	S	58505	Richmond City Hall Parking Lot	1/	37.993° N	122.115° W	B	NA	(9),(37)	94.3	0.11	0.13	0.04	(2),(4)	1, 3, 7	
89.10.17	Loma Prieta	7.0	S	1448	Martinez VA Hospital	1/	34.059° N	117.748° W	B	0.0	(13),(4),(39)	11.5	0.19	0.21	0.10	(2)	1, 2, 3, 4, 7, 8	
90.02.28	Upland	5.6	S	23525	Pomona, 4th and Locust FF	1/	34.104° N	117.574° W	C	NA	(13),(4)	13.2	0.23	0.24	0.16	(2)	1, 3, 7	
90.02.28	Upland	5.6	S	23487	Rancho Cucamonga, Law & Just. Ctr.	1/	34.177° N	118.076° W	C	0.9	(4),(39)	18.0	0.17	0.14	0.15	(2)	1, 2, 3, 4, 7, 8	
91.06.28	Sierra Madre	5.6	R	24402	Altadena, Eaton Canyon Park	1/	34.115° N	118.190° W	B	1.4	(13),(4),(39)	22.9	0.23	0.22	0.19	(2)	1, 2, 3, 4, 5, 6	
91.06.28	Sierra Madre	5.6	R	24401	San Marino, Southwestern Academy	1/	34.037° N	118.178° W	B/C	4.3	(13),(4),(39)	31.7	0.29	0.28	0.07	(2)	1, 2, 3, 4, 5, 6	
91.06.28	Sierra Madre	5.6	R	24400	Los Angeles, Obregon park	1/	34.131° N	116.314° W	B	0.2	(2),(3)	7.1	0.18	0.14	0.16	(11)	1, 3, 5	
92.06.28	Landers	7.3	S	22170	Joshua Tree Fire Station	1/	34.053° N	116.572° W	B	NA	(4)	18.0	0.28	0.42	0.18	(8)	1, 3, 7	
92.06.28	Landers	7.3	S	MVH	Monrovia valley	1/	34.852° N	116.658° W	C	NA	(4)	18.6	0.15	0.25	0.16	(2)	1, 3, 7	
92.06.28	Landers	7.3	S	NA	Coolwater SCE	1/	34.903° N	116.823° W	C	NA	(4)	22.4	0.09	0.09	0.11	(2)	1, 2, 3, 4, 7, 8	
92.06.28	Landers	7.3	S	22074	Yermo Fire Station	1/	33.829° N	116.501° W	C	0.7	(13),(4),(3)	28.2	0.15	0.17	0.18	(2)	1	
92.06.28	Landers	7.3	S	12025	Palm Springs Airport	1/	33.982° N	116.508° W	B/C	NA	(13),(4)	30.6	0.10	0.12	0.05	(2)	1	
92.06.28	Landers	7.3	S	12149	Desert Hot Springs, Pierson Blvd Fire Sta.	1/0	33.717° N	116.158° W	B/C	NA	(13),(4)	61.8	0.12	0.11	0.06	(2)	1, 3, 7	
92.06.28	Landers	7.3	S	12026	Indio, Coachella Canal	1/	35.268° N	116.664° W	C	NA	(4)	62.2	0.05	0.09	0.08	(2)	1, 3, 7	
92.06.28	Landers	7.3	S	24577	Fort Irwin	1/	33.729° N	116.979° W	C	NA	(13),(4)	66.5	0.05	0.05	0.04	(2)	1, 3, 5	
92.06.28	Landers	7.3	S	12351	Hemet, Station Av Fire Station	1/	33.324° N	116.663° W	C	NA	(2)	66.5	0.11	0.11	0.06	(2)	1, 3, 7	
92.06.28	Landers	7.3	S	12168	Puente La Cruz, USFS Storage Bldg	1/	35.272° N	116.068° W	B	NA	(4)	66.1	0.09	0.13	0.05	(2)	1, 3, 7	
92.06.28	Landers	7.3	S	32075	Baker Fire Station	1/	35.002° N	117.650° W	C	NA	(4)	68.2	0.04	0.05	0.05	(2)	1, 3, 5	
92.06.28	Landers	7.3	S	33083	Boron, Fire Station	1/	34.887° N	117.047° W	B/C	NA	(13),(4)	93.8	0.04	0.05	0.05	(2)	1, 2, 3, 4, 7, 8	
92.06.28	Landers	7.3	S	23559	Banow, Vineyard & H St	1/	33.951° N	117.446° W	B	NA	(4)	98.1	0.05	0.07	0.04	(2)	1, 3, 5	
92.06.28	Landers	7.3	S	13123	Riverside Airport	1/	34.055° N	117.748° W	C	0.0	(13),(4),(39)	117.0	0.05	0.05	0.03	(2)	1, 2, 3, 4, 7, 8	
92.06.28	Landers	7.3	S	23525	Pomona, 4th and Locust FF	1/	33.969° N	117.709° W	C	NA	(13),(4)	124.1	0.03	0.04	0.02	(2)	1, 2, 3, 4, 5, 6	
92.06.28	Landers	7.3	S	13122	Featherly Park, Park Maint. Bldg.	6/1	34.146° N	118.147° W	B	0.8	(4),(39)	142.4	0.04	0.06	0.02	(2)	1, 2, 3, 4, 7, 8	
92.06.28	Landers	7.3	S	24541	Pasadena, 6-story office bldg	1/	33.924° N	118.187° W	C	9.1	(13),(4),(39)	158.0	0.07	0.05	0.02	(2)	1, 2, 3, 4, 5, 6	
92.06.28	Landers	7.3	S	14368	Downey, County Maintenance Bldg	1/	34.037° N	118.178° W	B/C	4.3	(13),(4),(39)	162.1	0.03	0.05	0.01	(2)	1, 2, 3, 4, 5, 6	
92.06.28	Landers	7.3	S	24400	Los Angeles, Obregon Park	1/	33.905° N	118.279° W	B	4.3	(13),(4),(39)	166.0	0.04	0.05	0.02	(2)	1, 2, 3, 4, 5, 6	
92.06.28	Landers	7.3	S	14198	Inglewood, Union Oil Yard	1/	33.923° N	118.260° W	C	6.7	(13),(4),(39)	171.5	0.04	0.06	0.03	(2)	1, 2, 3, 4, 5, 6	
92.06.28	Landers	7.3	S	24438	Los Angeles, 116th St School	1/	34.180° N	118.534° W	B	0.3	(9),(39)	174.0	0.60	0.84	0.54	(2)	1, 3, 5	
92.06.28	Landers	7.3	S	24514	Tarzana, Cedar Hills Nursery	1/	34.326° N	118.444° W	B	NA	(13)	6.7	0.58	0.59	0.55	(2)	1	
94.01.17	Northridge	6.7	R	24279	Symar, Olive View Medical Center	1/	34.390° N	118.530° W	B/C	NA	(4)	7.5	0.34	0.31	0.55	(2)	1, 2, 3, 4, 7, 8	
94.01.17	Northridge	6.7	R	24079	Newhall, LA County Fire Station	1/	34.236° N	118.439° W	C	4.3	(13),(21)	10.2	0.45	0.27	0.28	(9)	1, 2, 3, 4, 5, 6	
94.01.17	Northridge	6.7	R	24087	Alheta, Northhoff Ave. Fire Station	1/	34.221° N	118.421° W	B	3.7	(22),(21)	11.7	0.44	0.27	0.28	(9)	1, 2, 3, 4, 7, 8	
94.01.17	Northridge	6.7	R	USC006	Sun Valley, 13248 Roscoe Blvd.	1/	34.209° N	118.517° W	C	3.2	(22),(21)	13.2	0.39	0.35	0.80	(9)	1, 2, 3, 4, 7, 8	
94.01.17	Northridge	6.7	R	USC003	Northridge, 17645 Satecity	1/	34.212° N	118.608° W	C	1.2	(22),(21)	15.9	0.39	0.35	0.42	(9)	1, 2, 3, 4, 7, 8	
94.01.17	Northridge	6.7	R	USC005	Caroga Park, 7769 Topanga Canyon Rd.	1/	34.564° N	118.642° W	B	10.0	(12),(32),(31),(3)	22.9	0.17	0.51	0.22	(2)	1, 2, 3, 4, 5, 6	
94.01.17	Northridge	6.7	R	24278	Castaic, Old Ridge Route	1/	34.571° N	118.560° W	B	NA	(4)	22.9	0.57	0.26	0.12	(2)	1, 3, 5	
94.01.17	Northridge	6.7	R	24607	Lake Hughes, Array Station 12A	1/	34.068° N	118.439° W	B/C	0.6	(7),(4),(39)	24.5	0.28	0.47	0.27	(2)	1, 2	
94.01.17	Northridge	6.7	R	24888	Los Angeles, UCLA Grounds	1/	34.068° N	118.418° W	C	4.0	(13),(25),(4),(39)	25.5	0.26	0.22	0.12	(2)	1, 2, 3, 4, 7, 8	
94.01.17	Northridge	6.7	R	24389	Century City, LA County Club North	1/	34.090° N	118.359° W	C	2.3	(7),(10),(30),(31),(39)	26.0	0.23	0.39	0.14	(2)	1, 2, 3, 4, 7, 8	
94.01.17	Northridge	6.7	R	24303	Los Angeles, Hollywood Storage PE Lot	1/	34.228° N	118.881° W	B/C	NA	(4)	28.3	0.29	0.19	0.15	(2)	1	
94.01.17	Northridge	6.7	R	24283	Moorpark, Ventura Co. Fire Dept. Garage	1/	34.011° N	118.490° W	B	2.1	(4),(39)	28.6	0.88	0.37	0.23	(2)	1, 2, 3, 4, 5, 6	
94.01.17	Northridge	6.7	R	24538	Santa Monica, City Hall Grounds	1/	34.009° N	118.361° W	B/C	4.9	(13),(4),(39)	32.0	0.24	0.17	0.09	(2)	1, 2	
94.01.17	Northridge	6.7	R	24157	Los Angeles, Belwin Hills	1/	34.043° N	118.271° W	B/C	NA	(4)	33.3	0.10	0.19	0.07	(2)	1, 3, 7	
94.01.17	Northridge	6.7	R	24812	Los Angeles, Pico & Sembois	1/	34.059° N	118.248° W	C	NA	(4)	33.4	0.13	0.18	0.10	(2)	1, 3, 5	
94.01.17	Northridge	6.7	R	24611	Los Angeles, Temple & Hope	1/	34.013° N	118.800° W	B	NA	(4)	36.0	0.13	0.09	0.09	(2)	1, 3, 5	
94.01.17	Northridge	6.7	R	24398	Malibu, Point Dume School	1/	34.062° N	118.985° W	B	NA	(4)	36.0	0.49	0.21	0.12	(2)	1, 3, 5	
94.01.17	Northridge	6.7	R	24805	Los Angeles, Univ. Hospital Grounds	1/	34											

Table A-1. Strong-Motion Database

Earthquake Yr./Mo./Dy	Location	Mw	Fault Type	Station Design.	Location	Structure Code	Latitude	Longitude	Site Class	Alluvial Depth	Geologic Reference	Closest Approach Distance [km]	PGA [g]			Strong-Motion Data Source	Database Subsets
													H1	H2	V		
94.01.17	Northridge	6.7	R	24401	San Marino, Southwest Academy	1/	34.115° N	118.130° W	B	1.4	(13),(4),(39)	36.8	0.12	0.15	0.09	(2)	1, 2, 3, 4, 5, 6
94.01.17	Northridge	6.7	R	24592	Los Angeles, City Terrace	1/	34.053° N	118.171° W	B	NA	(4)	38.4	0.28	0.32	0.13	(2)	1, 3, 5
94.01.17	Northridge	6.7	R	24461	Alhambra, Fremont School	1/	34.070° N	118.150° W	B	1.5	(13),(4),(39)	38.5	0.10	0.08	0.05	(2)	1, 2, 3, 4, 5, 6
94.01.17	Northridge	6.7	R	24400	Los Angeles, Obregon Park	1/	34.037° N	118.178° W	B/C	4.3	(13),(4),(39)	39.2	0.35	0.41	0.11	(2)	1, 2
94.01.17	Northridge	6.7	R	14403	Los Angeles, 118th St School	1/	33.929° N	118.260° W	C	8.7	(13),(4),(39)	43.3	0.20	0.14	0.06	(2)	1, 2, 3, 4, 7, 8
94.01.17	Northridge	6.7	R	25282	Campanillo	1/	34.208° N	119.078° W	C	NA	(4)	43.5	0.12	0.12	0.05	(2)	1, 3, 7
94.01.17	Northridge	6.7	R	14568	Downey, County Maintenance Bldg	1/	33.924° N	118.167° W	C	9.1	(13),(4),(39)	48.7	0.18	0.22	0.13	(2)	1, 2, 3, 4, 7, 8
94.01.17	Northridge	6.7	R	25147	Point Mugu, Naval Air Station	1/	34.113° N	119.119° W	C	NA	(17),(4)	49.3	0.14	0.18	0.06	(2)	1, 3, 7
94.01.17	Northridge	6.7	R	24475	Lancaster, Fox Airfield Grounds	1/	34.739° N	118.214° W	C	NA	(4)	52.5	0.06	0.08	0.05	(2)	1, 3, 7
94.01.17	Northridge	6.7	R	24586	Neenach, Sacatara Creek	1/	34.848° N	118.536° W	C	NA	(4)	53.2	0.06	0.07	0.05	(2)	1, 3, 7
94.01.17	Northridge	6.7	R	14560	Long Beach, City Hall Grounds	1/	33.768° N	118.196° W	B	3.0	(4),(39)	60.0	0.04	0.05	0.02	(2)	1, 2, 3, 4, 5, 6
94.01.17	Northridge	6.7	R	25340	Ventura, Harbor and California	1/	34.276° N	119.293° W	C	NA	(4)	61.3	0.05	0.08	0.03	(2)	1, 3, 7
94.01.17	Northridge	6.7	R	14578	Seal Beach, Parking Lot	1/	33.757° N	118.084° W	C	NA	(6)	66.8	0.06	0.08	0.04	(2)	1, 3, 7
94.01.17	Northridge	6.7	R	13610	Newport Beach, Newport & Coast Hwy	1/	33.623° N	117.931° W	B/C	NA	(4)	86.7	0.11	0.08	0.02	(2)	1
94.01.17	Northridge	6.7	R	23597	Phelan, Wilson Ranch Road	1/	34.467° N	117.520° W	B/C	NA	(4)	87.2	0.05	0.06	0.04	(2)	1
94.01.17	Northridge	6.7	R	23672	San Bernardino, CSUSB Grounds	1/	34.183° N	117.321° W	B/C	NA	(4)	104.7	0.03	0.07	0.02	(2)	1
94.01.17	Northridge	6.7	R	23542	San Bernardino, E & Hospitality	1/	34.065° N	117.292° W	C	NA	(2)	109.9	0.08	0.10	0.04	(2)	1, 3, 7
94.01.17	Northridge	6.7	R	13680	Hemet, Ryan Airfield	1/	33.731° N	117.023° W	C	NA	(2),(4)	146.3	0.06	0.06	0.03	(2)	1, 3, 7
94.01.17	Northridge	6.7	R	12673	San Jacinto, CDF Fire Station	1/	33.787° N	116.958° W	C	NA	(4)	149.2	0.08	0.10	0.02	(2)	1, 3, 7

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Appendix B. Ground-Motion Equation Sets

The general form of the ground-motion model, as given in equation (2), is as follows.

$$\ln Y = p_1 + p_2 M + p_3 \ln (R + p_4 \exp \{p_5 M\}) + p_6 S + p_7 F + p_8 D \quad (2)$$

- where
- Y ≡ ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV)
 - M ≡ moment magnitude, M_w
 - R ≡ closest distance from the site to the fault rupture surface
 - S ≡ site classification code: $S = 0$ for site class B, $S = 1$ for site class C
 - F ≡ fault-type code: $F = 0$ for strike-slip, $F = 1$ for reverse
 - D ≡ alluvial depth
 - p_i ≡ regression parameters, $i = 1$ to 8

Different seismic hazard analysis scenarios may arise depending upon which, if any, of the proper values of the variables S , F , and D are known. To provide ground-motion equations appropriate for the different scenarios, 16 variations of equation (2) were developed. Table 3 is repeated here; it indicates the database subset which was utilized to develop each equation set, the hazard analysis scenarios for which it is applicable, and which (if any) of the terms of the general regression model are not present.

Table 3. Ground-Motion Equation Sets

Regression Equation Set No.	Database Subset No.	Applicable Ground-Motion Modeling Scenario							Term Present, Eqn. (2)			
		Site Class (within B-C range)			Fault-Type		Alluvial Depth		$p_6 S$	$p_7 F$	$p_8 D$	
		Unknown	Class B	Class C	Unknown	Known	Unknown	Known	n	y	n	y
1	1	◆		and	◆		and	◆		◆		◆
2	2	◆		and	◆		and	◆		◆		◆
3	1	◆		and		◆	and	◆		◆		◆
4	2	◆		and		◆	and	◆		◆		◆
5	3		(◆ or ◆)	and	◆		and	◆		◆		◆
6	4		(◆ or ◆)	and	◆		and	◆		◆		◆
7	3		(◆ or ◆)	and		◆	and	◆		◆		◆
8	4		(◆ or ◆)	and		◆	and	◆		◆		◆
9	5		◆	and	◆		and	◆		◆		◆
10	6		◆	and	◆		and	◆		◆		◆
11	5		◆	and		◆	and	◆		◆		◆
12	6		◆	and		◆	and	◆		◆		◆
13	7			◆ and	◆		and	◆		◆		◆
14	8			◆ and	◆		and	◆		◆		◆
15	7			◆ and		◆	and	◆		◆		◆
16	8			◆ and		◆	and	◆		◆		◆

Note: Database Subset No. refers to designation assigned in Table 2

Each equation set provides horizontal- and vertical-component ground-motion equations for PGA and for PSV at fourteen periods T in the band $0.04 \leq T \leq 4.00$ sec. The values of the parameters p_i in the general equation (2) to be used with each equation set are provided in the following pages, along with the standard errors of the regression.

Equation Set (1)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln (R + p_4^{c,T} \exp \{ p_5^{c,T} M \}) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: S = 0 for site class B, S = 1 for site class C
- F = fault-type code: F = 0 for strike-slip, F = 1 for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients						$\sigma_{\ln Y}^{H,T}$
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	
PGA	...	-1.826494	0.898703	-1.528388	1.805913	0.384652	...	0.500496
PSV(T, $\xi = 5\%$)	0.04	0.415852	0.828727	-1.529806	2.275710	0.335225	...	0.491588
	0.10	3.889594	0.811368	-1.928031	3.947718	0.319103	...	0.576350
	0.15	5.237534	0.762958	-1.992922	10.201714	0.188202	...	0.554117
	0.20	5.841993	0.726846	-1.962920	15.877590	0.134864	...	0.535507
	0.30	3.074889	0.889607	-1.566408	2.812896	0.327707	...	0.532991
	0.40	1.458764	0.966950	-1.311823	0.762335	0.456588	...	0.529098
	0.50	0.212295	1.102586	-1.220250	0.163758	0.649799	...	0.530063
	0.60	-0.418378	1.128076	-1.110254	0.092720	0.695204	...	0.545546
	0.80	-0.951663	1.189240	-1.068707	0.045227	0.795371	...	0.579128
	1.00	-1.246766	1.237005	-1.051439	0.033744	0.865451	...	0.574562
	1.50	-2.474145	1.329566	-0.938841	0.011997	0.954221	...	0.631785
	2.00	-4.376082	1.617826	-0.986933	0.003486	1.140356	...	0.660365
	3.00	-6.300801	1.822044	-0.905630	0.002038	1.172921	...	0.731170
PSV(T, $\xi = 5\%$)	4.00	-7.049439	1.860690	-0.825680	0.002028	1.138160	...	0.755865

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients						$\sigma_{\ln Y}^{V,T}$
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	
PGA	...	-0.420255	0.567468	-1.523217	64.413473	-0.230552	...	0.608076
PSV(T, $\xi = 5\%$)	0.04	1.086234	0.655359	-1.563409	29.353282	-0.135606	...	0.686514
	0.10	3.476641	0.800828	-1.958642	12.147261	0.072498	...	0.701957
	0.15	3.856017	0.579190	-1.601048	51.714540	-0.172180	...	0.675994
	0.20	3.516854	0.523822	-1.390493	103.403636	-0.295485	...	0.646573
	0.30	2.570071	0.579469	-1.182356	33.776868	-0.110130	...	0.642234
	0.40	0.944152	0.748781	-1.044290	8.006099	0.068917	...	0.625417
	0.50	-0.214903	0.884953	-0.970706	3.010809	0.209564	...	0.594918
	0.60	-1.731805	1.046352	-0.847555	0.041267	0.819865	...	0.631281
	0.80	-1.403330	0.899436	-0.701899	0.024722	0.754776	...	0.636414
	1.00	-2.181922	1.055373	-0.743692	0.014854	0.868208	...	0.622854
	1.50	-4.355019	1.487144	-0.896605	0.007058	1.108591	...	0.609350
	2.00	-4.333284	1.423001	-0.844245	0.010587	0.983350	...	0.651698
	3.00	-6.271883	1.698933	-0.831494	0.014414	0.954196	...	0.765945
PSV(T, $\xi = 5\%$)	4.00	-7.202396	1.776405	-0.771832	0.012625	0.921229	...	0.826970

Equation Set (2)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln (R + p_4^{c,T} \exp \{ p_5^{c,T} M \}) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: S = 0 for site class B, S = 1 for site class C
- F = fault-type code: F = 0 for strike-slip, F = 1 for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{\ln Y}^{H,T}$
PGA	...	-0.925593	1.116971	-2.011655	1.877203	0.457525	0.047708	0.458657
PSV(T, $\xi = 5\%$)	0.04	1.082789	1.092776	-2.016675	1.931618	0.449201	0.050021	0.452815
:	0.10	5.004488	1.323057	-2.789442	2.926544	0.456240	0.046876	0.517700
:	0.15	7.228633	1.088598	-2.769144	6.941401	0.332718	0.042427	0.521840
:	0.20	9.184486	1.268612	-3.214297	10.879958	0.312801	0.047817	0.508698
:	0.30	4.763761	1.052274	-2.095003	3.981025	0.363815	0.033463	0.477981
:	0.40	2.588286	0.980640	-1.560662	1.371389	0.446447	0.045827	0.485773
:	0.50	1.409793	1.119591	-1.485708	1.019778	0.474518	0.062125	0.456288
:	0.60	0.606499	1.125760	-1.332519	0.887478	0.453334	0.067838	0.468326
:	0.80	-0.205291	1.183967	-1.230206	0.349114	0.565715	0.059224	0.471848
:	1.00	-1.857786	1.452333	-1.221642	0.049229	0.894565	0.050016	0.417780
:	1.50	-3.252566	1.455728	-0.944487	0.006746	1.087308	0.050871	0.475660
:	2.00	-4.236469	1.536262	-0.877410	0.003793	1.128387	0.061122	0.525022
:	3.00	-6.112303	1.750267	-0.828712	0.002839	1.140328	0.065419	0.594884
PSV(T, $\xi = 5\%$)	4.00	-6.669831	1.707610	-0.660340	0.002664	1.068951	0.073473	0.635562

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{\ln Y}^{V,T}$
PGA	...	1.152136	0.895727	-2.283947	10.810002	0.166843	0.014882	0.571390
PSV(T, $\xi = 5\%$)	0.04	2.634447	0.895512	-2.250953	8.341168	0.159267	0.022771	0.619068
:	0.10	5.399237	1.438647	-3.157725	4.493495	0.355489	0.024786	0.641381
:	0.15	6.078216	1.071609	-2.681960	8.804304	0.251388	0.041648	0.586767
:	0.20	6.514754	1.072366	-2.679984	11.127699	0.239038	0.023149	0.572392
:	0.30	4.829629	0.605686	-1.706696	12.907796	0.129995	0.011077	0.512384
:	0.40	2.772716	0.880077	-1.599645	5.265655	0.268893	-0.007752	0.533358
:	0.50	1.251533	0.818546	-1.186216	6.443529	0.161247	-0.014986	0.503701
:	0.60	0.464308	0.869439	-1.087370	1.182470	0.396176	-0.013204	0.509657
:	0.80	-1.149074	1.101089	-1.059543	0.039830	0.891801	-0.032011	0.483399
:	1.00	-1.906826	1.105279	-0.853945	0.017349	0.946270	-0.045880	0.481034
:	1.50	-2.869474	1.120952	-0.672083	0.002232	1.095861	-0.025317	0.507517
:	2.00	-3.825313	1.269568	-0.729762	0.008178	0.962295	0.014824	0.551933
:	3.00	-5.403820	1.475101	-0.678949	0.016319	0.901165	0.036935	0.680365
PSV(T, $\xi = 5\%$)	4.00	-7.045648	1.654034	-0.611025	0.005226	0.992985	0.044167	0.775762

Equation Set (3)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln(R + p_4^{c,T} \exp\{p_5^{c,T}M\}) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y ≡ ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M ≡ moment magnitude, M_w
- R ≡ closest distance from site to fault rupture surface in km
- S ≡ site classification code: S = 0 for site class B, S = 1 for site class C
- F ≡ fault-type code: F = 0 for strike-slip, F = 1 for reverse
- D ≡ depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{ln Y}^{H,T}$
PGA	...	-2.973118	0.958510	-1.418787	0.553905	0.517553	...	0.315526	...	0.478714
PSV(T, $\xi = 5\%$)	0.04	-0.635901	0.895135	-1.442779	0.782726	0.458198	...	0.285575	...	0.473636
	0.10	2.353312	0.866985	-1.746517	1.494712	0.418542	...	0.330451	...	0.555963
	0.15	3.537834	0.832342	-1.796019	4.376252	0.269973	...	0.297057	...	0.537258
	0.20	4.468647	0.879321	-1.933220	7.654564	0.227807	...	0.293602	...	0.517855
	0.30	1.819319	0.948682	-1.434008	0.868946	0.455707	...	0.300989	...	0.514802
	0.40	0.578677	1.030081	-1.252742	0.195845	0.622161	...	0.295721	...	0.511218
	0.50	0.166936	1.020238	-1.126046	0.137744	0.613623	...	0.296822	...	0.511627
	0.60	-0.441868	1.037171	-1.008007	0.031536	0.771329	...	0.301932	...	0.528284
	0.80	-0.689891	1.064474	-0.978399	0.041903	0.723009	...	0.290644	...	0.563296
	1.00	-1.371869	1.244671	-1.054250	0.033576	0.854741	...	0.154686	...	0.570128
	1.50	-2.501831	1.333911	-0.941424	0.012339	0.949305	...	0.022116	...	0.632752
2.00	-4.290447	1.608647	-0.989652	0.004231	1.115607	...	-0.035913	...	0.663050	
3.00	-6.047354	1.813332	-0.936756	0.003538	1.119950	...	-0.125432	...	0.730024	
PSV(T, $\xi = 5\%$)	4.00	-6.874815	1.837996	-0.815348	0.002453	1.115448	...	-0.141722	...	0.754417

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{ln Y}^{V,T}$
PGA	...	-1.466121	0.689488	-1.506439	24.802573	-0.104816	...	0.258775	...	0.596982
PSV(T, $\xi = 5\%$)	0.04	-0.046408	0.799312	-1.561229	8.240786	0.041384	...	0.289366	...	0.674300
	0.10	1.760928	0.968362	-1.886887	3.569343	0.225988	...	0.393451	...	0.678612
	0.15	2.060786	0.772517	-1.549102	11.621561	0.017530	...	0.421300	...	0.647763
	0.20	1.974263	0.693616	-1.354106	26.300197	-0.124369	...	0.396846	...	0.620281
	0.30	0.834308	0.744055	-1.106999	4.989823	0.114100	...	0.451235	...	0.607422
	0.40	-0.211548	0.876085	-1.027099	1.304037	0.302065	...	0.390883	...	0.598705
	0.50	-1.426904	1.032462	-0.967958	0.225565	0.565802	...	0.386362	...	0.567428
	0.60	-1.691264	0.959589	-0.780651	0.020540	0.820167	...	0.413754	...	0.599416
	0.80	-1.838584	0.963794	-0.750940	0.023557	0.748164	...	0.428735	...	0.599963
	1.00	-2.498612	1.101946	-0.778979	0.014676	0.858907	...	0.311232	...	0.604093
	1.50	-4.632581	1.518225	-0.896530	0.003976	1.180458	...	0.132974	...	0.605124
2.00	-4.381900	1.431232	-0.849722	0.011116	0.975125	...	0.037943	...	0.652444	
3.00	-6.245191	1.696495	-0.828662	0.015736	0.942859	...	-0.037438	...	0.767560	
PSV(T, $\xi = 5\%$)	4.00	-7.147568	1.769768	-0.764497	0.013939	0.909260	...	-0.074572	...	0.828362

Equation Set (4)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln (R + p_4^{c,T} \exp \{ p_5^{c,T} M \}) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: S = 0 for site class B, S = 1 for site class C
- F = fault-type code: F = 0 for strike-slip, F = 1 for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{ln Y}^{H,T}$
PGA	...	-1.960277	1.021393	-1.731595	1.227342	0.458359	...	0.238326	0.043569	0.448174
PSV(T, $\xi = 5\%$)	0.04	-0.031636	0.997907	-1.723962	1.216450	0.451377	...	0.259392	0.045536	0.439811
	0.10	3.297923	1.139206	-2.303488	1.880561	0.457264	...	0.263310	0.042407	0.506628
	0.15	5.283693	0.970835	-2.315736	4.694957	0.332737	...	0.211927	0.038933	0.515572
	0.20	6.239964	1.069607	-2.513069	7.157238	0.304225	...	0.212421	0.044055	0.504250
	0.30	3.311867	0.951260	-1.734332	2.482151	0.361226	...	0.230472	0.029180	0.469199
	0.40	1.783361	0.940439	-1.381161	0.790918	0.465951	...	0.222735	0.042029	0.477410
	0.50	0.624709	1.086751	-1.320682	0.552067	0.499784	...	0.253467	0.057972	0.443688
	0.60	-0.143094	1.113835	-1.200014	0.456611	0.480093	...	0.281853	0.063294	0.452547
	0.80	-0.656123	1.166387	-1.145141	0.247885	0.557262	...	0.254024	0.055221	0.459254
	1.00	-2.004512	1.421215	-1.166965	0.036341	0.909203	...	0.124965	0.048009	0.415449
	1.50	-3.131335	1.474613	-0.983956	0.008569	1.087556	...	-0.109444	0.052898	0.474631
	2.00	-3.878540	1.500693	-0.869760	0.005468	1.108444	...	-0.300950	0.064036	0.508515
	3.00	-5.550138	1.681191	-0.812373	0.007749	1.026301	...	-0.343696	0.068632	0.578424
PSV(T, $\xi = 5\%$)	4.00	-6.323197	1.679342	-0.651413	0.004152	1.086283	...	-0.359481	0.077406	0.616248

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{ln Y}^{V,T}$
PGA	...	0.436427	0.870379	-2.135827	10.347691	0.143971	...	0.138384	0.012434	0.570163
PSV(T, $\xi = 5\%$)	0.04	2.265602	0.888983	-2.183771	8.185176	0.146035	...	0.097932	0.021190	0.619735
	0.10	4.477458	1.358108	-2.913281	3.927480	0.346932	...	0.138209	0.022259	0.640882
	0.15	4.461552	0.978435	-2.303463	7.213977	0.224701	...	0.219484	0.037446	0.581157
	0.20	4.247641	0.744983	-1.892818	13.385628	0.093196	...	0.205382	0.014260	0.570750
	0.30	3.634259	0.568296	-1.461322	12.825104	0.056164	...	0.241970	0.006301	0.503001
	0.40	2.264642	0.845202	-1.473512	4.850673	0.245067	...	0.104319	-0.009831	0.533337
	0.50	0.498748	0.800597	-1.041802	7.411392	0.052896	...	0.251045	-0.019889	0.492800
	0.60	-0.368418	0.848713	-0.929327	0.605519	0.373632	...	0.339558	-0.019471	0.488047
	0.80	-1.413291	1.010911	-0.921639	0.018309	0.889328	...	0.332785	-0.038584	0.461304
	1.00	-2.096829	1.103633	-0.824906	0.011375	0.968928	...	0.140340	-0.048150	0.476676
	1.50	-2.692949	1.098252	-0.664876	0.004207	1.023370	...	-0.109709	-0.024254	0.507001
	2.00	-3.702465	1.257321	-0.709443	0.003725	1.110361	...	-0.218134	0.016386	0.541946
	3.00	-5.780729	1.609142	-0.750101	0.012615	1.045014	...	-0.407816	0.043784	0.649770
PSV(T, $\xi = 5\%$)	4.00	-6.638539	1.611389	-0.608355	0.008560	0.980659	...	-0.294329	0.047485	0.766518

Equation Set (5)

$$\ln Y^{aT} = p_1^{aT} + p_2^{aT}M + p_3^{aT} \ln(R + p_4^{aT} \exp\{p_5^{aT}M\}) + p_6^{aT}S + p_7^{aT}F + p_8^{aT}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: S = 0 for site class B, S = 1 for site class C
- F = fault-type code: F = 0 for strike-slip, F = 1 for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients						$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{ln Y}^{H,T}$
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$			
PGA	...	-2.787721	1.032112	-1.534951	0.717696	0.530994	0.210306	0.476146
PSV(T, $\xi = 5\%$)	0.04	-0.599470	0.961561	-1.521269	0.837291	0.491723	0.197585	0.467455
	0.10	2.849412	1.004369	-1.994636	1.729899	0.462844	0.274861	0.547736
	0.15	4.748527	0.938137	-2.138764	5.774321	0.302408	0.259702	0.525700
	0.20	6.254352	1.042144	-2.428963	9.798588	0.274104	0.233821	0.507596
	0.30	1.988346	1.012084	-1.536163	1.343317	0.434903	0.214532	0.500580
	0.40	0.665723	1.066675	-1.311833	0.309477	0.596545	0.228421	0.516513
	0.50	0.108715	1.044495	-1.144128	0.164764	0.623111	0.255154	0.526697
	0.60	-1.093715	1.218170	-1.118899	0.050525	0.797242	0.228446	0.536233
	0.80	-0.867446	1.093852	-0.975996	0.048238	0.741039	0.244843	0.573700
	1.00	-1.820460	1.313539	-1.058491	0.036297	0.868850	0.237031	0.565670
	1.50	-3.096112	1.417525	-0.951799	0.012963	0.965539	0.237982	0.620702
	2.00	-5.284766	1.834076	-1.127883	0.005414	1.169954	0.365500	0.635486
	3.00	-6.647735	1.918545	-1.015715	0.005007	1.110406	0.383238	0.712264
PSV(T, $\xi = 5\%$)	4.00	-7.471061	1.948893	-0.899025	0.005526	1.070108	0.362112	0.738777

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients						$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{ln Y}^{V,T}$
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$			
PGA	...	-1.017423	0.663159	-1.563180	35.980089	-0.131541	0.268116	0.598805
PSV(T, $\xi = 5\%$)	0.04	0.883645	0.699459	-1.635801	31.454772	-0.135981	0.387184	0.665080
	0.10	3.405820	0.801140	-1.991944	14.419155	0.047112	0.330342	0.689820
	0.15	3.427922	0.638985	-1.639597	34.311269	-0.103102	0.331214	0.656969
	0.20	3.178900	0.599475	-1.458262	62.640531	-0.205290	0.227204	0.629254
	0.30	1.886921	0.688969	-1.210684	12.637607	0.047933	0.127228	0.632620
	0.40	0.309031	0.878118	-1.097943	2.098725	0.300157	0.061585	0.615605
	0.50	-0.755943	0.968205	-0.978550	1.265573	0.343607	0.024359	0.599089
	0.60	-1.986970	1.087306	-0.860841	0.017564	0.954548	0.038759	0.637193
	0.80	-1.341437	0.916909	-0.745289	0.025140	0.794821	-0.035566	0.641046
	1.00	-2.120266	1.077798	-0.788798	0.014581	0.922117	-0.047609	0.621221
	1.50	-3.313020	1.265139	-0.824610	0.011732	0.965734	0.081809	0.620567
	2.00	-4.545243	1.497095	-0.921039	0.011247	1.026582	0.124268	0.653182
	3.00	-6.552699	1.783845	-0.906212	0.015430	0.999277	0.144260	0.766232
PSV(T, $\xi = 5\%$)	4.00	-7.392247	1.799476	-0.778070	0.013515	0.931120	0.147589	0.834813

Equation Set (6)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln (R + p_4^{c,T} \exp \{ p_5^{c,T} M \}) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: S = 0 for site class B, S = 1 for site class C
- F = fault-type code: F = 0 for strike-slip, F = 1 for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{\ln Y}^{H,T}$
PGA	...	-1.895854	1.159895	-1.906878	1.184828	0.508597	0.158566	...	0.039787	0.437772
PSV(T, $\xi = 5\%$)	0.04	0.117335	1.133274	-1.908447	1.210491	0.500865	0.151692	...	0.042350	0.433445
:	0.10	3.749339	1.408525	-2.698676	1.968552	0.506723	0.252957	...	0.037649	0.493222
:	0.15	6.275421	1.161677	-2.720033	5.498722	0.362696	0.287764	...	0.030977	0.491920
:	0.20	8.110289	1.338022	-3.137898	8.920299	0.336644	0.241695	...	0.038122	0.486423
:	0.30	3.551440	1.096635	-1.948918	2.421452	0.415324	0.157746	...	0.024862	0.458351
:	0.40	1.927179	1.014568	-1.500978	0.898598	0.495706	0.195379	...	0.037162	0.472384
:	0.50	0.978224	1.132263	-1.433145	0.746797	0.506893	0.099792	...	0.058614	0.461113
:	0.60	0.123009	1.147509	-1.272629	0.532229	0.509609	0.040735	...	0.065428	0.474686
:	0.80	-0.559846	1.207822	-1.199266	0.234712	0.612466	0.048375	...	0.057421	0.478584
:	1.00	-2.263159	1.494407	-1.208334	0.031866	0.955805	0.088997	...	0.048097	0.421450
:	1.50	-3.327187	1.466315	-0.966685	0.012328	1.005897	0.129478	...	0.048211	0.476798
:	2.00	-4.564634	1.544654	-0.861043	0.003402	1.132342	0.328478	...	0.053100	0.503742
:	3.00	-6.271920	1.706041	-0.788675	0.005418	1.003499	0.452452	...	0.056882	0.564915
PSV(T, $\xi = 5\%$)	4.00	-6.817475	1.690330	-0.659633	0.004659	0.984982	0.426588	...	0.066450	0.610257

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{\ln Y}^{V,T}$
PGA	...	0.478575	0.962048	-2.261099	8.275468	0.203742	0.170791	...	0.009990	0.576233
PSV(T, $\xi = 5\%$)	0.04	2.021977	0.975973	-2.269520	6.212511	0.207487	0.291655	...	0.016067	0.614969
:	0.10	4.800225	1.550978	-3.214835	3.711393	0.390121	0.290267	...	0.017959	0.638745
:	0.15	5.552711	1.095756	-2.631953	7.660410	0.267173	0.140272	...	0.036343	0.595166
:	0.20	5.715579	1.018200	-2.479719	10.431655	0.225780	0.026048	...	0.017690	0.568293
:	0.30	4.352338	0.577076	-1.570616	13.100457	0.094328	-0.133538	...	0.008807	0.504904
:	0.40	2.250287	0.855143	-1.459349	4.427120	0.259042	-0.136423	...	-0.008975	0.530693
:	0.50	1.023721	0.799708	-1.094297	6.406105	0.120518	-0.231852	...	-0.013138	0.494612
:	0.60	0.478709	0.813131	-0.991258	1.158308	0.350032	-0.234438	...	-0.012007	0.499304
:	0.80	-0.987655	1.041056	-0.991241	0.032814	0.879558	-0.218572	...	-0.030796	0.473957
:	1.00	-1.837322	1.093054	-0.839372	0.016433	0.934871	-0.132996	...	-0.045043	0.479322
:	1.50	-2.930992	1.112075	-0.669749	0.004734	0.965576	0.151251	...	-0.029185	0.504181
:	2.00	-3.726446	1.230340	-0.708449	0.003878	1.059438	0.155064	...	0.010191	0.549557
:	3.00	-5.614011	1.484269	-0.682160	0.017123	0.897857	0.259376	...	0.034858	0.673793
PSV(T, $\xi = 5\%$)	4.00	-7.182161	1.657145	-0.627111	0.006338	0.984802	0.279164	...	0.041492	0.773138

Equation Set (7)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln (R + p_4^{c,T} \exp \{ p_5^{c,T} M \}) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: S = 0 for site class B, S = 1 for site class C
- F = fault-type code: F = 0 for strike-slip, F = 1 for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{ln Y}^{H,T}$
PGA	...	-3.682981	1.048296	-1.419542	0.342687	0.593107	0.231204	0.307178	...	0.456027
PSV(T, $\xi = 5\%$)	0.04	-1.444034	0.989498	-1.427005	0.420028	0.552299	0.217216	0.286451	...	0.449758
	0.10	1.452624	1.006257	-1.776129	0.789506	0.527877	0.296161	0.321017	...	0.529116
	0.15	2.947740	0.967142	-1.876925	2.674755	0.365764	0.277498	0.273850	...	0.512118
	0.20	3.797223	1.036507	-2.018345	4.578807	0.326014	0.250514	0.263253	...	0.494792
	0.30	0.969349	1.034973	-1.405230	0.624128	0.496075	0.235313	0.308876	...	0.481481
	0.40	-0.020076	1.097554	-1.252242	0.150858	0.660515	0.249941	0.307720	...	0.497955
	0.50	-0.959503	1.217197	-1.206966	0.055196	0.784470	0.281880	0.339516	...	0.502022
	0.60	-1.595945	1.246869	-1.095264	0.030839	0.825545	0.253590	0.344335	...	0.513415
	0.80	-1.302631	1.137974	-0.981676	0.031521	0.772832	0.268046	0.311102	...	0.554858
	1.00	-2.181092	1.351909	-1.057922	0.021732	0.928495	0.250440	0.184402	...	0.558472
	1.50	-4.344454	1.670077	-1.051580	0.007951	1.086046	0.247649	0.069048	...	0.615469
	2.00	-5.304621	1.747688	-1.000325	0.002976	1.197313	0.362098	-0.004484	...	0.636919
	3.00	-6.622133	1.925991	-1.019458	0.006357	1.090327	0.377962	-0.074118	...	0.713194
PSV(T, $\xi = 5\%$)	4.00	-6.439521	1.742149	-0.803943	0.005855	1.006544	0.347583	-0.128097	...	0.744734

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{ln Y}^{V,T}$
PGA	...	-2.219481	0.787144	-1.522146	16.800628	-0.046877	0.283754	0.289824	...	0.585491
PSV(T, $\xi = 5\%$)	0.04	-0.396054	0.844219	-1.611074	12.837068	-0.028035	0.405532	0.334279	...	0.648936
	0.10	1.612459	0.957945	-1.894247	5.860012	0.141872	0.351950	0.413471	...	0.665353
	0.15	1.533558	0.822602	-1.558951	11.272702	0.015681	0.354912	0.444947	...	0.626649
	0.20	1.542334	0.752164	-1.380485	26.551342	-0.128634	0.247691	0.387630	...	0.605605
	0.30	0.265437	0.814392	-1.106173	3.793672	0.147029	0.149662	0.441099	...	0.601444
	0.40	-0.640059	0.955510	-1.052736	0.862049	0.380680	0.080000	0.348630	...	0.595937
	0.50	-1.648846	1.046999	-0.948643	0.488990	0.427148	0.044578	0.379218	...	0.574644
	0.60	-2.521400	1.135271	-0.869586	0.017417	0.903728	0.063751	0.433557	...	0.605228
	0.80	-2.559682	1.127003	-0.834868	0.015204	0.883060	-0.005826	0.448799	...	0.606628
	1.00	-2.545782	1.127890	-0.806318	0.014951	0.882006	-0.029397	0.310008	...	0.604193
	1.50	-4.515074	1.543483	-0.990903	0.005865	1.148640	0.096328	0.155889	...	0.611949
	2.00	-4.591737	1.501827	-0.921105	0.012261	1.009155	0.126165	0.028280	...	0.654355
	3.00	-6.507521	1.780567	-0.904163	0.017762	0.981797	0.141697	-0.050790	...	0.768139
PSV(T, $\xi = 5\%$)	4.00	-8.402034	2.009070	-0.867110	0.008660	1.050751	0.151746	-0.025302	...	0.829157

Equation Set (8)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln (R + p_4^{c,T} \exp \{ p_5^{c,T} M \}) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: S = 0 for site class B, S = 1 for site class C
- F = fault-type code: F = 0 for strike-slip, F = 1 for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{\ln Y}$
PGA	...	-2.744812	1.077356	-1.678289	0.775010	0.514537	0.192870	0.245794	0.035211	0.425829
PSV(T, $\xi = 5\%$)	0.04	-0.791941	1.051772	-1.671470	0.766266	0.508120	0.189106	0.266159	0.037408	0.418894
:	0.10	2.212340	1.221897	-2.242692	1.209446	0.514127	0.290996	0.290450	0.032296	0.478475
:	0.15	4.269983	1.034441	-2.249298	3.429838	0.369431	0.319532	0.248123	0.026500	0.481907
:	0.20	5.209203	1.134796	-2.440668	5.495904	0.334676	0.269727	0.237064	0.033619	0.477636
:	0.30	2.436908	1.011586	-1.668167	1.532734	0.418415	0.188880	0.233305	0.020243	0.448711
:	0.40	1.204089	0.974111	-1.340593	0.512187	0.517249	0.230928	0.241411	0.032603	0.461953
:	0.50	0.251978	1.104080	-1.290327	0.409234	0.532254	0.141385	0.273709	0.053708	0.446437
:	0.60	-0.516174	1.139763	-1.171566	0.292395	0.533667	0.086702	0.288917	0.060356	0.458439
:	0.80	-0.973020	1.193942	-1.130911	0.172925	0.603576	0.089939	0.258804	0.052997	0.465927
:	1.00	-2.395634	1.464671	-1.160501	0.023992	0.969556	0.108630	0.128380	0.045880	0.419081
:	1.50	-3.304606	1.471783	-0.961716	0.010443	1.042147	0.112105	-0.107861	0.049239	0.477426
:	2.00	-4.184121	1.501460	-0.847053	0.008202	1.022173	0.285184	-0.256523	0.056028	0.494719
:	3.00	-6.030547	1.705080	-0.804609	0.005309	1.068762	0.408600	-0.259210	0.060866	0.551307
PSV(T, $\xi = 5\%$)	4.00	-6.568532	1.674798	-0.652718	0.003166	1.104478	0.378852	-0.281094	0.070104	0.598142

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{\ln Y}$
PGA	...	-0.265110	0.930956	-2.105163	7.844418	0.179499	0.193771	0.163279	0.006873	0.573918
PSV(T, $\xi = 5\%$)	0.04	1.505604	0.960918	-2.171684	6.040860	0.188147	0.314012	0.150697	0.013347	0.613640
:	0.10	3.625903	1.435420	-2.892284	3.071738	0.380715	0.314431	0.190954	0.014179	0.635812
:	0.15	3.956292	0.999208	-2.257056	6.209988	0.239944	0.171634	0.244273	0.031345	0.587972
:	0.20	4.468856	0.913391	-2.149757	10.028279	0.179664	0.045220	0.164865	0.013609	0.566144
:	0.30	3.573333	0.568035	-1.434275	12.893824	0.047586	-0.105310	0.190456	0.005107	0.499734
:	0.40	2.040653	0.845497	-1.414469	4.250864	0.249483	-0.128537	0.055110	-0.010047	0.532161
:	0.50	0.532146	0.804787	-1.027427	6.789834	0.059363	-0.200567	0.194075	-0.016713	0.488746
:	0.60	-0.161128	0.826508	-0.915606	0.709383	0.342336	-0.186441	0.290147	-0.017125	0.483774
:	0.80	-1.284679	1.005764	-0.919576	0.019391	0.876831	-0.171387	0.291246	-0.036236	0.455442
:	1.00	-2.018028	1.097218	-0.818204	0.011350	0.961438	-0.115265	0.105115	-0.047030	0.478718
:	1.50	-2.819066	1.116179	-0.686540	0.003078	1.087299	0.132352	-0.107085	-0.027010	0.500927
:	2.00	-3.950598	1.290066	-0.718690	0.002947	1.131886	0.119099	-0.214437	0.013467	0.539929
:	3.00	-6.027019	1.632305	-0.762058	0.013231	1.037168	0.197880	-0.375626	0.041843	0.647313
PSV(T, $\xi = 5\%$)	4.00	-6.836353	1.615381	-0.607479	0.009157	0.961684	0.236317	-0.252200	0.044164	0.764499

Equation Set (9)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln (R + p_4^{c,T} \exp \{ p_5^{c,T} M \}) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: $S = 0$ for site class B, $S = 1$ for site class C
- F = fault-type code: $F = 0$ for strike-slip, $F = 1$ for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{\ln Y}^{H,T}$
PGA	...	-2.788816	1.083898	-1.664933	0.707328	0.495853	0.512829
PSV($T, \xi = 5\%$)	0.04	-1.131621	1.064646	-1.603504	0.482076	0.534007	0.500078
	0.10	1.922701	1.280780	-2.219585	0.750588	0.577036	0.573296
	0.15	4.038685	1.273050	-2.472638	4.053933	0.358246	0.557202
	0.20	4.769454	1.042985	-2.229934	8.803985	0.226976	0.536325
	0.30	0.379647	1.203289	-1.508531	0.394318	0.563735	0.544030
	0.40	1.267698	0.992687	-1.387504	1.238820	0.333826	0.560330
	0.50	-0.260832	1.193668	-1.335564	0.091568	0.684649	0.594013
	0.60	0.322824	1.013788	-1.171220	10.250271	-0.063997	0.620264
	0.80	0.778887	0.805584	-0.944971	441.348158	-0.891756	0.706759
	1.00	-2.180062	1.368885	-1.085105	0.026836	0.864184	0.635404
	1.50	-2.906396	1.349051	-0.904186	0.023762	0.802783	0.684178
2.00	-3.915624	1.484747	-0.925547	0.021264	0.825384	0.702187	
3.00	-5.599219	1.649287	-0.813291	0.022213	0.810870	0.770092	
PSV($T, \xi = 5\%$)	4.00	-6.504493	1.721184	-0.728078	0.054943	0.696101	0.743540

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{\ln Y}^{V,T}$
PGA	...	-1.908120	0.784642	-1.578886	60.541179	-0.254580	0.561273
PSV($T, \xi = 5\%$)	0.04	-0.993340	1.011749	-1.698632	2.940387	0.210184	0.605406
	0.10	3.696014	0.902516	-2.207327	24.862105	-0.021694	0.646358
	0.15	2.769831	0.946909	-1.963181	15.230470	0.037613	0.600471
	0.20	3.144321	0.488987	-1.339467	1020.727462	-0.736072	0.562656
	0.30	2.906451	0.856123	-1.686463	26.650402	-0.006950	0.648353
	0.40	0.187352	1.003687	-1.305215	1.941374	0.289952	0.617719
	0.50	0.782077	0.813542	-1.142128	108.207463	-0.363402	0.646774
	0.60	-2.096018	1.266625	-1.128617	0.072995	0.767891	0.675332
	0.80	-2.218989	1.271748	-1.101685	0.058953	0.767712	0.672424
	1.00	-3.531512	1.503780	-1.095060	0.025170	0.954013	0.665477
	1.50	-3.388237	1.435326	-1.070858	0.030044	0.896974	0.586906
2.00	-4.196881	1.463912	-0.969876	0.021912	0.882104	0.587994	
3.00	-6.661736	1.798592	-0.900619	0.024747	0.873355	0.740270	
PSV($T, \xi = 5\%$)	4.00	-7.572208	1.841813	-0.776934	0.057415	0.715533	0.774544

Equation Set (10)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln(R + p_4^{c,T} \exp\{p_5^{c,T}M\}) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g ; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: $S = 0$ for site class B, $S = 1$ for site class C
- F = fault-type code: $F = 0$ for strike-slip, $F = 1$ for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{ln Y}^{H,T}$
PGA	...	-2.975972	1.227410	-1.826005	0.380017	0.653480	0.088530	0.521704
PSV($T, \xi = 5\%$)	0.04	-1.186804	1.206466	-1.789849	0.330341	0.664250	0.089930	0.514579
	0.10	3.558792	1.440038	-2.707265	1.728860	0.531916	0.083526	0.579104
	0.15	3.318964	1.731238	-2.910269	1.228889	0.594872	0.097973	0.553521
	0.20	3.515965	1.832299	-2.975927	1.318012	0.597221	0.105846	0.538029
	0.30	2.322878	1.250004	-1.965869	0.610081	0.608388	0.075645	0.538924
	0.40	1.113333	1.111242	-1.515351	0.282792	0.649108	0.077654	0.508098
	0.50	-0.485406	1.345685	-1.491065	0.091843	0.801033	0.123208	0.479628
	0.60	-0.562702	1.165825	-1.194658	0.110654	0.702911	0.101819	0.543346
	0.80	-0.083439	0.984456	-1.009230	0.239936	0.518725	0.080042	0.578846
	1.00	-2.282153	1.491504	-1.220639	0.034404	0.935634	0.071760	0.460305
	1.50	-2.725069	1.424097	-1.007494	0.027306	0.939001	0.039470	0.483458
2.00	-3.815055	1.515648	-0.952742	0.015385	0.999464	0.045058	0.508216	
3.00	-5.105444	1.576954	-0.808169	0.046226	0.789838	0.041625	0.634938	
PSV($T, \xi = 5\%$)	4.00	-6.354649	1.832419	-0.922558	0.030480	0.939452	0.041799	0.697312

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{ln Y}^{V,T}$
PGA	...	-1.745624	1.258930	-2.238205	1.681324	0.445083	0.073718	0.524466
PSV($T, \xi = 5\%$)	0.04	-0.074963	1.247277	-2.206172	1.080728	0.479615	0.062950	0.607714
	0.10	3.037641	1.576617	-2.920754	1.319271	0.533158	0.078502	0.579639
	0.15	1.159517	2.263270	-3.303948	0.911895	0.628358	0.101764	0.479091
	0.20	1.685676	1.628569	-2.552692	1.430087	0.513860	0.098227	0.465015
	0.30	3.954387	1.038534	-2.095539	2.979234	0.406017	0.055404	0.548947
	0.40	1.651880	1.087724	-1.687173	1.703544	0.429518	0.032979	0.667411
	0.50	0.783063	1.191835	-1.634659	1.410876	0.446215	0.037507	0.638982
	0.60	-0.437303	1.573796	-1.861511	0.474193	0.663463	0.031699	0.527261
	0.80	-1.696306	1.388415	-1.364766	0.111526	0.776249	-0.000778	0.515933
	1.00	-3.597728	1.876025	-1.603209	0.042114	0.989815	0.018430	0.528956
	1.50	-2.515605	1.323412	-1.098184	0.033257	0.901461	0.009161	0.461807
2.00	-3.957006	1.484074	-1.034928	0.021273	0.943302	0.009717	0.534376	
3.00	-6.225355	1.803000	-1.015070	0.050851	0.824879	0.007102	0.746333	
PSV($T, \xi = 5\%$)	4.00	-8.035118	2.206232	-1.214621	0.066910	0.865049	0.021768	0.858834

Equation Set (11)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln(R + p_4^{c,T} \exp(p_5^{c,T}M)) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA),
or peak 5 percent-damped pseudo-spectral velocity (PSV);
units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: $S = 0$ for site class B, $S = 1$ for site class C
- F = fault-type code: $F = 0$ for strike-slip, $F = 1$ for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{\ln Y}^{H,T}$
PGA	...	-3.229398	1.058631	-1.569282	0.555772	0.502083	...	0.246531	...	0.500355
PSV($T, \xi = 5\%$)	0.04	-1.493416	1.039559	-1.522169	0.395433	0.535196	...	0.240741	...	0.487849
:	0.10	1.308554	1.183205	-2.013513	0.585070	0.577923	...	0.291273	...	0.560151
:	0.15	2.795351	1.245220	-2.233571	2.664349	0.388011	...	0.236726	...	0.547434
:	0.20	3.517292	1.053323	-2.030828	5.764815	0.258966	...	0.205956	...	0.529178
:	0.30	-0.159937	1.157771	-1.370789	0.229350	0.592318	...	0.309128	...	0.524558
:	0.40	0.763372	1.009554	-1.329585	0.758015	0.378668	...	0.226178	...	0.551544
:	0.50	-0.484874	1.173649	-1.288407	0.088343	0.659828	...	0.267115	...	0.581656
:	0.60	-0.730714	1.119583	-1.128444	1.370645	0.202240	...	0.315513	...	0.603128
:	0.80	-0.412443	0.946502	-0.937018	325.351657	-0.916925	...	0.418869	...	0.668120
:	1.00	-2.389088	1.344771	-1.038823	0.022485	0.850346	...	0.294596	...	0.619869
:	1.50	-3.148562	1.351275	-0.883364	0.018389	0.805251	...	0.267464	...	0.672821
:	2.00	-4.114766	1.475830	-0.900053	0.025749	0.755808	...	0.282090	...	0.690049
:	3.00	-6.043688	1.685735	-0.801787	0.013854	0.844482	...	0.291622	...	0.756917
PSV($T, \xi = 5\%$)	4.00	-6.785239	1.745056	-0.719497	0.040240	0.719221	...	0.157421	...	0.740978

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{\ln Y}^{V,T}$
PGA	...	-2.777517	0.873859	-1.542786	29.313166	-0.160838	...	0.208319	...	0.554450
PSV($T, \xi = 5\%$)	0.04	-1.543074	1.047073	-1.653648	1.889054	0.259351	...	0.197220	...	0.600268
:	0.10	2.011563	1.014549	-2.058034	12.213589	0.053198	...	0.359792	...	0.624498
:	0.15	1.144293	1.054101	-1.819363	6.278832	0.133932	...	0.378567	...	0.573359
:	0.20	1.776488	0.666243	-1.332980	178.868558	-0.480565	...	0.329434	...	0.541644
:	0.30	0.813255	0.998376	-1.499581	7.421244	0.134084	...	0.383800	...	0.623120
:	0.40	-0.235440	1.020187	-1.258865	1.323526	0.325556	...	0.174487	...	0.614509
:	0.50	0.028004	0.905740	-1.131307	36.888452	-0.209692	...	0.185911	...	0.643288
:	0.60	-2.291067	1.237300	-1.074666	0.062961	0.756007	...	0.248750	...	0.666156
:	0.80	-2.460049	1.244583	-1.045379	0.047929	0.756300	...	0.300661	...	0.657639
:	1.00	-3.690274	1.449928	-1.022106	0.019590	0.947230	...	0.301460	...	0.649932
:	1.50	-3.477299	1.413406	-1.038323	0.027333	0.889701	...	0.147568	...	0.584247
:	2.00	-4.321775	1.451576	-0.944296	0.019156	0.877036	...	0.170777	...	0.583487
:	3.00	-6.930307	1.785811	-0.861179	0.018352	0.872870	...	0.333657	...	0.722341
PSV($T, \xi = 5\%$)	4.00	-7.999473	1.873483	-0.758510	0.035303	0.751569	...	0.248938	...	0.764899

Equation Set (12)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln (R + p_4^{c,T} \exp \{ p_5^{c,T} M \}) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: S = 0 for site class B, S = 1 for site class C
- F = fault-type code: F = 0 for strike-slip, F = 1 for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients									
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{ln Y}^{H,T}$	
PGA	...	-3.438716	1.064905	-1.561987	0.321121	0.609753	...	0.280577	0.079106	0.510114	
PSV(T, $\xi = 5\%$)	0.04	-1.647263	1.047523	-1.530232	0.273347	0.620739	...	0.293666	0.080130	0.500896	
:	0.10	1.621750	1.149988	-2.059471	0.826683	0.553212	...	0.416721	0.071282	0.556546	
:	0.15	1.921374	1.421424	-2.334328	0.794180	0.591839	...	0.325302	0.087794	0.543871	
:	0.20	2.310864	1.464855	-2.366754	1.014116	0.567785	...	0.270557	0.096824	0.528672	
:	0.30	1.650729	0.987419	-1.538759	0.445397	0.555187	...	0.278617	0.066522	0.529563	
:	0.40	0.905411	1.033991	-1.388657	0.244439	0.629116	...	0.128276	0.073497	0.510368	
:	0.50	-0.648045	1.257772	-1.361937	0.070190	0.788589	...	0.164481	0.117822	0.479357	
:	0.60	-0.853291	1.116790	-1.092971	0.089299	0.670684	...	0.224815	0.094713	0.538663	
:	0.80	-0.527344	0.970331	-0.929313	0.292487	0.398540	...	0.305814	0.070271	0.565890	
:	1.00	-2.407397	1.411386	-1.109214	0.023552	0.937149	...	0.176104	0.065991	0.457812	
:	1.50	-2.724998	1.436501	-1.019332	0.026961	0.948828	...	-0.038394	0.040855	0.488773	
:	2.00	-3.778425	1.535854	-0.976907	0.016162	1.010442	...	-0.085977	0.048010	0.513143	
:	3.00	-4.949367	1.585589	-0.835488	0.054454	0.795634	...	-0.140836	0.046186	0.639955	
PSV(T, $\xi = 5\%$)	4.00	-6.271135	1.990673	-1.104244	0.040989	0.985296	...	-0.317117	0.052907	0.691268	

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients									
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{ln Y}^{V,T}$	
PGA	...	-2.137768	1.166247	-2.066330	1.758670	0.406429	...	0.155372	0.068244	0.525213	
PSV(T, $\xi = 5\%$)	0.04	-0.319928	1.189360	-2.100133	1.122773	0.452080	...	0.128142	0.058463	0.611568	
:	0.10	2.209228	1.337061	-2.511478	1.278082	0.484613	...	0.292905	0.068251	0.569062	
:	0.15	0.665751	2.064686	-3.001224	0.836162	0.611992	...	0.151046	0.096520	0.479159	
:	0.20	1.632934	1.607519	-2.519910	1.432867	0.509265	...	0.017748	0.097571	0.470315	
:	0.30	2.941213	0.815381	-1.661896	3.218954	0.309727	...	0.238623	0.047027	0.544001	
:	0.40	1.798776	1.120817	-1.750044	1.702403	0.443681	...	-0.049630	0.034686	0.674710	
:	0.50	0.804862	1.198130	-1.645614	1.405009	0.449549	...	-0.009391	0.037842	0.646354	
:	0.60	-0.363992	1.606969	-1.911277	0.489891	0.667659	...	-0.032574	0.032815	0.533128	
:	0.80	-1.897051	1.308849	-1.241638	0.092559	0.753703	...	0.187640	-0.006993	0.513911	
:	1.00	-3.631091	1.850152	-1.568477	0.038388	0.995507	...	0.034305	0.017309	0.534805	
:	1.50	-2.334508	1.401436	-1.208727	0.044850	0.913271	...	-0.235660	0.017041	0.454505	
:	2.00	-3.838157	1.539424	-1.108019	0.024409	0.968853	...	-0.207828	0.016777	0.533039	
:	3.00	-6.322026	1.932611	-1.130873	0.027935	0.982444	...	-0.294537	0.018324	0.741991	
PSV(T, $\xi = 5\%$)	4.00	-8.707985	2.576310	-1.509543	0.035019	1.054745	...	-0.310699	0.034863	0.857161	

Equation Set (13)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln(R + p_4^{c,T} \exp\{p_5^{c,T}M\}) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: $S = 0$ for site class B, $S = 1$ for site class C
- F = fault-type code: $F = 0$ for strike-slip, $F = 1$ for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{\ln Y}^{H,T}$
PGA	...	-3.171847	1.105604	-1.495235	0.315897	0.670390	0.432386
PSV($T, \xi = 5\%$)	0.04	-0.631375	1.014424	-1.529255	0.504387	0.591804	0.428633
	0.10	2.469940	0.990540	-1.851245	0.975031	0.535067	0.499553
	0.15	3.585989	0.851706	-1.763100	2.015547	0.409572	0.466280
	0.20	5.910391	1.174911	-2.463648	5.807310	0.368350	0.464664
	0.30	2.581938	0.913354	-1.471862	2.134150	0.362448	0.466536
	0.40	0.347917	1.079412	-1.205913	0.109990	0.739537	0.457061
	0.50	-0.432626	1.167464	-1.141107	0.113232	0.707330	0.457979
	0.60	-1.361394	1.234138	-1.017782	0.016784	0.933752	0.473798
	0.80	-1.339666	1.237347	-1.017965	0.029211	0.857198	0.496888
	1.00	-1.795883	1.345774	-1.049509	0.021121	0.960719	0.532648
	1.50	-4.119871	1.625452	-0.969601	0.008157	1.060542	0.567066
	2.00	-4.682409	1.615584	-0.872844	0.002119	1.139593	0.598645
	3.00	-6.364540	1.838184	-0.883308	0.001993	1.120150	0.667339
PSV($T, \xi = 5\%$)	4.00	-7.006554	1.825652	-0.744807	0.001555	1.063866	0.710394

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{\ln Y}^{V,T}$
PGA	...	-0.559472	0.620993	-1.558580	42.672176	-0.170980	0.621805
PSV($T, \xi = 5\%$)	0.04	1.961399	0.561932	-1.619466	266.066139	-0.508069	0.692416
	0.10	2.914728	0.713356	-1.742818	39.399670	-0.203078	0.719386
	0.15	3.177836	0.537737	-1.395497	73.871073	-0.304972	0.674064
	0.20	2.786204	0.635713	-1.392705	93.213717	-0.305204	0.679197
	0.30	1.189784	0.641793	-0.974013	35.770574	-0.194924	0.589851
	0.40	0.046961	0.817867	-0.943034	4.276434	0.143474	0.575021
	0.50	-1.406940	1.039507	-0.916857	0.489645	0.508104	0.535768
	0.60	-1.797061	0.931776	-0.669255	0.119863	0.492423	0.600575
	0.80	-1.753020	0.959881	-0.716849	0.017301	0.885265	0.592254
	1.00	-2.221688	1.049198	-0.733990	0.011650	0.946231	0.557499
	1.50	-3.894661	1.272470	-0.675605	0.002920	1.123433	0.601683
	2.00	-4.260013	1.381443	-0.775603	0.010486	1.019084	0.661584
	3.00	-7.246310	1.863324	-0.831041	0.008931	1.089968	0.765604
PSV($T, \xi = 5\%$)	4.00	-7.794594	1.814333	-0.675976	0.005623	1.038565	0.841187

Equation Set (14)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln(R + p_4^{c,T} \exp\{p_5^{c,T}M\}) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y ≡ ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g ; units of PSV: [PSV] = cm/sec
- M ≡ moment magnitude, M_w
- R ≡ closest distance from site to fault rupture surface in km
- S ≡ site classification code: $S = 0$ for site class B, $S = 1$ for site class C
- F ≡ fault-type code: $F = 0$ for strike-slip, $F = 1$ for reverse
- D ≡ depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{ln Y}^{H,T}$
PGA	...	-2.015820	1.153071	-1.852876	0.750521	0.563234	0.025951	0.406245
PSV($T, \xi = 5\%$)	0.04	0.364718	1.093490	-1.885195	0.974180	0.524345	0.029432	0.406711
	0.10	3.731035	1.201834	-2.431032	1.633315	0.494372	0.028169	0.456326
	0.15	5.472566	0.950331	-2.287954	3.752345	0.360953	0.003618	0.450936
	0.20	6.686006	1.264552	-2.776806	5.703215	0.370405	0.017663	0.460511
	0.30	3.049698	1.125886	-1.864507	1.542490	0.462788	0.001928	0.421470
	0.40	1.454307	1.086857	-1.469933	0.343272	0.622287	0.017469	0.475111
	0.50	0.927555	1.125258	-1.398409	0.647430	0.507691	0.035606	0.458459
	0.60	0.325510	1.095849	-1.251366	0.955739	0.384565	0.049201	0.457015
	0.80	-1.842585	1.447201	-1.256686	0.026909	0.944773	0.048728	0.454654
	1.00	-2.568056	1.535995	-1.207010	0.013423	1.057494	0.047540	0.441020
	1.50	-3.387277	1.447946	-0.929657	0.014107	0.931298	0.062390	0.475158
	2.00	-4.139228	1.471404	-0.804379	0.004391	0.981605	0.066278	0.493842
	3.00	-6.042967	1.698842	-0.752102	0.003704	0.938893	0.070925	0.517036
PSV($T, \xi = 5\%$)	4.00	-6.393076	1.621853	-0.567337	0.001769	0.905683	0.085956	0.562746

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{ln Y}^{V,T}$
PGA	...	1.077007	0.636199	-1.944540	31.146485	-0.072968	-0.016066	0.621109
PSV($T, \xi = 5\%$)	0.04	3.761469	0.504742	-1.965056	130.081121	-0.352853	-0.014180	0.649008
	0.10	4.781791	0.923477	-2.420698	12.082899	0.091979	-0.013047	0.695227
	0.15	5.699239	0.539835	-1.950878	28.089125	-0.048326	-0.001192	0.661795
	0.20	4.022075	0.413019	-1.376288	97.287931	-0.374042	-0.033745	0.658784
	0.30	4.026279	0.442460	-1.366140	46.966826	-0.190756	-0.013939	0.497968
	0.40	1.773771	0.697511	-1.195841	12.669448	-0.006028	-0.026550	0.481226
	0.50	0.132259	0.787930	-0.936336	11.781855	-0.063293	-0.032734	0.447273
	0.60	-0.029126	0.689685	-0.746210	1.652385	0.032242	-0.045091	0.467070
	0.80	-0.613553	0.829203	-0.813486	0.016326	0.866179	-0.043985	0.442880
	1.00	-1.709742	0.956814	-0.692547	0.006802	0.917881	-0.068927	0.435347
	1.50	-3.374020	1.149316	-0.592391	0.001597	1.055927	-0.032309	0.515452
	2.00	-4.314197	1.327705	-0.709110	0.009266	0.965933	0.024308	0.557013
	3.00	-5.874392	1.477967	-0.583182	1.231425	0.176819	0.060847	0.635854
PSV($T, \xi = 5\%$)	4.00	-5.907418	1.407442	-0.489391	25.479222	-0.382398	0.068681	0.746116

Equation Set (15)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln (R + p_4^{c,T} \exp \{ p_5^{c,T} M \}) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: $S = 0$ for site class B, $S = 1$ for site class C
- F = fault-type code: $F = 0$ for strike-slip, $F = 1$ for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{\ln Y}^{H,T}$
PGA	...	-3.936006	1.131331	-1.407338	0.132766	0.762349	...	0.222134	...	0.423165
PSV($T, \xi = 5\%$)	0.04	-1.357995	1.047913	-1.454411	0.238403	0.672454	...	0.191981	...	0.422150
	0.10	1.583506	1.007164	-1.732633	0.566841	0.581291	...	0.222755	...	0.492162
	0.15	2.655102	0.891582	-1.658653	1.057827	0.474372	...	0.182737	...	0.461422
	0.20	3.797246	1.150935	-2.090457	2.707418	0.429144	...	0.197572	...	0.458925
	0.30	1.757966	0.964154	-1.399615	1.068671	0.433155	...	0.191845	...	0.460916
	0.40	-0.341344	1.143885	-1.184304	0.040152	0.855455	...	0.283950	...	0.441457
	0.50	-1.250108	1.262700	-1.132481	0.023059	0.904682	...	0.322455	...	0.439150
	0.60	-1.308109	1.174087	-0.985260	0.021467	0.826603	...	0.291465	...	0.453652
	0.80	-1.678807	1.291551	-1.039450	0.019647	0.909185	...	0.172694	...	0.490725
	1.00	-1.862627	1.355410	-1.053069	0.023326	0.942043	...	0.038112	...	0.534120
	1.50	-4.012763	1.608815	-0.962336	0.008855	1.051218	...	-0.060261	...	0.568799
	2.00	-4.228495	1.526558	-0.822749	0.003465	1.038725	...	-0.213967	...	0.597052
	3.00	-5.897752	1.747070	-0.834153	0.002207	1.057780	...	-0.216479	...	0.663833
PSV($T, \xi = 5\%$)	4.00	-6.735449	1.776420	-0.716986	0.001572	1.042894	...	-0.163145	...	0.709761

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{\ln Y}^{V,T}$
PGA	...	-1.725546	0.764676	-1.548369	18.364196	-0.067773	...	0.285830	...	0.611885
PSV($T, \xi = 5\%$)	0.04	0.526387	0.759216	-1.635060	88.127107	-0.363091	...	0.373243	...	0.676089
	0.10	1.213682	0.945844	-1.766289	11.354476	-0.041997	...	0.503098	...	0.688235
	0.15	1.813103	0.719528	-1.407318	24.071591	-0.166183	...	0.398727	...	0.654138
	0.20	1.524304	0.783045	-1.372194	42.942093	-0.226298	...	0.326083	...	0.667097
	0.30	0.064487	0.781080	-0.972995	11.989361	-0.084220	...	0.361985	...	0.570768
	0.40	-0.956925	0.939824	-0.942038	1.182603	0.289582	...	0.346593	...	0.557090
	0.50	-2.410384	1.145846	-0.898898	0.091114	0.700216	...	0.370380	...	0.512922
	0.60	-2.719244	1.097141	-0.757253	0.025803	0.743329	...	0.477149	...	0.564777
	0.80	-2.465013	1.092663	-0.798560	0.016309	0.891525	...	0.416272	...	0.565420
	1.00	-2.582774	1.108626	-0.767066	0.012798	0.915573	...	0.235089	...	0.550268
	1.50	-4.140309	1.330392	-0.706613	0.003076	1.158884	...	0.041385	...	0.601047
	2.00	-4.136549	1.366826	-0.766809	0.012379	1.006751	...	-0.119997	...	0.663141
	3.00	-6.962224	1.810674	-0.799058	0.010925	1.065098	...	-0.156364	...	0.762751
PSV($T, \xi = 5\%$)	4.00	-7.663303	1.787413	-0.650318	0.006922	1.017589	...	-0.115178	...	0.843241

Equation Set (16)

$$\ln Y^{c,T} = p_1^{c,T} + p_2^{c,T}M + p_3^{c,T} \ln (R + p_4^{c,T} \exp(p_5^{c,T}M)) + p_6^{c,T}S + p_7^{c,T}F + p_8^{c,T}D$$

- Y = ground-motion parameter—either peak ground acceleration (PGA), or peak 5 percent-damped pseudo-spectral velocity (PSV); units of PGA: [PGA] = g; units of PSV: [PSV] = cm/sec
- M = moment magnitude, M_w
- R = closest distance from site to fault rupture surface in km
- S = site classification code: $S = 0$ for site class B, $S = 1$ for site class C
- F = fault-type code: $F = 0$ for strike-slip, $F = 1$ for reverse
- D = depth to basement in km

Horizontal Component

Ground-Motion Parameter, $Y^{H,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{H,T}$	$p_2^{H,T}$	$p_3^{H,T}$	$p_4^{H,T}$	$p_5^{H,T}$	$p_6^{H,T}$	$p_7^{H,T}$	$p_8^{H,T}$	$\sigma_{ln Y}$
PGA	...	-2.722823	1.105448	-1.683807	0.425417	0.602958	...	0.183172	0.024488	0.400847
PSV($T, \xi = 5\%$)	0.04	-0.453568	1.049550	-1.701597	0.531022	0.566631	...	0.198691	0.027820	0.399926
	0.10	2.906963	1.136391	-2.222362	1.154496	0.511344	...	0.152023	0.026946	0.454405
	0.15	4.343899	0.897721	-2.040535	2.539610	0.373475	...	0.177455	0.002064	0.447224
	0.20	4.693018	1.134157	-2.302479	3.463922	0.384564	...	0.193536	0.015694	0.456061
	0.30	2.068472	1.071210	-1.641261	0.841904	0.493745	...	0.227100	0.000130	0.412342
	0.40	0.601977	1.068338	-1.317495	0.101083	0.731723	...	0.318640	0.015198	0.456408
	0.50	0.004364	1.133078	-1.265281	0.201827	0.607581	...	0.326197	0.033414	0.437589
	0.60	-0.538696	1.144968	-1.175065	0.243775	0.525161	...	0.287624	0.047093	0.441287
	0.80	-2.111858	1.431323	-1.207824	0.017393	0.969773	...	0.220262	0.048159	0.443601
	1.00	-2.591393	1.521160	-1.192724	0.013925	1.038309	...	0.074162	0.047710	0.442444
	1.50	-3.325758	1.446800	-0.921095	0.010806	0.991039	...	-0.142566	0.061650	0.472638
	2.00	-3.596462	1.384646	-0.752811	0.006434	0.946916	...	-0.377579	0.065902	0.465185
	3.00	-5.665602	1.634462	-0.702852	0.002569	1.010255	...	-0.307991	0.070062	0.497140
PSV($T, \xi = 5\%$)	4.00	-6.133363	1.575226	-0.534287	0.001843	0.882245	...	-0.182212	0.085524	0.559842

Vertical Component

Ground-Motion Parameter, $Y^{V,T}$	Period T [sec]	Attenuation Equation Coefficients								
		$p_1^{V,T}$	$p_2^{V,T}$	$p_3^{V,T}$	$p_4^{V,T}$	$p_5^{V,T}$	$p_6^{V,T}$	$p_7^{V,T}$	$p_8^{V,T}$	$\sigma_{ln Y}$
PGA	...	0.669394	0.644169	-1.888426	31.869138	-0.095439	...	0.103491	-0.016777	0.623173
PSV($T, \xi = 5\%$)	0.04	3.434138	0.525762	-1.939816	136.682583	-0.373788	...	0.102450	-0.014754	0.651286
	0.10	3.956864	0.910670	-2.270291	12.072915	0.055295	...	0.200860	-0.014682	0.693841
	0.15	4.726885	0.552329	-1.806007	29.716172	-0.105575	...	0.229435	-0.003107	0.657990
	0.20	3.911835	0.500999	-1.520666	118.213507	-0.343504	...	0.173085	-0.030268	0.679890
	0.30	3.362888	0.490733	-1.311406	65.800181	-0.260923	...	0.178855	-0.015658	0.491104
	0.40	1.365266	0.720016	-1.159498	11.266933	-0.019657	...	0.132407	-0.027266	0.480460
	0.50	-0.486943	0.844271	-0.915951	9.014459	-0.081244	...	0.262286	-0.033728	0.433697
	0.60	-0.595461	0.780898	-0.799571	2.048703	-0.004686	...	0.396638	-0.044864	0.428560
	0.80	-1.051411	0.879716	-0.829975	0.011837	0.864018	...	0.326888	-0.044304	0.419317
	1.00	-1.922788	0.987203	-0.709449	0.006926	0.893816	...	0.164202	-0.068869	0.431408
	1.50	-3.225348	1.114051	-0.559256	0.001821	0.991820	...	-0.109495	-0.033227	0.519725
	2.00	-3.991431	1.294437	-0.696769	0.010816	0.985673	...	-0.272321	0.024126	0.546003
	3.00	-4.708215	1.361602	-0.623787	21.502256	-0.119324	...	-0.361965	0.062479	0.619146
PSV($T, \xi = 5\%$)	4.00	-5.626069	1.365882	-0.467435	12.966326	-0.248231	...	-0.202523	0.068878	0.744878