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Update of the Accounting Surface Along the Lower Colorado River

Scientific Investigations Report 2008–5113

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By Stephen M. Wiele, Stanley A. Leake, Sandra J. Owen-Joyce, and Emmet H. McGuire

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Mark D. Myers, Director

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Conversion Factors

Inch/Pound to SI

Multiply	By	To obtain
Length		
foot (ft)	0.3048	meter (m)
mile (mi)	1.609	kilometer (km)
Volume		
acre-foot (acre-ft)	1,233	cubic meter (m^3)
acre-foot (acre-ft)	0.001233	cubic hectometer (hm^3)
Flow rate		
foot per day (ft/d)	0.3048	meter per day (m/d)
cubic foot per second (ft^3/s)	0.02832	cubic meter per second (m^3/s)

Vertical coordinate information is referenced to the "National Vertical Geodetic Datum of 1929 (NGVD 29)."

Horizontal coordinate information is referenced to the "North American Datum of 1927 (NAD 27)."

Elevation, as used in this report, refers to distance above the vertical datum.

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Abstract

The accounting-surface method was developed in the 1990s by the U.S. Geological Survey, in cooperation with the Bureau of Reclamation, to identify wells outside the flood plain of the lower Colorado River that yield water that will be replaced by water from the river. This method was needed to identify which wells require an entitlement for diversion of water from the Colorado River and need to be included in accounting for consumptive use of Colorado River water as outlined in the Consolidated Decree of the United States Supreme Court in *Arizona v. California*. The method is based on the concept of a river aquifer and an accounting surface within the river aquifer. The study area includes the valley adjacent to the lower Colorado River and parts of some adjacent valleys in Arizona, California, Nevada, and Utah and extends from the east end of Lake Mead south to the southerly international boundary with Mexico. Contours for the original accounting surface were hand drawn based on the shape of the aquifer, water-surface elevations in the Colorado River and drainage ditches, and hydrologic judgment. This report documents an update of the original accounting surface based on updated water-surface elevations in the Colorado River and drainage ditches and the use of simple, physically based ground-water flow models to calculate the accounting surface in four areas adjacent to the free-flowing river.

Introduction

The accounting-surface method was developed in the 1990s by the U.S. Geological Survey (USGS), in cooperation with the Bureau of Reclamation (Reclamation), to identify wells outside the flood plain of the lower Colorado River that yield water that will be replaced by water from the river (Wilson and Owen-Joyce, 1994; Owen-Joyce and others, 2000). Prior to the development of the accounting-surface method, water pumped from many wells outside the flood plain was not included when accounting for consumptive use of river water. A method was needed to identify which wells pump water that will be replaced by water from the Colorado River and need to be included in accounting for consumptive use of

Colorado River water as outlined in the Consolidated Decree of the United States Supreme Court in *Arizona v. California*, 547 U.S.150 (2006). The method is based on the concept of a river aquifer and an accounting surface within the river aquifer. The study area includes the valley adjacent to the lower Colorado River and parts of some adjacent valleys in Arizona, California, Nevada, and Utah and extends from the east end of Lake Mead south to the southerly international boundary with Mexico (fig. 1). Nearly 15 years have passed since the development of the original accounting surface. Prior to the issuance of a proposed rule to define the accounting procedure, an update of the accounting surface is needed for use in the process of Decree accounting for the following reasons:

1. The original accounting surface was generated on the basis of water-surface profiles of the lower Colorado River computed for the highest median monthly projected discharge for 1992–2001 and assuming delivery of full allocations of river water to users in the United States. Since that time, historical data are available that represent the current and anticipated future operation of the Colorado River for the delivery of full allocation of river water to users in the United States and treaty deliveries to Mexico.

2. The original water-surface profiles were generated with a surface-water model representing river-channel conditions surveyed between 1980 and 1988. More recent river cross-sectional information is available, and the target elevations for Lakes Mohave and Havasu have changed slightly since the original accounting surface was developed.

3. The original accounting surface in parts of the Parker and Palo Verde areas was based on water-surface elevations in drainage ditches or wells along the edge of the flood plain that represented regulated flow conditions of the late 1980s and early 1990s. Furthermore, the elevations from the drainage ditches used in the Palo Verde Valley were based on a nonstandard vertical datum, adding an error to the elevation of the accounting surface in that area.

4. Improved ground-water flow modeling is now available that will allow efficient construction of an accounting surface tied to the river in reaches not adjacent to reser-
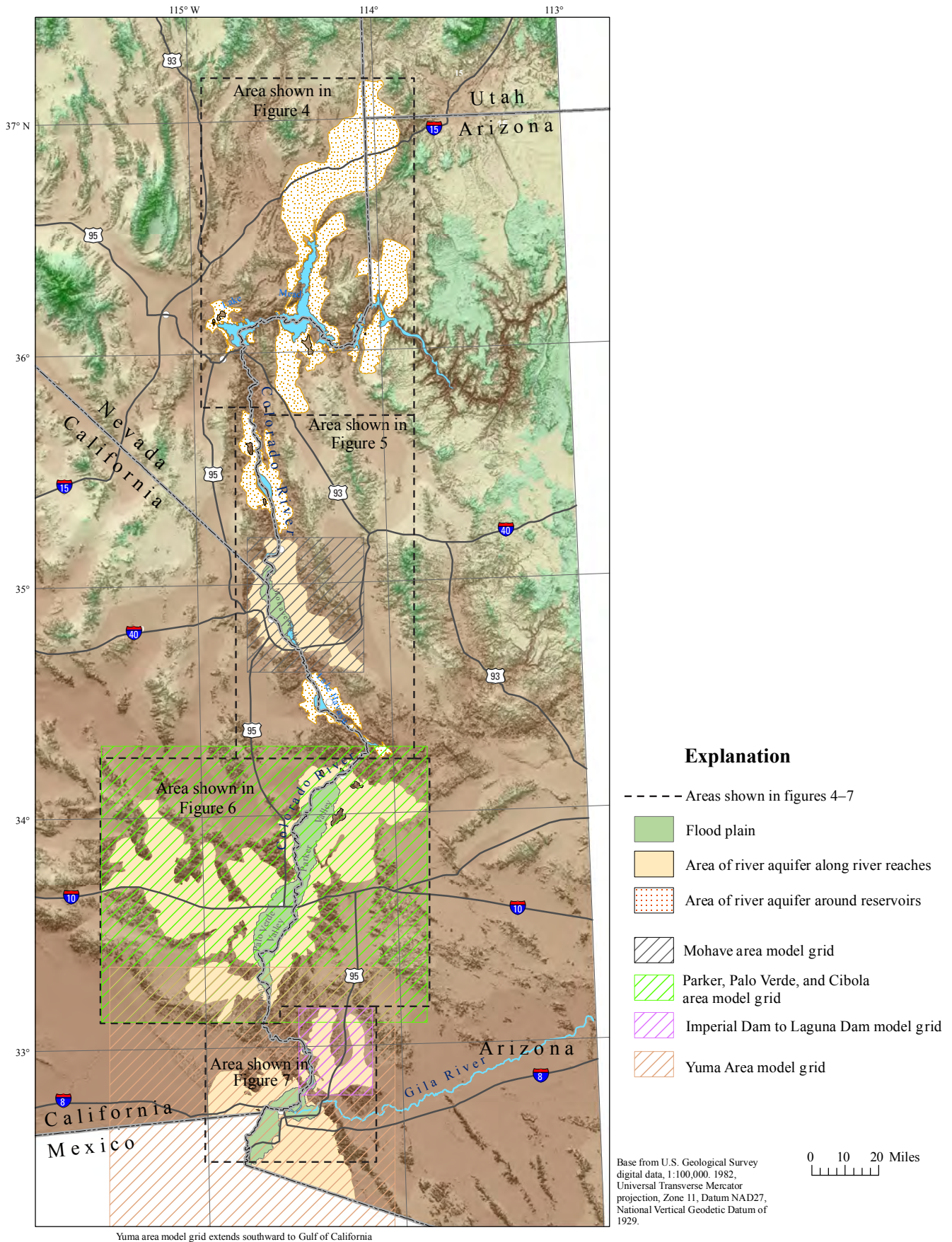
Figure 1. Map showing the lower Colorado River and areal extent of the river aquifer.

voirs. An accounting surface computed with a physically based model is an improvement on the original accounting surface which was hand-drawn based on hydrologic judgment and can be easily replicated and quickly updated as required.

Legal Framework

The Colorado River Compact of 1922 apportions the waters of the Colorado River between the upper basin and the lower basin (U.S. Congress, 1948, p. A17-A22). The requirement for participation of the USGS and Reclamation is stated in Article V:

> The chief official of each signatory State charged with the administration of water rights, together with the Director of the United States Reclamation Service and the Director of the United States Geological Survey shall cooperate, ex-officio:
>
> (a) To promote the systematic determination and coordination of the facts as to flow, appropriation, consumption, and use of water in the Colorado River Basin, and the interchange of available information in such matters.

Water in the lower Colorado River is apportioned among the States of California, Arizona, and Nevada by the Boulder Canyon Project Act of December 21, 1928 (U.S. Congress, 1948, p. A213–A225) and confirmed by the Consolidated Decree (U.S. Supreme Court, 2006) in terms of consumptive use. The decree is specific about the responsibility of the Secretary of the Interior to account for consumptive use of water from the mainstream. Consumptive use is defined to include "water drawn from the mainstream by underground pumping." Article V of the Consolidated Decree (U.S. Supreme Court, 2006) states in part:

> The United States shall prepare and maintain, or provide for the preparation and maintenance of, and shall make available, annually and at such shorter intervals as the Secretary of the Interior shall deem necessary or advisable, for inspection by interested persons at all reasonable times and at a reasonable place or places, complete, detailed and accurate records of: * * *
>
> * * * (B) Diversions of water from the mainstream, return flow of such water to the stream as is available for consumptive use in the United States or in satisfaction of the Mexican treaty obligation, and consumptive use of such water. These quantities shall be stated separately as to each diverter from the mainstream, each point of diversion, and each of the States of Arizona, California, and Nevada; * * *

Article I of the decree defines terminology and states in part:

> (A) "Consumptive use" means diversions from the stream less such return flow thereto as is available for consumptive use in the United States or in satisfaction of the Mexican treaty obligation;
>
> (B) "Mainstream" means the mainstream of the Colorado River downstream from Lee Ferry within the United States, including the reservoirs thereon;
>
> (C) Consumptive use from the mainstream within a state shall include all consumptive uses of water of the mainstream, including water drawn from the mainstream by underground pumping, and including but not limited to, consumptive uses made by persons, by agencies of that state, and by the United States for the benefit of Indian reservations and other federal establishments within the state; * * *

Ground water in the river aquifer beneath the flood plain is considered to be Colorado River water, and water pumped from wells on the flood plain is presumed to be river water and is accounted for as Colorado River water. Drainage ditches that lie along the edge of the flood plain contain a mixture of river water (recharged on the flood plain from the application of diverted irrigation water) and tributary water.

Purpose and Scope

This report documents the updates to the data and method used to generate the accounting surface in previous reports (Wilson and Owen-Joyce, 1994; Owen-Joyce and others, 2000) and presents the updated accounting surface needed to identify wells outside the flood plain of the lower Colorado River that yield water that will be replaced by water from the Colorado River. The report describes the process to update the accounting surface using simple, physically based ground-water flow models and contains maps (figs. 4–7) that show the elevation and contours of the updated accounting surface. Site-specific data were collected where needed to update the accounting surface.

Data Collection

The USGS collected hydrologic data for the study during 2007–08. Most field work was done along the drainage ditches on the flood plain in Parker and Cibola Valleys in Arizona, in Palo Verde Valley in California, and in the Yuma area in Arizona and California. Additional data were collected along reaches of the river between Parker and Headgate Rock Dams and from upstream of Imperial Dam to the northerly international boundary (NIB) with Mexico. Water-surface elevations in drainage ditches were determined by use of Global Positioning System (GPS) surveys (Remondi, 1985). The data are stored in a database of the Arizona Water Science Center of the U.S. Geological Survey, Tucson, Arizona.

Precise GPS was used to collect water-level elevation data in the drainage ditches of agricultural areas along the lower Colorado River in Parker, Cibola, and Palo Verde Valleys and in the Yuma area. Field collection of data for the Palo Verde Valley drainage-ditch survey was conducted during the weeks of August 13 and 27, 2007. Data for the drainage ditches in Parker Valley were collected during the weeks of August 27 and September 10, 2007. Data for the Cibola Valley drainage ditches were collected during the week of November 5, 2007. Data for the drainage ditches in the Yuma area were collected January 30–31, 2008. Precise GPS was also used to collect data for specific reaches along the Colorado River. Data for the river between Parker Dam and Headgate Rock Dam were collected January 24, 2008. Data for the river in the Yuma area were collected the week of February 4, 2008.

Survey methods included collecting survey data by using Real-Time Kinematic (RTK)-Infill and static GPS. RTK GPS was used to collect edge-of-water or staff-gage elevations in the drainage ditches. RTK base-station positions were located at higher topographic locations near the drainage ditches. The base-station positions were selected by virtue of line-of-sight capability with the area of the drainage ditch to be surveyed. Because most survey points within the drainage ditches were obscured from the base station by the embankments, two technicians conducted the survey for safety and to ensure line-of-site radio link between the RTK base station and rover unit. One technician entered the drainage ditch to place the rover GPS antenna pole at the edge of water or, when available, on top of a staff gage, while the second technician remained at the top of the drainage ditch with the rover radio receiver. Down-to-water measurements were made from the top-of-staff gage or other measuring-point positions.

Static GPS methods included the occupation of surrounding survey benchmarks that have coordinates published by the National Geodetic Survey (NGS). Data collected from the static occupations were used to tie-in, correct, and check the coordinates of individual RTK base-station positions for each of the individual drainage-ditch surveys. In addition, individual base-station positions from each of the drainage ditches were surveyed to a single benchmark located just west of the right bank cableway tie-back at the Colorado River below Palo Verde Dam (USGS 09429100) streamflow-gaging station. Selected top-of-staff measuring points and surrounding NGS benchmarks, when available, were reoccupied with RTK GPS to check for survey accuracy and repeatability. The accuracy of the surveyed elevations was ±0.20 feet.

Various precise GPS methods were used to collect water-level elevation data depending on the conditions that existed in those areas. Traditional RTK and faststatic techniques were not feasible due to line-of-sight problems and the absence of an established faststatic base station in the area between Parker and Headgate Rock Dams. Data were collected at eleven points along this reach of the river by treating each point as a base station and obtaining an Online Positioning User Service (OPUS) solution for each point. In the Yuma area, where there is an established base station surveyed in at the Yuma USGS office, data were collected at 28 points in drainage ditches and at 6 wells using the faststatic technique. Along the river in the reach upstream from Laguna Dam, 15 elevation points were collected by using the faststatic technique with the Yuma base station at the USGS office. Along the river in the Yuma area downstream from Laguna Dam to the NIB with Mexico, the RTK technique was used to collect data at 14 points by using both the AMVD and COCO base stations, which are developed benchmarks established by the City of Yuma.

Previous Investigations

The accounting-surface method is described for two areas in separate reports—the area upstream from Laguna Dam in Wilson and Owen-Joyce (1994) and the area downstream from Laguna Dam in Owen-Joyce and others (2000). Previous geohydrologic studies of the lower Colorado River valley from Davis Dam to Yuma defined and described the formations that constitute the river aquifer, discussed the geologic structures and framework of the lower Colorado River valley, and described the occurrence and movement of ground water (Metzger, 1965, 1968; Metzger and Loeltz, 1973; Metzger and others, 1973; Olmsted and others, 1973). The major emphasis of these studies was the ground-water flow system beneath the flood plain and its relation to the Colorado River because few wells were available outside the flood plain to provide water levels or samples for chemical analysis. Refinement of the hydrogeologic framework, updated maps of ground-water flow, estimates of ground-water storage in the mound under Yuma Mesa, water-chemistry analyses, and water-budget components are topics covered in a recent study of the Yuma area (Dickinson and others, 2006). Additional work to develop procedures to apply the accounting-surface method to water-level data from wells applied geographic information system (GIS) methods to identify areas where wells pump water that will be replaced by water from the Colorado River (Spangler and others, 2007).

Acknowledgments

Jeffrey C. Addiego, Douglas B. Blatchford, William B. Greer, Ruth M. Thayer, Shana G. Tighi, and Dennis E. Watt of the Bureau of Reclamation provided information regarding discharges and water-surface profiles in the Colorado River and guidance concerning the use of drainage ditches in the ground-water modeling. The Colorado River Indian Tribes kindly provided access to their reservation. Palo Verde Irrigation District provided maps and water-level data for their staff gages in drainage ditches.

Accounting-Surface Method

The accounting-surface method was developed to identify wells outside the flood plain of the lower Colorado River that yield water that will be replaced by water from the river

(Wilson and Owen-Joyce, 1994; Owen-Joyce and others, 2000). The method is based on the concept of a river aquifer and an accounting surface within the river aquifer. The method provides a uniform criterion for all users pumping water from wells by determining whether the elevation of the static water table at a well is above or below the accounting surface. The elevation of the static water table at a well is determined by measuring the elevation of the static water level in the well. The static water level is the level of the water in a well that is not being affected by ground-water withdrawal or the level to which water will rise in a tightly cased well under its full pressure head. Wells that have a static water-level elevation equal to or below the accounting surface are presumed to yield water that will be replaced by water from the river. Wells that have a static water-level elevation above the accounting surface are presumed to yield water that will be replaced by water from precipitation and inflow from tributary valleys (fig. 2). Ground water in the river aquifer beneath the flood plain is considered to be Colorado River water regardless of water levels. Water pumped from wells on the flood plain is presumed to be river water and is accounted for as Colorado River water.

The accounting surface is defined to represent the elevation and slope of the static water table in the river aquifer outside the flood plain and the reservoirs of the Colorado River that would exist if the water in the river aquifer were derived only from the river (Wilson and Owen-Joyce, 1994). The accounting surface extends outward from the edges of the flood plain or a reservoir to the subsurface boundary of the river aquifer. Initial attempts to compare the water level in wells to the accounting surface were stymied by the inability to obtain water levels in every well (Spangler and others, 2007). Consequently, a method was devised by Spangler and others (2007) to estimate the water surface from available data and a new category—near the accounting surface—was added to the existing categories of well water levels—at, below, or above the accounting surface. GIS methods were used to create maps from measured water-level data that were then used to delineate areas where the water levels in wells were above or below the accounting surface. Estimations of water elevation can be made for wells without a measured water level (Spangler and others, 2007) from these maps. Water levels in wells were measured with calibrated steel or electrical tapes that are accurate to within tenths or hundredths of a foot. A differential GPS was used to determine land-surface elevations to within an operational accuracy of ± 0.43 ft, resulting in calculated water-level elevations having a 95-percent confidence interval of ± 0.84 ft. GIS interpolation tools were used to delineate areas within the river aquifer where water-level elevations are presumed to be above, below, and near (within ± 0.84 ft at the 95-percent confidence interval) the elevation of the accounting surface.

The criterion in the accounting-surface method for all users pumping water from wells was changed by Spangler and others (2007) to determining whether the elevation of
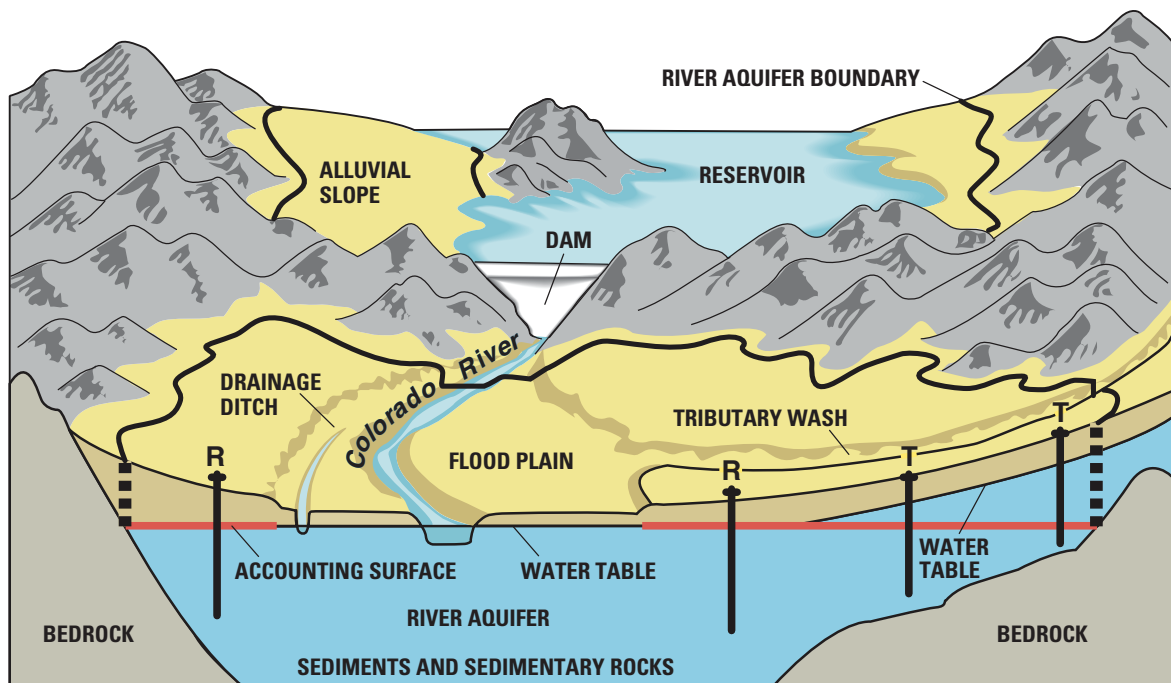
Figure 2. Schematic diagram showing the river aquifer and accounting surface (red line) of the lower Colorado River. Wells labeled "R" have a static water-level elevation equal to or below the accounting surface and are presumed to yield water that will be replaced by water from the river. Wells labeled "T" have a static water-level elevation above the accounting surface and are presumed to yield water that will be replaced by water from precipitation and inflow from tributary valleys (Modified from Wilson and Owen-Joyce, 1994).

the static water table at a well is above, near, or below the accounting surface. Wells that have a static water-level elevation near, equal to, or below the accounting surface are presumed to yield water that will be replaced by water from the river. Wells that have a static water-level elevation above the accounting surface are presumed to yield water that will be replaced by water from precipitation and inflow from tributary valleys.

River Aquifer

The boundary of the area that contains the accounting surface was defined as the river aquifer and delineated in the previous studies (Wilson and Owen-Joyce, 1994; Owen-Joyce and others, 2000). The river aquifer consists of permeable, partly saturated sediments and sedimentary rocks that are hydraulically connected to the Colorado River so that water can move between the river and the aquifer in response to withdrawal of water from the aquifer or differences in water-level elevations between the river and the aquifer. The subsurface limit of the river aquifer is the nearly impermeable bedrock of the bottom and sides of the basins that underlie the Colorado River valley and adjacent tributary valleys, which is a barrier to ground-water flow. For this study the boundary of the river aquifer remains the same as defined previously (fig. 1).

The river aquifer beneath the area where the accounting surface exists can be divided into two areas. The first area is where the water table is controlled by reservoirs, and the second area is where the water table is controlled by the Colorado River, drainage ditches on the flood plain, or both. In areas controlled by reservoirs, the accounting surface is set at a constant elevation defined by a representative reservoir level specified by Reclamation. In areas controlled by the Colorado River, drainage ditches, or both, the accounting surface varies depending on the shape of the aquifer and the surface-water elevations.

Generation of the Accounting Surface

The accounting surface adjacent to free-flowing reaches of the river between reservoirs published by Wilson and Owen-Joyce (1994) and Owen-Joyce and others (2000) was represented by hand-drawn contours based on surface-water profiles. In Parker and Palo Verde Valleys, drainage ditches or wells along the edge of the flood plain were used to define the level of the accounting surface. Reclamation considers the water levels in the drainage ditches to represent the level of Colorado River water beneath the flood plain. Adjacent to reservoirs, the accounting surface is flat, and is set to an elevation of the adjacent reservoir defined by the annual high water-surface elevation used by Reclamation to operate the reservoirs under normal flow conditions.

The general strategy for updating the accounting surface was as follows:

1. The extent of the river aquifer and area over which the accounting surface was defined by Wilson and Owen-Joyce (1994) and Owen-Joyce and others (2000) was retained.

2. Water-surface profiles of the Colorado River and drainage ditches used in defining the accounting surface were updated using the most recent information available. Drainage ditches were used in Parker, Palo Verde, and Cibola Valleys in defining the accounting surface.

3. Water-surface elevations in reservoirs were updated on the basis of current operating conditions for Lakes Mead, Mohave, and Havasu.

4. Contours of the accounting surface adjacent to free-flowing reaches of the Colorado River were generated using simple steady-state ground-water models that simulate two-dimensional flow, using a constant transmissivity value, with river and drainage-ditch elevations as boundary conditions.

The discharges along the free-flowing reaches of the Colorado River and the water-surface elevations in reservoirs used to define the accounting surface were specified by Reclamation. Implementation of this general strategy is discussed in more detail in the following sections.

Criteria for Establishing Reservoir Water-Surface Elevations and Colorado River Flow Conditions Used to Generate the Accounting Surface

The water-surface elevations in the Colorado River, reservoirs, and drainage ditches satisfy the following criteria (Jeffrey C. Addiego, Bureau of Reclamation, written commun., 2007):

- The Colorado River is flowing under normal operating conditions. Normal operating conditions exist when releases from the reservoirs are being made to accommodate downstream requirements where each State is using its full apportionment (consumptive use in Arizona + California + Nevada equals 7.5 million acre-feet) and a treaty-specified 1.5 million acre-feet is being delivered to Mexico (approximately 1.36 million acre-feet at the NIB with Mexico and 0.14 million acre-feet at the land boundary near San Luis). Flow and (or) river stage values can be either historical or modeled values, and should exclude flood flows from the lower basin tributaries and side-wash inflows.

- The hydraulic influence of the Colorado River under normal operating conditions is defined by the mean stage of the Colorado River (excluding reservoirs) during the highest flow month of the year (the flow that should be used to calculate the river stage is the mean monthly flow for the highest flow month of the year).

- The elevations used for the reservoirs (Lakes Mohave and Havasu) are the high monthly target elevation for the year used when operating under normal operating

conditions—644 feet for Lake Mohave and 448.7 feet for Lake Havasu.

- The maximum elevation of the accounting surface for Lake Mead is the top of the spillway gates in their fixed (down) position (1,205.4 feet). This elevation corresponds to an elevation (and corresponding area) in the vicinity of Lake Mead where a well would have the potential to pump Colorado River water. Whether a well would be considered to pump Colorado River water in the Lake Mead area would depend upon the actual lake elevations during the accounting year.

- The flows and river stage values account for major diversions from and return flows to the river at their respective locations. These diversions and return flows include, at minimum, the diversion at Headgate Rock Dam and major drainage ditches from the Colorado River Indian Reservation, the diversion at Palo Verde Diversion Dam and major drainage ditches from the Palo Verde Irrigation District, the diversions from Lake Havasu by the Central Arizona Project and the Metropolitan Water District canals, the diversions at Imperial Dam and major returns below Imperial Dam. As many diversions and return flow points are used as practical given the available data and the practical influence upon the resultant values.

Areas of the River Aquifer Controlled by Reservoirs

The accounting surface elevations in the river aquifer surrounding Lake Havasu, Lake Mohave, and Lake Mead are determined by the reservoir levels. Reclamation has determined that the accounting-surface elevations are 448.7 ft for Lake Havasu, 644.0 ft for Lake Mohave, and 1205.4 ft for Lake Mead.

Areas of the River Aquifer Controlled by the Colorado River, Drainage Ditches, or Both

River Reaches

Along reaches of the Colorado River without irrigation on the flood plain where the river loses water to the aquifer, the accounting surface is determined by the water surface of the Colorado River. Under predevelopment conditions and where the flood plain is not irrigated with diverted river water, ground-water levels in areas outside the flood plain that are higher than the Colorado River are caused only by tributary ground-water inflow. In this case, the river controls the elevation of the water table under the flood plain, and the accounting surface would be lower than the higher water level caused by tributary ground-water inflow. Water pumped from a well with a static water level above the accounting surface would be deemed tributary water, and an entitlement would not be needed.

Table 1. Discharges used to determine the water-surface elevation of the Colorado River used in the ground water-flow models.

[River miles start at the southerly international boundary with Mexico and increase upstream (Bureau of Reclamation, 2001)]

| Colorado River gaging station	River mile	Discharge, in cubic feet per second
Below Hoover Dam	342.0	17,634
Below Davis Dam	275.5	17,069
At Big Bend	264.7	19,567
Below Parker Dam	192.2	12,370
Forebay at Headgate Rock Dam	177.7	11,402
At Parker	175.3	11,970
At Water Wheel	151.5	11,157
Below Palo Verde Diversion Dam	132.7	10,924
At Taylor Ferry	106.4	9,825
At Lower Cibola Bridge	86.9	10,399
Above Imperial Dam	49.2	10,222
Below Imperial Dam	49.2	549
Below Laguna Dam	41.7	716
Below Yuma Main Canal Wasteway	29.4	1,527
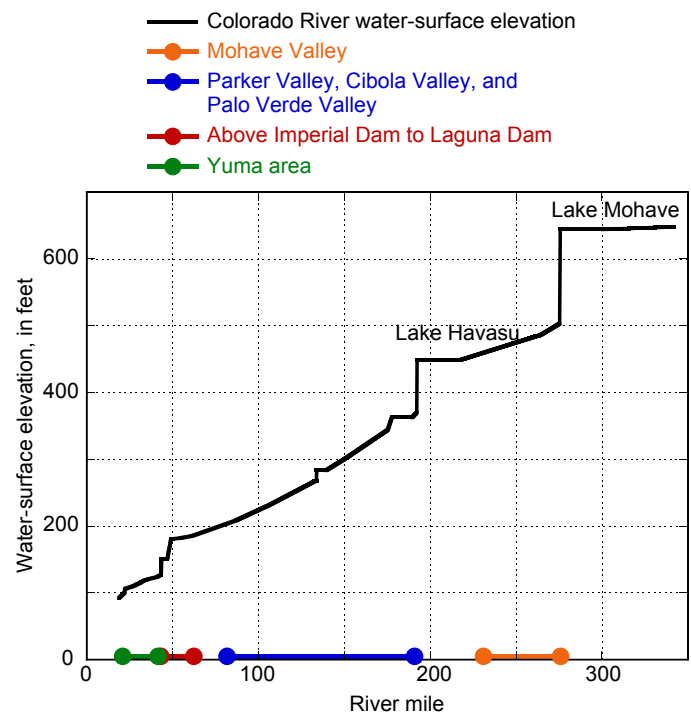
Figure 3. Water surface profile of the Colorado River used in the ground-water flow models. The approximate extent of the accounting surface in each of the four modeled areas is also displayed as a function of river mile. River miles (Bureau of Reclamation, 2001) start at the southerly international boundary with Mexico and increase upstream.

Table 2. Streamflow-gaging station data used to define the water-surface elevation used in the ground-water flow models.

[Agency: USGS, U.S. Geological Survey; Reclamation, Bureau of Reclamation; IBWC, International Boundary and Water Commission]

Colorado River gaging station	UTM coordintate[1], in meters		Elevation, in feet	River mile	Agency
	Easting	Northing			
Below Davis Dam (09423000)	721369	3895914	503.17	275.40	USGS
Big Bend	717750	3884573	486.60	264.70	Reclamation
Below Needles Bridge	721649	3855318	462.20	243.30	Reclamation
RS41 (below Topock Marsh)	731394	3844023	454.14	233.60	Reclamation
Below Parker Dam (09427500)	763366	3798537	370.45	192.20	USGS
Forebay at Headgate Rock Dam	750315	3783939	362.62	177.70	Reclamation
Parker	748190	3781783	344.00	175.30	Reclamation
Water Wheel	728171	3756367	302.63	151.50	Reclamation
Below Palo Verde Diversion Dam	732289	3732777	267.02	132.70	Reclamation
Taylor Ferry (TFLC)	720531	3701245	231.57	106.40	Reclamation
Lower Cibola Bridge	716492	3676582	208.38	86.90	Reclamation
Below Imperial Dam (09429500)	736985	3640727	180.72	49.20	USGS
Below Laguna Dam (0942600)	732742	3633016	127.06	41.7	USGS
Below Yuma Main Canal Wasteway (09521100)	720849	3623858	113.22	29.50	USGS
Above Rockwood Weir	713707	3622116	106.80	23.10	IBWC
Above Morelos Dam	712976	3620783	105.00	22.10	IBWC
Below Morelos Dam	712939	3620723	100.20	22.11	IBWC
Eleven-mile gage	711163	3616163	92.20	18.80	IBWC

[1] Universal Transverse Mercator, Zone 11 coordinates, North American Datum of 1927.

A calibrated and documented step-backwater model was not available for the study area, and development of such a model was beyond the scope of this study. Reclamation reevaluated the discharges below dams and streamflow-gaging stations along the river used to establish the water-surface elevations according to the criteria described above (Douglas B. Blatchford, Bureau of Reclamation, written commun., 2007) and produced the discharges in table 1. The water-surface profile of the Colorado River was based on a profile linearly interpolated between streamflow-gaging stations that was provided by Reclamation (Shana G. Tighi, written commun., 2008) that included streamflow data collected at gaging stations operated by Reclamation, USGS, and the International Boundary and Water Commission (IBWC). That profile was modified by additional water-surface elevation measurements made by the USGS. Water-surface measurements were made where the linearly interpolated profile deviated significantly from the profiles used in the previous studies (Wilson and Owen-Joyce, 1994; Owen-Joyce and others, 2000). The water-surface profile of the Colorado River used in the ground-water flow models is shown in figure 3. Data from streamflow-gaging stations and USGS measurements are listed in tables

2 and 3. Tables containing the water-surface elevations in the drainage ditches, the path of the Colorado River in Universal Transverse Mercator (UTM) coordinates, and the digitized surface elevations used to represent the Colorado River south of Eleven-mile gage are in the appendixes (available only online at http://pubs.usgs.gov/sir/2008/5113/appendixes/).

Mohave Valley—The water-surface profile was determined from the stage-discharge relations at four streamflow-gaging stations at river miles 275.4 (Colorado River below Davis Dam), 264.7 (Colorado River at Big Bend), 243.4 (Colorado River below Needles Bridge), and 233.6 (Colorado River near Topock [at RS41]), and the elevation of Lake Havasu.

Parker Valley and Palo Verde Valley—The linearly interpolated profile was based on streamflow-gaging station data at river miles 192.2 (Colorado River below Parker Dam), 177.7 (Colorado River Forebay above Headgate Rock), 175.3 (Colorado River at Parker), 151.5 (Colorado River at Water Wheel), 132.7 (Colorado River below Palo Verde Diversion Dam), 106.4 (Colorado River at Taylor Ferry), and 86.9 (Colorado River at Lower Cibola Bridge) and Reclamation GIS coverages of the

Table 3. Colorado River water-surface elevation measurements used to define the water-surface elevation used in the ground-water flow models.

River mile	UTM Coordinates[1], in meters		Elevation, in feet
	Easting	**Northing**	
191.74	763447	3798135	368.3
190.77	763714	3796614	367.1
189.00	763681	3794281	362.2
188.56	763001	3794254	365.1
187.31	761528	3793982	365.0
186.14	760340	3793027	364.9
184.34	758613	3790855	365.2
182.92	757106	3789138	365.0
181.33	755824	3787245	364.8
179.68	754201	3785508	364.8
178.07	752000	3784263	365.0
47.86	736669	3639365	156.2
47.72	736624	3639178	150.8
46.81	736281	3638125	150.8
44.43	735168	3635338	150.8
40.98	731882	3631582	[2]122.9
38.47	731591	3627923	[2]121.3
35.93	731039	3624107	[2]119.8
34.46	729226	3622800	[2]118.6
34.31	728990	3622783	[2]118.4
33.39	727531	3622993	[2]117.7
31.55	724802	3623457	[2]114.7
31.41	724567	3623385	[2]114.8
30.98	723908	3623357	[2]114.1
29.70	721982	3623768	[3]112.0
28.88	720667	3623899	[3]111.1
28.20	719580	3623936	[3]110.1
26.69	717318	3624355	[3]108.5
25.83	716056	3624828	[3]107.7

[1] Universal Transverse Mercator, Zone 11 coordinates, North American Datum of 1927.

[2] These elevations were increased by 0.8 ft for use in the ground-water model to account for difference in discharge specified for the accounting surface and the discharge during the stage measurements. See the section Laguna Dam to Eleven-mile gage for further explanation.

[3] These elevations were increased by 1.2 ft for use in the ground-water model to account for difference in discharge specified for the accounting surface and the discharge during the stage measurements. See the section Laguna Dam to Eleven-mile gage for further explanation.

extent of the Palo Verde Dam and Headgate Rock Dam forebays (Shana Tighi, Bureau of Reclamation, written commun. 2008). Water-surface elevation measurements were made between Parker Dam and Headgate Rock Dam during this study because the interpolated profile showed the forebay behind Headgate Rock Dam extending upstream to around river mile 189, whereas the earlier profile had a sloping water surface, resulting in higher water-surface elevations. The measurements supported the extent of the forebay represented in the interpolated profile.

Above Imperial Dam to Laguna Dam—The linearly interpolated profile was based on a streamflow-gaging station at river mile 49.2 (Colorado River below Imperial Dam) and Reclamation GIS coverages of the Imperial and Laguna Dam forebays. Water-surface elevation measurements were made above Imperial Dam and showed that the linearly interpolated profile overestimates the extent of the forebay. The measurements, which show a sloping water surface, were used in the ground-water model. The measurements were made at a discharge of about 6,000 ft^3/s, whereas the discharge specified for the accounting surface in this reach was 10,222 ft^3/s. Stage-discharge relations were not available for this reach and a correction for the difference between the two discharges was not made. Water-surface measurements made between Imperial Dam and Laguna Dam supported the linearly interpolated profile drawn from the Reclamation GIS coverage of the Laguna Dam forebay.

Yuma area—The following six streamflow-gaging stations were used for this reach: Colorado River below Laguna Dam, below Yuma Main Canal Wasteway, above Rockwood Weir at the NIB (International Boundary and Water Commission (IBWC) 095-219.00), immediately above Morelos Dam (IBWC 09-5220.21), immediately below Morelos Dam (IBWC 09-5220.41), and at Eleven-mile gage (IBWC 09-5221.00). The previous water-surface profile (Owen-Joyce and others, 2000) showed considerable variability in the water-surface profile between Laguna and Morelos Dams that was not represented by the linearly interpolated profile, so water-surface elevation measurements were made in this reach. The measurements were made at lower discharges than specified for the updated accounting surface. Corrections to water-surface elevations that account for the differences in discharge were estimated from the stage-discharge rating curves at the below Laguna Dam streamflow-gaging station and the below Yuma Main Canal Wasteway streamflow-gaging station. Between Laguna Dam and the below Yuma Main Canal Wasteway streamflow-gaging station, the updated accounting surface discharge was 716 ft^3/s, whereas the discharge measured during the water-surface elevation measurements was 440 ft^3/s. The water-surface profile between Laguna Dam and the below Yuma Main Canal Wasteway streamflow-gaging station was defined in the ground-water model by the measurements plus a correction of 0.8 ft, based on the stage-discharge rating curve at the below Laguna Dam streamflow-gaging station. In the reach between the below Yuma Main Canal Wasteway streamflow-gaging station and Morelos Dam, the discharge specified for the updated accounting surface was 1,526 ft^3/s. The discharges measured during the water-surface elevation

measurements were 670 and 778 ft³/s. The water-surface profile between the below Yuma Main Canal Wasteway streamflow-gaging station and the above Rockwood Weir streamflow-gaging station was defined in the ground-water model by the measurements plus a correction of 1.2 ft, based on the stage-discharge rating curve at the below Yuma Main Canal Wasteway streamflow-gaging station and the average of the measured discharges in that reach. An analysis of streamflow-gaging station records by Jeffrey C. Addiego (Bureau of Reclamation, written commun., 2008) provided the water-surface elevation at the above Rockwood Weir streamflow-gaging station. Just above and below Morelos Dam and at the Eleven-mile gage (at river mile 18.8), the average monthly high stages at the IBWC streamflow-gaging stations were used to establish the water-surface elevations. Below Eleven-mile gage, the land surface digitized along the path of the Colorado River was used as the water-surface boundary condition in the ground-water flow model.

Drainage Ditch Reaches

Along reaches of the Colorado River where water is diverted for irrigation on the flood plain, drainage ditches intercept return flow to the river and the river gains water from the aquifer. In these reaches, the accounting surface is defined by using the water-surface elevation in the drainage ditches along the edge of the flood plain. Flood-plain irrigation with diverted Colorado River water causes a higher ground-water level under the flood plain because irrigation water not consumptively used by crops percolates down to the water table and causes the water table to rise. There is a constant flow of irrigation with diverted river water, percolation to the drainage ditches or river, and flow in the drainage ditches to the river. Interception of the percolated irrigation water by a network of drainage ditches connected to the Colorado River keeps the water table from rising up into the root zone and this level is higher than it would be if controlled by the river. Because water in the drainage ditches is considered Colorado River water for accounting surface purposes, it warrants the same level of protection from depletion without an entitlement as water in the Colorado River and stored in reservoirs. Where drainage ditches intercept percolated irrigation water, the water-surface elevations in the drainage ditches were used to define the accounting surface. Drainage ditches along the edge of the flood plain could not be used in the Yuma area because the elevation of the water surface is controlled mainly by recharge from the unlined canals that are above the flood-plain elevation and run parallel to the edge of the flood plain rather than by percolation from irrigation on the flood plain. Water-surface elevations in the drainage ditches were determined from USGS measurements in 2007 and 2008.

Ground-Water Flow Models

In the previous studies, the accounting surface was hand-drawn using hydrologic judgment to extend water-surface elevations into the river aquifer based on the shape of the river aquifer. This study refines that procedure by using simple physically based steady-state numerical models to calculate the updated accounting surface.

The river aquifer was represented as a single model layer of uniform aquifer thickness and spatially invariant or constant transmissivity. Because transmissivity is the product of the aquifer thickness and hydraulic conductivity, hydraulic conductivity in the model also is constant. With the assumption of a spatially invariant or constant transmissivity throughout the model domain, the governing equation of steady-state flow in two dimensions is:

$$\frac{\partial^2 h}{\partial x^2} + \frac{\partial^2 h}{\partial y^2} = 0 \tag{1}$$

where h is hydraulic head and x and y represent Cartesian coordinates along orthogonal axes. Aquifer thickness and conductivity are not present in equation 1; the distribution of heads in the modeled river aquifer depends only on the aquifer boundaries and the specified water-surface elevations in the drainage ditches in the flood plain and in the Colorado River. Flow rates through the aquifer would depend on the aquifer transmissivity, but flow rates are not considered in this study.

The river aquifer adjacent to the Colorado River is unconfined. The assumption of constant transmissivity neglects spatial variations in transmissivity that would arise from the spatial variations in the vertical position of the water table. The assumption also neglects variations in transmissivity that occur from variations in the vertical position of the aquifer bottom as well as spatial variations in hydraulic conductivity. A more rigorous approach would be to use the nonlinear Boussinesq equation instead of the simpler linear Laplace equation (equation 1). That approach, however, would have required unavailable information on aquifer geometry and hydraulic properties. Use of equation 1 is in keeping with a long history in the field of ground-water hydrology of using simple linear equations with the assumption of homogeneous properties to approximate ground-water conditions and responses and is consistent with the overall parsimonious approach taken in the concept, definition, and application of an accounting surface.

The accounting surface in the four areas was modeled with MODFLOW 2000 (Harbaugh and others, 2000) using the water-surface elevations in the Colorado River and drainage ditches as constant-head boundaries. The grid spacing in the models was 0.25 mi along model rows and columns. General characteristics of the model grids are given in table 4 and the extent of the model grids is shown in figure 1. The path and distribution of Colorado River and drainage ditch water-surface elevations were established on the model grids using the RIVGRID program (Leake and Claar, 1999). The water-surface elevations defined by RIVGRID were then incorporated into the models as nodes with a constant head.

Areas of the river aquifer adjacent to the Colorado River for which the accounting surface was modeled include (1) Mohave Valley; (2) Parker, Palo Verde, and Cibola Valleys; (3) Imperial Dam to Laguna Dam; and (4) the Yuma area. Each area was modeled with a single horizontal layer of cells of thickness 500

ft and hydraulic conductivity 39.2 ft/day; however, as pointed out in the discussion of equation 1, the model predictions of the accounting surface are independent of thickness and hydraulic conductivity. Tests were carried out by varying hydraulic conductivity and thickness to verify that computed head distributions were independent of these parameters. Rows and columns of the model grids were oriented in east-west and north-south directions in the UTM, Zone 11, coordinate system.

Updated Accounting Surface

The accounting surface around reservoirs was updated using a reservoir elevation. The accounting surface is set at its maximum possible level of 1,205.4 ft in the river aquifer around Lake Mead (fig. 4) and has not changed from the original accounting surface. The accounting surface is set at 644.0 ft in the river aquifer around Lake Mohave (fig. 5), and at 448.7 ft in the river aquifer around Lake Havasu (fig. 5), the current high monthly target elevations for these reservoirs. These elevations are slightly different from the high monthly target elevation used for the original accounting surface. In the river aquifer between the major reservoirs, ground-water flow models with boundary conditions set by Colorado River and drainage ditch water-surface elevations were used to contour the accounting surface. The models computed water-level elevations over the entire river aquifer; however, only contours in the river aquifer where the accounting surface exists are shown for modeled areas (figs. 4–7). The updated accounting surface is shown on maps for Mohave Valley and adjacent tributary areas (fig. 5); for Parker, Palo Verde, and Cibola Valleys and adjacent tributary areas (fig. 6); and for the Yuma area upstream and downstream from Laguna Dam and adjacent tributary areas (fig. 7). The model grid in the Yuma area extends to the south of the accounting surface, but only contours in the area with the accounting surface are shown.

Summary

An update of the accounting surface developed in the 1990s to identify wells outside the flood plain of the lower Colorado River that yield water that will be replaced by water from the river was required as a result of changes in the ground and surface water systems and a datum correction to the water surface elevations in drainage ditches. The updated accounting surface will be used to identify which wells need an entitlement for diversion of water from the Colorado River and need to be included in accounting for consumptive use of Colorado River water as outlined in the Consolidated Decree of the United States Supreme Court in Arizona v. California, 547 U.S.150 (2006). Contours of the original accounting surface were hand drawn based on the shape of the aquifer, water-surface elevations in the Colorado River and drainage ditches, and hydrologic judgment.

The original accounting surface was updated based on updated water-surface elevations in the Colorado River and drainage ditches, and the use of a simple, physically based ground-water flow model to calculate the accounting surface. The water-surface elevation of the Colorado River was determined for discharges specified by Reclamation. The water-surface elevations were derived from a linearly interpolated profile between USGS and Reclamation streamflow-gaging stations and supplemented by IBWC streamflow-gaging stations downstream from the northerly international boundary with Mexico. In addition, water-surface elevations were measured where the linearly interpolated profile deviated significantly from the water-surface profile used to develop the original accounting surface. The USGS also measured water-surface elevations in drainage ditches in the Parker, Palo Verde, and Cibola Valleys.

The accounting surface was modeled with MODFLOW 2000 (Harbaugh and others, 2000) using the water-surface elevations in the Colorado River and drainage ditches as constant-head boundaries. Reaches of the river aquifer adjacent to the Colorado River for which the accounting surface was modeled include (1) Mohave Valley, (2) Parker, Palo Verde, and Cibola Valleys, (3) Imperial Dam to Laguna Dam, and (4) the Yuma area. The development and application of computer models will make further updating of the accounting surface, if necessary, a straightforward task. In the river aquifer adjacent to reservoirs, the accounting surface was determined by a reservoir elevation specified by Reclamation.

Table 4. Properties of ground-water flow models used to compute the accounting surface for areas along the lower Colorado River.

Area modeled	UTM Coordinates of northwest corner of model grid[1], in meters		Number of model rows	Number of model columns	Number of active cells[2]
	Easting	Northing			
Mohave Valley	706260.7	3897829.0	160	139	13,264
Parker, Palo Verde, and Cibola Valleys	636449.1	3797916.0	329	388	87,176
Imperial Dam to Laguna Dam	730975.8	3672261.3	103	88	4,702
Yuma Area	640414.6	3691950.0	511	340	69,814

[1] Universal Transverse Mercator, Zone 11 coordinates, North American Datum of 1927.

[2] Includes cells with computed head and constant-head cells used to represent water-surface features.
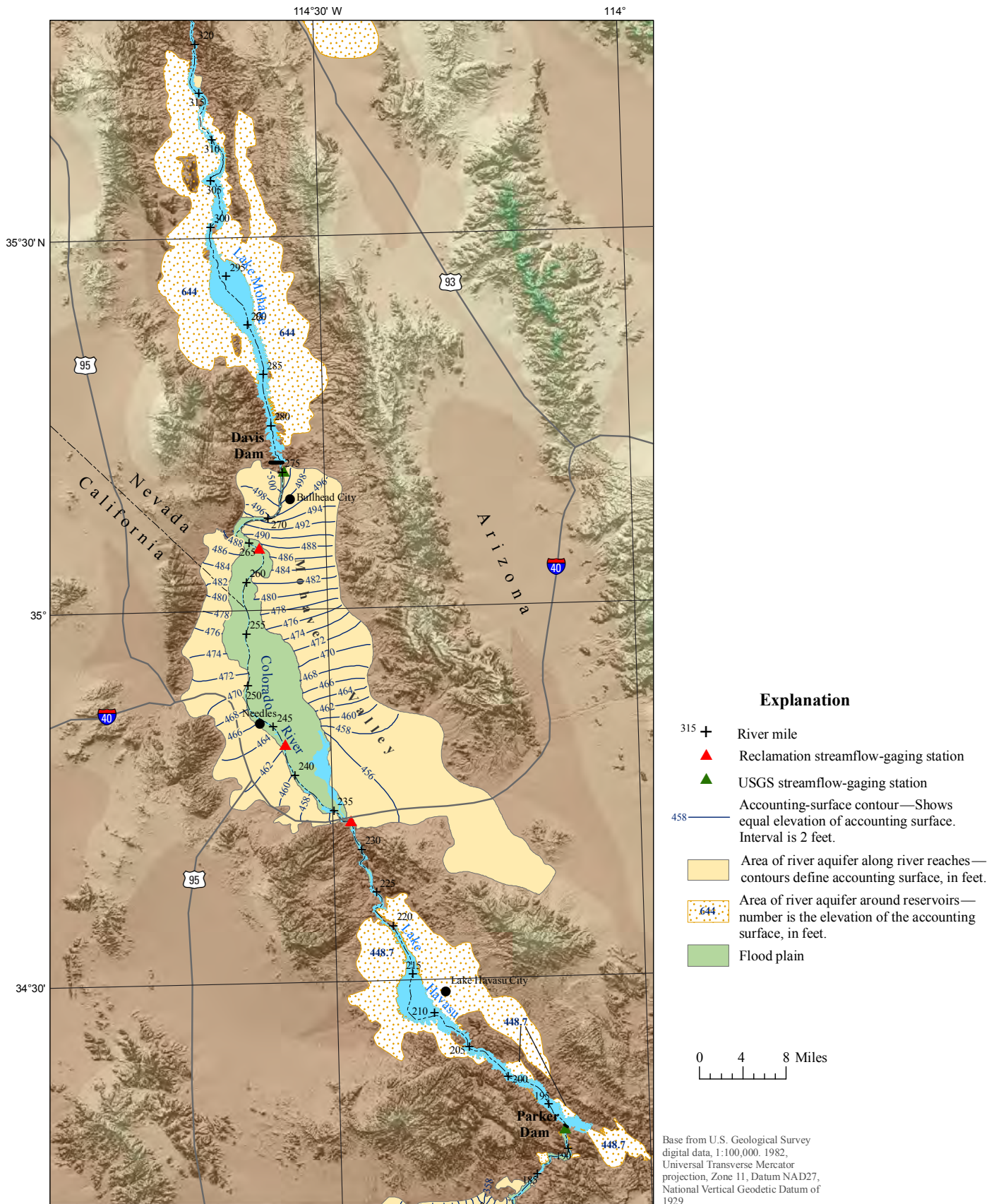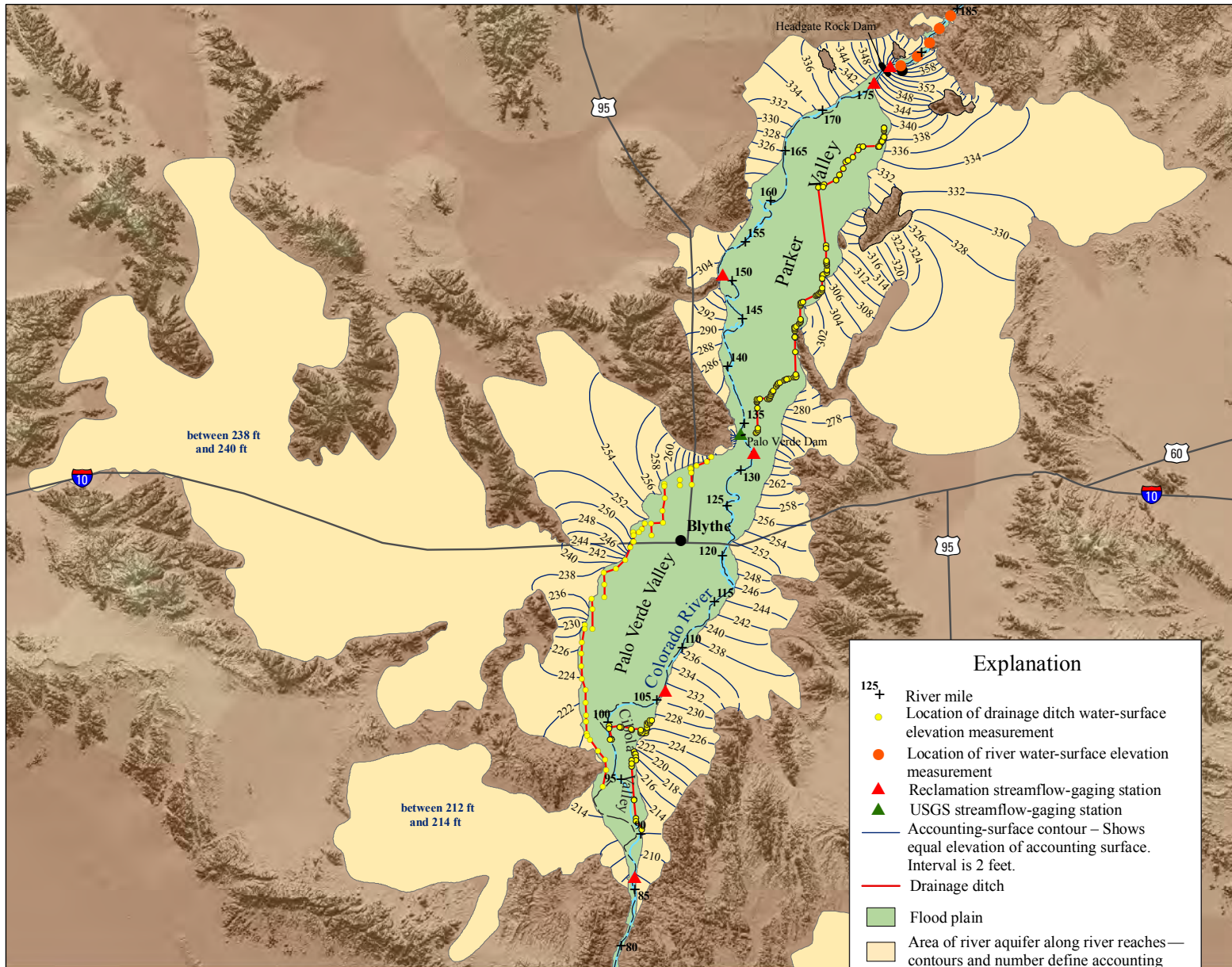
Figure 4. Map showing the accounting surface in the areas surrounding Lake Mead, Arizona, Utah, and Nevada.

Figure 5. Map showing the accounting surface in Mohave Valley and adjacent tributary areas in Arizona, California, and Nevada.
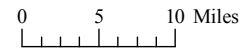
Figure 6. Map showing the accounting surface in Parker, Palo Verde, and Cibola Valleys and adjacent tributary areas in Arizona and California.
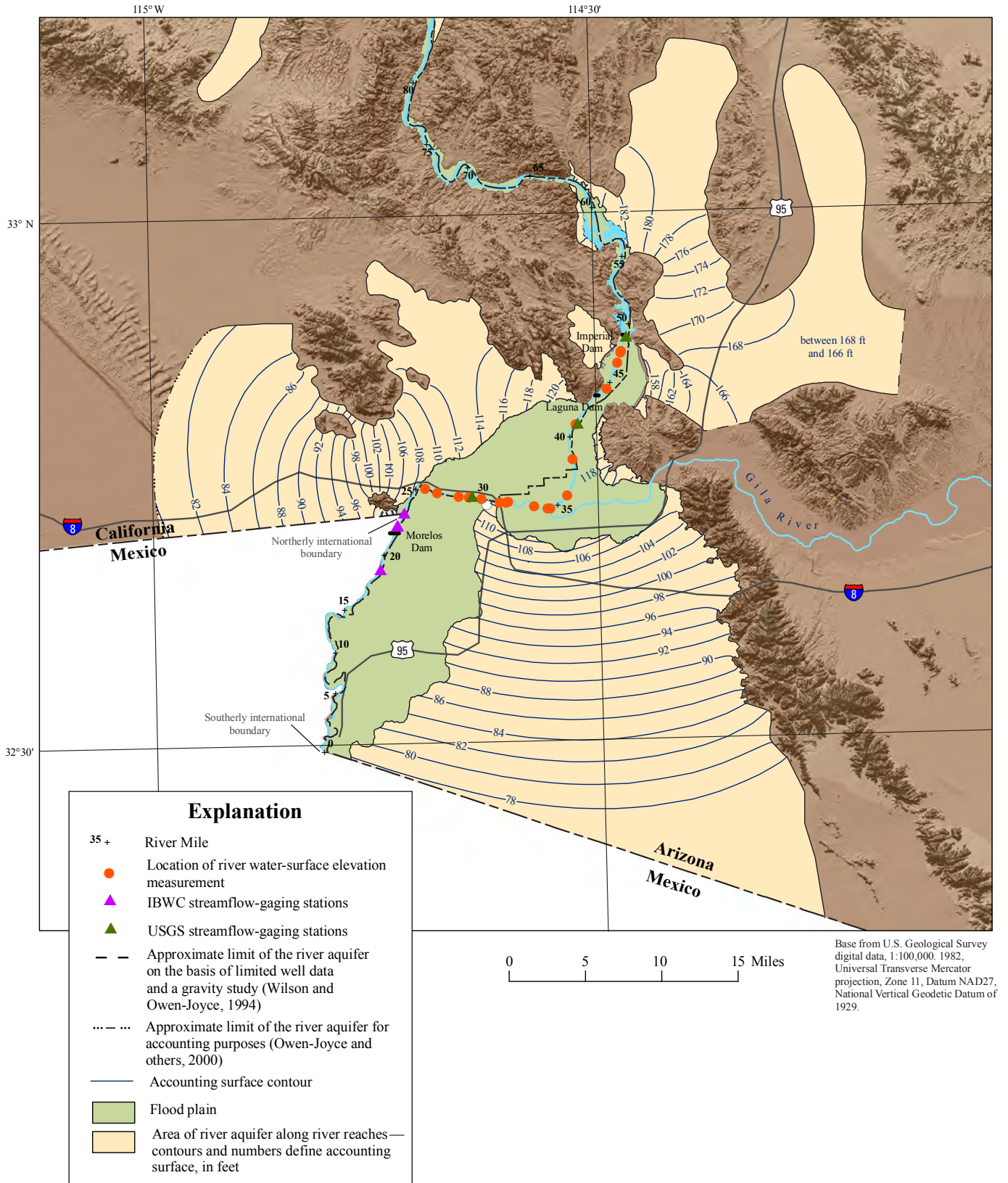
Figure 7. Map showing the accounting surface in the Yuma area upstream and downstream from Laguna Dam and adjacent tributary areas in Arizona and California.

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